FINAL
GEOTECHNICAL INVESTIGATION REPORT
PROPOSED CARSON FREEWAY, U.S. 395
(NORTH PART)
CARSON CITY, NEVADA

July 15, 1999

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July 15, 1999

File: 30-1348-15.003

Mr. Parviz Noori, P.E. Materials Division Nevada Department of Transportation 1263 South Stewart Street Carson City, Nevada 89712

SUBJECT: Final Geotechnical Investigation Report

Proposed Carson Freeway, U.S. 395 (North Part)

Carson City, Nevada

Dear Parviz:

The attached final report presents the results of our geotechnical investigation for the proposed Carson Freeway, U.S. 395. The proposed freeway is to be located approximately two kilometers east of the existing U.S. 395 at the north end of Carson City and extends from Duck Hill to U.S. 50 West at the south end of Carson City. This investigation was conducted for the northern five kilometer portion of the freeway from Duck Hill to U.S. 50 East including seven bridge structures, several retaining walls, on/off ramps, and realignment of several adjacent surface streets. It is understood that the Nevada Department of Transportation (NDOT) will conduct the geotechnical investigation for the southern end of the freeway. Our work consisted of subsurface exploration, laboratory testing, engineering analyses, and preparation of a draft report. The draft report was submitted to NDOT for review and comment. NDOT's review comments are incorporated into this final report.

Based on our work completed to date, we have drawn the following general conclusions:

- Site soils encountered in our field investigation generally consist of sandy soils with occasional interbedded clay zones. The near-surface sandy soils tend to be loose to medium dense, increasing in density with depth. The clay zones, where encountered, tend to be medium stiff to stiff and of moderate plasticity.
- Groundwater was encountered at depths as shallow as 1-1/2 meters below the existing ground surface south of Arrowhead Drive. The shallow groundwater may adversely impact construction in some areas of the project south of Arrowhead Drive.

- Two potential fill borrow areas were explored as part of our investigation at Duck Hill and Edmonds Drive. The Duck Hill area is composed of decomposed granite (DG), and the Edmonds Drive area composed of silty sands. Both areas are deemed to be adequate fill sources; however some blending of the silty fine sand portions of the south end of the Edmonds area must be performed to produce a good quality fill. We estimate that a 50-50 blend of the fine sand at the south end of Edmonds with the DG from the Duck Hill area or the northern Edmonds area will provide a suitable fill soil.
- We analyzed embankment stability assuming a material internal angle of friction of 34 degrees (as requested by NDOT) and a material internal angle of 38 degrees (based empirical values provided in Table 4-5, Publication No. FHWA-HI-97-013 for dense materials). Our results indicate slopes of inclinations of 2H:1V or less and up to 9.1 meters in height are stable under static and seismic conditions for materials with internal angles of friction of 34 to 38 degrees. Our results also indicate slopes constructed at a batter of 1.5H:1V are unstable during a design level seismic event for a cohesionless soil with an internal angle of friction of 34 degrees. We recommend that permanent fill be constructed to a maximum slope of 2H:1V.
- Our stability analysis performed using the XSTABL5.2 computer program indicates low factors of safety for abutment foundations supported within the embankments during a design seismic event. We recommend that all bridge structures be founded on deep foundations. Recommendations for driven pipe piles, sizes PP305, PP406 and PP457 are provided.
- Resitivity testing indicates potentially corrosive to moderately corrosive conditions at
 the bridge locations. Carbon steel corrosion rates are anticipated to be on the order of
 12 μm/yr. Assuming a design life of 100 years for the proposed bridge abutments and
 a uniform loss model, steel pipe piles will have an overall sacrificial wall thickness
 loss of less than 1.5 mm. We recommend a sacrificial wall thickness of 1.5 mm be
 used in determining the required pipe pile wall thickness.
- The potential for liquefaction of the site soils and the resultant settlement are expected to be minimal.

These and other conclusions and recommendations, along with restrictions and limitations on these conclusions, are discussed in the attached report.

We appreciate this opportunity to be of service to you, and look forward to future endeavors. If you have any questions regarding this report or need additional information or services, please feel free to call one of the undersigned in our Reno office.

Sincerely,

KLEINFELDER, INC.

Mark Doehring, P.E. 7/15/17

Project Engineer

Christine M. Welch, P.E. Regional Manager

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Enclosures:

Report (10 Bound)



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FINAL GEOTECHNICAL INVESTIGATION REPORT PROPOSED CARSON FREEWAY, U.S. 395 (NORTH PART) CARSON CITY, NEVADA

1. INTRODUCTION AND SCOPE

1.1 Project Description

This report presents the results of our geotechnical investigation for the alignment, structures, ramps, and surface street realignment portion of the proposed Carson Freeway, U.S. 395. The new U.S. 395 freeway will extend from approximately two kilometers east of the existing U.S. 395 at Duck Hill at the north end of Carson City, southward to the intersection of U.S. 50 West at the south end of Carson City. This study focused on the northern portion of the freeway from Duck Hill southward to U.S. 50 East. The Nevada Department of Transportation (NDOT) will conduct the geotechnical investigation for the southern portion of the freeway, south of U.S. 50 East. The project location is shown on the attached site plans (Figures A-1 through A-6, Appendix A).

We understand that the northern portion of the project will include construction of a five kilometer, multiple lane, limited access highway. Construction will include seven bridge structures at U.S. 395 and Bonanza Drive. Arrowhead Drive, Northgate Lane, Emerson Drive, East College Parkway, Carmine Street and U.S. 50 East. Construction will also include on/off ramps at U.S. 395, at Bonanza Drive, East College Parkway and the intersection of U.S. 50 East. Realignments of Bonanza Drive. Imus Road, Arrowhead Drive, and Lompa Lane are included in this project. Construction of drainage channels and retaining walls are planned at various areas throughout the proposed five kilometer freeway alignment, and additional drainage channels south of U.S. 50 East were also studied. Although the section of the project south of U.S. 50 East is not included in our overall geotechnical study, a portion of our scope of work included drilling, sampling, and laboratory testing in areas of the southern end of the project to evaluate drainage issues and potential fill sources.



1.2 Purpose and Scope of Work

The purpose of this study is to evaluate the feasibility of the proposed freeway construction with respect to the observed subsurface conditions, and to provide our geotechnical recommendations and opinions as outlined in NDOT's Task Order No. 2, dated June 30, 1997 and summarized below.

- General soil and groundwater conditions at the project site, with emphasis on how the conditions are expected to affect the proposed construction;
- Suggested specifications for earthwork construction, including site preparation recommendations, a discussion of reuse of existing near surface soils as structural or non-structural fill, and a discussion of remedial earthwork recommendations, if warranted;
- Recommendations for temporary excavations and trench backfill:
- Recommendations for permanent cut and fill slopes;
- Conventional shallow spread foundation design including soil bearing values. minimum footing depth, resistance to lateral loads and estimated settlements.
- Lateral earth pressures and drainage recommendations for short retaining structures; and
- Steel corrosion potential and concrete reactivity of site soils.

In addition, we have been asked to provided the following information for deep foundations, using closed end driven pipe piles, designations PP305, PP406 and PP457:

- Ultimate capacities for each bridge location;
- Recommended grade of steel and minimum wall thickness:
- Wave equations relating expected driving resistance for each bridge location; and
- Pile lateral load design parameters.

Our scope of services consisted of background review, site reconnaissance, field exploration, laboratory testing, engineering analyses, and preparation of a draft report. The draft report was



submitted to NDOT for review and comment. NDOT's review comments have been incorporated into this final report.

1.3 Authorization

Authorization to proceed with our work on this project was provided by Mr. Rod Johnson, P.E. and Mr. Dean Weltzel. P.E. on June 30, 1997 in the form of a signed Nevada Department of Transportation Task Order.

1.4 References

The following information was provided to Kleinfelder in the course of this study and serves as the basis of our understanding of the project type and scope.

- Several site plans prepared by the Nevada Department of Transportation. These
 drawings are the basis for the site plans shown on Figures A-1 through A-6, Plates A1 through A-14, Figures B-1 through B-3 and Plates B-1 through B-7 presented in
 Appendices A & B.
- Preliminary 300-scale site plan showing retaining wall locations.
- Preliminary retaining wall profile sheet.

In addition, the following published and unpublished references were reviewed during preparation of this report.

- "U.S. 395 Carson City Interim Bypass, Expressway 2000, Carson City, Nevada, Preliminary Roadway Design Report, Phase II Submittal," prepared by Lumos and Associates, Inc., dated August 1994.
- "Geotechnical Investigation for Hydraulics Analysis Report (Draft). Proposed Carson Freeway, Carson City, Nevada," prepared by Kleinfelder, Inc., dated January 26, 1998, File No.: 30-1348-15.001.
- "Geotechnical Investigation Report (Draft). Proposed Carson Freeway. U.S. 395.
 Carson City, Nevada." prepared by Kleinfelder, Inc., dated March 11, 1998. File No.: 30-1348-15.002.
- AASHTO Standard Specifications for Highway Bridges. 16th edition, 1996.



- "Design and Construction of Driven Pile Foundations," Publication No. FHWA-HI-97-013.
- "Earth Retaining Structures, Reference Manual (Draft)," FHWA Publication NHI Course No. 132326 Module 6, May 1998.
- "Corrosion / Degradation of Soil Reinforcements for Mechanically Stabilized Earth Walls and Reinforced Soil Slopes," Publication No. FHWA-SA-96-072.
- Ishihara, K. (1985), Stability of Natural Deposits During Earthquakes, Proceedings, Eleventh International Conference on Soil Mechanics and Foundation Engineering, San Francisco, August 12-16, pp. 321-376.
- Seed, H.B., Idriss, I.M., and Arango, I. (1983), Evaluation of Liquefaction Potential Using Field Performance Data, Journal of Geotechnical Engineering, ASCE, Vol. 109, No. 3, March, pp. 458-482.
- Seed, R.B., Harder, L.F. (1990), SPT-Based Analysis of Cyclic Pore Pressure Generation and Undrained Residual Strength, Proceedings, H. Bolton Seed Memorial Symposium, Vol. 2, ed: J. Michael Duncan, pp. 351-376.
- Tokimatsu, K and Seed, H.B. (1987), Evaluation of Settlements in Sands Due to Earthquake Shaking, Journal of Geotechnical Engineering, ASCE, Vol. 113, No. 8, August, pp. 861-878.
- Youd, T.L. and Garris, C.T., (1995), Liquefaction-Induced Ground-Surface Disruption, Journal of Geotechnical Engineering, ASCE, Vol. 121, No. 11, November, pp. 805-809.
- "Geotechnical Earthquake Engineering, Reference Manual." FHWA Publication HI-99-012. NHI Course No. 13239 December 1998.



2. METHODS OF STUDY

2.1 Field Exploration

Our selection of field exploration locations was based on the anticipated roadway alignment, bridge layouts, ramp and realignment layouts, and site access. The subsurface exploration consisted of drilling 40 alignment borings ranging in depth from 2 to 12 meters below existing grade. 23 drainage borings ranging in depth from 9 to 16 meters below existing grade and 27 ramp and roadway realignment borings ranging in depth from 2 to 8 meters below existing grade. These borings were advanced using an auger type drill rig. The subsurface exploration for the bridge structures consisted of drilling 14 borings to depths of 14 to 24 meters below existing grade using mud-rotary type drilling methods.

Locations of borings shown on the site plans in Appendices A and B were approximated by pacing and measuring from features shown on the site plans. Elevations shown on the boring logs were obtained by interpolation between contour lines shown on the site plans. These locations and elevations should be considered accurate only to the degree implied by the method used.

Soil conditions encountered are presented on boring logs as follows:

- Line Borings, Plates C-2 through C-41A, Appendix C;
- Structure Borings, Plates D-2 through D-15A, Appendix D;
- Drainage Borings, Plates E-2 through E-24, Appendix E: and
- Ramp and Realignment Borings, Plates F-2 through F-28, Appendix F.

A key to the boring logs is presented as the first plate of Appendices C through F (C-1, D-1, E-1 and F-1).

A field engineer logged the soil conditions exposed in the borings and collected bulk and relatively undisturbed driven samples for laboratory testing. Soil samples were obtained by driving a 50.8 mm ID, 63.5 mm OD Modified California (MC) Sampler containing thin brass liners, into the bottom of the boring. The number of blows required to drive the last 300 mm of



an 458 mm drive with a 64 kg hammer dropping 760 mm is recorded as the blows per 300 mm (Blow Count) on the boring logs. Based on our field experience, the Standard Penetration Test (SPT) Blow Count can be approximated from the MC Sampler Blow Count by multiplying the field count by a factor of 0.85. Blow counts shown on the logs represent field counts and have not been corrected for sampler type, hammer efficiency, and overburden pressure. When the sampler was withdrawn from the boring, the brass liners containing the samples were removed, examined for logging, labeled and sealed to preserve the natural moisture content for laboratory testing.

After borings were completed, they were backfilled with excavated soil using the equipment at hand. Where borings were drilled in existing pavement, the pavement structural sections were measured and the borings were backfilled to the bottom of the structural section with drill cuttings with the final few inches backfilled with quick setting low shrinkage grout.

Our field exploration also included the coring of the existing pavement along the freeway alignment at College Parkway, Carmine Street, and Lompa Lane. Our cores extended through the structural section to the top of the subgrade soil. The results of our coring are presented in Section 3.2.4 of this report.

2.2 <u>Laboratory Testing</u>

Laboratory testing is useful for evaluating both index and engineering properties of soils. Typical index tests evaluate soil moisture content, unit weight, soil particle gradation, and plasticity characteristics. Tests for engineering properties can assess soil strength, compressibility, swell potential, and potential for steel corrosion or adverse reactivity with Portland Cement Concrete. We performed laboratory testing on selected soil samples to assess the following:

- Soil Classification (AASHTO T11, T27, T89 and T90)
- Unit Weight and Moisture Content (ASSHTO T204 and T265)
- Consolidation (AASHTO T216)
- Direct Shear Strength (AASHTO T216)
- R-Value (Nevada Test Method T115)
- Permeability (California Test Method 220)



A number of consolidation tests were conducted to evaluate settlement. Some of the tests were conducted on sandy samples prior to the completion of classification testing based on visual evaluation of the tube samples. These tests were initiated because of the long time required to complete the tests. In some cases, the amount of consolidation was minimal. In some cases, however, fine sandy soils consolidated as much or more than the more clayey soils although by classification, they tended to be more sandy rather than fine grained.

The following analytical tests were performed by Acculabs, Inc. (formerly Chemax Laboratories) on selected samples obtained from borings in the proposed structure locations:

- Soluble Sulfate Content
- Resistivity and pH

Individual laboratory test results can be found on the boring logs and in Appendices at the end of this report as follows:

- Laboratory tests for Line Borings, Appendix C, Plates C-42 through C-89;
- Laboratory tests for Structure Borings, Appendix D, Plates D-16 through D-41;
- Laboratory tests for Drainage Borings, Appendix E, Plates E-25 through E-34; and
- Laboratory tests for Ramp and Realignment Borings Appendix F, Plates F-29 through F-55.



3. DISCUSSION

3.1 Site Conditions

The majority of the Carson Freeway alignment north of U.S. 50 crosses undeveloped land. Some small areas cross through single family home sites. In addition, near its intersection with U.S. 50, the alignment crosses through a mobile home park and a strip mall. The general project area is relatively flat with a slight overall gradient down toward the south, except at the far northern end of the project (Duck Hill) where the ground slopes sharply down to the south, and where a total relief of approximately 31 meters is present. The Carson Freeway south of U.S. 50 primarily crosses undeveloped land. The area is relatively flat in the Lompa Ranch area, but gently slopes to the north and west in the Edmonds Drive area. In general, vegetation consisted of a low to moderate cover of sagebrush and native grasses. Surface runoff consists of sheetflow to existing road ditches and detention ponds.

3.2 Subsurface Conditions

The following paragraphs summarize the results of our field exploration. The boring logs should be reviewed for a more detailed description of the subsurface conditions at the locations explored.

3.2.1 Line Borings

We generally encountered sandy soils along the total alignment between Duck Hill on the north, to U.S. 50 East on the southern end of the project. However, clay soils were observed in the following locations: in the area near Northgate Lane below 6 meters, in the area near Emerson Drive between approximately 1/2 and 2 meters, and south of College Parkway below 4 meters. The sandy soils tend to be medium dense and fine to medium grained, while the clayey soils tend to be medium stiff to stiff and moderately plastic. Groundwater was encountered during our exploration at depths ranging from 8 to 9 meters (elevation 1445 to 1446) below the ground surface south of Bonanza at the base of Duck Hill. The general groundwater level south of Arrowhead Drive was typically 1-1/2 meters below the ground surface (approximate elevation



1441.5). No groundwater was encountered during our exploration north of Bonanza. Fluctuations in the level of the groundwater and soil moisture conditions as noted in this report may occur due to variations in precipitation, land use, irrigation, and other factors. A key to soil classification and terms is presented in Appendix C, Plate 1. The logs of line borings are presented in Appendix C, Plates C-2 through C-41A.

3.2.2 Structure Borings

As with the line borings, we generally encountered sandy soils with occasional relatively thin clay zone in the structure borings. The clay zones were more dominant in the borings drilled for the Northgate Lane structures. The clayey soils, although moderately plastic, tended to be very stiff to hard in consistency. In general, the near surface sandy soils were relatively loose to depths of 1-1/2 to 2 meters below the existing ground surface. Below this depth, soils encountered consisted of medium dense to very dense sandy soils with some interbedding of very stiff clayey soils. Groundwater levels were not be measured in any of the structure borings because all of the structure borings were drilled using the mud rotary drilling method. This method of drilling was used so that accurate blow counts could be measured for liquefaction analyses. Groundwater levels were measured in the other (line, ramp, and realignment and drainage) borings. Based on the groundwater measurements in these borings, groundwater is expected to be in excess of 6 to 8 meters (elevation 1460 to 1458) in the U.S. 395 over-crossing area, approximately 3 to 4 meters (elevation 1442 to 1441) in the Arrowhead Drive area, and 1-1/2 to 2 meters in areas of the remaining structures. A key to soil classification and terms is presented in Appendix D. Plate D-1. The logs of structure borings are presented in Appendix D. Plates D-2 through D-15A.

3.2.3 Drainage Borings

Drainage borings were drilled south of U.S. 50 in two general areas. The first area was in the Lompa Ranch area and the State Prison. In general, the groundwater level ranged from 1 to 2-1/2 meters below the ground surface. The soil types encountered were generally sandy soils. The exception was on the Lompa Ranch itself. One boring encountered clays up to 4 meters below the ground surface, with a second boring encountering clays within the first meter.

The second set of drainage borings south of U.S. 50 was drilled along the west side of Edmonds Avenue. The purpose of these borings, in addition to drainage issues, was to evaluate the subsurface conditions for the possible construction of the freeway below grade and the possible



use of the excavated soil for fill in the north of U.S. 50 section of freeway. In general, sandy soils were encountered in the borings. The near-surface soils tended to be more clayey toward the south, although all of the soils encountered in the borings were classified as sand, silty sand, or clayey sand. No groundwater was observed in these borings up to the depth drilled of over 15 meters. A key to soil classification and terms is presented in Appendix E, Plate E-1. The logs of drainage borings are presented in Appendix E, Plates E-2 through E-24.

3.2.4 Ramp and Realignment Borings and Cores

Subsurface conditions consisted of sandy soils. The density of the sandy soils varied from loose to medium dense. Groundwater was encountered at depths in the range of 1 to 2 meters (elevation 1413 to 1412) in the U.S. 50 and the north side of College Parkway ramp areas (elevation 1429 to 1428). This was also the typical depth to groundwater for the realignment of Lompa Lane (elevation 1415 to 1411) and Arrowhead Drive (elevation 1446 to 1445). In the realignment area for Imus Road groundwater was not encountered to the maximum depth explored of 3-1/2 meters below the existing ground surface. No groundwater was encountered south of College Parkway to the maximum depth explored of about 4-1/2 meters. This was also the case for the U.S. 395 ramps where the maximum depth explored was 8 meters. A key to soil classification and terms is presented in Appendix F, Plate F-1. The logs of ramp and realignment borings are presented in Appendix F, Plates F-2 through F-28.

Pavement cores were obtained for the realignment of College Parkway. Carmine Street, and Lompa Lane. Cores were taken at random locations within the areas proposed for realignment. Presented in Table 1 is a summary of the results of our coring study.

TABLE 1

CORING OF EXISTING PAVEMENTS

STREET NAME	Core No.	AC THICKNESS	AB THICKNESS	Remarks
College Parkway	1	115 mm	75 mm	Eastbound
	2	120 mm	75 mm	Eastbound
	3	95 mm	130 mm	Turn lane
	4	100 mm	175 mm	Turn lane
	5	125 mm	75 mm	Westbound

STREET NAME	Core No.	AC THICKNESS	AB THICKNESS	REMARKS
	6	H15 mm	90 mm	Westbound
Carmine Street	7	145 mm	305 mm	Westbound
	8	125 mm	305 mm	Westbound
	9	120 mm	455 mm*	Eastbound
	10	125 mm	480 mm*	Eastbound
Lompa Lane	11	75 mm	610 mm**	Northbound
	12	75 mm	455 mm**	Southbound
	13	75 mm	305 mm	Southbound
	14	80 mm	305 mm	Northbound

^{*}Very sandy base. Not expected to meet NDOT requirements for base.

3.2.5 Groundwater Measurements

Groundwater levels for the line, ramp, and drainage borings, along with corresponding surface elevations, boring depths, and project stationing, are provided in Table 2. Locations of alignment and bridge borings are also shown on the site plans in Appendices A and B, respectively. Groundwater levels were not measured in any of the structural borings since the borings were drilled using rotary mud methods.

TABLE 2
MEASURED GROUNDWATER LEVELS

Boring Number	Surface Elevation (m)	Boring Depth (m)	GROUNDWATER ELEVATION	STATION
BL-1	1414.27	1.98	NFWE	227+00
BL-2	1417.32	1.98	NFWE	332+00
BL-3	1417.32	3.51	1416.20	337+00
BL-4	1420.37	5.03	1416.67	342+00
BL-5	1420.37	6.55	1417.97	347+00
BL-6	1420.37	6.55	1419.27	352+00
BL-7	1423.42	6.55	1419.92	357+00
BL-8	1424.94	3.51	NFWE	362+00

^{**}Minimum thickness. Could not hand auger to bottom of base.

BORING NUMBER	SURFACE ELEVATION (M)	BORING DEPTH (M)	GROUNDWATER	CT. TIC.
			ELEVATION	STATION
BL-9	1425.55	3.51	NFWE	367+00
BL-10	1435.00	4.27	1432.00	372+00
BL-11	1432.56	7.16	1429.56	377+00
BL-12	1431.34	6.55	1428.14	382+00
BL-13	1431.34	6.55	1429.54	387+00
BL-14	1432.56	10.97	1431.06	393+00
BL-15	1432.56	11.13	1430.86	397+00
BL-16	1432.56	11.13	1430.86	402+00
BL-17	1434.08	7.16	1432.28	407+00
BL-18	1435.91	8.08	1434.21	424+00
BL-19	1435.61	8.08	1434.21	429+00
BL-20	1439.27	9.60	1437.77	439+00
BL-21	1435.61	10.21	1434.11	434+00
BL-22	1437.13	6.55	1435.73	444+00
BL-23	1440.48	10.21	1438.68	449+00
BL-24	1441.70	6.55	1440.00	454+00
BL-25	1443.23	8.08	1441.53	458+00
BL-26	1443.23	9.60	1441.53	462+00
BL-27	1447.80	8.69	1444.30	467+00
BL-28	1452.68	9.60	1445.08	474+00
BL-29	1455.72	9.60	1446.32	478+00
BL-30	1460.60	9.60	NFWE	483+00
BL-31	1469.14	9.60	NFWE	489+00
BL-32	1475.84	8.08	NFWE	494+00
BL-33	1480.11	9.60	NFWE	498+00
BL-34	1417.32	5.03	1415.82	339+00
BL-35	1420.37	6.55	1418.27	349+00
BL-36	1435.61	6.55	1433.91	404+00
BL-37	1440.48	6.55	1438.98	451+00
BL-38	1514.25	12.34	NFWE	503+00
BL-39	1530.10	9.75	NFWE	511+00
BL-40	1527.66	12.34	NFWE	521+00
BR-1	1497.18	3.20	NFWE	18+00
BR-2	1476,45	3.51	NFWE	8+00

Boring Number	SURFACE ELEVATION (M)	BORING DEPTH (M)	GROUNDWATER ELEVATION	STATION
BR-3	1469.14	1.98	NFWE	3+00
BR-4	1475.23	8.08	NFWE	15+00
BR-5	1470.36	3.51	NFWE	10+00
BR-6	1466.09	1.98	NFWE	6+50
BR-7	1466.09	3.51	NFWE	22+00
BR-8	1458.77	3.51	NFWE	18+00
BR-9	1456.33	3.51	NFWE	15+00
BR-10	1447.80	3.51	NFWE	5+50
BR-11	1429.51	3.51	1427.81	3+00
BR-12	1431.04	6.55	1429.24	8+00
BR-13	1430.43	1.98	NFWE	1+00
BR-14	1430.43	3.51	NFWE	4+00
BR-15	1431.65	4.72	NFWE	12+00
BR-16	1430.73	3.51	NFWE	9+00
BR-17	1430.12	3.51	NFWE	4+00
BR-18	1447.19	3.51	NFWE	5+00
BR-19	1446.58	3.51	1445.38	2+00
BR-20	1417.32	3.51	1415.62	20+00
BR-21	1414.27	3.51	1412.77	6+00
BR-22	1412.75	3.51	1411.55	1+00
BR-23	1412.75	3.51	1410.65	2+00
BR-24	1413.06	3.51	1412.16	6+00
BR-25	1414.88	3.51	1413.98	10+00
BR-26	1414.27	3.51	1412.57	10+00
BR-27	1413.05	3.51	1411.55	5+00
BD-1	1444.75	15.70	NFWE	144+00
BD-2	1444.75	15.39	NFWE	138+50
BD-3	1444.14	15.70	NFWE	135+00
BD-4	1444.75	15.54	NFWE	130+00
BD-5	1444.14	15.70	NFWE	125+00
BD-6	1409.70	9.45	1408.20	254+00
BD-7	1409.70	9.45	1408.20	253+00
BD-8	1409.70	9.60	1408.50	254+00
BD-9	1417.32	12.65	1415.92	343+86

Boring Number	SURFACE ELEVATION (M)	BORING DEPTH (M)	GROUNDWATER ELEVATION	STATION
BD-10	1417.62	12.65	1416.92	344+60
BD-11	1417.62	12.65	1415.82	349+83
BD-12	1417.62	12.65	1416.12	350+89
BD-13	1417.02	12.65	1415.32	337+00
BD-14	1417.32	12.65	1415.82	337+90
BD-15	1410.00	9.60	1408.80	254+75
BD-16	1410.31	9.60	1407.91	262+50
BD-17	1410.31	9.60	1408.21	262+50
BD-18	1410.61	9.60	1408.81	277+50
BD-19	1410.61	9.60	1408.81	227+50
BD-20	1410.61	9.60	1409.11	282+50
BD-21	1410.61	9.60	1409.41	303+50
BD-22	1410.61	9.60	1409.41	308+50
BD-23	1411.22	9.45	1409.42	312+80

NFWE=No free water encountered

BL=Line boring

BR=Ramp boring

BD=Drainage boring

3.3 Liquefaction

Liquefaction is a phenomena where soils undergo a sudden loss of strength as a result of increased pore pressure induced by ground shaking. Soil conditions considered most susceptible to liquefaction are saturated sands, coarse silts and gravel with little or no fines.

The boring logs at Arrowhead Drive (Borings BS-03 & BS-04), at Northgate Lane (Borings BS-05 & BS-06), and at Emerson Drive (Borings BS-07 & BS-08) (Appendix D) indicate that these sites are underlain by material consisting of loose to medium dense sands, silty sands, and sandy silts. The depth to groundwater is approximately 1.5 meters. Because of the shallow groundwater at these structure locations, these structures were analyzed for liquefaction. The liquefaction analyses were performed according to the method proposed by Seed et. al. (1983) using the field SPT N-values. We have performed liquefaction analyses for the Design Level Earthquake (DLE) which is defined as a ground motion having 10 percent probability of exceedance in 50 years. Assuming Magnitude 6.9 associated with the Genoa fault and a peak



ground acceleration of 0.4g, the results of our analyses indicate that some of the layers consisting of sands and silty sands will likely liquefy under the design level earthquake. These liquifiable soil layers for the design level of ground shaking within typical borings are summarized in Table 3.

TABLE 3

DEPTH OF LIQUEFACTION AND UNDRAINED RESIDUAL STRENGTH

	Depth (m)			iā.	Undrained
Boring	From	То	Thickness (m)	(N ₁) _{60-cs} *	Residual Strength (kN/m²)
BS-03	1.5	3.0	1.1	15	0.31
BS-04	1.5	3.0	1.8	12	0.18
BS-05	4.9	5.8	1.5	22	0.67
BS-07	8.5	10.1	2.7	25	>0.7

^{*} For the definition of $(N_1)_{60-es}$, see Seed and Harder (1990).

If liquefaction occurs, foundations resting on or within the liquefiable layers may undergo settlement. This will result in reduction of foundation stiffness and capacities. In some cases, the soil layer may liquefy but the residual shear strength may still be enough so that the earthquake induced settlements will be negligible. Therefore, we have also estimated the residual strengths for each zone of liquefaction. The residual strengths were estimated based on the data presented by Seed and Harder (1990) in which $(N_1)_{60-cs}$ is the SPT penetration resistance in blows per 300 mm corrected for overburden pressure, applied energy to the drill rods, and fines content.

3.4 Earthquake Induced Settlement and Ground Surface Disruption

During an earthquake, saturated sands tend to settle and densify as a result of pore pressure dissipation. A layer may or may not liquefy, nevertheless, it can experience settlement during an earthquake. Another major concern during an earthquake is some form of ground surface disturbance or ground failure. The ground failure can be in the form of sand boils, small ground fissures, ground oscillation such as buckled pavements, curbs, broken pipelines, etc., and lateral ground displacement. One of the major reasons for ground surface disruption is insufficient cover thickness of non-liquefiable layer over a liquefiable layer (Ishihara, 1985; Youd and Garris, 1995).



We have estimated the earthquake induced settlement based on the method proposed by Tokimatsu and Seed (1987). The ground surface disruption has been estimated using the criteria proposed by Ishihara (1985). The results of our settlement analysis as well as potential for ground disruption for each boring is presented in Table 4. The results of our analysis show that the Arrowhead Drive site can experience settlements on the order of 25 to 35 mm during a seismic event. However, the earthquake induced settlements will be minimal at Northgate Lane and Emerson Drive. The settlement amount may increase at the Arrowhead Drive location in the event the sand manifests as sand boils or the groundwater table is higher than assumed in the analysis. Also, ground surface disruption is likely to occur only at the Arrowhead Drive location during a seismic event. This may result in an increase in settlement locally beneath footings.

TABLE 4
EARTHQUAKE INDUCED SETTLEMENTS

Boring	Settlement (mm)	Ground Surface Disruption
BS-03	25	Yes
BS-04	30	Yes
BS-05	13	No
BS-07	0	No



4. CONCLUSIONS

The following conclusions are based on the data collected during this assessment <u>and are subject</u> to the limitations stated in this report. These conclusions may change if additional information becomes available. Based on the results of our study, no severe soil or groundwater constraints were observed which would preclude the planned construction. The following is a summary of our conclusions.

- Site soils encountered in our field investigation generally consist of sandy soils with occasional interbedded clay zones. The near-surface sandy soils tend to be loose to medium dense, increasing in density with depth. The clay zones, where encountered, tend to be medium stiff to stiff and of moderate plasticity.
- Groundwater was encountered at depths as shallow as 1-1/2 meters below the existing ground surface south of Arrowhead Drive. The shallow groundwater may adversely impact construction in some areas of the project south of Arrowhead Drive.
- Two potential fill borrow areas were explored as part of our investigation at Duck Hill and Edmonds Drive. The Duck Hill area is composed of decomposed granite (DG), and the Edmonds Drive area composed of silty sands. Both areas are deemed to be adequate fill sources; however some blending of the silty fine sand portions of the south end of the Edmonds area must be performed to produce a good quality fill. We estimate that a 50-50 blend of the fine sand at the south end of Edmonds with the DG from the Duck Hill area or the northern Edmonds area will provide a suitable fill soil.
- We analyzed embankment stability assuming a material internal angle of friction of 34 degrees (as requested by NDOT) and a material internal angle of 38 degrees (based empirical values provided in Table 4-5, Publication No. FHWA-HI-97-013 for dense materials). Our results indicate slopes of inclinations of 2H:1V or less and up to 9.1 meters in height are stable under static and seismic conditions for materials with internal angles of friction of 34 to 38 degrees. Our results also indicate slopes constructed at a batter of 1.5H:1V are unstable during a design level seismic event for a cohesionless soil with an internal angle of friction of 34 degrees. We recommend that permanent fill be constructed to a maximum slope of 2H:1V.
- Our stability analysis performed using the XSTABL5.2 computer program indicates low factors of safety for abutment foundations supported within the embankments during a design seismic event. We recommend that all bridge structures be founded



on deep foundations. Recommendations for driven pipe piles, sizes PP305, PP406 and PP457 are provided.

- Resitivity testing indicates potentially corrosive to moderately corrosive conditions at the bridge locations. Carbon steel corrosion rates are anticipated to be on the order of 12 μm/yr. Assuming a design life of 100 years for the proposed bridge abutments and a uniform loss model, steel pipe piles will have an overall sacrificial wall thickness loss of less than 1.5 mm. We recommend a sacrificial wall thickness of 1.5 mm be used in determining the required pipe pile wall thickness.
- The potential for liquefaction of the site soils and the resultant settlement are expected to be minimal.

Specific recommendations for project design and construction including mitigation of potential problems described above are presented in Section 5.0.



5. RECOMMENDATIONS

5.1 Standard Specifications

Specifications or Standard Specifications as referenced to in this report, mean the "Standard Specifications for Road and Bridge Construction, State of Nevada, Department of Transportation." 1996 Edition.

5.2 Site Clearing and Preparation

Prior to construction, surface vegetation and organic soils should be stripped and removed from the alignment or stockpiled for use in non-structural areas. Clearing and grubbing should be performed in accordance with Section 201 of NDOT Standard Specifications for Road and Bridge Construction. Although it will vary considerably along the project length, it appears 150 mm can be used as a reasonable estimate for average depth of stripping. Deeper stripping/grubbing of organic soils, tree roots, etc., may be required in localized areas. Tree root balls should be removed and the resulting voids backfilled with adequately compacted backfill soil. All man-made debris and improvements including structures such as foundations, dump fills and trash should be removed from the alignment. Brush may be disposed of beneath fill slopes of roadway embankments provided the embankment height is greater than one meter and NDOT requirements are followed.

The geotechnical engineer should be present during stripping and surface preparation operations to observe stripping and grubbing depths, and to evaluate whether buried obstacles such as underground utilities, wells, and foundations are present. Special care should be exercised in evaluating whether loose utility backfills exist which could adversely affect the planned pavements and structures. Any existing wells should be abandoned in accordance with local requirements. Excavations resulting from removal operations should be cleaned of all loose material and widened as necessary to permit access to compaction equipment.

Dust control will be the responsibility of the contractor. A dust control plan should be prepared by NDOT or the contractor prior to the start of grading.



5.3 Earthwork

5.3.1 Fill Sources

Two areas of possible fill source were evaluated as part of this investigation, the area at the north end of the project (Duck Hill) and the area south of U.S. 50 along Edmonds Drive. Based on the borings and laboratory tests on samples from these areas, we have developed the following conclusions and recommendations:

- The potential fill source at Duck Hill consists of relatively clean sand or decomposed granite (DG) (see Appendix C for boring logs and laboratory test results) and is an acceptable borrow source. The majority of the site soils including topsoil contain less than 25% fines. Any layers with excessive fines are anticipated to be limited and should be readily mixed during construction to obtain an acceptable fill material. The material in this area can be excavated with conventional earthmoving equipment. An occasional large boulder may be encountered which will require special handling.
- The potential fill source along Edmonds Drive south of the Lompa Ranch is also a viable borrow source. The soil types in this area are generally sandy. Toward the south end of the area explored as part of this study (see Appendix E for boring logs and laboratory test results), the silt content and the fineness of the sandy soils encountered in our borings resulted in fairly low R-Value test results. Some special handling of some of these fine silty sand zones, such as mixing with other sandy soils or the DG from Duck Hill, may become necessary. We recommend a 50-50 mixture of these fine sandy soils with either DG from the Duck Hill area or the more sandy soils of the northern portions of the Edmonds area. We believe this mixture will provide a suitable fill material. Alternating loads of these materials on grade with mixing as part of the filling and compaction process should provide suitable mixing. It is not expected that this will be a major construction issue if these soils are chosen as borrow sources.

5.3.2 General Site Grading

Site preparation and grading should conform to the requirements contained in this report and in the standard specifications. We anticipate that site grading can be performed with conventional



earthmoving equipment. Prior to fill placement, the exposed native soils should be scarified to a minimum depth of 150 mm, moisture conditioned within 2% of optimum moisture, and compacted to a minimum of 90% relative compaction in accordance with the AASHTO T180 compaction test method. Fill soils, which have been similarly moisture conditioned can then be placed and compacted to the designed lines and grades.

Shallow groundwater and the percentage of fines within near surface soils at Highway U.S. 50 and Arrowhead Drive may dictate the need for soil stabilization prior to the placement of embankment fill. Suitable subgrade support can be accomplished with a geogrid reinforced aggregate raft or with the creation of a construction platform using pit run gravel. The actual design will depend on conditions encountered during construction. In general, with the exception of shallow groundwater at some locations, we do not believe that the near surface soils are unstable or pose major construction difficulties.

5.3.3 Recommended Permanent Slope Angles

Although no borrow source(s) has been formerly designated for construction of embankments for the Carson Freeway, material properties similar to the soil conditions encountered in the Duck Hill and Edmonds Drive areas can be expected. Based on these conditions, we have conducted slope stability analyses for the conditions shown in Table 5. We have analyzed embankment stability assuming a material internal angle of friction of 34 degrees (as requested by NDOT) and also assuming a material internal angle of 38 degrees (based on empirical values provided in Table 4-5, Publication No. FHWA-HI-97-013 for dense materials). Based on laboratory results, fill materials were assumed to have an average cohesion value of 6 kPa. The embankment stability was also analyzed assuming a non-cohesive sand fill.

The stability analyses were performed using the XSTABL5.2 computer program, which analyzes circular and/or block failure surfaces and their attendant Factor of Safety (FS) by the modified Bishop or Janbu methods. The program analyzes two-dimensional cross-sections of the slope using available information on the subsurface structure, strength of the various earth materials, surcharge or seismic loading conditions, and groundwater conditions. The computed FS is the ratio of forces tending to resist movement to the forces tending to drive movement. A FS less than 1 suggests the slope is unstable. From a practical perspective, a FS of at least 1.5 is used as an acceptance criteria for the static load case, and a minimum factor of 1.1 is used for the seismic case. The results of these analyses are shown on Plates 1 through 4a for static and seismic states using both 34 and 38 degrees, at the end of Section 5.0, and are also presented below in Table 5.

TABLE 5

RESULTS OF EMBANKMENT STABILITY ANALYSES

Soil Type	Soil Properties	Embankment Height & Slope	Seismic Coefficients	Factor of Safety for c=0	Factor of Safety for c=6 kPa
Fill (assumed average properties)	$\gamma = 19.63 \text{ kN/m}^3$ $\phi = 34^\circ \& 38^\circ$ $c=0 \text{ kPa} \text{ and } 6 \text{ kPa}$	9.1 meters	4		
Native (average measured properties)	$\gamma = 18.69 \text{ kN/m}^3$ $\gamma = 20.26 \text{ kN/m}^3$ (saturated) $c = 9 \text{ kPa}$ $\phi = 31^\circ$ Water level @1.5m				
	For φ = 34°	2H:1V 1.5H:1V	0V.0H 0V.0.175H 0V.0H	1.62 1.06 1.16	2.15 1.41 1.73
	For φ = 38°	2H:1V 1.5H:1V	0V.0.175H 0V.0H 0V,0.175H 0V.0H	0.80 1.88 1.10 1.34	1.25 2.25 1.50 2.04
		1.211.1 v	0V,0.175H	0.93	1.42

Our results indicate the slopes are globally stable under seismic and static loading conditions for construction slopes of 1.5H:1V and 2H:1V constructed with fill material with an average cohesion value of 6 kPa. Embankments constructed with non-cohesive materials can be expected to experience shallow instability (surface sloughing). In areas where clean sand is placed on the surface embankments, periodic maintenance should be anticipated.

Drainage and runoff primarily affect satisfactory slope performance. Care must be taken that drainage is not directed to flow over slope faces. Interceptor (brow) ditches should be



constructed at the tops of slopes in order to collect and divert runoff which otherwise would flow over slope face. Slope faces should be protected against erosion resulting from direct rain impact and melting snow. Consideration should be given to permanent measures such as concrete, geosynthetics and vegetation.

Clay layers were encountered in some of our line borings. An analysis of these clay layers indicates that long-term settlement should not be an issue in the performance of fill slopes along the alignment. The clay layers encountered are relatively thin and stiff to very stiff. Relatively loose surface sandy soils are expected to experience the majority of their settlement during construction.

5.3.4 Embankment Fill Stability Supporting Shallow Spread Foundations

We have analyzed fill stability assuming bridge abutment foundations would be supported within the embankment fill. The abutment foundations were assumed to be 2.5 meters wide, with an embedment depth of 1.5 meters and a design bearing pressure of load of 191.5 kPa. We analyzed the embankment stability assuming a material internal angle of friction of 34 degrees and 38 degrees, a non-cohesive material and a material cohesion of 6 kPa and an embankment inclination of 2H:1V. Each case was analyzed for static and seismic loading conditions. The results of these analyses are shown on Plates 5 through 8 and summarized below in Table 5a.



 $\underline{TABLE\ 5a}$ Results of Embankment Stability Supporting Abutment Foundations

Soil Type	Soil Properties	Embankment Height & Slope	Seismic Coefficients	Factor of Safety for c=0	Factor of Safety for c=6 kPa
Fill	$\gamma = 19.63$	9.1 meters			
	kN/m³	!			
(assumed	c=6 kPa				
average					
Properties)	φ = 34° &				
22.	38°				
 Native	$\gamma = 18.69$				
	kN/m³				
(average	$\gamma = 20.26$				
measured	kN/m³				
	(saturated)				
Properties)	c= 9 kPa		90		İ
	ψ = 31°				
	Water				
	level@1.5m	E. 145-711-			
	For φ = 34°	2H:1V	H0,V0	1.30	1.50
			0V,0.175H	0.87	1.13
	For φ = 38°	2H:1V	0V,0H	1.53	1.59
			0V.0.175H	1.00	1.20

Our results indicate the slopes are globally stable under static and seismic conditions for slopes materials with a minimum cohesion of 6 kPa and constructed at a maximum batter of 2H:1V. However, for a cohesionless fill material with an internal angle of friction 34 degrees the factor of safety is insufficient for both the static and seismic cases. Cohesionless materials with an internal angle of friction of 38 degrees are also considered unstable during a design earthquake event. Deep foundations are recommended for the support of bridge abutment foundations.



5.3.5 Temporary Trench Excavation and Backfill

It appears that excavations for footings and utility trenches can be readily made with either a conventional backhoe or excavator in either native soil or compacted imported fill. We expect the trench walls for excavations less than I-1/2 meters deep to stand nearly vertical without significant sloughing provided that proper moisture contents are maintained. The contractor should evaluated and verify the stability of each trench prior to occupation by construction personnel. At some locations shoring or sloping of trench walls may be necessary to protect personnel and provide temporary stability. All excavations above the water table should comply with current OSHA safety requirements for Type B soils, which allows for a maximum vertical slope of 1H:1V for excavations less than 6 meters in height (Federal Register 29 CFR, Part 1926). In areas of shallow groundwater, or where water is freely seeping from trench walls, excavations should comply with OSHA safety requirements for Type C soils, which allows for a maximum allowable slope of 1.5H:1V. Shoring and/or dewatering may be required. Sloping or benching of excavations greater than 6 meters in depth will need to be evaluated on a case by case basis.

During wet weather, runoff water should be prevented from entering excavations. Water should be collected and disposed of outside the construction limits. Heavy construction equipment, building materials, excavated soil, and vehicular traffic should not be allowed within a distance of one-third the slope height from the top of any excavation. Backfills for trenches or other excavations within pavement areas, beneath concrete slabs, and adjacent to foundations should be compacted in 150 to 200 mm layers with mechanical tampers. Jetting and flooding should not be permitted. We recommend all backfill be compacted to a minimum compaction of 90% of the maximum dry density as determined by Nevada Test Method T101.

5.3.6 Soil Shrinkage

Shrinkage will generally occur where native soils are excavated and placed in compacted fills. Also, shrinkage can be expected in the Duck Hill and Edmonds Drive borrow areas. We estimate that shrinkage will range from 5% to 15% as a result of volume change due to compactive effort. The oversize materials may also contribute to loss of volume where screening is required prior to placement in structural fill areas or trench backfill. Provisions should be made for replacing the volume of oversize materials with additional imported structural fill material.



Alternately, oversize rocks up to 300 mm in diameter may be placed in deep fill areas, at depths greater than 1 meter below utilities and foundations. Incorporation of oversize rocks into structural fills will require special construction techniques to prevent nesting. The geotechnical engineer or his representative should observe all procedures for placement of oversize materials within fills.

The shrinkage estimates discussed in this report are based on a comparison of in-place density tests and estimated maximum densities. The actual volume of soils tested is minuscule when compared to the volume of materials, which will be involved in earthwork operations. NDOT and the Contractor are warned against relying on these estimates for bidding purposes or quantity calculations. In general, the Contractor's past experience in the area is a better predictor of volume changes during earthwork operations.

5.4 Foundations

5.4.1 Alternative Foundation Types

Our stability analysis performed using the XSTABL 5.2 computer program indicates low factors of safety for abutment foundations supported within the embankments during a design seismic event. We recommend that all bridge structures be founded on deep foundations. Recommendations for driven pipe piles. sizes PP305, PP406 and PP457 are provided below Recommendations for conventional foundations for retaining walls are provided in Section 5.5.2.

5.4.2 Deep Foundation Design Parameters

Included in Plates 9 through 29 at the end of Section 5.0, are design charts for ultimate capacities with depth for single driven closed end pipe piles, designations PP305, PP406, and PP457. Design charts have been provided for each bridge location and are based on conditions encountered during our subsurface exploration. In accordance with AASHTO Standard Specifications for Highway Bridges, 16th edition, 1996, Section 4.3.2, ultimate bearing capacities have been limited to six meters above the bottom of our subsurface exploration at each location. The ultimate capacity line has been extended below the six meter limit in each chart to reflect an inferred ultimate bearing which could be achieved; however, additional subsurface exploration is required to verify this extrapolation.

For piles driven through embankment fill, capacities for PP305, PP406, and PP457 pile types may be increased by 7 kN, 15 kN and 21 kN, respectively. Piles driven through embankment fills should extend to a minimum of three meters into the original ground surface unless a competent bearing layer is encountered at a shallower depth. The maximum size rock within the embankment fill should not exceed 150 mm in areas where pile foundations are to be constructed.

In accordance with AASHTO Standard Specifications for Highway Bridges, 16th edition, 1996, Table 4.5.6.2A, we recommend that allowable pile capacities be calculated using a factor of safety of 2.75. This factor of safety value is consistent with the use of the 1997 version of the Wave Equation Pile Analysis (WEAP) program. For construction control which might include a static load test, a factor of safety of 2.0 may be used.

We recommend that pipe piles be Grade 2 steel (ASTM A252-96) with a minimum wall thickness of 7.9 mm. The actual wall thickness will need to be determined by the structural engineer. A sacrificial wall thickness of 1.5 mm should be considered when determining the actual wall thickness to account for possible corrosion of the steel piles by the native site soils (see Appendix D, Plates D-40 and D-41).

Wave equations relating expected driving resistance with depth for each bridge location are provided below in Table 6. Driving resistances were calculated assuming various Delmag diesel hammers and wood block cushions. Maximum allowable driving resistance is estimated to be approximately 217 MPa or 0.9F_x.

TABLE 6
SUMMARY OF WEAP ANALYSIS

Boring Location	Pile Length (m)	Pile Type	Ultimate Pile Capacity (kN)	Required Yield Strength (MPa)	Hammer Description	Blows per Meter	Driving Stress (MPa)	Allowable Driving Stress (MPa)
BS-I	14	PP305 7.9mm wall	1229	241	Delmag D 36-32	186	200.4	217
BS-I	14	PP406 9.5mm wall	1966	241	Delmag D 62-22	283	191.9	217
BS-1	14	PP457 9.5mm wall	2222	241	Delmag D 80-23	168	208.1	217
BS-4	18	PP305 7.9mm wall	1229	241	Delmag D 36-32	170	196.8	217



Boring Location	Pile Length (m)	Pile Type	Ultimate Pile Capacity (kN)	Required Yield Strength (MPa)	Hammer Description	Blows per Meter	Driving Stress (MPa)	Allowable Driving Stress (MPa)
BS-4	18	PP406 9.5mm wall	1966	241	Delmag D 62-22	245	188.7	217
BS-4	18	PP457 9.5mm wall	2222	241	Delmag D 80-23	137.7	201.7	217
BS-5	18	PP305 7.9mm wall	1229	241	Delmag D 36-32	152	196.1	217
BS-5	18	PP406 9.5mm wall	1966	241	Delmag D 62-22	234	187.2	217
BS-5	18	PP457 9.5inniwait	2222	241	Delmag D 80-23	132	199.3	217
BS-8	14	PP305 7.9mm wall	1229	241	Delmag D 36-32	185	200.3	217
BS-8	14	PP406 9.5mm wall	1966	241	Delmag* D 62-22	285	191.9	217
BS-8	14	PP457 9.5mm wall	2222	241	Delmag D 80-23	154	205.6	217
BS-10	18	PP305 7.9mm wall	1229	241	Delmag D 36-32	289	200.4	217
BS-10	18	PP406 9.5mm wall	1966	241	Delmag D 62-22	. 468	192.7	217
BS-10	18	PP457 9.5mm wall	2222	241	Delmag D 80-23	244	205.9	217
BS-11	. 18	PP305 7.9mm wall	1229	241	Delmag D 36-32	251	198.4	217
BS-11	18	PP406 9.5mm wall	1966	241	Delmag D 62-22	385	190.6	217
BS-11	18	PP457 9.5mm wall	2222	241	Delmag D 80-23	197	206.7	217
BS-14	15	PP305 7.9mm wall	1229	241	Delmag D 36-32	188	200.5	217
BS-14	15	PP406 9.5mm wall	1966	241	Delmag D 62-22	286	192.4	217
BS-14	15	PP457 9.5mm wall	2222	241	Delmag D 80-23	151	206.0	217

Notes: Hammer efficiency is assumed to be 80 percent. A factor of safety of 2.75 is used for ultimate pile capacities.

Soil parameters for laterally loaded pile analysis are provided in Table 7. Design parameters were estimated based on corrected blow counts, laboratory index test results and recommendations provided in Tables 9-12 and 7-13 of FHWA *Design and Construction of Driven Pile Foundations, Workshop Manual* - Volume 1, December 1996. As noted in Table 7, where cohesionless soils were encountered in our field investigation, soil parameters consisting of friction angles were used in our analyses. Where clayey soils were encountered, values of cohesion and strain at 50 percent of the maximum principal stress were used in the analyses.

TABLE 7
PILE LATERAL LOAD DESIGN PARAMETERS

Structure Location	Elevation (M)	Groundwater Elevation (M)	Dry Density (kN/m³)	Soil Parameters	K (kN/m³)
U.S. 395	1460.1	NFWE (1)			
	1.462.6		19	φ = 32°	24,430
	1462.6		19	φ = 35°	24,430
	1457.1				
	1454.1		21	φ = 35°	24,430
	14,74.1		21	φ = 40°	24,430
	1451.6			,	
Arrowhead	1446.0		20	1 300	0.17100
Drive	1441.0	1441.0	20	φ = 32°	24,430
			11	φ = 32°	16,300
	1430.0		8	$C_u = 80 \text{ kPa}$	136,000 (static)
	[O	$\varepsilon_{\rm u} = 80 \text{ kf a}$ $\varepsilon_{\rm 50} = 0.007$	54.300 (static)
	1427.0			20	
Northgate	1436.5		••		
Lane			20	$C_u = 50 \text{ kPa}$ $\varepsilon_{50} = 0.007$	136,000 (static) 54,300 (cyclic)
	1434.0	1434.0		2 50 - 0.007	54,500 (cyche)
			10	φ = 32°	5.430
	1432.5		10	φ = 32°	16,300
	1428.0			Ψ., 32	10.500
			10	$C_{ii} = 100 \text{ kPa}$	136,000 (static)
	1426.0			$\varepsilon_{50} = 0.005$	54.300 (cyclic)
			10	φ = 36°	33.900
	1417.5				_
Emerson Drive	1433.2		20	φ = 32°	24.420
שוועכ	1432.2	1432.2	20	ψ – 32-	24,430
			10	$C_u = 25 \text{ kPa}$	27.150

		Groundwater			
Structure	Elevation	Elevation	Dry Density	Soil	K
Location	(M)	(M)	(kN/m³)	Parameters	(kN/m³)
-	1430.2			$\varepsilon_{50} = 0.01$	
			10	ф = 33°	16,300
	1424.2		10	$C_u = 50 \text{ kPa}$	136,000 (static)
	1423.2			$\varepsilon_{50} = 0.007$	54.300 (cyclic)
	1418.2		10	φ = 33°	16,300
			10	$C_u = 150 \text{ kPa}$ $\varepsilon_{50} = 0.005$	271.000 (static) 108,500 (cyclic)
	1415.2		10	φ = 34°	16,300
	1414.2			T	10,500
College	1430.1		17	ф=33	24,430
Parkway	1428.0	1428.0	17	ф=33	16,300
	1413.1		16	C _u =70 kPa ε ₅₀ =0.007	136,000 (static) 54,300 (seismic)
	1408.8		18	ф=40	33,900
Carmine Street	1417.3		19		
Sileet	1415.3	1415.3		ф = 33°	24,430
	1401.8		10	φ = 33°	16,300
			10	$C_u = 300 \text{ kPa}$ $\varepsilon_{50} = 0.004$	543,000 (static) 217,000 (cyclic)
	1398.3			246	9
U.S 50	1411.2		14	φ = 30°	24.450
	1409.4	1409.4	16	φ = 35°	16,300
	1406.8		17	φ = 37°	33,900
	1404.7		18	$C_n = 96 \text{ kPa}$ $\varepsilon_{50} = 0.005$	271,000 (static) 108,500 (seismic)
	1403.3		17	ф=40	33,900
	1400.6		17	$C_{ii} = 96 \text{ kPa}$	271,000 (static)



Structure Location	Elevation (M)	Groundwater Elevation (M)	Dry Density (kN/m³)	Soil Parameters	K (kN/m³)
				$\varepsilon_{50} = 0.005$	108,500 (seismic)
	1397.8		17	φ=35	25,100

(1) NFWE = No Free Water Encountered

The recommended driving shoe to be used on pipe piles is presented on Plate 30 at the end of Section 5.0.

5.5 Retaining Structures

5.5.1 Lateral Earth Pressure

Retaining walls are currently planned for seven locations along the proposed alignment. These locations are designated as Hot Springs, Bike Path #1, Emerson and Northgate, Broadleaf East, Bike Path #2, South of Arrowhead, and LV3. Lateral earth pressures will be imposed on all subterranean structures, including retaining walls and foundations. Table 8 presents a list of soil parameters which we recommend for use in the design of these structures.

TABLE 8

LATERAL EARTH PRESSURES

Backfill Angle	Earth Pressure	Earth Pressure Coefficient	Equivalent Fluid Density
0°	Active	0.25	5.0 kN/m ³
	At-rest	0.44	8.8 kN/m ³
	Passive	7.24	144.8 kN/m ³
	Active (K _{AII})	0.56	11.2 kN/m ³
	Passive (K _{PE})	5.95	119.0 kN/m ³
26° (2:1)	Active	0.40	8.0 kN/m ³
	Passive	19.30	392.0 kN/m ³
	Active (K _{AE})	1.36	27.2 kN/m ³
	Passive (K _{pE})	138.86*	2,777.2 kN/m ³
	Friction Coefficient (tan δ)		0.4

Angle of Internal Friction (φ)	34°
Wall Friction Angle (δ)	22°
Unit Weight (γ)	20 kN/m³

^{*}Rotation into 2:1 slope

Earth pressures provided above were calculated in accordance with recommendations provided in the Federal Highway Administration *Earth Retaining Structures*. *Reference Manual (Draft)*, dated May 1998. As noted, an angle of internal friction of 34 degrees is recommended per NDOT's request. Seismic lateral earth pressure design parameters were calculated using the Mononobe-Okabe analysis as outlined in the National Cooperative Highway Research Program Report 343 (*Manuals for the Design of Bridge Foundations, Part 3 – Engineering Manual for Retaining Walls and Abutments*) December 1991.

The at-rest case is applicable for braced walls where rotational movement is confined to less than 0.001 H. If greater movement is possible, the active case applies. These values do not include hydrostatic pressures that might be caused by groundwater or surface water trapped behind a structure. Where backfill is placed against structures such as retaining walls, we recommend that non-expansive, free-draining materials meeting NDOT filter criteria be used in the zone immediately adjacent to the structure to reduce hydrostatic forces. The free-draining material should have a minimum lateral thickness of 600 mm. Alternately, the use of pre-manufactured drainage panels should be considered. Furthermore, adequate drainage of the backfill in the form of subdrains and/or weepholes should be provided at the base of the wall. If weepholes are constructed, they should be on a maximum of three-meter centers vertical spacing, five-meter centers horizontal spacing, and have a minimum diameter of 102 mm. All weepholes should be backed with a minimum of 0.06 cubic meters of Type 2 drain backfill encased in geofabric, Mirafi 160N, 180N, or equal.

General backfill should be non-expansive material conforming to NDOT specifications for granular backfill. The lateral loads computed using the values in Table 8 assume that the non-expansive backfill will extend laterally at least one-half of the wall height. If this condition does not apply, the design values may require revision. This backfill should be compacted to 95% of maximum dry density and within 2% of the optimum moisture content as determined by AASHTO T99. Over-compaction should be avoided as the increased compactive effort will result in lateral pressures higher than those recommended above. Heavy equipment or other



loads should not be allowed in within one-third of the wall height unless planned for in the structural design.

5.5.2 Bearing Capacity

Presented in Plates 31 through 37 at the end of Section 5.0 are our recommended retaining wall foundation bearing pressures and associated estimated settlements as a function of footing width and embedment depth. Plates 31 through 37 are for the seven locations where retaining walls are planned for construction. Bearing Capacities were calculated in accordance with AASHTO Standard Specifications for Highway Bridges, 16th edition, 1996, Section 4.4.7.

Foundations should bottom on firm native soil or compacted structural fill. Any loose soil in the bottom of the footing excavation should be recompacted to at least 95% relative compaction or removed to expose firm unyielding material.

The allowable bearing pressures were calculated using a factor of safety of 3.0 in accordance with AASHTO procedures. The allowable bearing pressures are net values; therefore, the weight of the foundation and backfill may be neglected when computing dead loads.

If seismic loading is evaluated in accordance with AASHTO Standard Specifications for Highway Bridges 16th edition. 1996. Section 3, we recommend using an acceleration coefficient of 0.35g and a Site Coefficient (S) of 1.2 which is applicable to a Type II Soil Profile. The allowable bearing capacity for seismic loading can be determined by multiplying the allowable bearing pressure in Plates 31 through 37 by 3.0 (to determine the ultimate bearing capacity) and dividing by a factor of safety of 1.5.

The data presented on Plates 31 through 37 were developed based on conditions encountered in adjacent borings. The effects of shallow groundwater were considered in the ultimate/allowable bearing values and calculated settlements as presented in the design Plates. Assumed groundwater levels for each location are presented below in Table 9.

TABLE 9

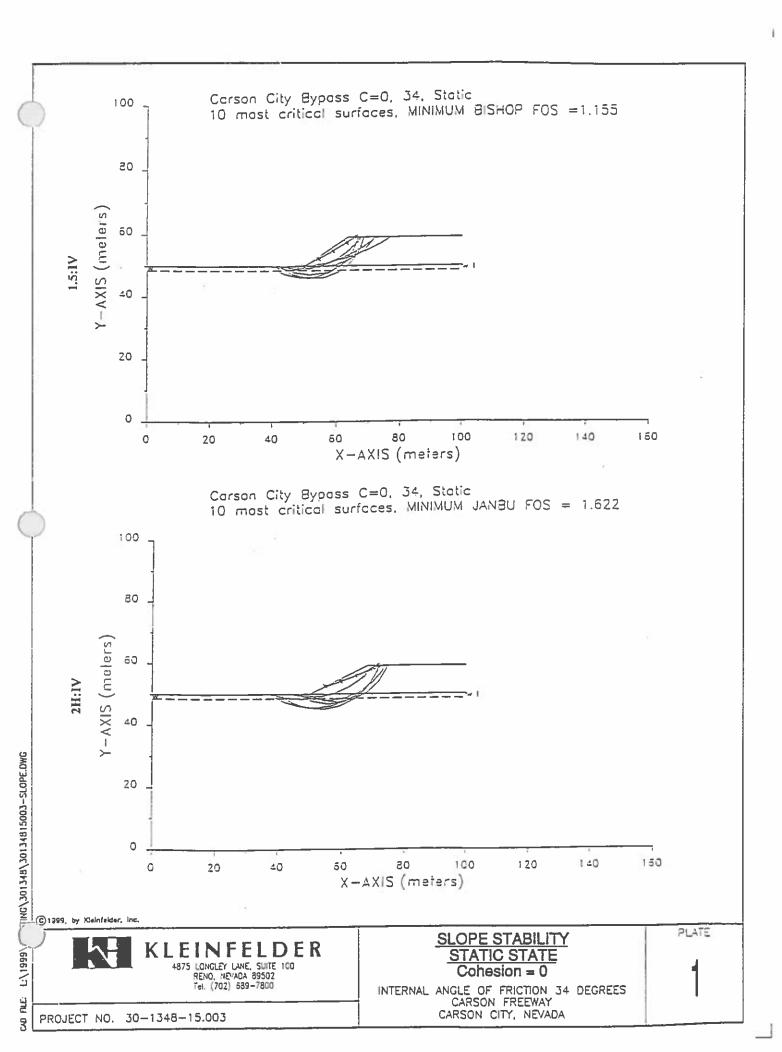
Assumed Groundwater Levels for Retaining Wall Design

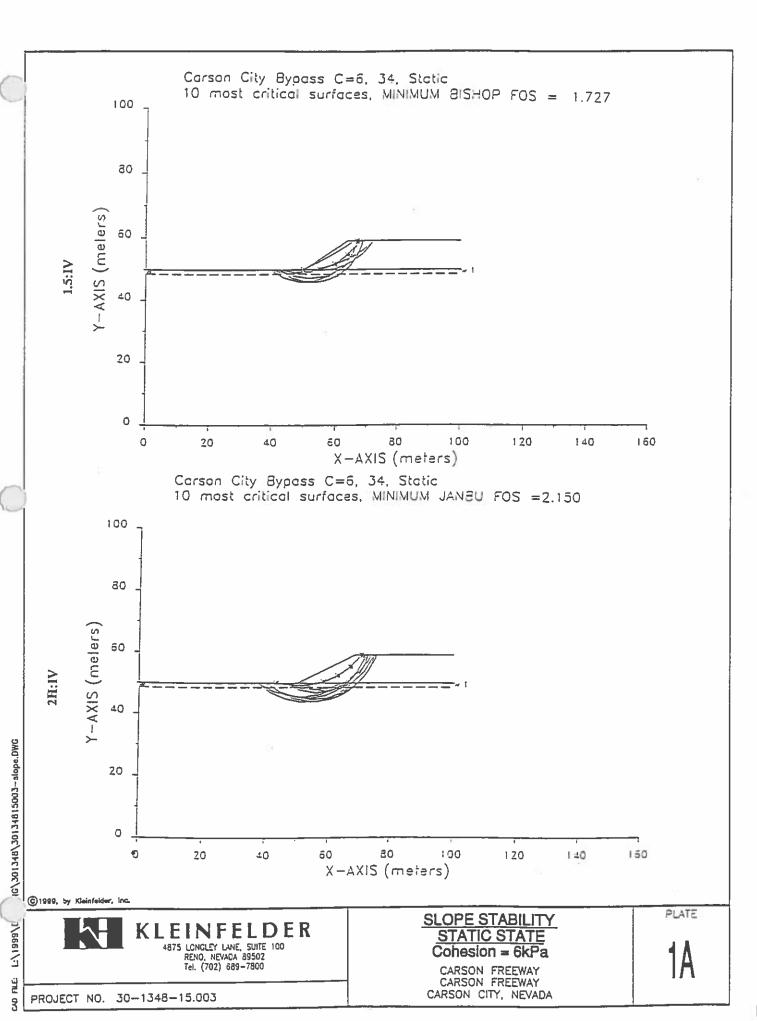
Wall Location	Adjacent Boring	Groundwater Depth, (m)
Hot Springs	BL-15	1.7
Bike Path #1	BL-16	1.7
Emerson and Northgate	BL-17	1.8
Broadleaf East	BL-22	1.4
Bike Path #2	BL-25	1.7
South of Arrowhead	BL-25	1.7
LV3	BL-40	

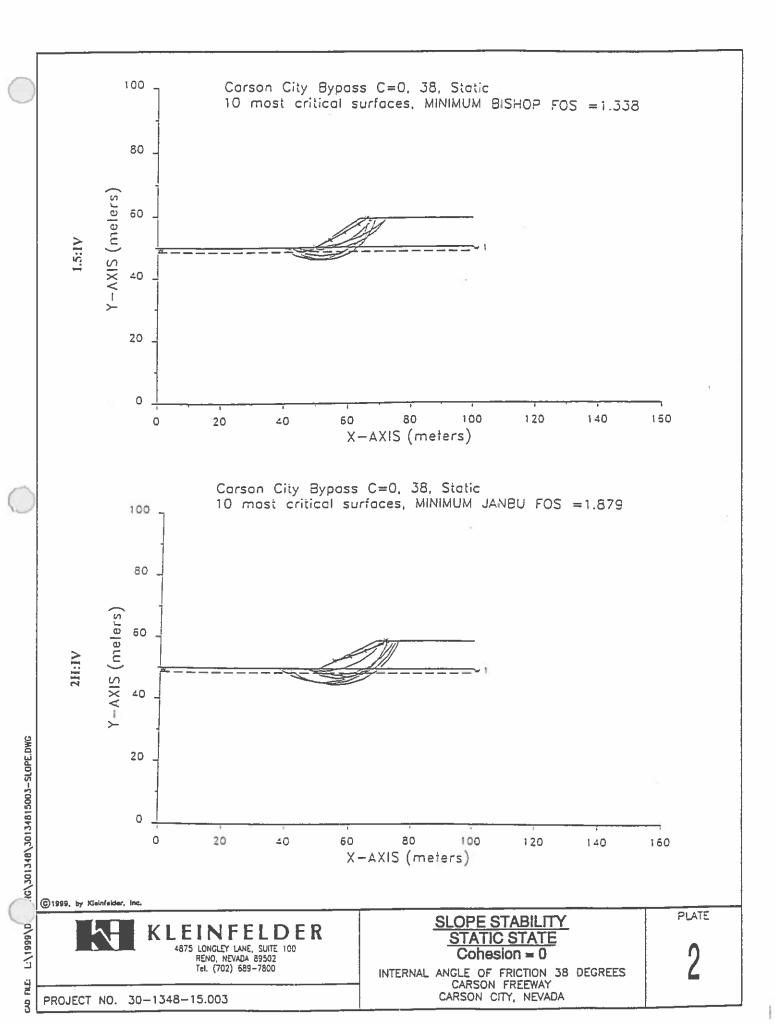
5.6 Steel and Concrete Reactivity

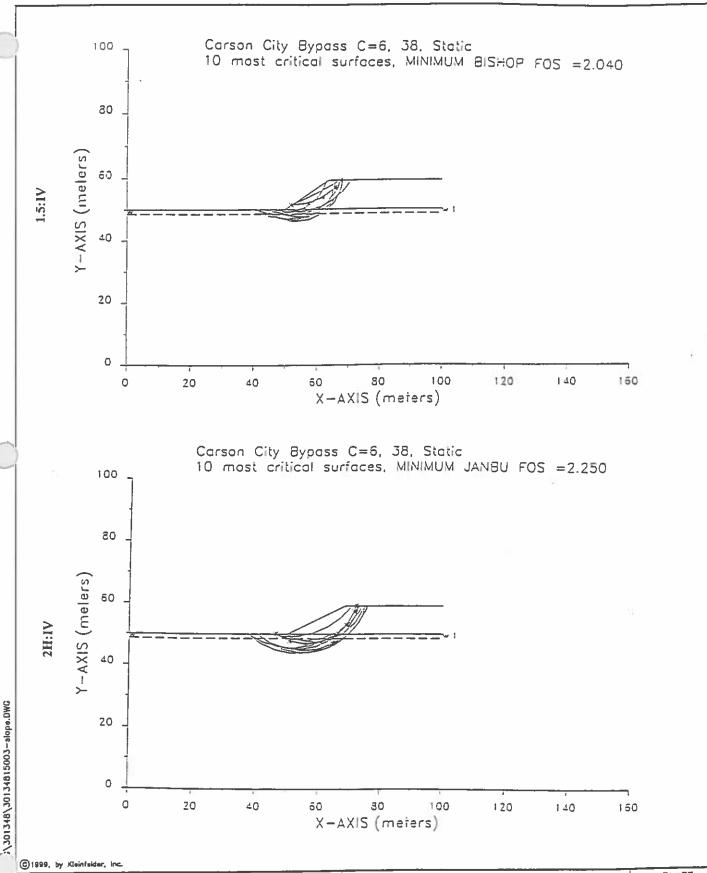
Analytical testing of selected soil samples from structure borings was performed to assess the potential for adverse reactivity with concrete and corrosivity with steel. Soluble sulfate tests were performed to evaluate potential sulfate attack against Portland Cement Concrete. Soluble sulfate contents were observed to be less than 0.02 percent. Therefore, the potential for sulfate attack appears to be negligible and conventional Type II cement may be used according to data furnished by Cement Industry Technical Committee of California and Acculabs, Inc.

Resistivity tests are used as an indication of possible steel corrosion activity. Generally, the lower the native resistivity of the soils, the more likely that galvanic currents may occur and corrosion result. Resistivity values for the near-surface native soils are on the order of 500 to 5000 ohm-cm with a typical value of less than 2000 ohm-cm and; therefore, appear to be corrosive to moderately corrosive where metal will be in contact with native soils. Carbon steel corrosion rates are anticipated to be on the order of 12 μ m/yr per exposed side. Assuming a design life of 100 years for the proposed bridge abutments and a uniform loss model, steel pipe piles will have a sacrificial wall thickness of less than 1.5 mm. We recommend a sacrificial wall thickness of 1.5 mm be used in determining the required pipe pile wall thickness. Consideration should be given to epoxy coating steel reinforcing, since this material could be exposed to the additional corrosive potential from deicing salts during the winter months. The coating thickness should be on the order of 450 μ m.









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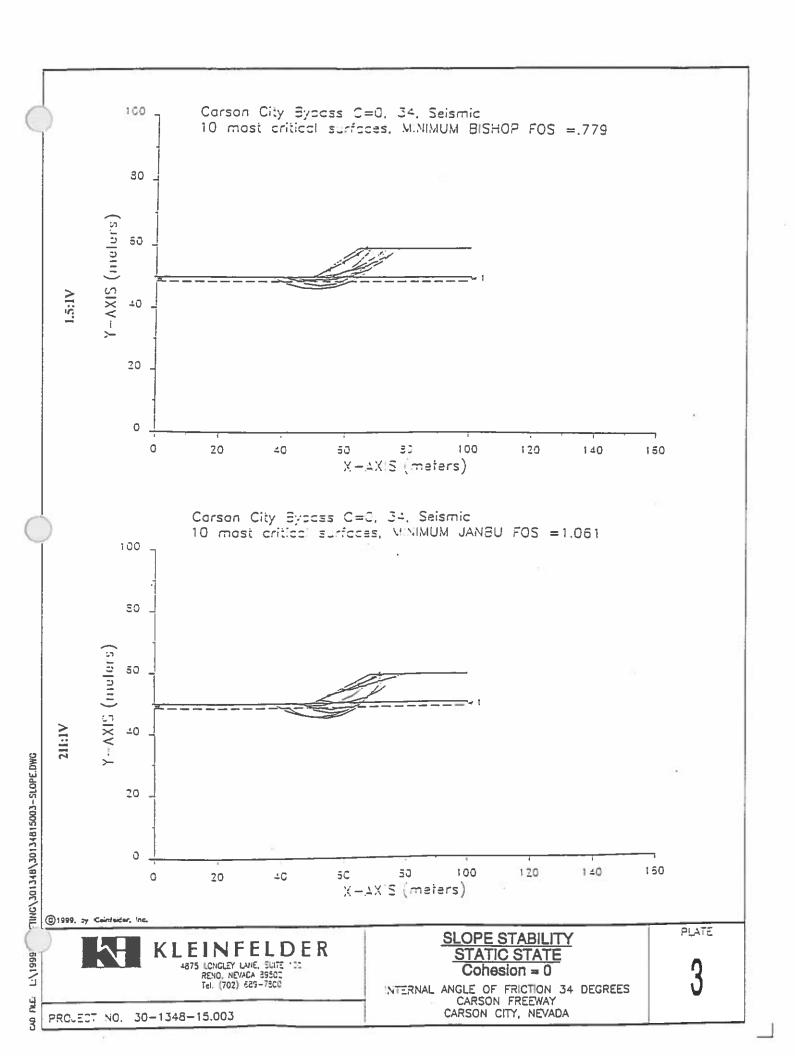
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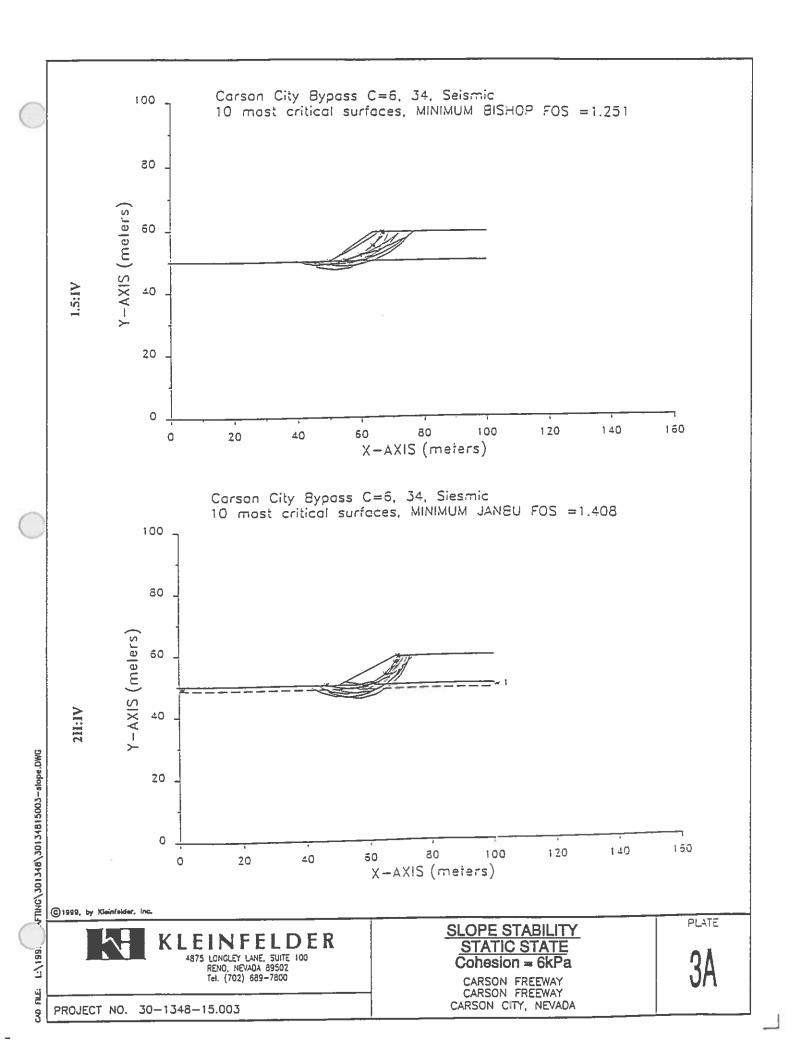
SLOPE STABILITY STATIC STATE Cohesion = 6kPa

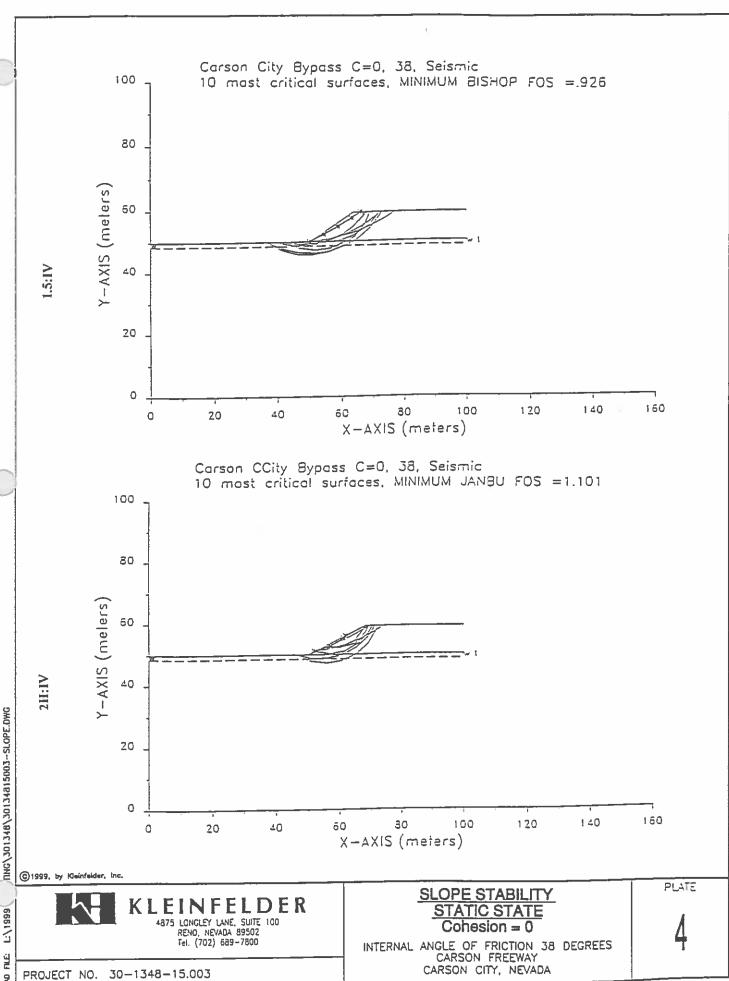
INTERNAL ANGLE OF FRICTION 38 DEGREES
CARSON FREEWAY
CARSON CITY, NEVADA

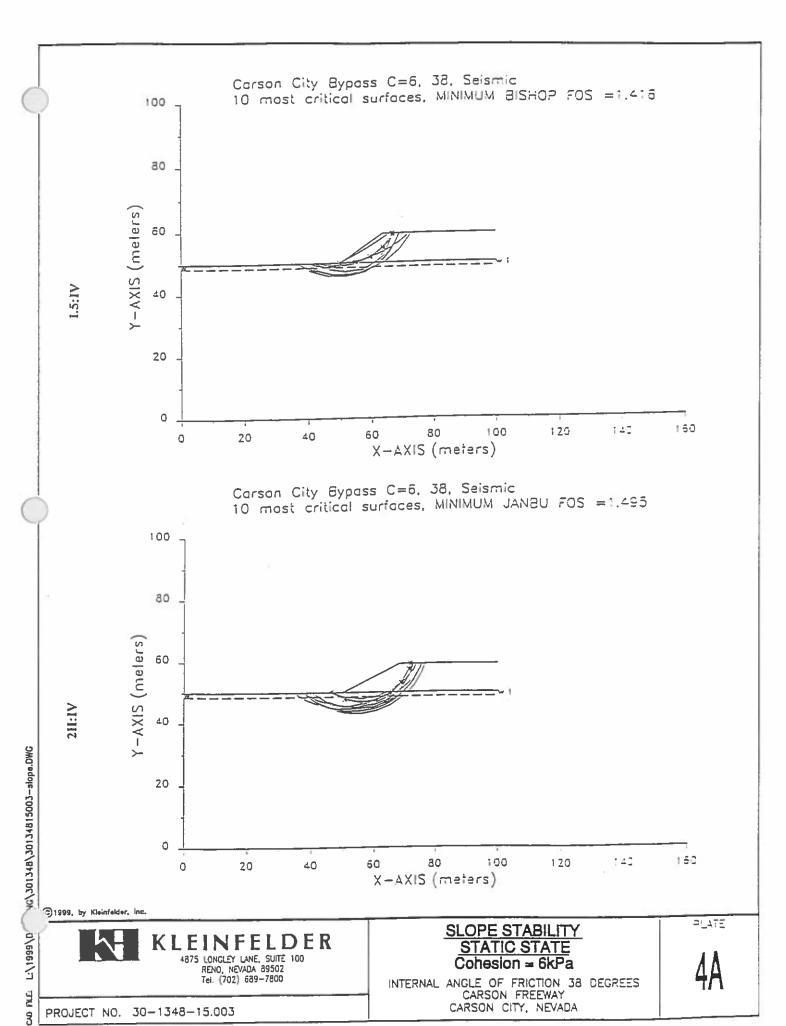
PLATE

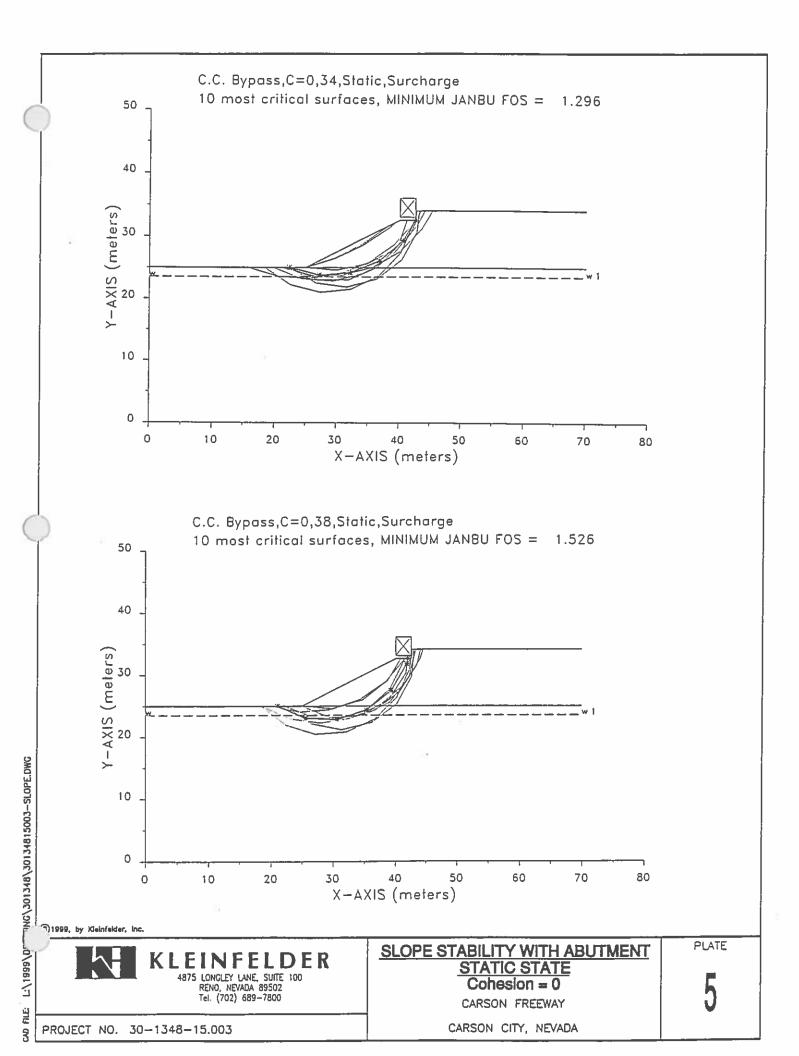
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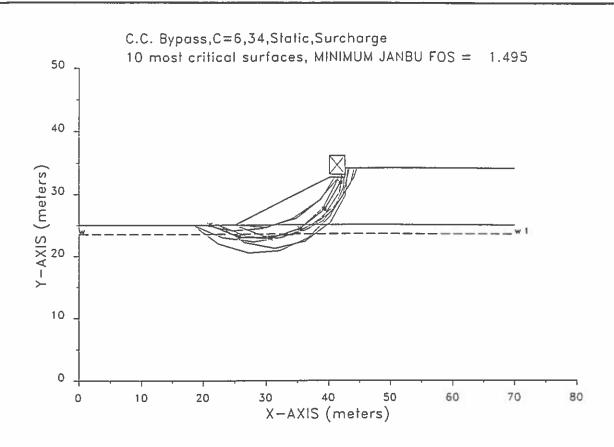


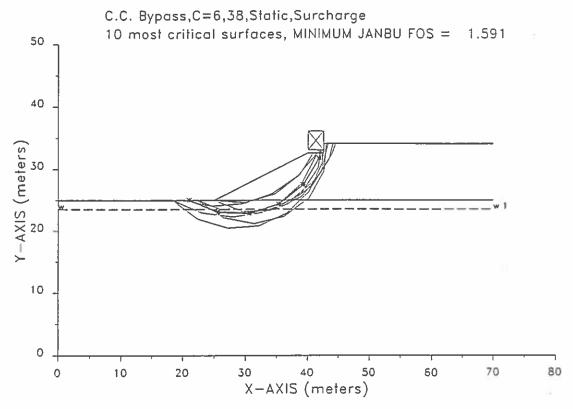












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4875 LONGLEY LANE, SUITE 100 RENO, NEVADA 89502 Tel. (702) 689-7800 SLOPE STABILITY WITH ABUTMENT

STATIC STATE

Cohesion = 6kPa

CARSON FREEWAY

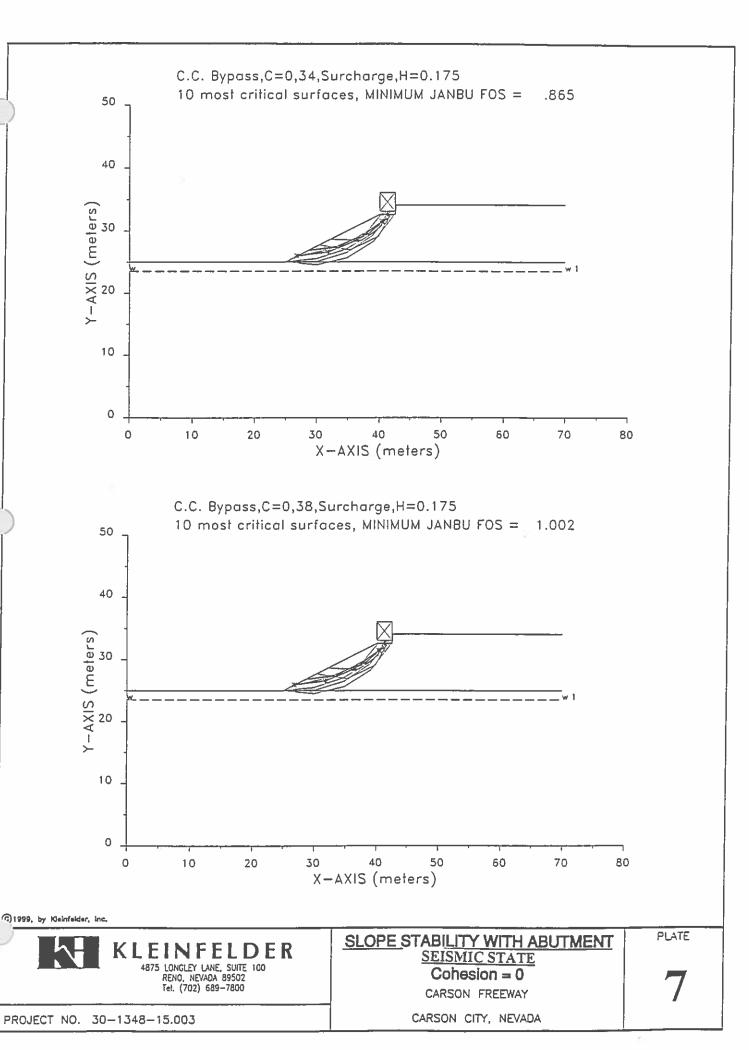
CARSON FREEWAY
CARSON CITY, NEVADA

PLATE

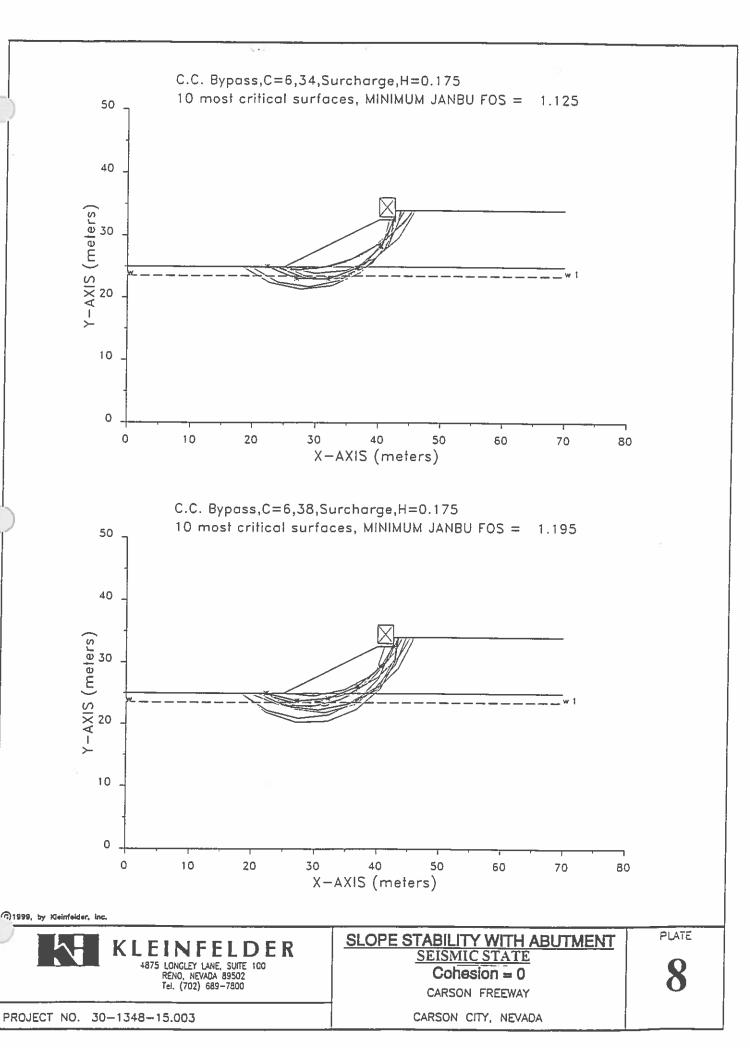
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PILE DESIGN, PP305 **ULTIMATE BEARING CAPACITY** US 395

Inferred Capacity

12

Pipe Pile, PP305, Ultimate Bearing Capacity

Usable Curve

6

3

Depth (m)

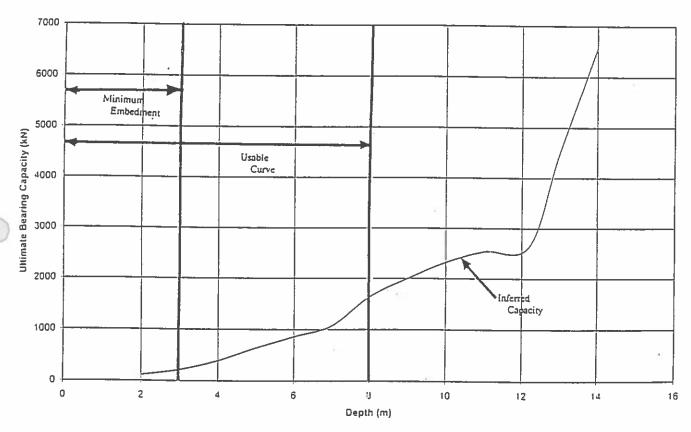
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CARSON FREEWAY

CARSON CITY, NEVADA

PLATE

16



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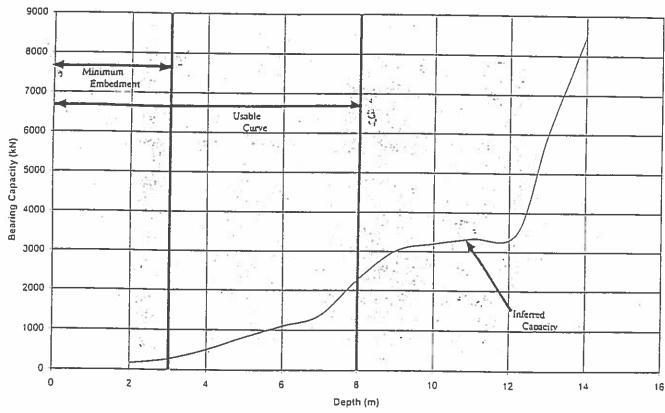
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PILE DESIGN, PP406 **ULTIMATE BEARING CAPACITY US 395**

CARSON FREEWAY

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CARSON FREEWAY

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PLATE

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PILE DESIGN, PP305 **ULTIMATE BEARING CAPACITY** ARROWHEAD DRIVE

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CARSON CITY, NEVADA

PLATE

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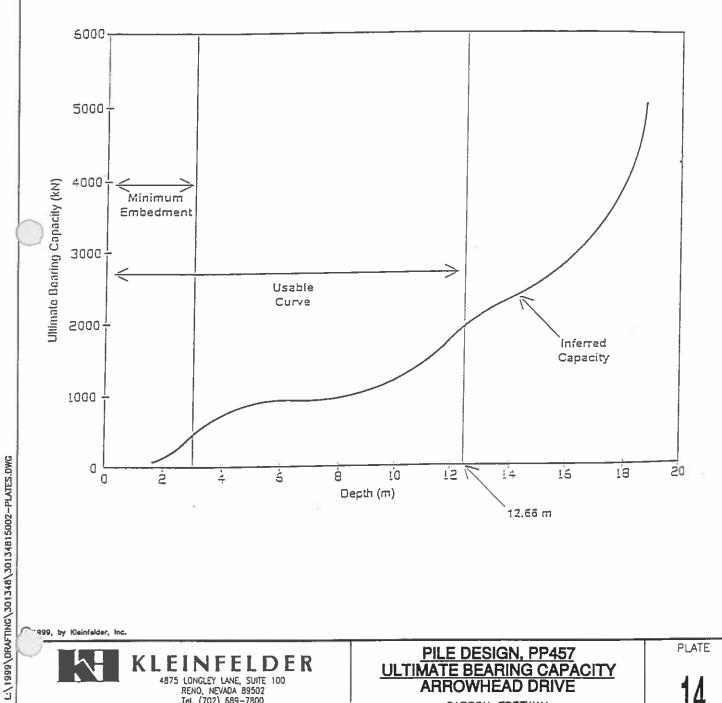
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PILE DESIGN, PP406 ULTIMATE BEARING CAPACITY ARROWHEAD DRIVE

CARSON FREEWAY

CARSON CITY, NEVADA

PLATE





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PILE DESIGN, PP305 **ULTIMATE BEARING CAPACITY** NORTHGATE LANE

CARSON FREEWAY

CARSON CITY, NEVADA

PLATE

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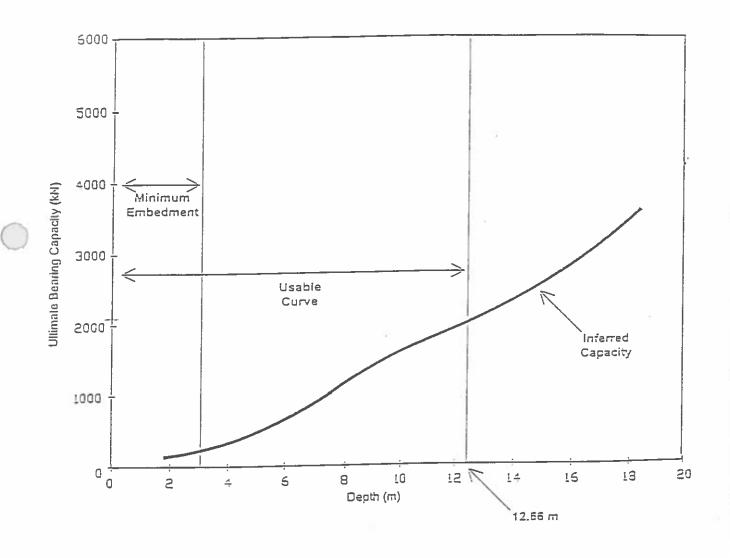


PILE DESIGN, PP406 **ULTIMATE BEARING CAPACITY** NORTHGATE LANE

CARSON FREEWAY

CARSON CITY, NEVADA

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PILE DESIGN, PP457 ULTIMATE BEARING CAPACITY NORTHGATE LANE

CARSON FREEWAY

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PILE DESIGN, PP305 **ULTIMATE BEARING CAPACITY EMERSON DRIVE**

CARSON FREEWAY CARSON CITY, NEVADA PLATE

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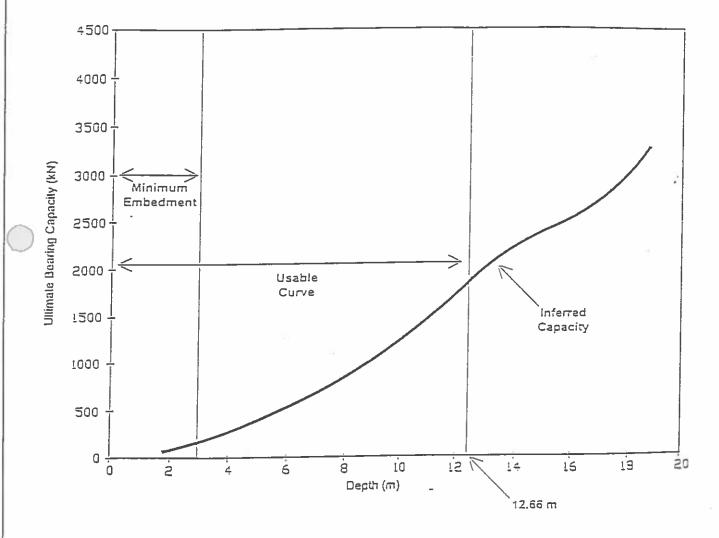
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CARSON FREEWAY

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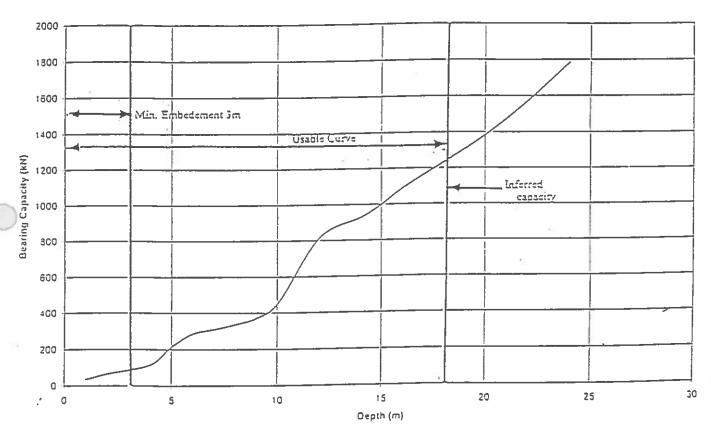
PILE DESIGN, PP457 ULTIMATE BEARING CAPACITY EMERSON DRIVE

CARSON FREEWAY

CARSON CITY, NEVADA

PLATE

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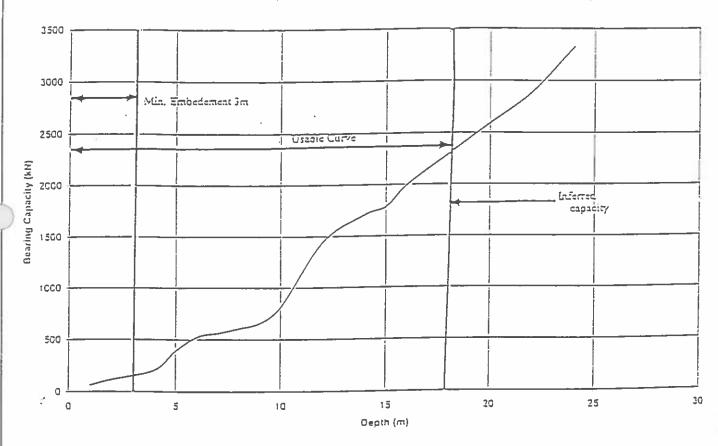
PILE DESIGN, PP305 ULTIMATE BEARING CAPACITY COLLEGE PARKWAY

CARSON FREEWAY

CARSON CITY, NEVADA

PLATE

85-13A, Pipe Pile PP406, Ultimate Searing Capacity



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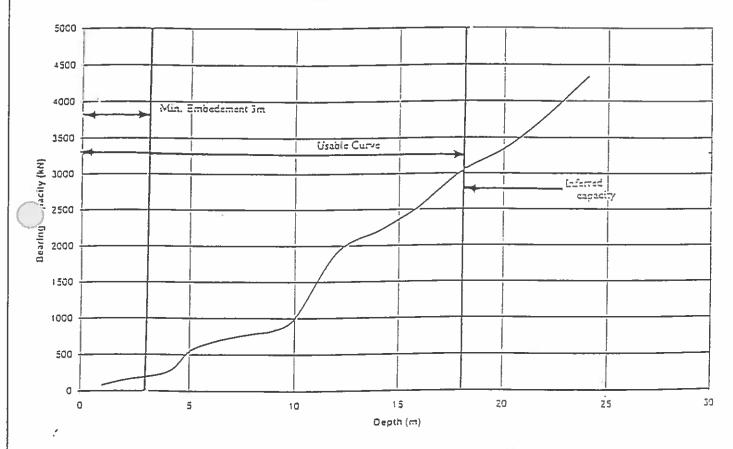
PILE DESIGN, PP406 ULTIMATE BEARING CAPACITY COLLEGE PARKWAY

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BS-13A, Pipe Pile PP457, Ultimate Bearing Capacity



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PILE DESIGN, PP305 ULTIMATE BEARING CAPACITY CARMINE STREET

CARSON FREEWAY

CARSON CITY, NEVADA

PLATE

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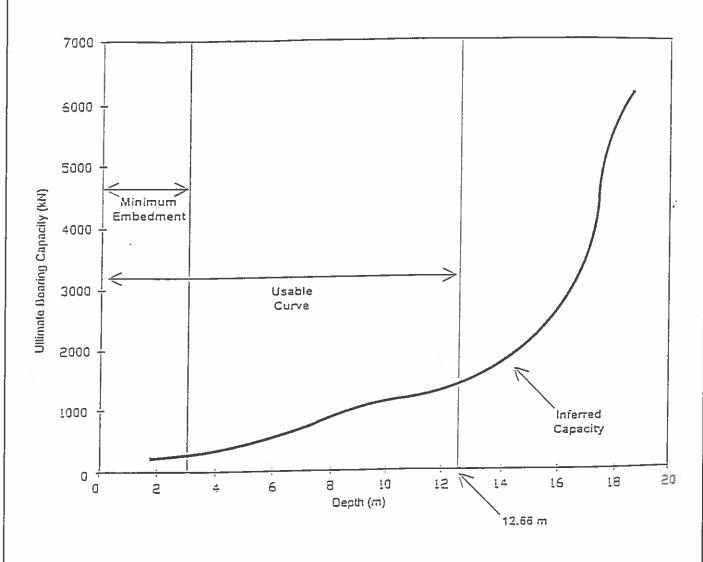


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PILE DESIGN, PP406 ULTIMATE BEARING CAPACITY CARMINE STREET

CARSON FREEWAY CARSON CITY, NEVADA 25

PLATE



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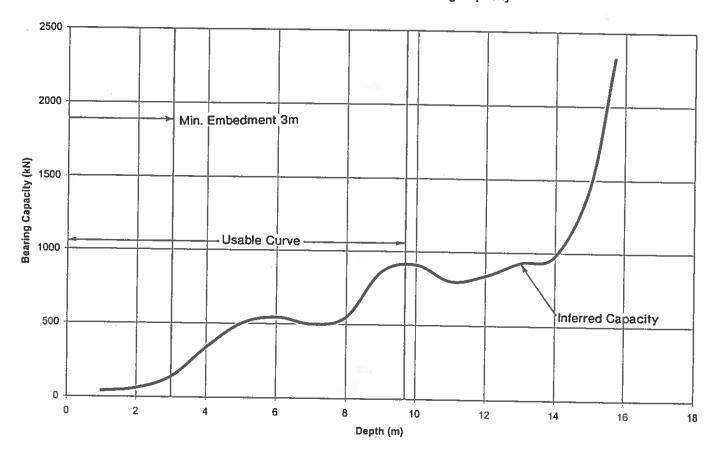


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PILE DESIGN, PP457 **ULTIMATE BEARING CAPACITY** CARMINE STREET

CARSON FREEWAY CARSON CITY, NEVADA

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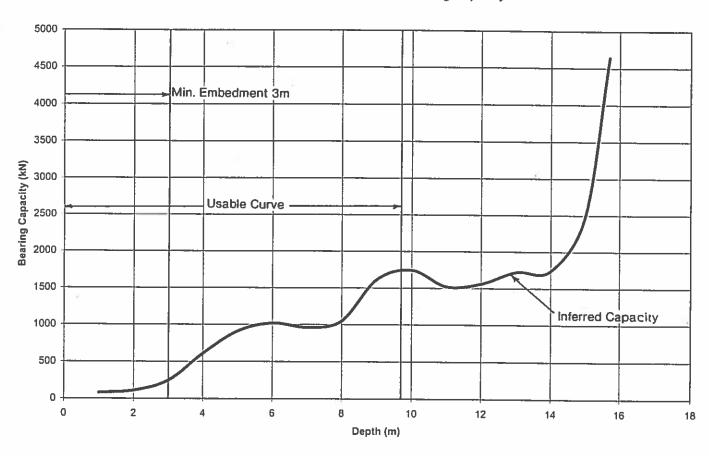
CARSON FREEWAY

CARSON CITY, NEVADA

PLATE

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4875 LONGLEY LANE, SUITE 100 RENO, NEVADA 89502 Tel. (775) 689-7800 PILE DESIGN, PP406
ULTIMATE BEARING CAPACITY
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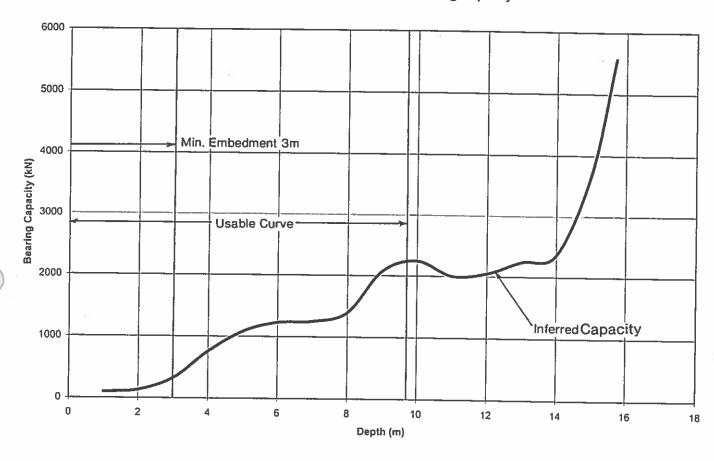
CARSON FREEWAY

CARSON CITY, NEVADA

PLATE

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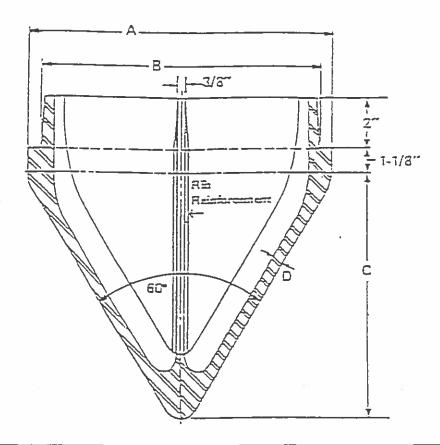
CARSON FREEWAY

CARSON CITY, NEVADA

PLATE

29

MATERIAL: CAST STEEL ASTM AZZ 65/25 HEAT-TREATED



PIPE O.D.	А	В	C	D
8-5/8"	8-3/4"	7-1/2"	7-1/8"	1/2"
9-5/8"	9-3/4"	7-1/2"	7-1/8"	1/2"
10-3/4"	10-7/8"	9-3/4"	g"	1/2"
12"	12-1/8"	11"	10-3/8"	1/2"
12-3/4"	12-7/8"	11-3/4"	10-3/4"	1/2"
13-3/8"	13-1/2"	11-11/16"	11-3/8"	1/2"
14"	14-1/8"	13"	11-13/16"	9/16"
16"	16-1/8"	15"	13-1/2"	9/16"
18"	18-1/8"	17"	1E-1/4"	5/8"
20"	20-1/8"	10"	17"	5/8"
22"	22-1/8"	21"	18-7/8"	5/6"
24"	24-1/5"	23"	20-3/8"	5/8"

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PIPE PILE DRIVING SHOE

CARSON FREEWAY CARSON CITY, NEVADA PLATE

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PROJECT NO. 30-1348-15.002

Footing Width (m)

		0.5	1.0	1.5	2.0
lh (m)	0.5	144	148	147	174
l Depl	1.0	131	185	147	175
dmen	1.5	156	223	172	200
Embedment Depth (m)	2.0	181	260	197	225

Footing Width (m)

		0.5	1.0	1.5	2.0
Embedment Depth (m)	0.5	1	2	3	4
t Dept	1.0	2	4	4	5
dmen	1.5	3	6	5	7
Embe	2.0	4	8	7	9

Elastic Settlement (mm)

Allowable Bearing Capacity (kN/m2)

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RETAINING WALL DESIGN ALLOWABLE LOAD AND SETTLEMENT HOT SPRINGS

CARSON FREEWAY

CARSON CITY, NEVADA

		0.5	1.0	1.5	2.0
h (m)	0.5	90	124	159	158
l Depl	1.0	146	156	162	174
dnnen	1.5	150	150	163	179
Embedment Depth (m)	2.0	120	136	152	168

Footing Width (m)

		0.5	1.0	1.5	2.0
h (m)	0.5	2	4	6	7
Dept	1.0	5	7	9	11
lment	1.5	6	8	11	14
Embedment Depth (m)	2.0	5	9	12	15

Elastic Settlement (mm)

Allowable Bearing Capacity (kN/m2)

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RETAINING WALL DESIGN ALLOWABLE LOAD AND SETTLEMENT **BIKE PATH #1**

CARSON FREEWAY

CARSON CITY, NEVADA

		0.5	1.0	1.5	2.0
lh (m)	0.5	61	70	84	87
Depl	1.0	77	86	101	104
lment	1.5	94	103	117	120
Embedment Depth (m)	2.0	111	119	134	137
_					

Allowablo Dearing Capacity (kN/m2)

Footing Width (m)

	0.5	1.0	1.5	2.0
0.5	1	1	2	2
1.0	1	2	3	3
1.5	2	3	4	4
2.0	2	4	5	6
	1.0	0.5 1 1.0 1 1.5 2	0.5 1 1 1.0 1 2 1.5 2 3	0.5 1 1 2 1.0 1 2 3 1.5 2 3 4

Elastic Sottlement (mm)

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RETAINING WALL DESIGN ALLOWABLE LOAD AND SETTLEMENT **EMERSON & NORTHGATE**

CARSON FREEWAY

CARSON CITY, NEVADA

		0.5	1.0	1.5	2.0
(m) H	0.5	60	74	91	109
Dept	1.0	72	89	106	122
iment	_{::} 1.5	100	116	133	150
Embedment Depth (m)	2.0	127	144	161	178

Faating Width (m)

		0.5	1.0	1.5	2.0
h (m)	0.5	1	2	4	5
Dept	1.0	2	4	6	8
dment	1.5	4	6	9	12
Embedment Depth (m)	2.0	6	9	13	16

Elastic Settlement (mm)

Allowablo Bearing Capacity (kN/m2)

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RETAINING WALL DESIGN ALLOWABLE LOAD AND SETTLEMENT BROADLEAF EAST

CARSON FREEWAY

CARSON CITY, NEVADA

PLATE

34

CAD FILE: L'\1999\D. NG\301348\30134815002-PLATES.DWG

PROJECT NO. 30-1348-15.002

		0.5	1.0	1.5	2.0
(m)	0.5	87	113	121	132
Depth	1.0	128	120	129	141
ment	1.5	100	117	133	150
Embedmenl Deplh (m)	2.0	128	145	161	178
ш	1				

Allowable Bearing Capacity (kN/m2)

Footing Width (m)

		0.5	1.0	1.5	2.0
(m)	0.5	2	4	5	6
Deptil	1.0	2	5	7	9
ment	1.5	4	6	9	12
Embedment Depth (m)	2.0	6	9	13	16

Elastic Settloment (mm)

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RETAINING WALL DESIGN ALLOWABLE LOAD AND SETTLEMENT BIKE PATH #2

CARSON FREEWAY

CARSON CITY, NEVADA

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PROJECT NO. 30-1348-15.002

		0.5	1.0	1.5	2.0
(m) r	0.5	44	61	77	64
Dept	1.0	72	89	105	122
Iment	1.5	100	117	133	150
Embedment Depth (m)	2.0	128	145	161	178

Allowable Bearing Capacity (kN/m2)

Footing Width (m)

		0.5	1.0	1.5	2.0
Embedment Depth (m)	0.5	1	2	3	4
	1.0	2	4	6	8
	1.5	4	6	9	12
Embec	2.0	6	9	13	16

Elastic Settlement (mm)

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RETAINING WALL DESIGN ALLOWABLE LOAD AND SETTLEMENT SOUTH OF ARROWHEAD

CARSON FREEWAY

CARSON CITY, NEVADA

PLATE

CAD FILE: L'\1999\DK...36\301348\30134815002-PLATES.DWG

PROJECT NO. 30-1348-15.002

		0.5	1.0	1.5	2.0
Embedment Depth (m)	0.5	117	163	209	255
	1.0	188	234	280	326
	1.5	258	304	350	397
	2.0	329	375	421	467

Allowable Bearing Capacity (kN/m2)

Footing Width (m)

		0.5	1.0	1.5	2.0
Embedment Depth (m)	0.5	1	3	4	6
	1.0	3	5	8	10
	1.5	5	8	12	15
Embec	2.0	7	12	16	21

Elastic Sottlement (mm)

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RETAINING WALL DESIGN ALLOWABLE LOAD AND SETTLEMENT LV3

CARSON FREEWAY

CARSON CITY, NEVADA

JG 301348 30134815002-PLATES.DWG CAD FILE: L:\1999\0+

PROJECT NO. 30-1348-15,002



6. ADDITIONAL SERVICES

6.1 Project Bid Documents

It has been our experience during the bidding process, that contractors often contact us to discuss the geotechnical aspects of the project. Informal contacts between Kleinfelder and an individual contractor could result in incorrect or incomplete information being provided to the contractor. Therefore, we recommend a pre-bid meeting be held to answer any questions about the report prior to submittal of bids. If this is not possible, questions or clarifications regarding this report should be directed to NDOT or his designated representative. After consultation with Kleinfelder, NDOT(or representative) should provide clarifications or additional information to all contractors bidding the job.

6.2 <u>Construction Observation/Testing and Plan Review</u>

The recommendations made in this report are based on the assumption that an adequate program of tests and observations will be made during construction to verify compliance with these recommendations. These tests and observations should include, but not necessarily be limited to, the following:

- Observations and testing during site preparation and earthwork.
- Observation of footing trench excavations.
- Observation and testing of construction materials.
- Consultation as may be required during construction.

We also recommend that project plans and specifications be reviewed by us to verify compatibility with our conclusions and recommendations. Additional information concerning the scope and cost of these services can be obtained from our office.

The review of plans and specifications and the field observation and testing by Kleinfelder are an integral part of the conclusions and recommendations made in this report. If we are not retained for these services, the Client agrees to assume Kleinfelder's responsibility for any potential claims that may arise during construction.



7. LIMITATIONS

Recommendations contained in this report are based on our field explorations, laboratory tests, and our understanding of the proposed construction. The study was performed using a mutually agreed upon scope of work. It is our opinion that this study was a cost-effective method to evaluate the subject site and evaluate some of the potential geotechnical concerns. More detailed, focused, and/or thorough investigations can be conducted. Further studies will tend to increase the level of assurance, however, such efforts will result in increased costs. If the Client wishes to reduce the uncertainties beyond the level associated with this study. Kleinfelder should be contacted for additional consultation.

The soils data used in the preparation of this report were obtained from borings made for this investigation. It is possible that variations in soils exist between the points explored. The nature and extent of soil variations may not be evident until construction occurs. If any soil conditions are encountered at this site which are different from those described in this report, our firm should be immediately notified so that we may make any necessary revisions to our recommendations. In addition, if the scope of the proposed project, locations of structures, or structural loads change from the description given in this report, our firm should be notified.

This report has been prepared for design purposes for specific application to the Carson Freeway Project in accordance with the generally accepted standards of practice at the time the report was written. No warranty, express or implied, is made.

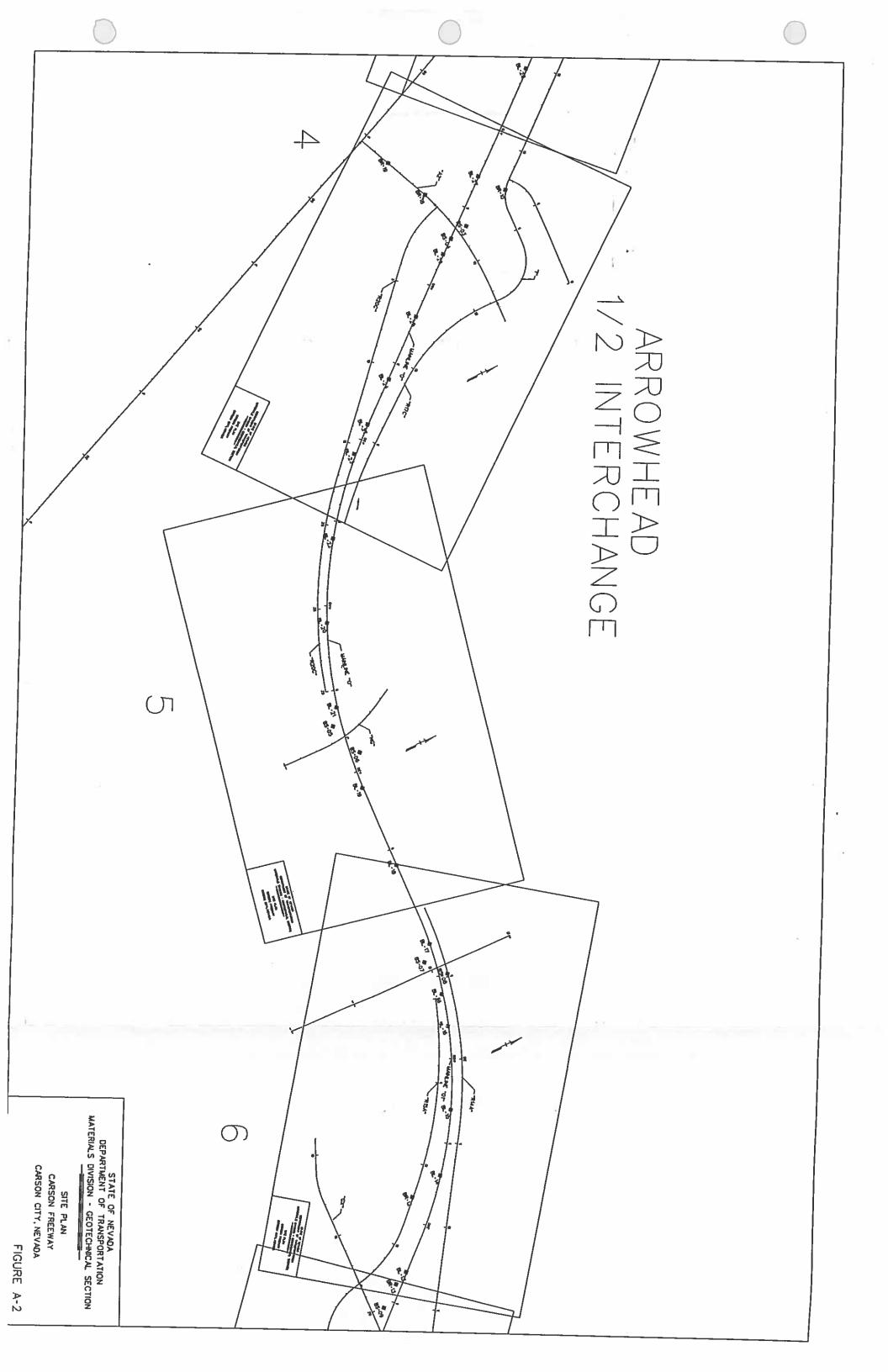
This report may be used only by the Client and for the purposes stated, within a reasonable time from its issuance. Land use, site conditions (both on- and off-site), or other factors including advances in man's understanding of applied science may change over time and could materially affect our findings. Therefore, this report should not be relied upon after 36 months from its issue. Kleinfelder should be notified if the project is delayed by more than 24 months from the date of this report so that a review of site conditions can be made, and recommendations revised if appropriate.

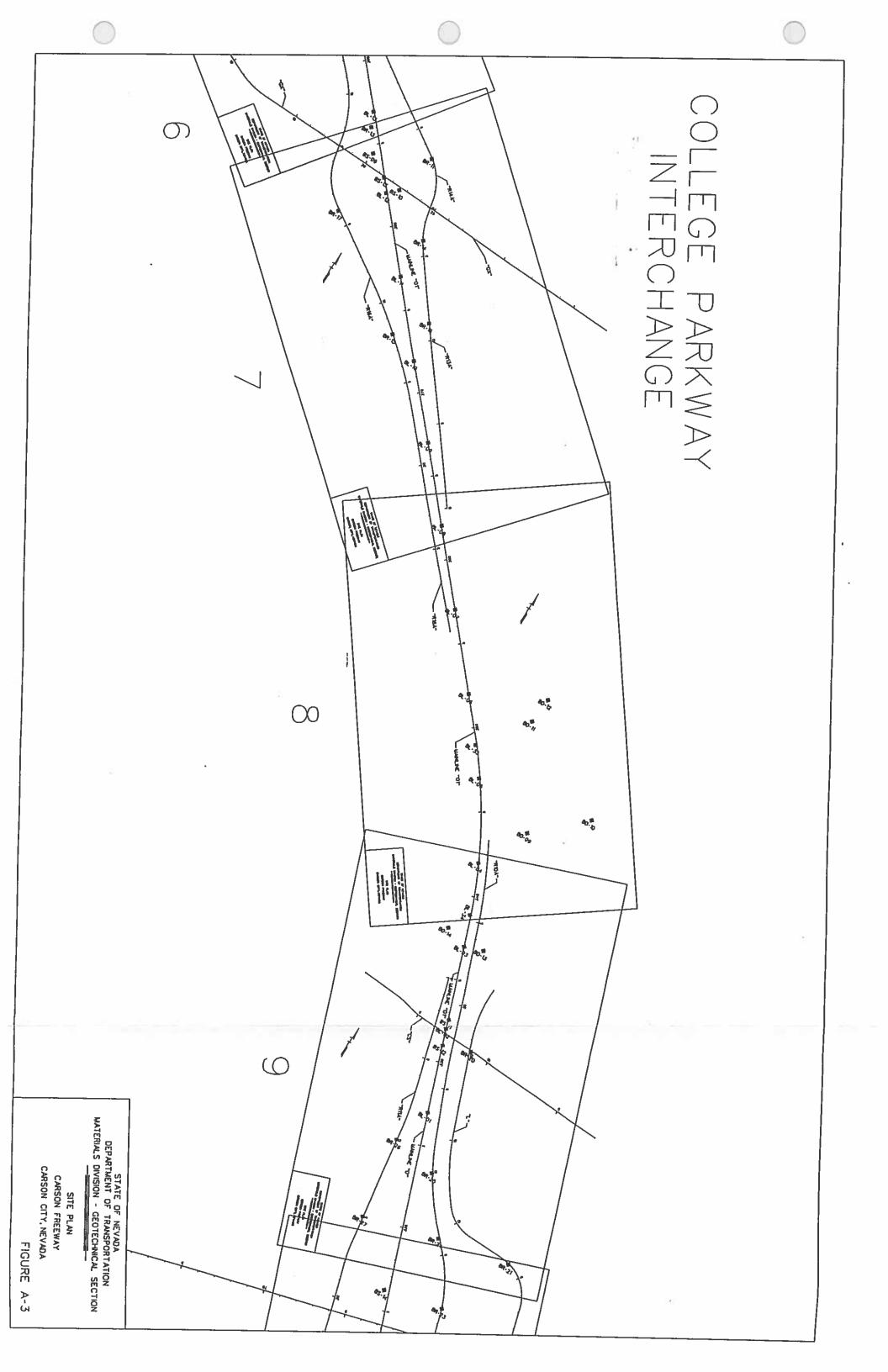
It is the CLIENT'S responsibility to see that all parties to the project including the designer, contractor, subcontractors, etc., are made aware of this report in its entirety. The use of information contained in this report for bidding purposes should be done at the Contractor's option and risk. Any party other than the Client who wishes to use this report shall notify Kleinfelder of such intended use. Based on the intended use of the report, Kleinfelder may require that additional work be performed and that an updated report be issued. Non-compliance with any of these requirements by the Client or anyone else will release Kleinfelder from any liability resulting from the use of this report by any unauthorized party.

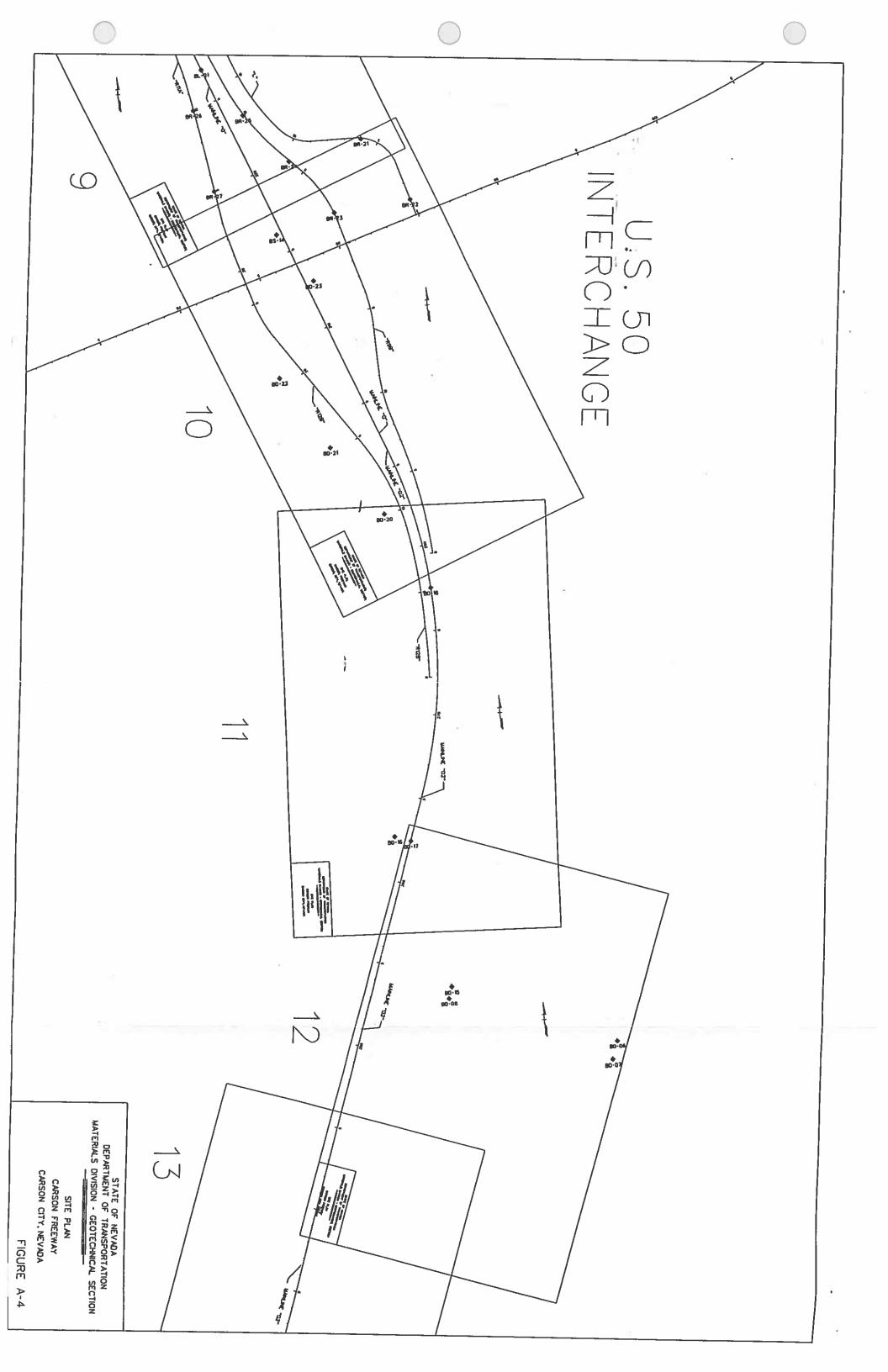
APPENDIX A

Project Site Plans

W STATE OF NEVADA
DEPARTMENT OF TRANSPORTATION
MATERIALS DIVISION - GEOTECHNICAL SECTION SITE PLAN CARSON FREEWAY CARSON CITY, NEVADA FIGURE A-1





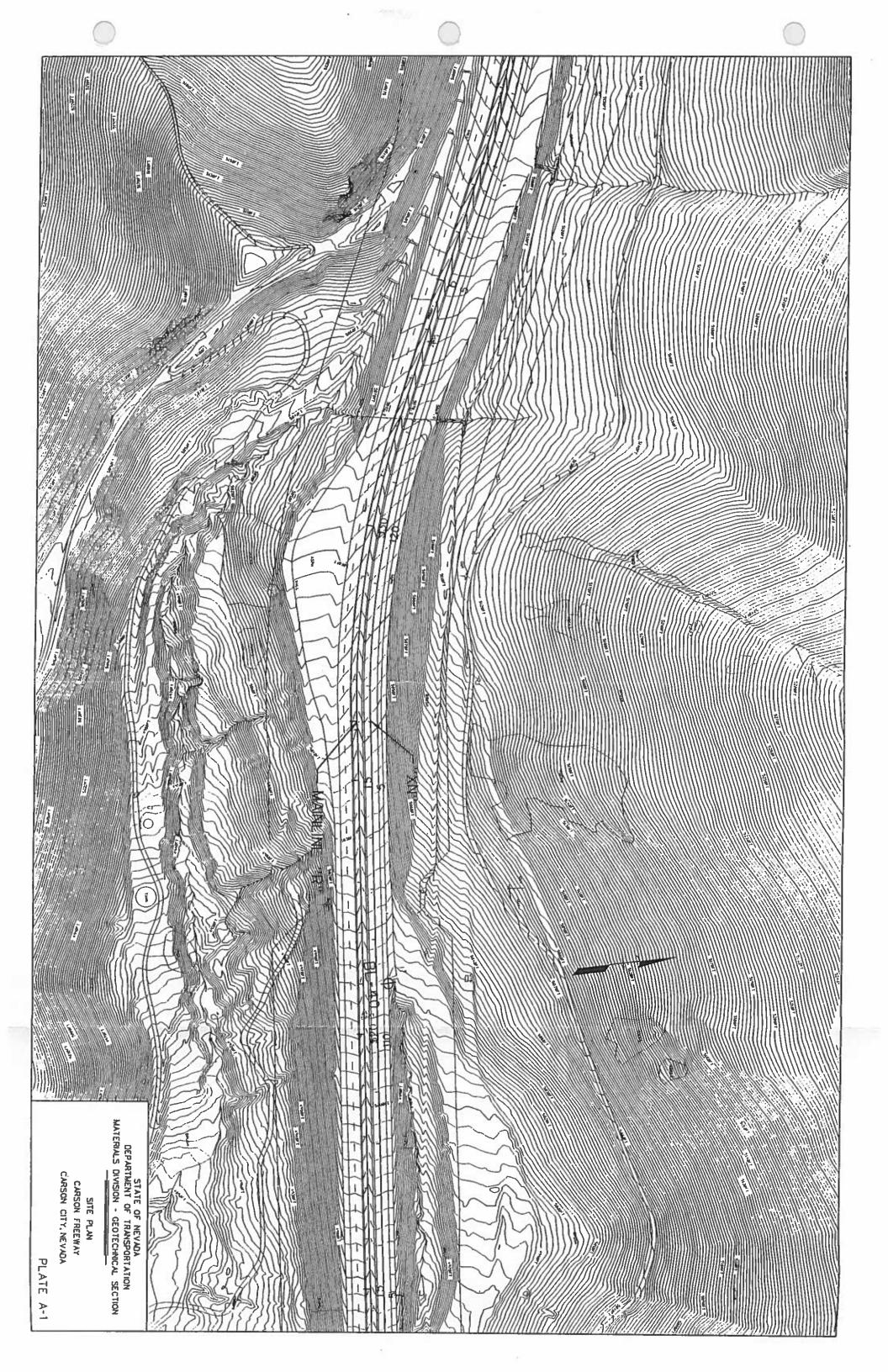


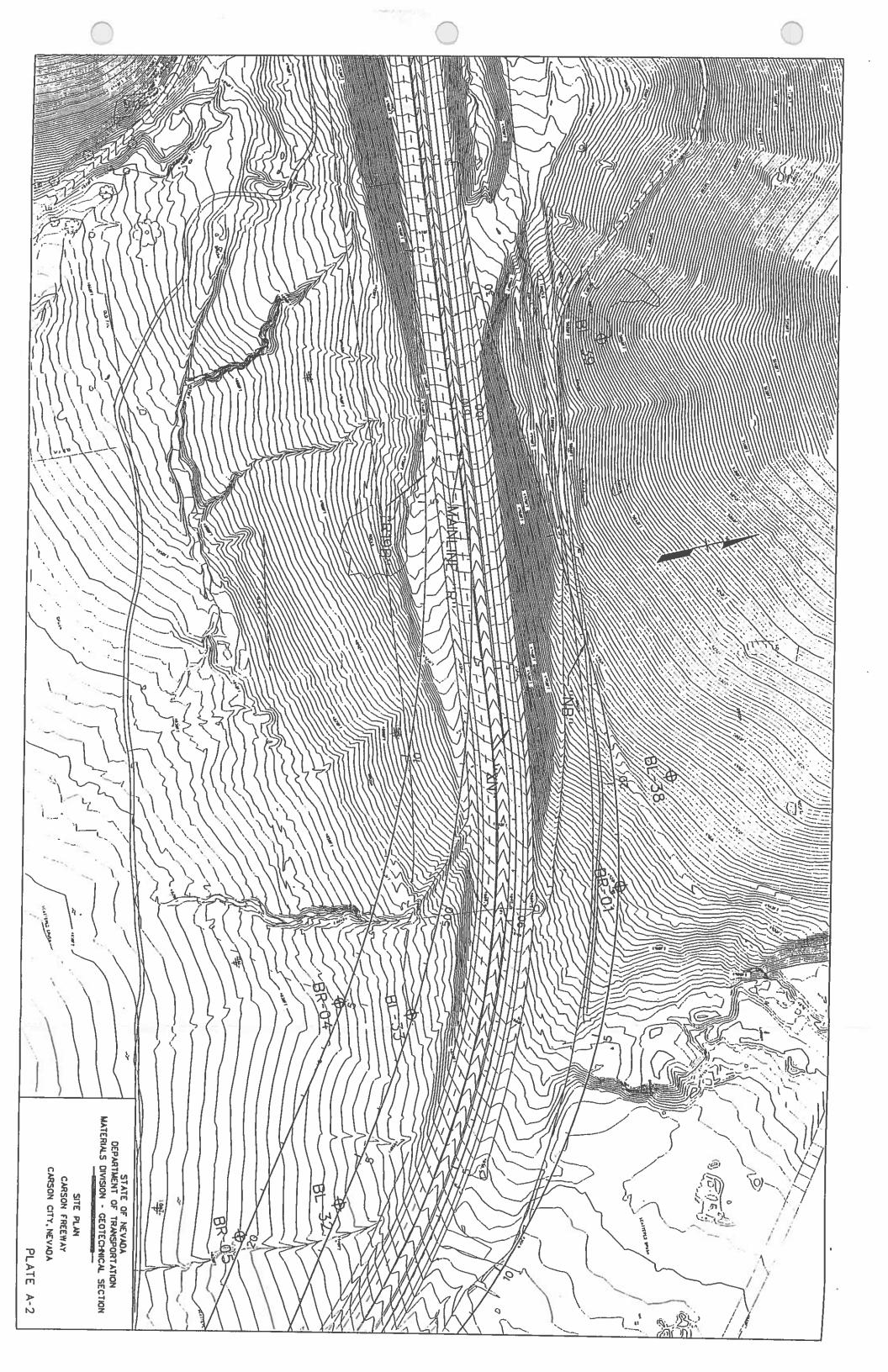
13 FAIRVIEW STATE OF NEVADA
DEPARTMENT OF TRANSPORTATION
MATERIALS DIVISION - GEOTECHNICAL SECTION SITE PLAN CARSON FREEWAY CARSON CITY, NEVADA FIGURE A-5

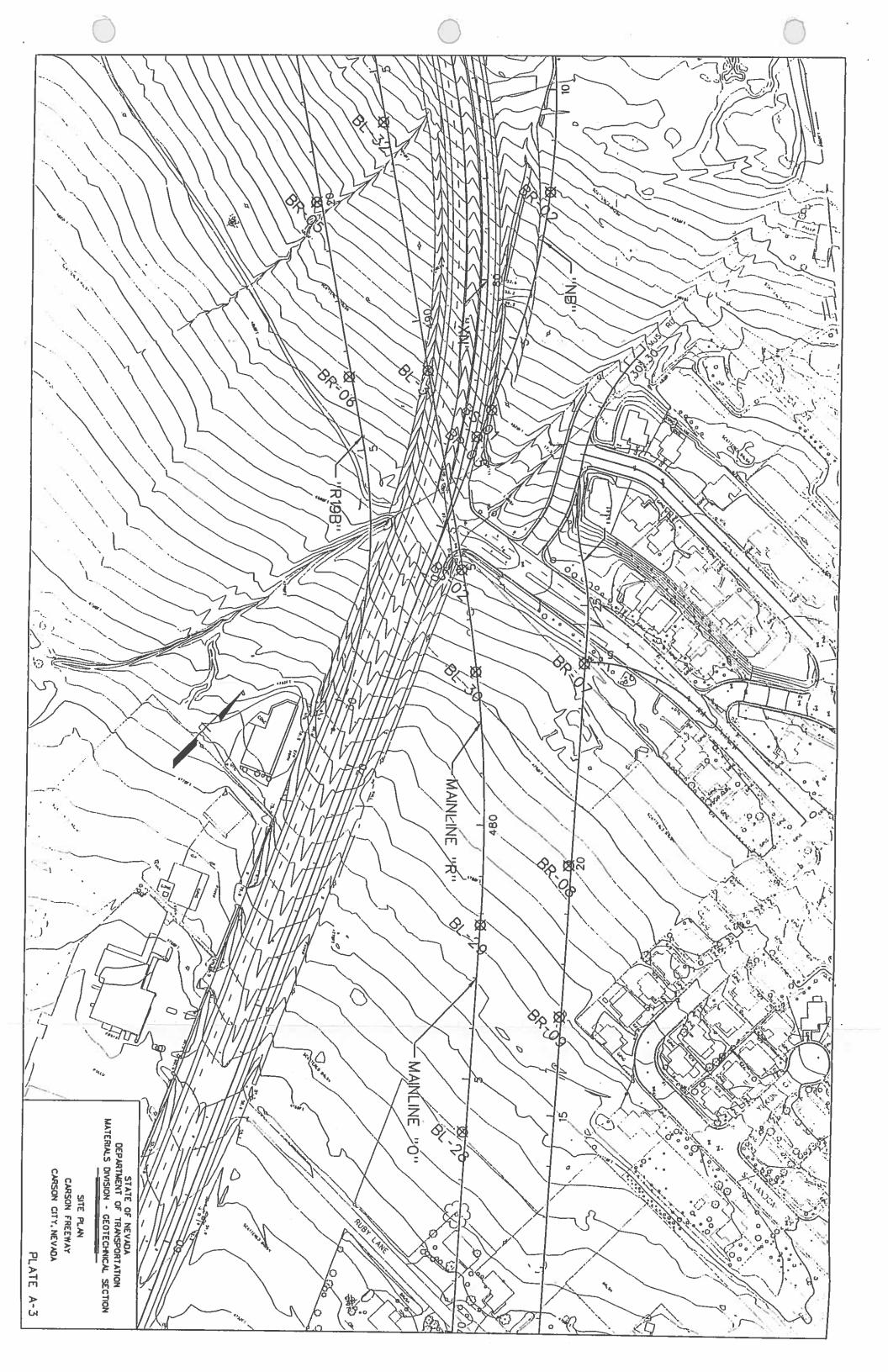
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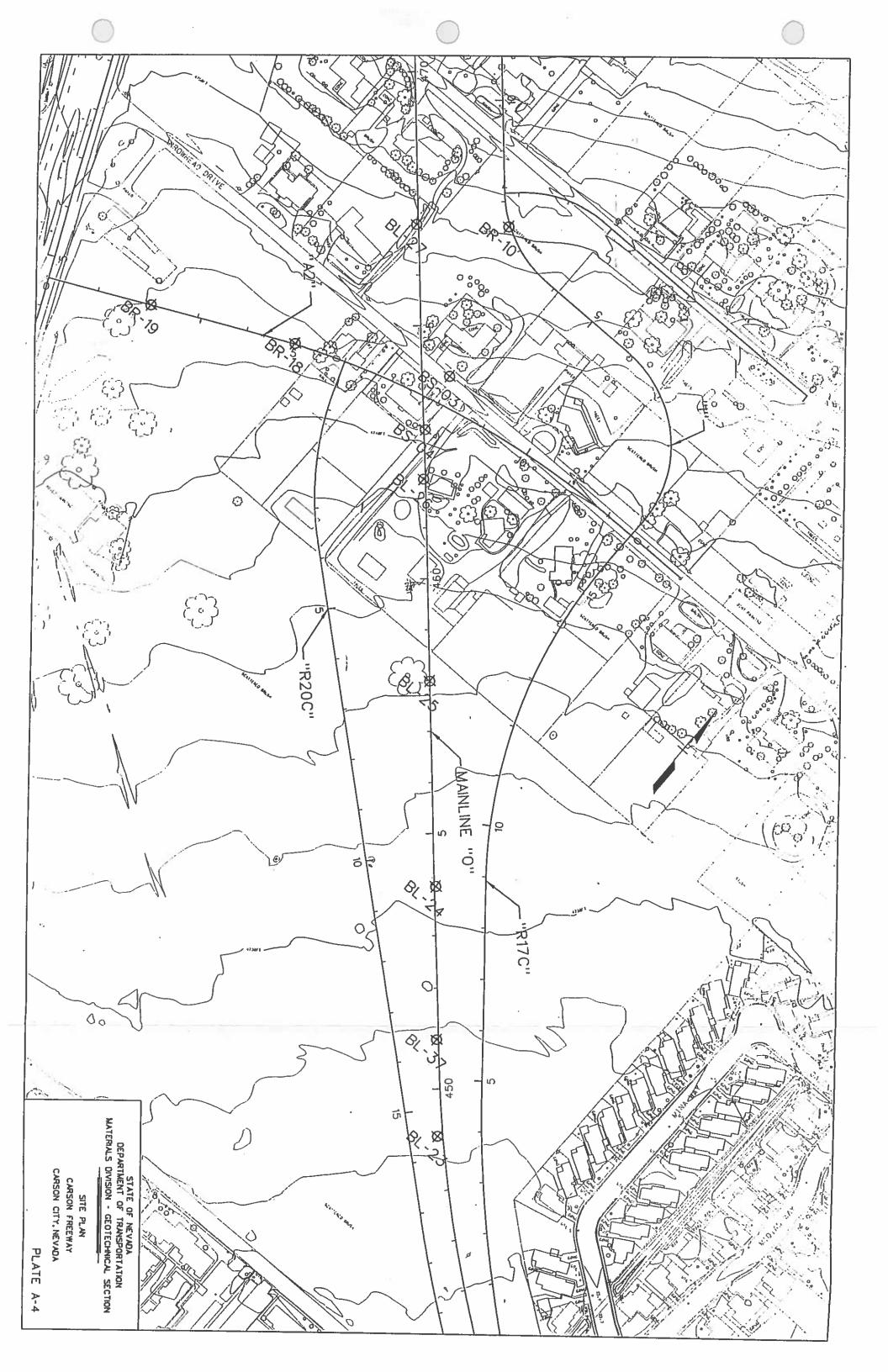
DEPARTMENT OF TRANSPORTATION

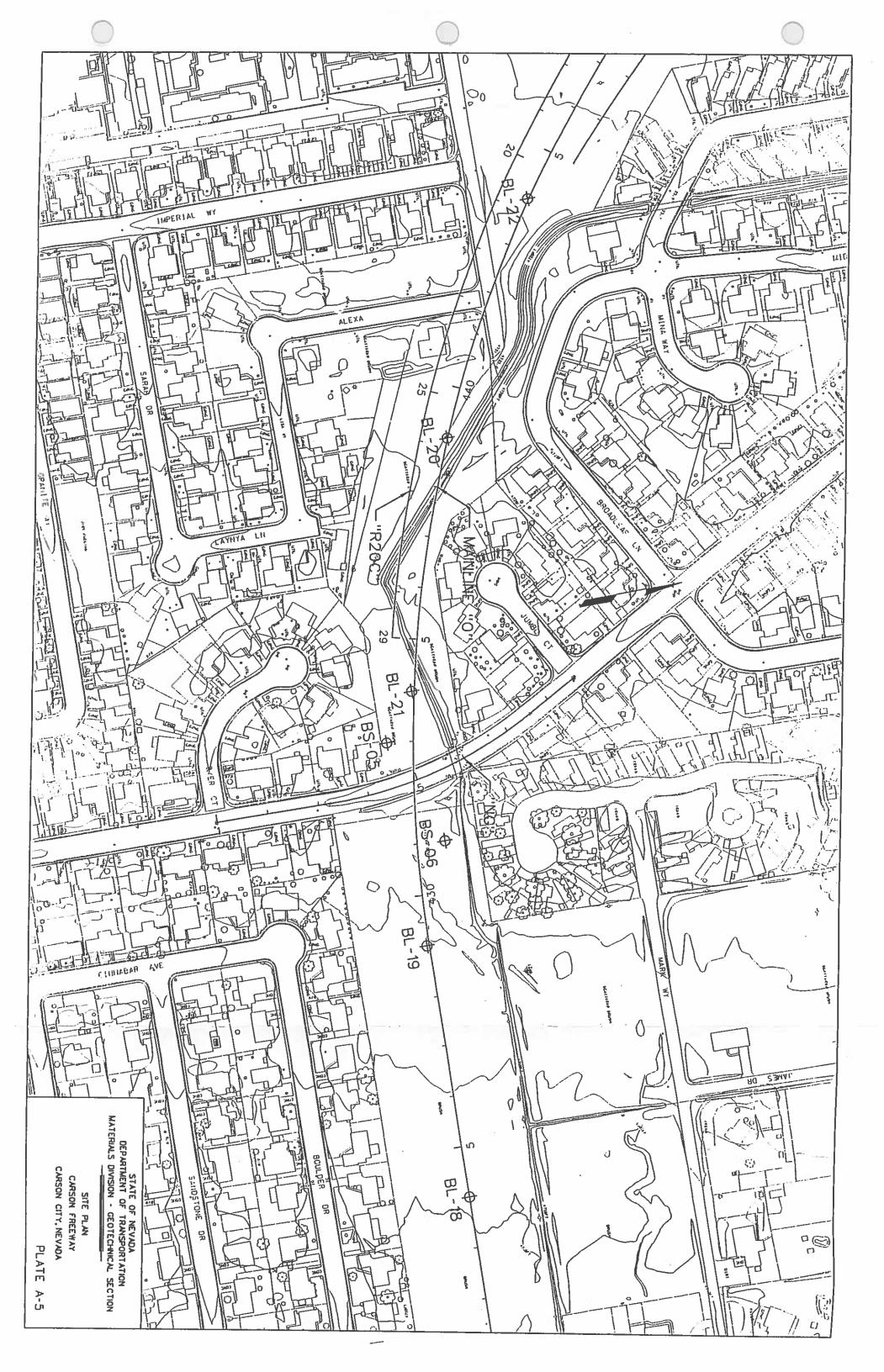
MATERIALS DIVISION - GEOTECHNICAL SECTION SITE PLAN CARSON FREEWAY CARSON CITY, NEVADA FIGURE A-6

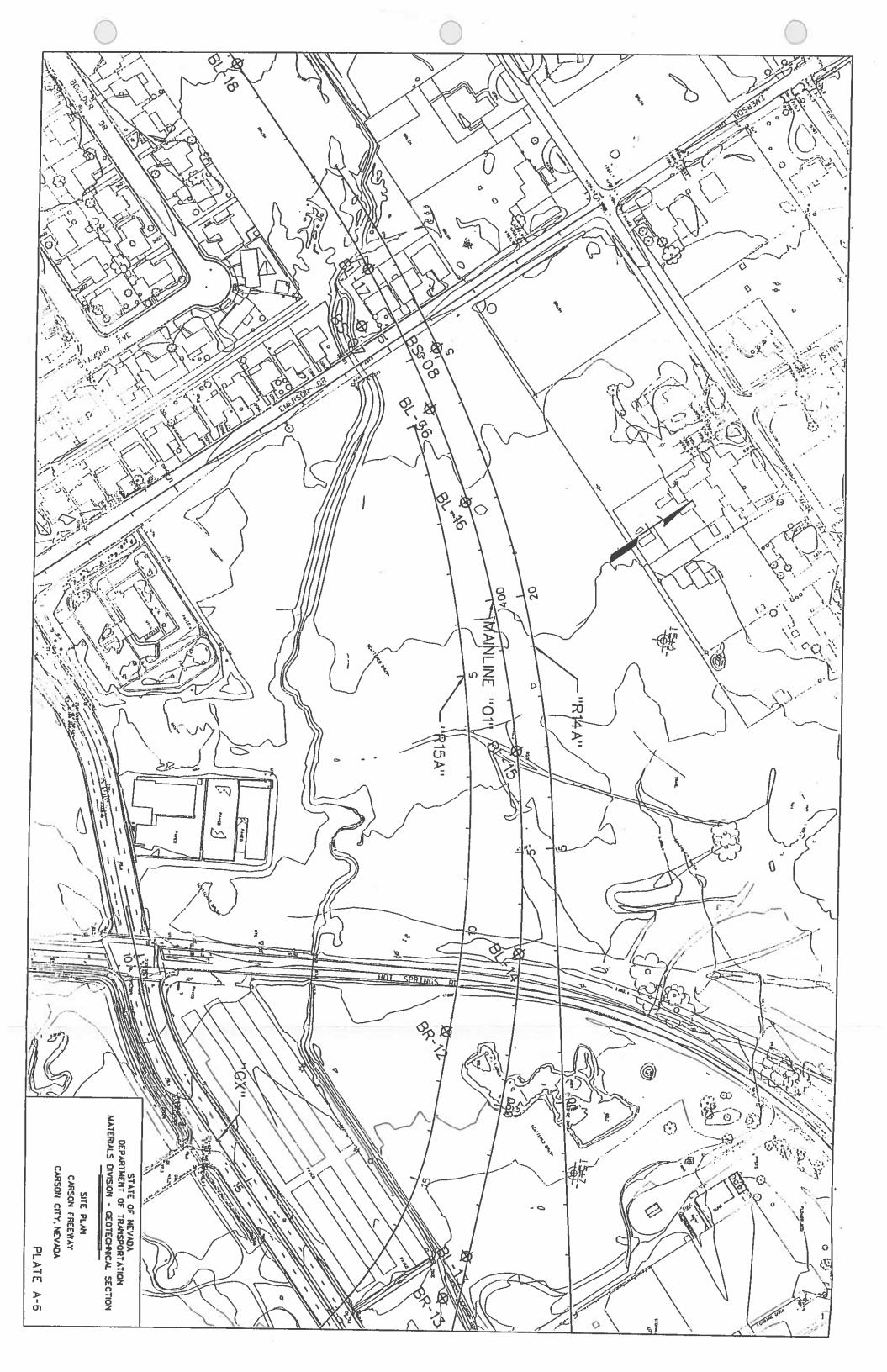


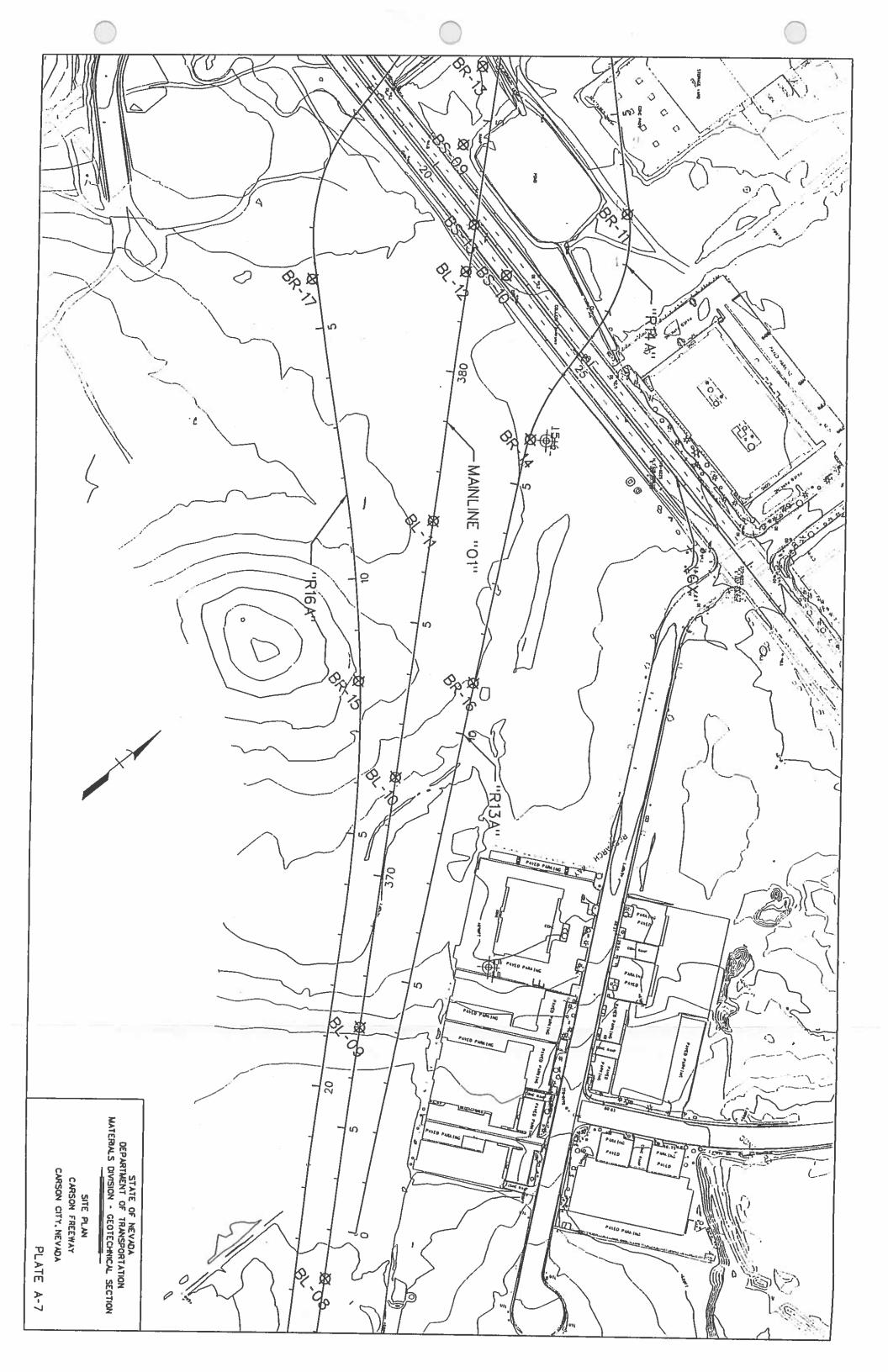


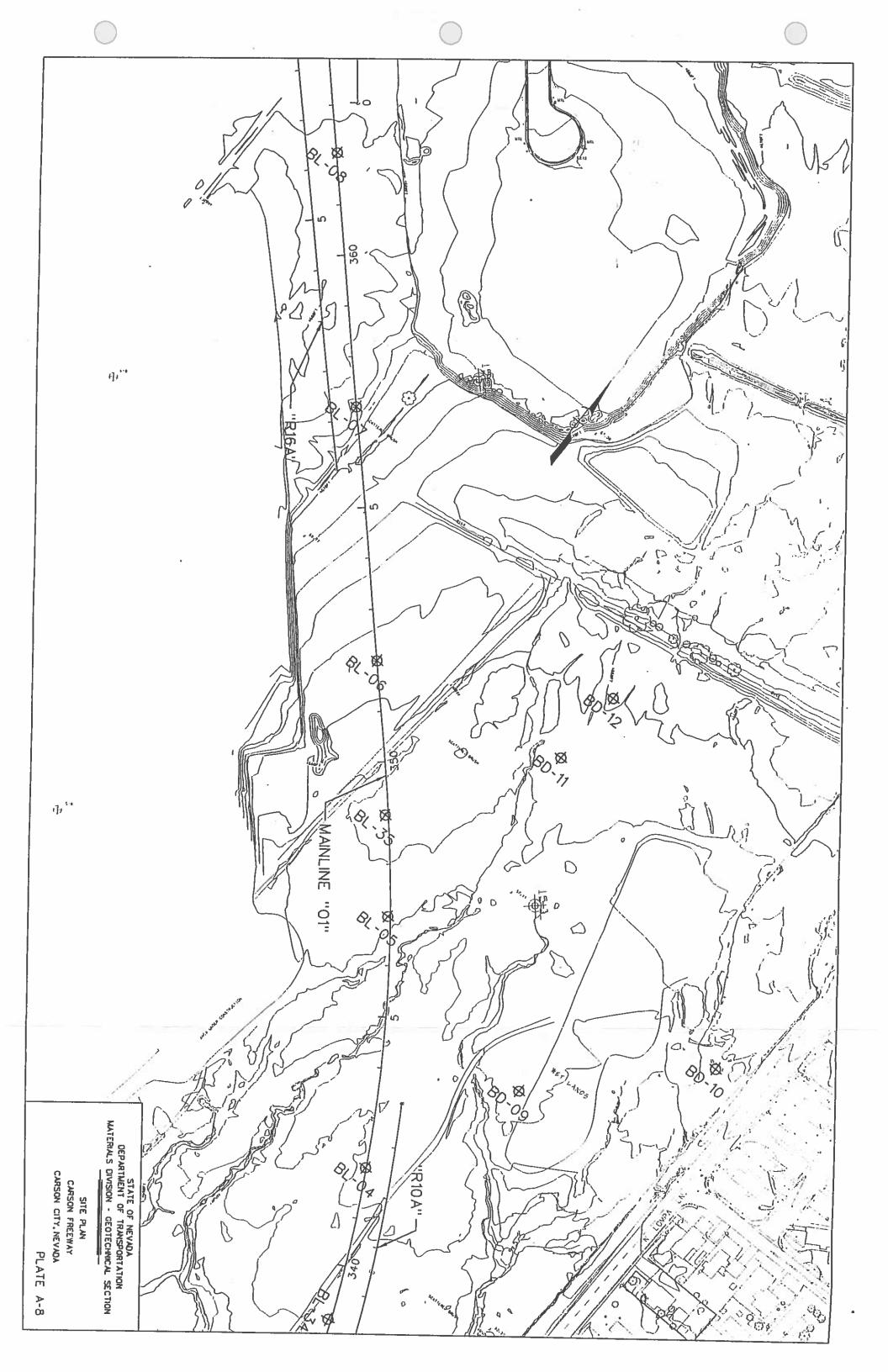


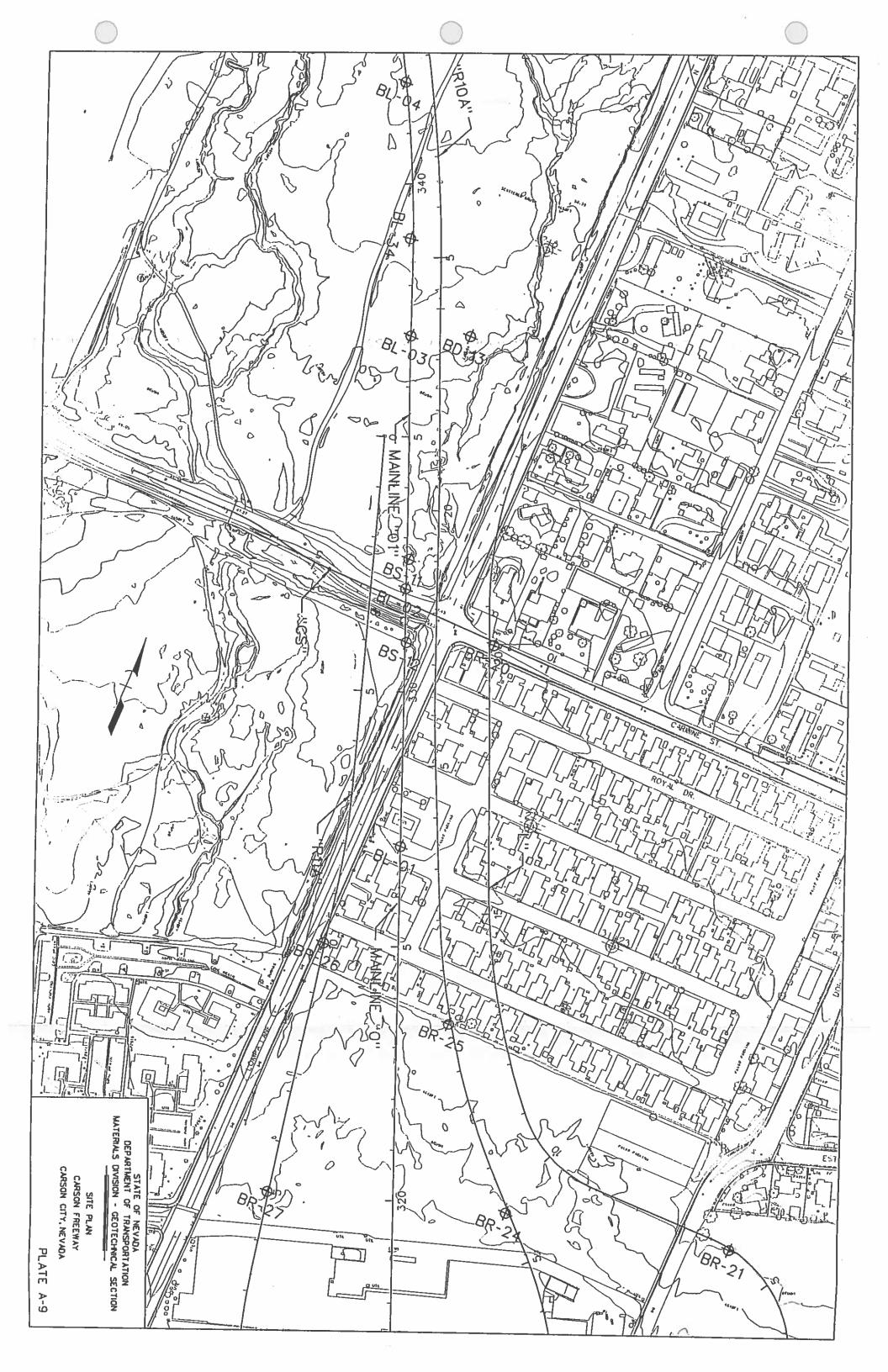




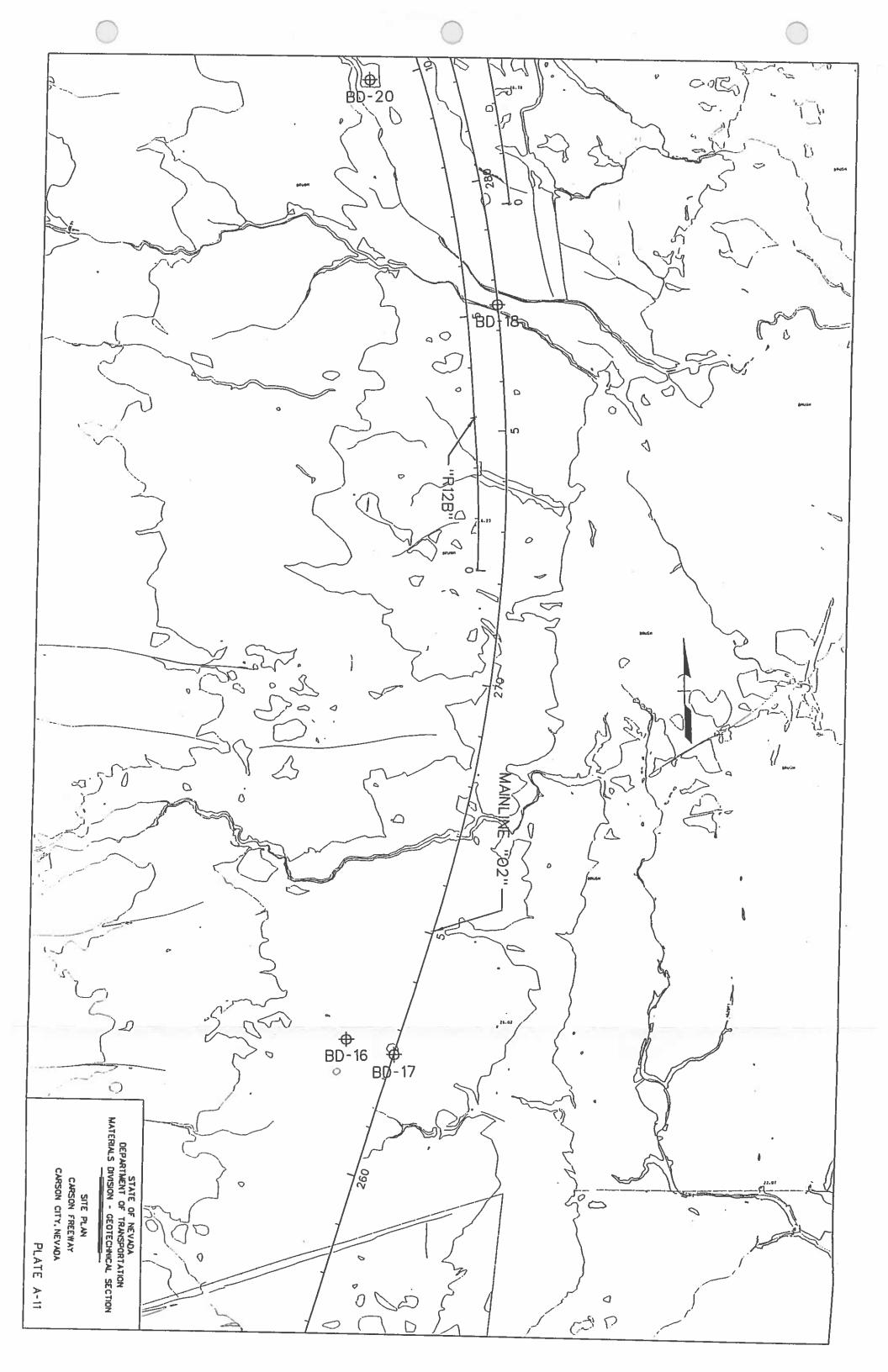


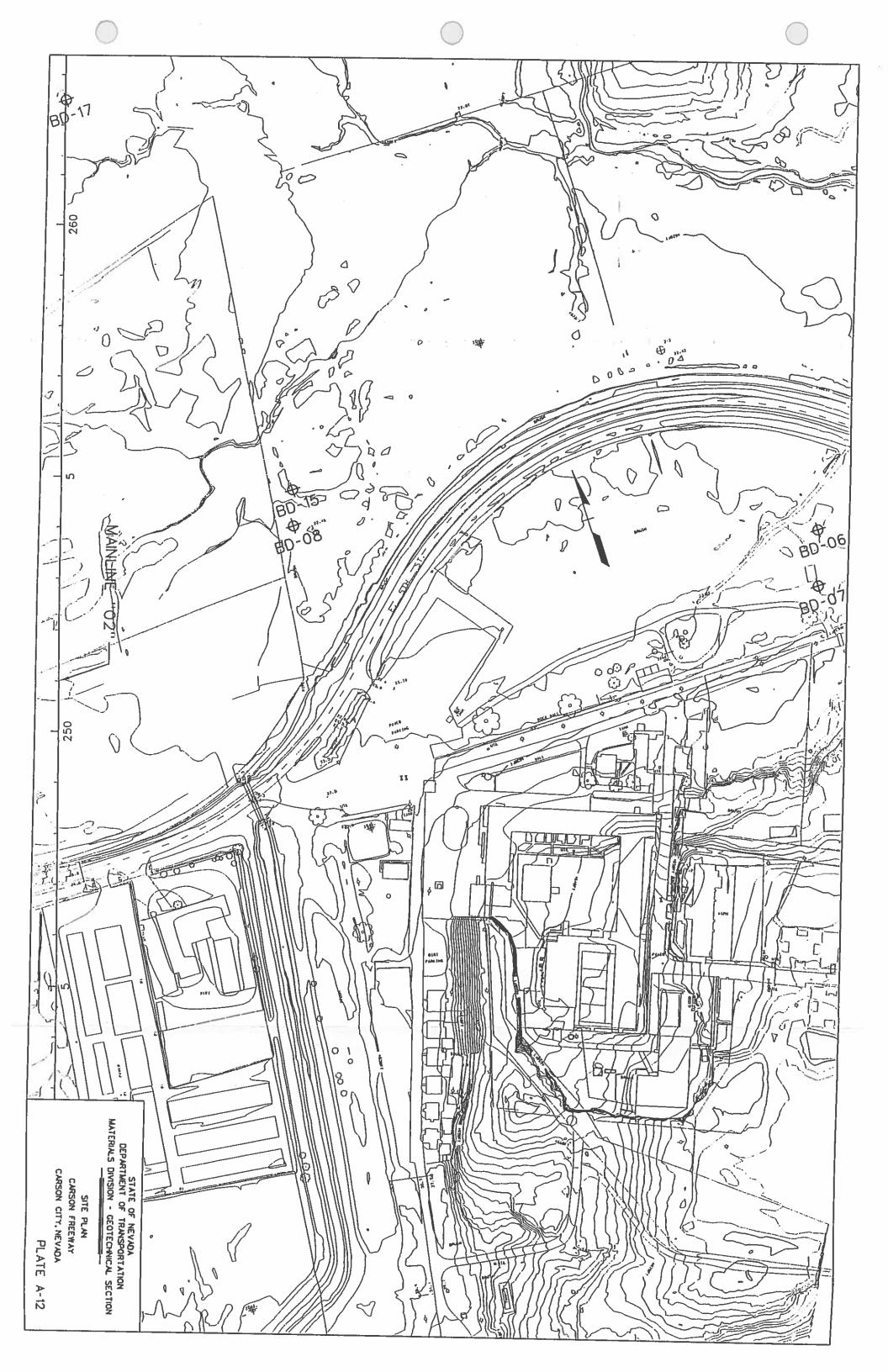


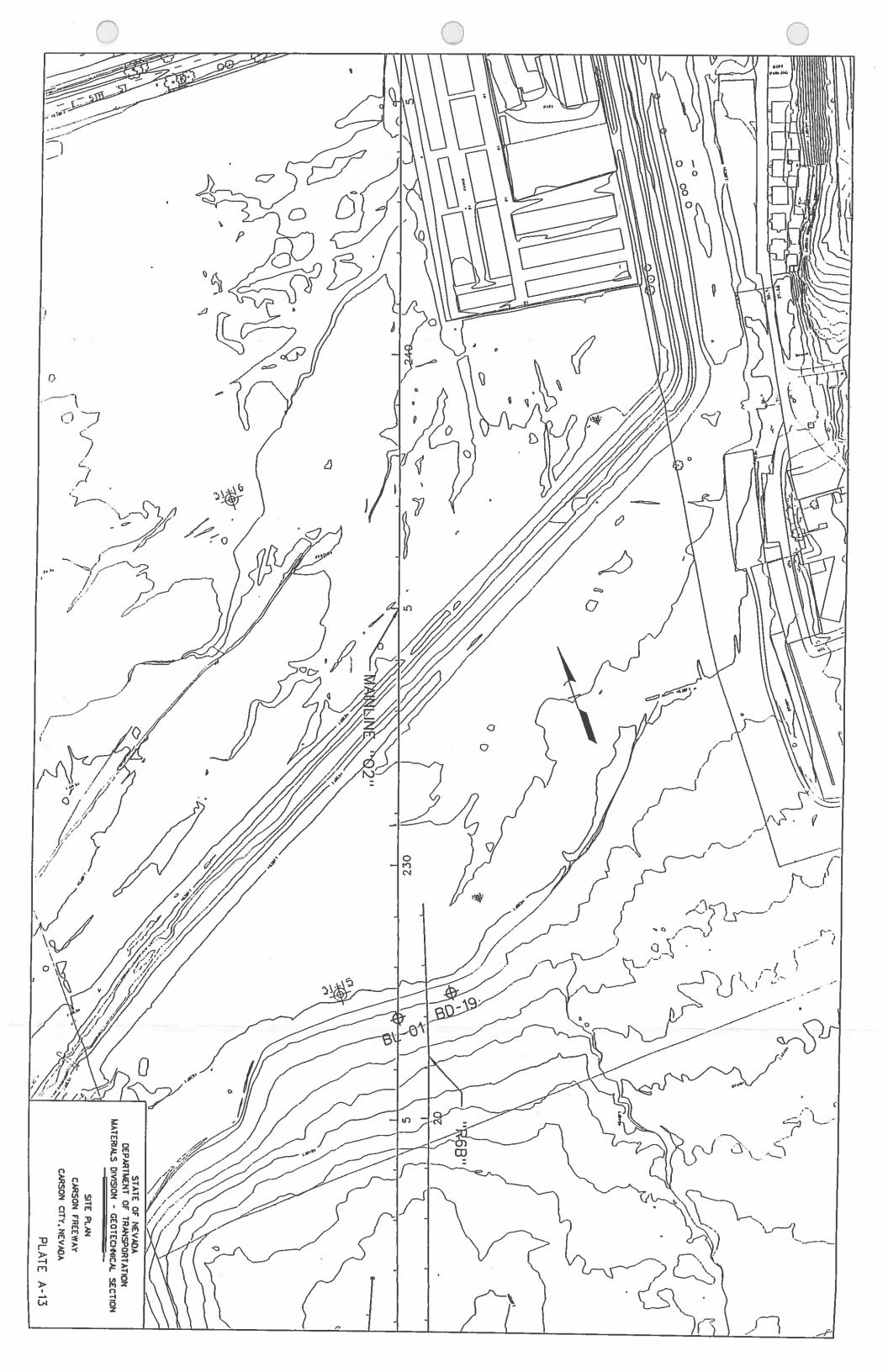


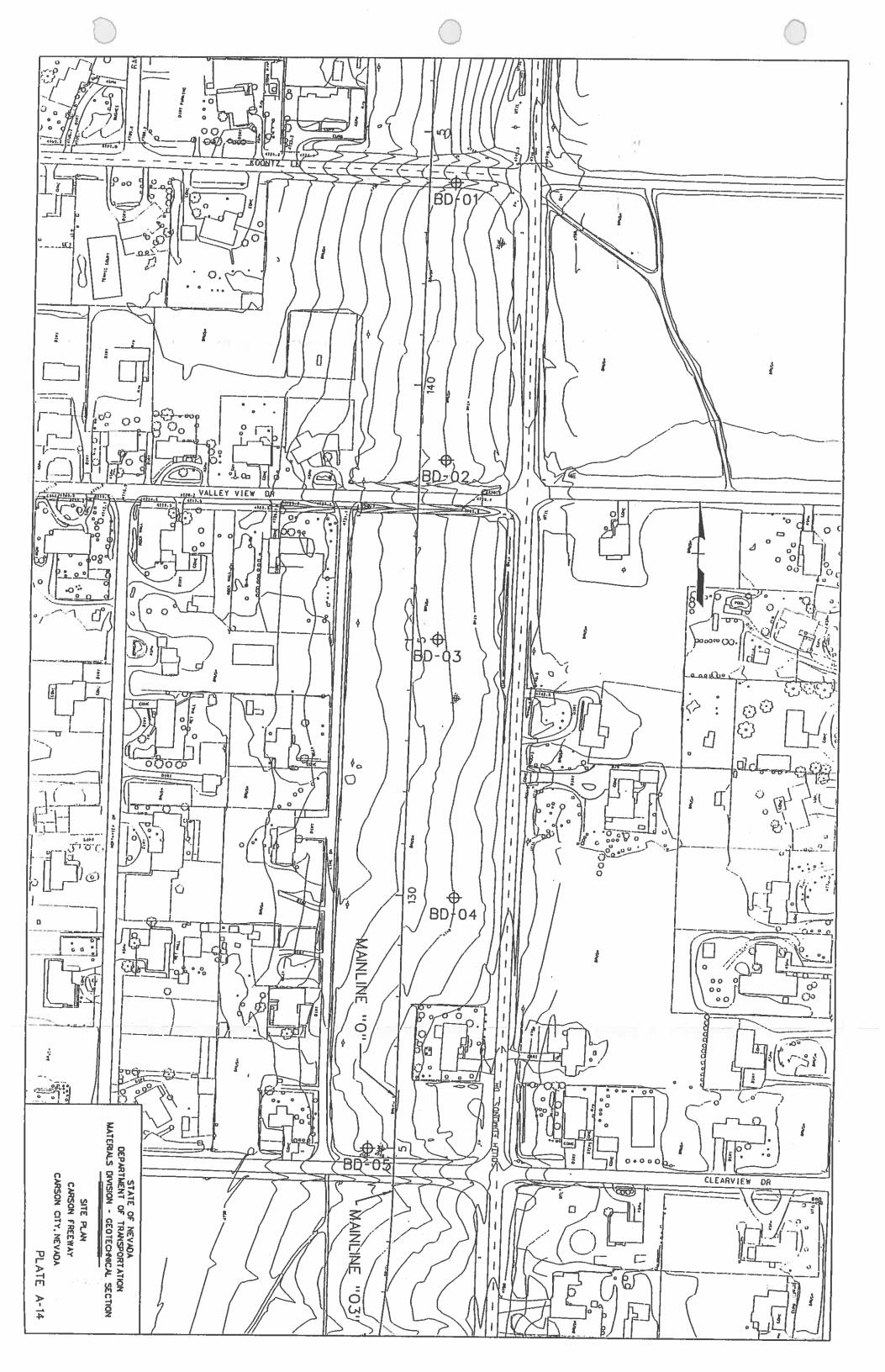






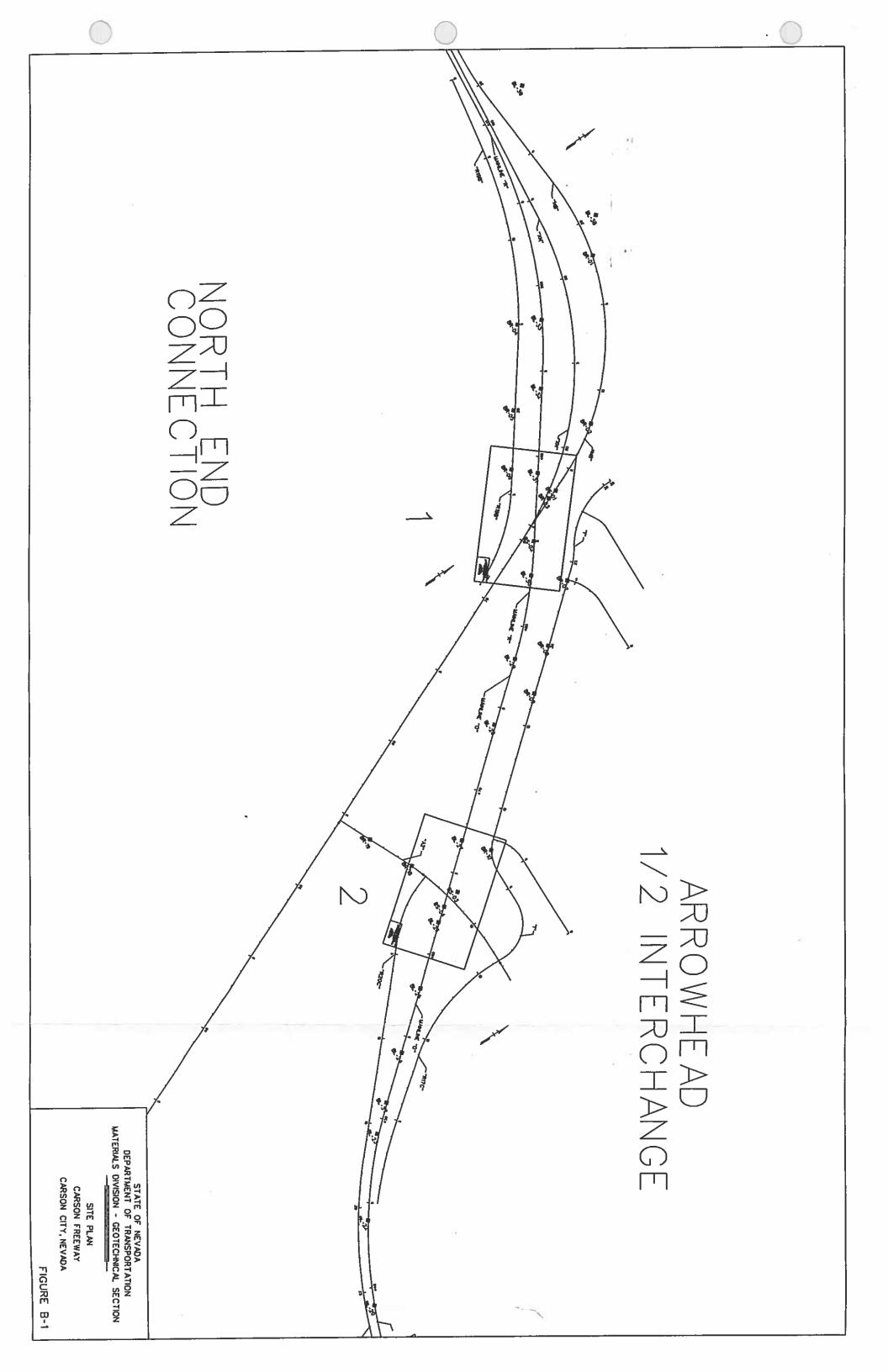


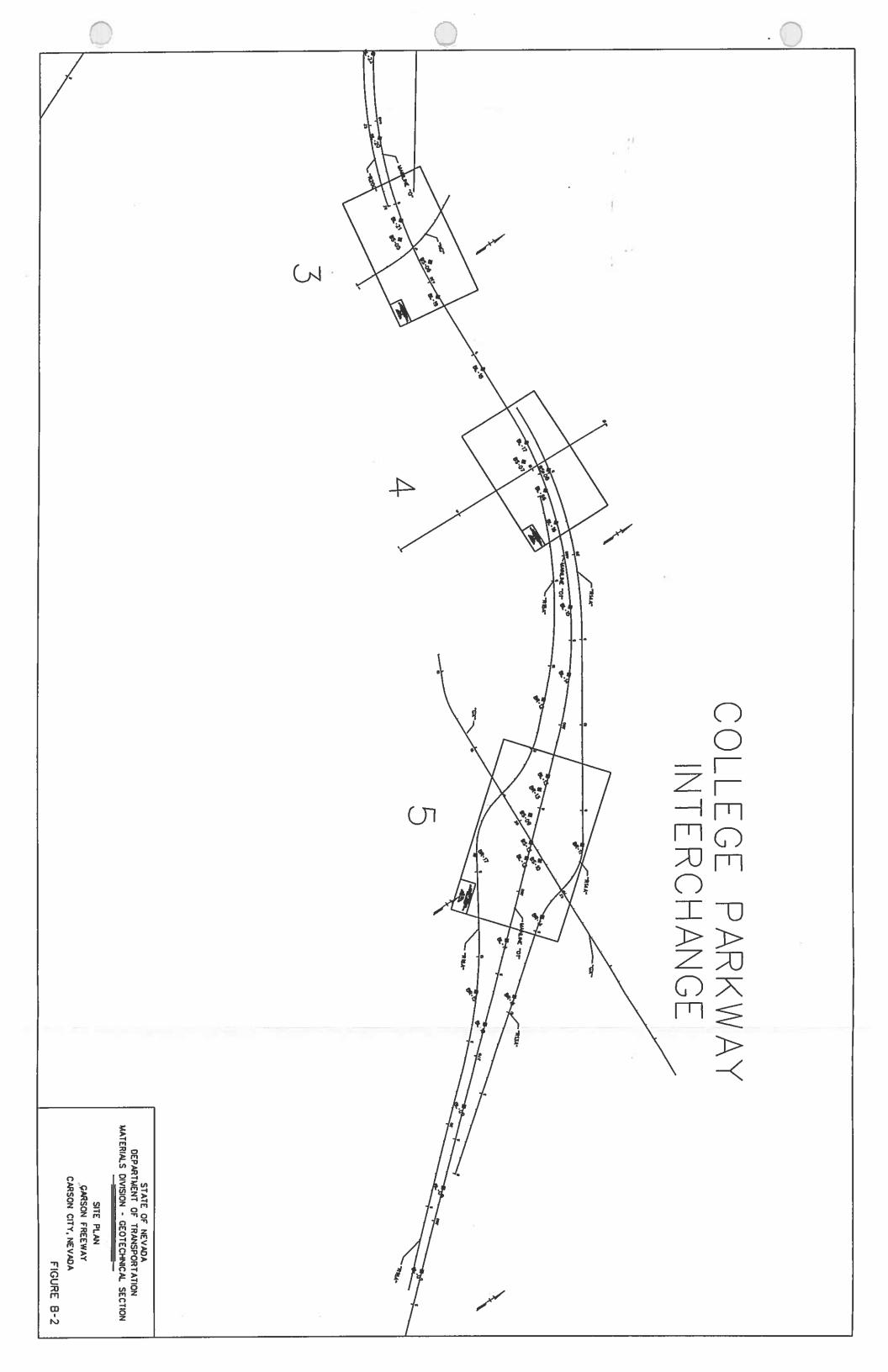


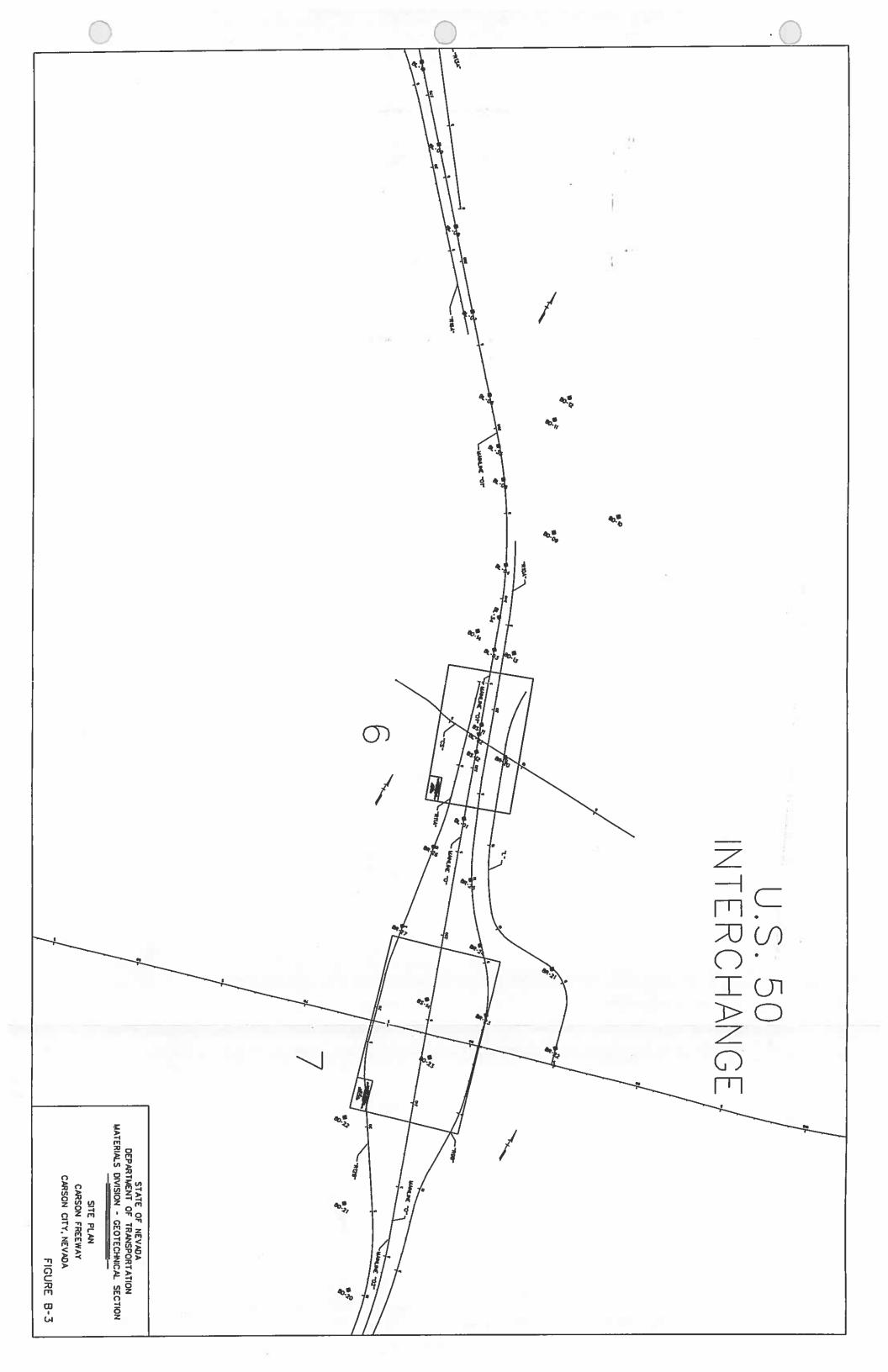


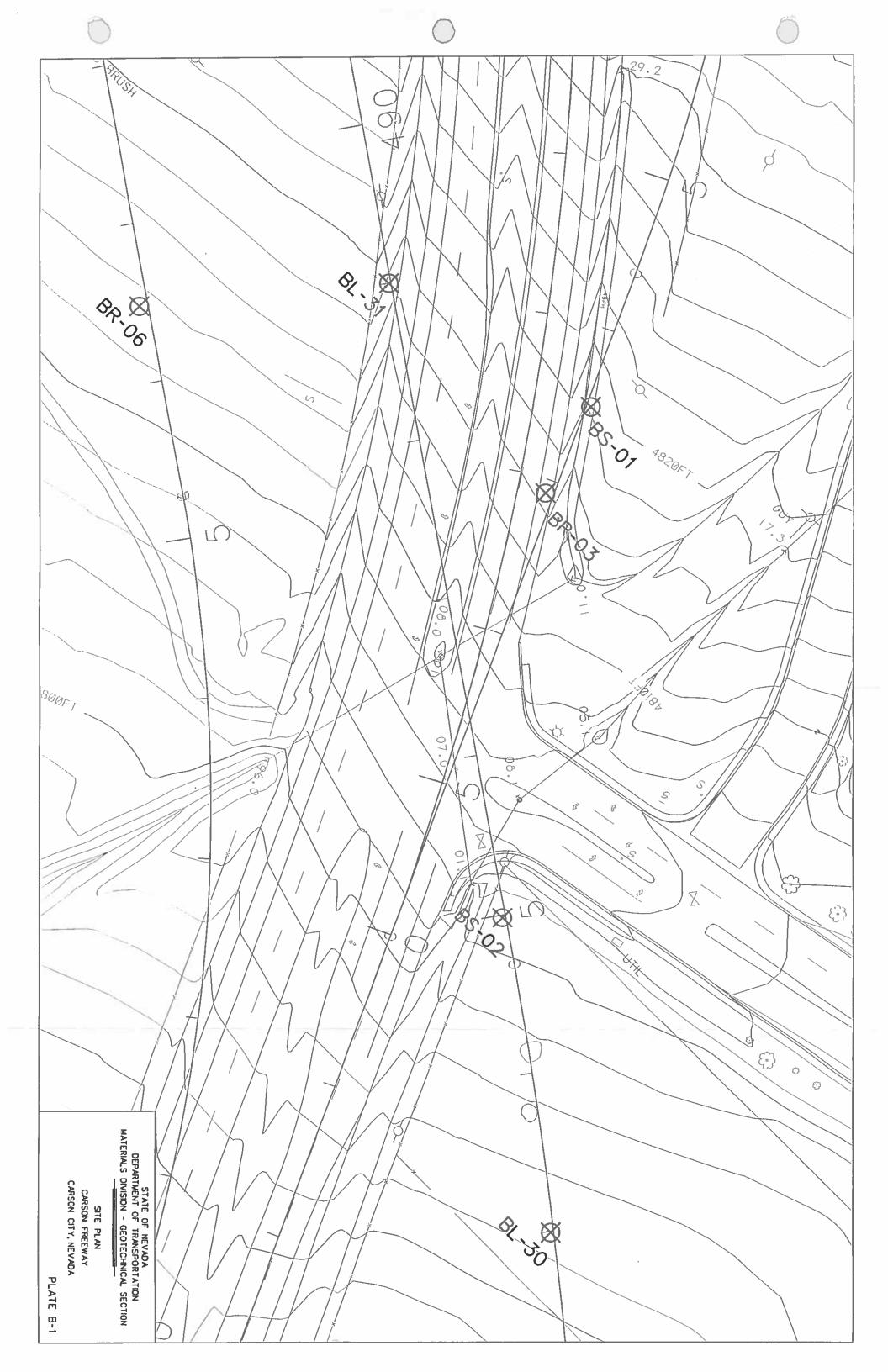
APPENDIX B

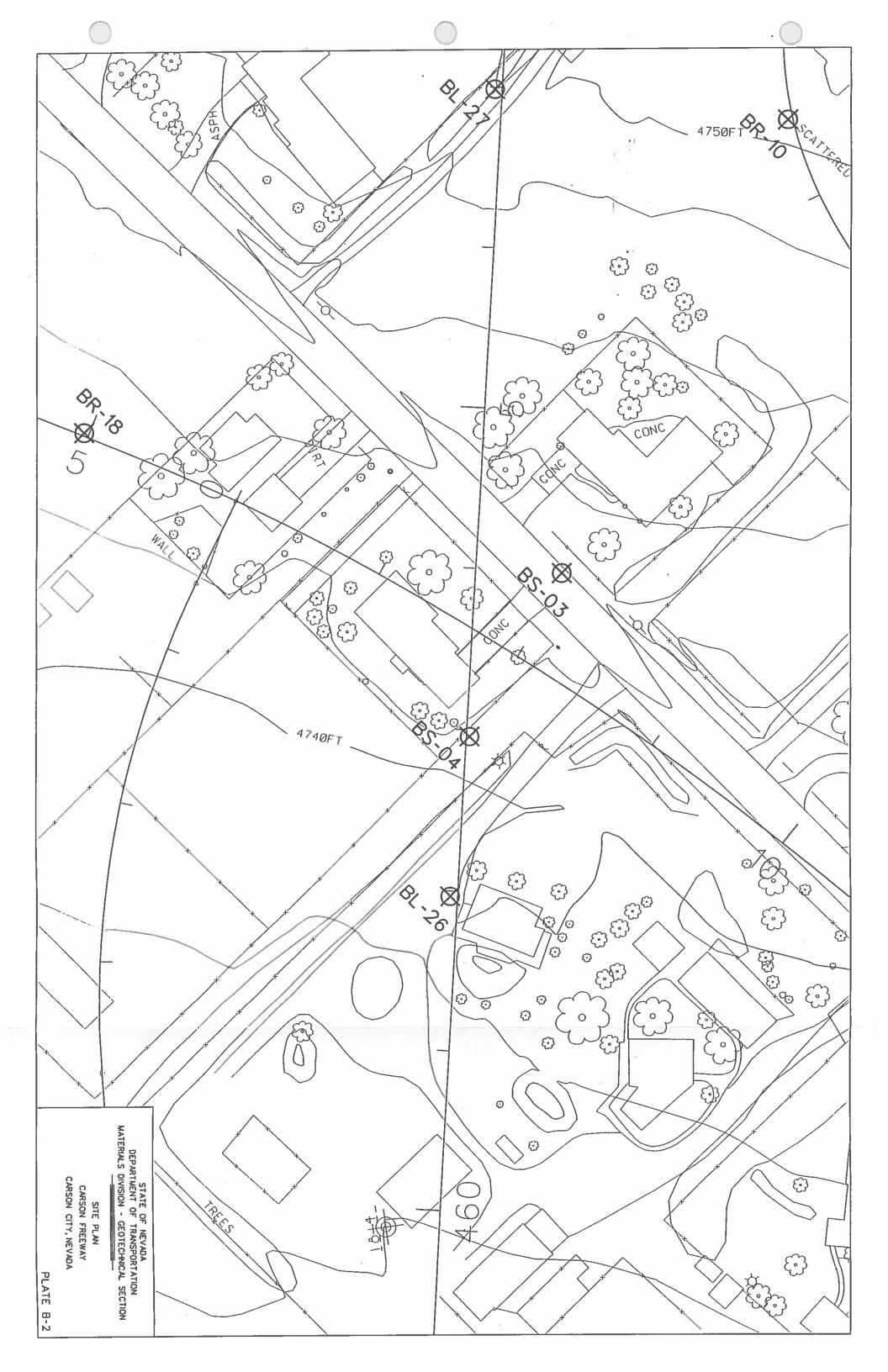
Bridge Structure Site Plans

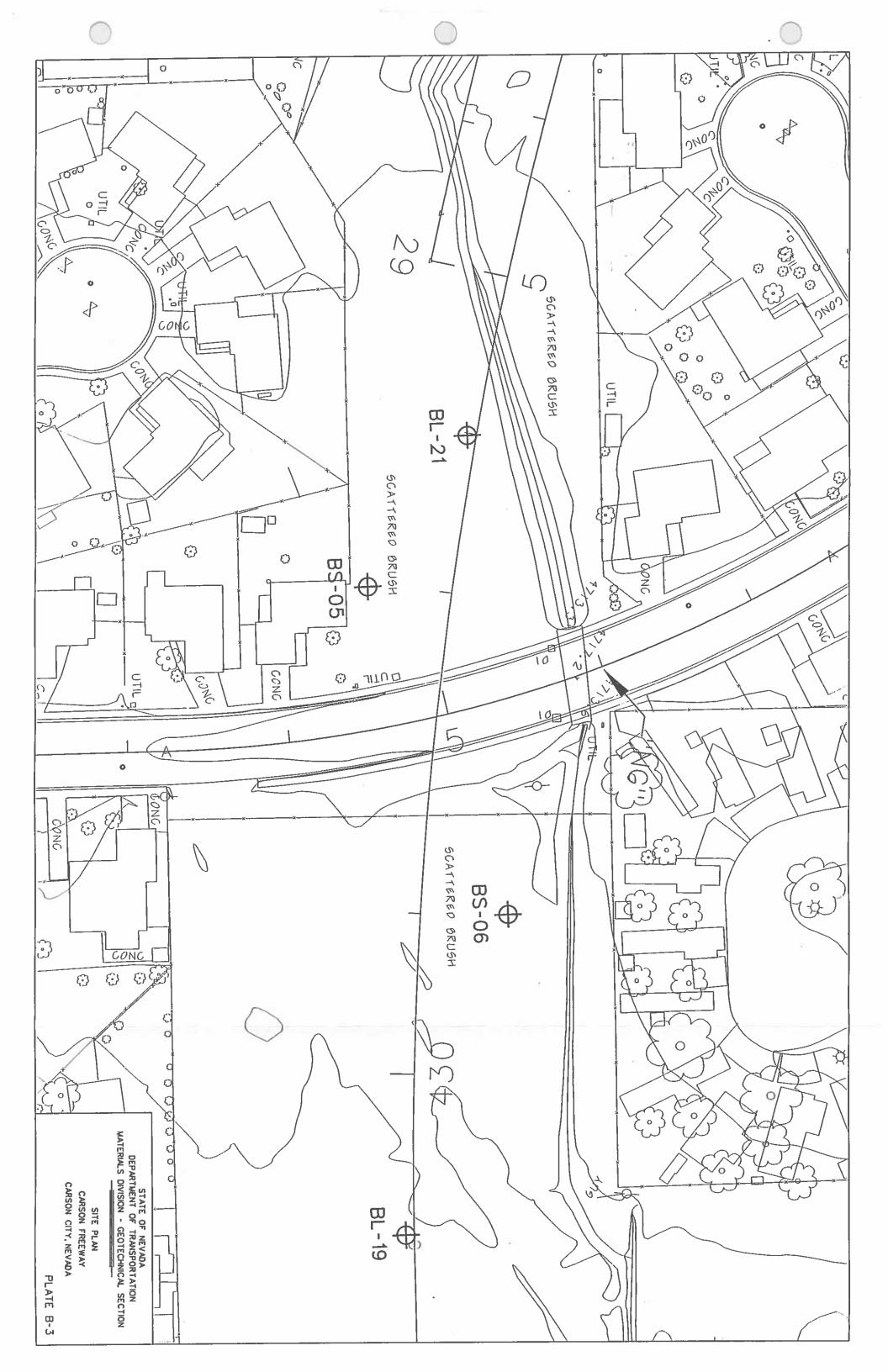


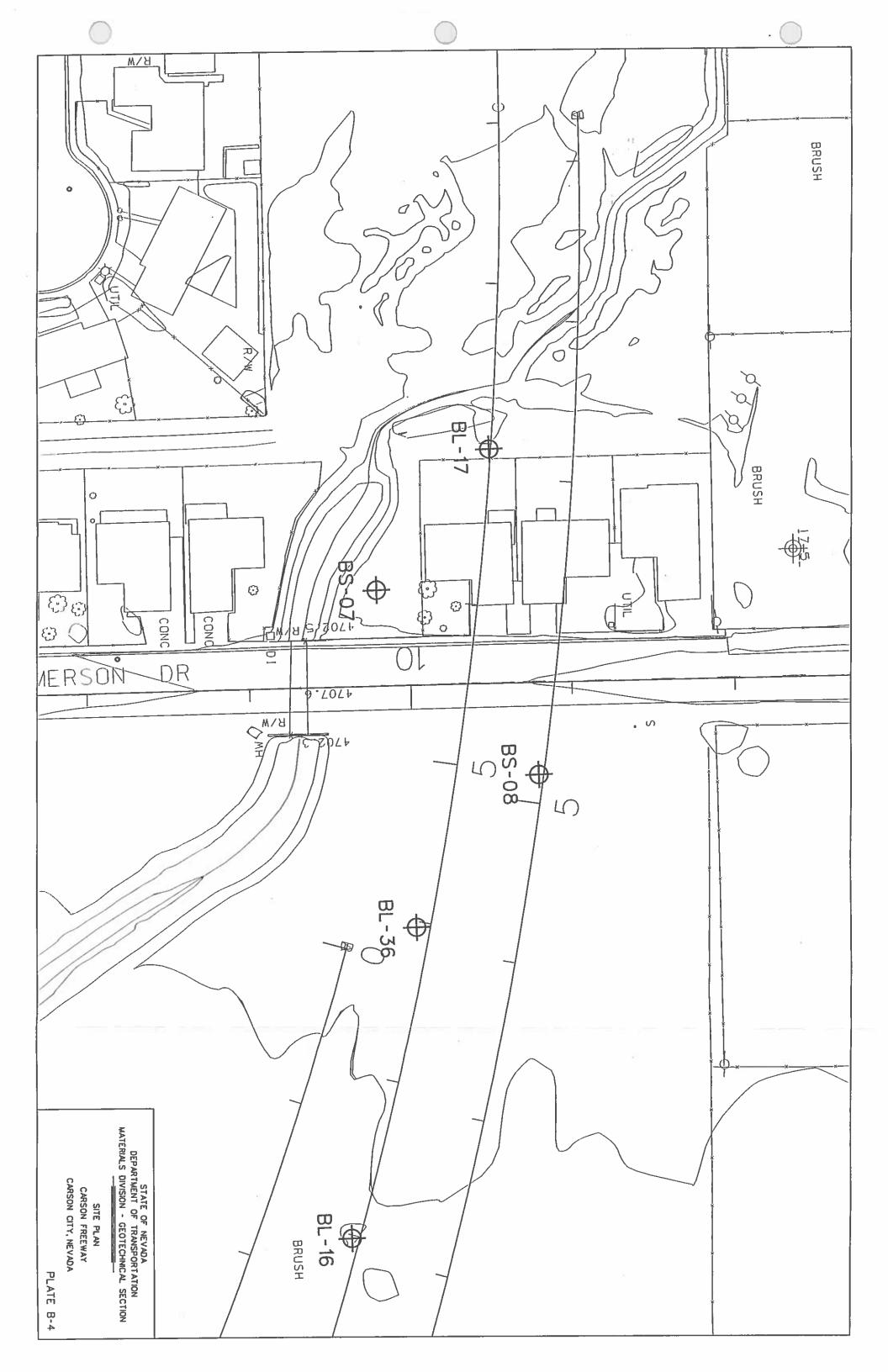


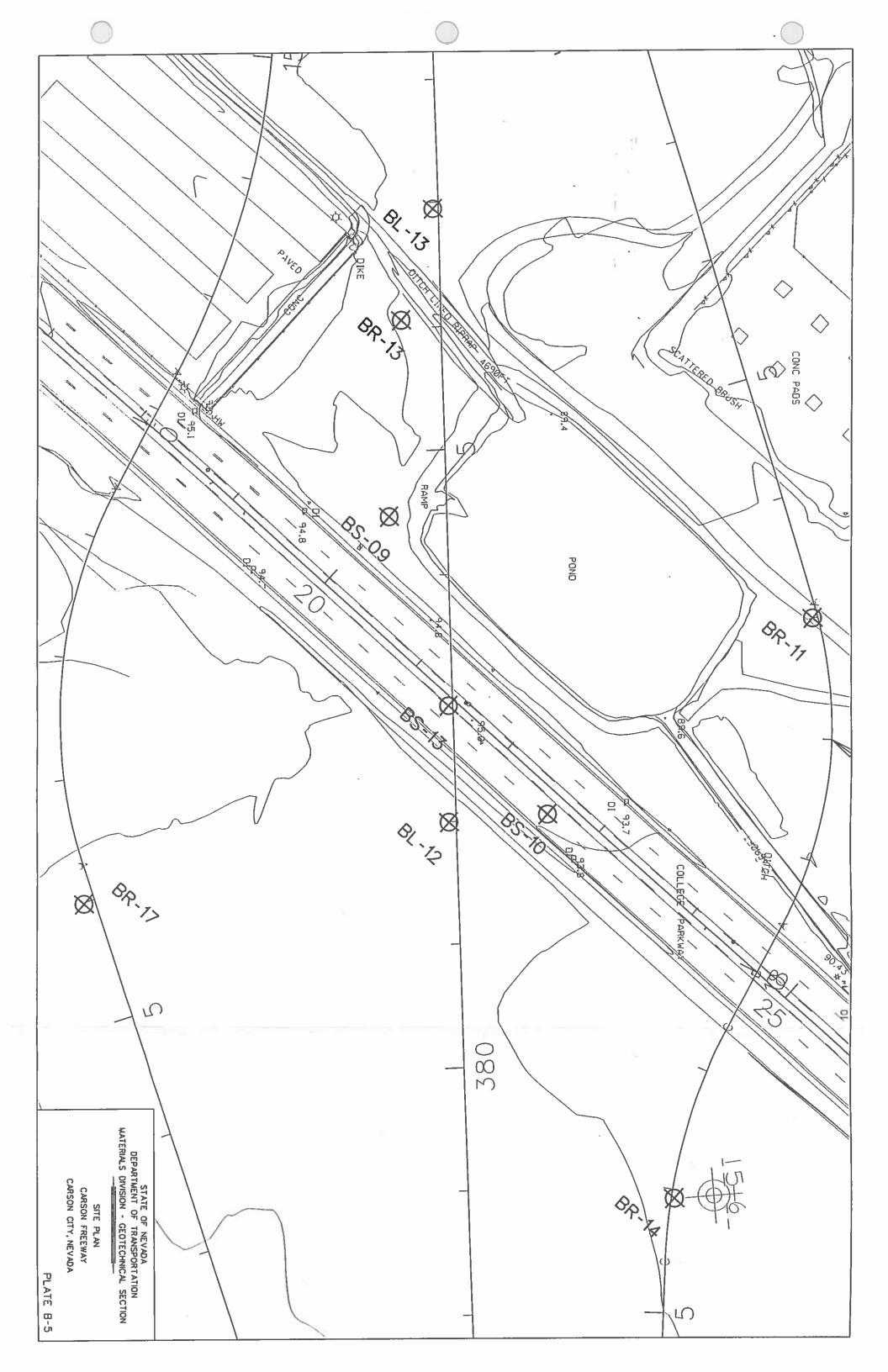


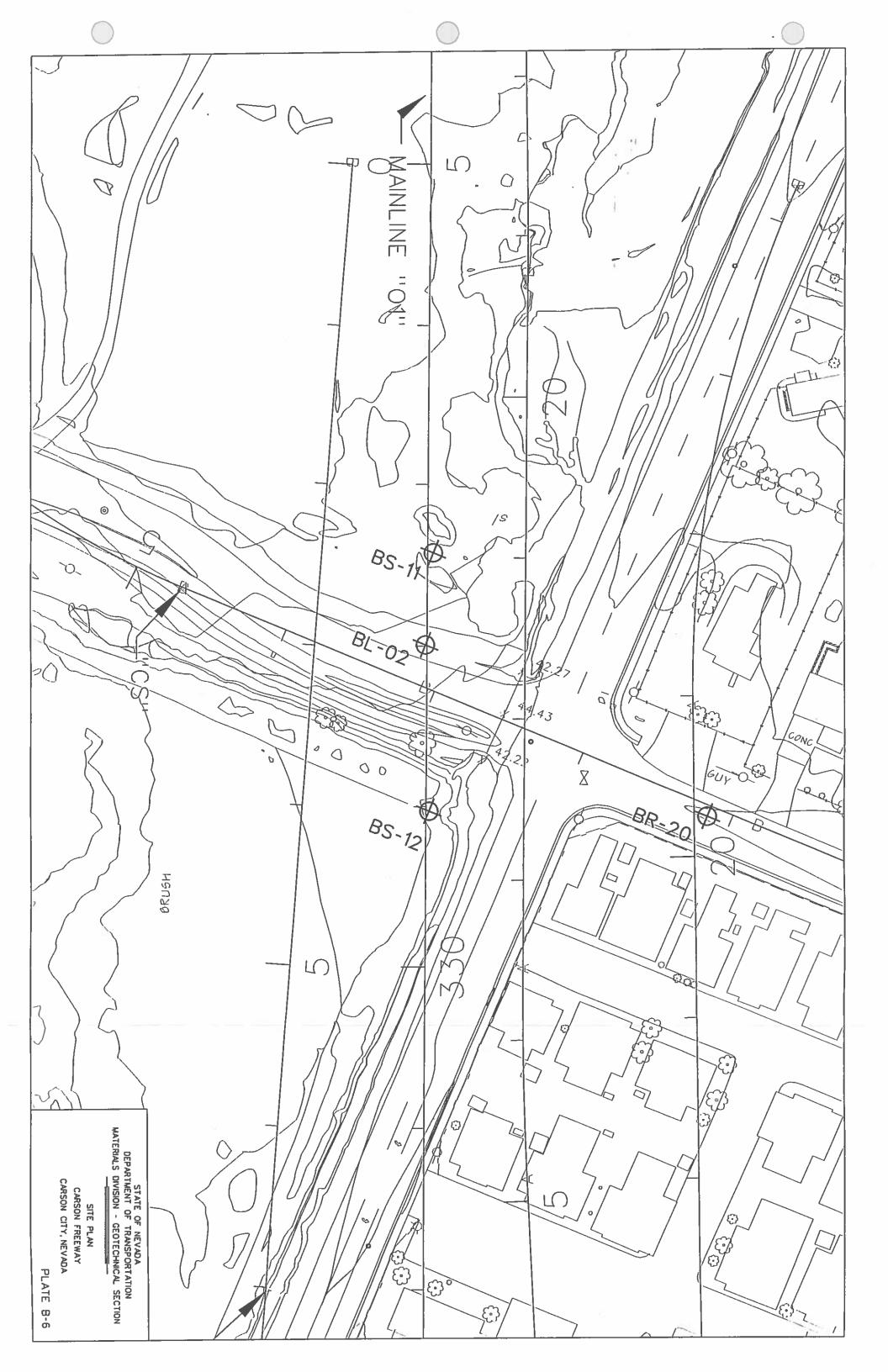


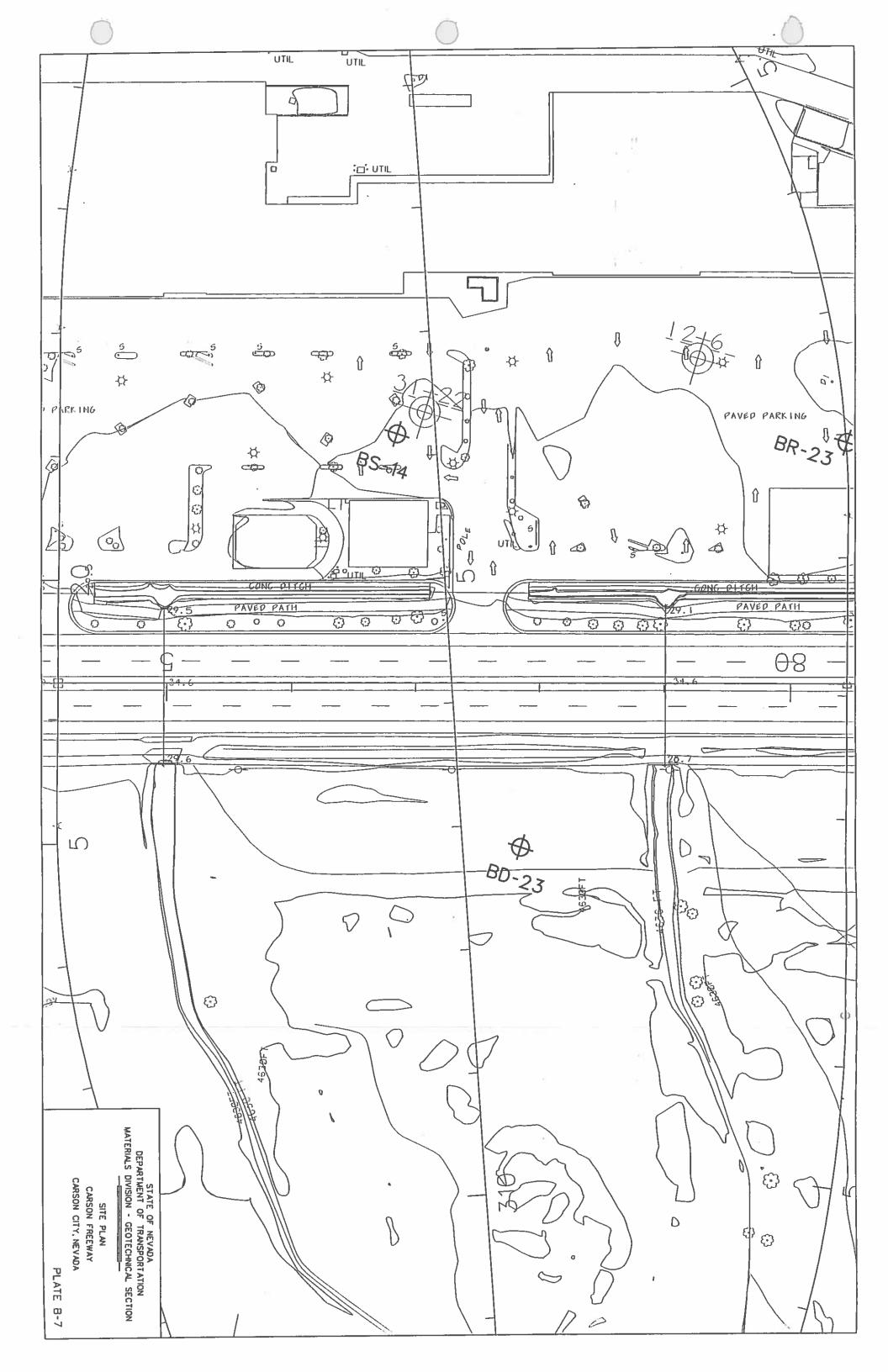












APPENDIX D

Structure Borings and Laboratory Test Results

USCS GROUP	TYPICAL SOIL DESCRIPTION
GW	Well graded gravels, gravel-sand mixtures, little or no fines
GP	Poorly graded gravels, gravel-sand mixtures, little or no fines
GM	Silty gravels, poorly graded gravel-sand-silt mixtures
GC	Clayey gravels, poorly graded gravel-sand-clay mixtures
SW	Well graded sands, gravelly sands, little or no fines
SP	Poorly graded sands, gravelly sands, little or no fines
SM	Silty sands, poorly graded sand-silt mixtures
SC	Clayey sands, poorly graded sand-clay mixtures
ML	Inorganic silts and very fine sands, rock flour, silty or clayey fine sands with slight plasticity
CL	Inorganic clays of low to medium plasticity, gravelly clays, sandy clays, silty clays, lean clays
OL	Organic silts and organic silt-clays of low plasticity
MH	Inorganic silts, micaceous or diatomaceous fine sandy or silty soils, elastic silts
СН	Inorganic clays of high plasticity, fat clays
OH	Organic clays of medium to high plasticity
CE	Caliche
PT	Peat and other highly organic soils

MOISTURE CONDITION CRITERIA

SOIL CEMENTATION CRITERIA

Description	Criteria	Description	Criteria
Dry	Absence of moisture, dusty, dry to	Weak	Crumbles or breaks with handling or
-	touch.		little finger pressure.
Moist	Damp, no visible water.	Moderate	Crumbles or breaks with considerable finger pressure.
Wet	Visible free water, usually below water table.	Strong	Will not crumble or break with finger pressure.

	TANDARD PENETRATION	CLASSIFICA	TION*	
GF	RANULAR SOIL	CL	AYEY SOIL	
BLOWS/0.3m	DENSITY	BLOWS/0.3m	CONSISTENCY	
0-4	VERY LOOSE	0-1	VERY SOFT	
5-10	LOOSE	2-4	SOFT	
11 - 30	MEDIUM DENSE	5-8	MEDIUM STIFF	
31 – 50	DENSE	9 – 15	STIFF	
OVER 50	VERY DENSE	16 – 30	VERY STIFF	
*Standard Penetrat	ion Test (N) 63.5 Kg hammer	31 – 60	HARD	
760mm free fall on :	50.8mm O.D. x 35mm I.D. sampler.	OVER 60	VERY HARD	

Blow counts on California Split Spoon (N_{cas}) can be converted to N_{spt} by: (N_{cas})(0.563) = N_{spt}

Blow counts from Automatic Hammer can be converted to Standard $N_{\rm spt}$ by: $(N_{\rm Automatic\ Hammer})(1.33) = N_{\rm spt}$

TEST	Γ ABBREVIATIONS			SAMPLER NOTATION
CD CH CM CU D DS E G H HC K	CONSOLIDATED DRAINED CHEMICAL (CORROSIVENESS) COMPACTION CONSOLIDATED UNDRAINED DISPERSIVE SOILS DIRECT SHEAR EXPANSIVE SOIL SPECIFIC GRAVITY HYDROMETER HYDRO-COLLAPSE PERMEABILITY	O OC PI RQD RV S SL U UU UW W	ORGANIC CONTENT CONSOLIDATION PLASTICITY INDEX ROCK QUALITY DESIGNATION R-VALUE SIEVE ANALYSIS/-200 WASH SHRINKAGE LIMIT UNCONFINED COMPRESSION UNCONSOLIDATED UNDRAINED UNIT WEIGHT MOISTURE CONTENT	CPT CONE PENETRATION CS CONTINUOUS SAMPLER(1) MC MODIFIED CA SPLIT SPOON (2) P PUSHED (NOT DRIVEN) PB PTICHER BARREL RC ROCK CORE(3) SH SHELBY TUBE (4) SPT STANDARD PENETRATION TEST TP TEST PIT (1) LD. = 82mm with tube; \$8.9mm w/o tube (2) LD. = 61.5mm (3) NXW (4) LD. = 73mm

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KEY TO SOIL CLASSIFICATION AND TERMS

CARSON FREEWAY
CARSON CITY, NEVADA

PLATE

D-1

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PROJECT NO. 30-1348-15.002

LOG OF BORING

CARSON FREEWAY

CARSON CITY, NEVADA

PLATE

D-2

EXPLORATION LOG START DATE: 4/29/97 SHEET 2 OF 2 4/29/97 END DATE: STATION STA 488+00 JOB DESCRIPTION _ CARSON FREEWAY 30.48 M RIGHT OFFSET EAST STRUCTURE U.S. 395 J. FORGA LOCATION ENGINEER B\$-01 EQUIPMENT _CME 85 **BORING** KLEINFELDER 30-1348-15.002 GROUNDWATER LEVEL OPERATOR _SPECTRUM E.A. # DATE | DEPTH | ELEV. GROUND ELEV 1466.09 m MUD ROTARY HAMMER DROP SYSTEM MANUAL BACKFILLED YES DATE 4/29/97 DEPTH ELEV. BLOWS/ Recovery LAB TESTS MATERIAL DESCRIPTION NO. TYPE REMARKS <u>1498)1</u> (m)10.67 G MC 51 W,UW,\$ 10.85 Encountered some fine gravel. 32% fines - 11 11,13 1455.1 SC RED BROWN CLAYEY SAND moist, very dense, red brown, fine to coarse grained sand, estimated 20 to 30% fines, moderate to high plasticity fines 1454.1 12 MC 50/140 W,UW Becoming more sandy, estimated 10 to 20% 12.65 1453.1 13 MC 50/140 W,UW Could not measure 1452.1 144,02 14.02 water level due to use of mud rotary B.O.H.

1448.1 18

1451.1

1450.1

1449.1

1447.1

.. 19

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LOG OF BORING

CARSON FREEWAY

CARSON CITY, NEVADA

PLATE

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4875 LONGLEY LANE, SUITE 100 RENO, NEVADA 89502 Tel. (702) 689-7800 LOG OF BORING
CARSON FREEWAY

0774 1171121

PLATE

D-3

PROJECT NO. 30-1348-15.002

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KLEINFELDER

4/29/97 START DATE: __

4/29/97

JOB DESCRIPTION CARSON FREEWAY

WEST STRUCTURE U.S. 395

LOCATION BORING

END DATE:

30-1348-15.002 E.A. # GROUND ELEV 1466.09 m

HAMMER DROP SYSTEM MANUAL

EXPLORATION LOG

GROUNDWATER LEVEL

DATE | DEPTH | ELEV.

STA 485+00

SHEET 2 OF 2

STATION OFFSET

J. FORGA **ENGINEER**

EQUIPMENT _CME 85

OPERATOR SPECTRUM

DRILLING METHOD

MUD ROTARY

BACKFILLED YES DATE 4/29/97

				H DHOP			_	BACKPILLED 1E3 L	AIE -7/53/31
ELEV. 1.498)1	DEPTH (m)	NO.	TYPE	IBLOWS/	Recovery (%)	LAB TESTS	USC:	MATERIAL DESCRIPTION	REMARKS
	}								
	t					:			
	10.67	G	MC	50/292		w,uw,s			27% fines
1455.1	111.13								
	ł							0.	
1454.1	12								
	12.19	Н	МС	127					
	12.65							Low plasticity fines	
1453.1	13								
	-								
	13.72								
1452.1	- 13.87 _ 14	1	MC	127				13.87	Could not measure water level due to
1432.1	F '*						,		use of mud rotary
	-								
	-								
1451.1	_ 15								
	-				ĺ				
						ĺ			
1450.1	_ 16								
	[
	-					ľ			
1449.1	17								
	-								
	[1						
1448.1	_ 18								
	-								
1447.1	- 10			-					
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PROJECT NO. 30-1348-15.002

CARSON FREEWAY

LOG OF BORING

CARSON CITY, NEVADA

PLATE

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PROJECT NO. 30-1348-15.002

LOG OF BORING

CARSON FREEWAY

CARSON CITY, NEVADA

D-4

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LOG OF BORING CARSON FREEWAY

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PROJECT NO. 30-1348-15.002

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PROJECT NO. 30-1348-15.002

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RENO, NEVADA 89502 Tel. (702) 689-7800

LOG OF BORING CARSON FREEWAY

CARSON CITY, NEVADA

PLATE

CARSON FREEWAY

CARSON CITY, NEVADA

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PROJECT NO. 30-1348-15.002

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					4/30/07	0		EXPLORATION LOG		
_		_	START END DA	DATE:	4/30/97				STATION STA 433+0	SHEET 1 OF 2
				· · · · · · ·		SON FREEWA	AY		OFFSET 15.24 M LE	
			LOCATI			RUCTURE, N		ATE LANE	ENGINEER J. FORGA	
	.,,		BORING	OI -	BS-05				EQUIPMENT CME 85	
KL	EINFELD	EA		. –	30-1348-1	5.002		GROUNDWATER LEVEL	OPERATOR SPECTRUM	Λ
			E.A. #	ID ELEV_				DATE DEPTH ELEV.	DRILLING METHOD MUD ROTA	DV.
				R DROP S					BACKFILLED YES D	
									BACKFIELED 160 D	ATE
ELEV.	DEPTH (m)	NO.	TYPE	AMPLE BLOWS/ 300mm	Recovery	LAB TESTS	USC S	MATERIAL DE	SCRIPTION	REMARKS
							SM	LIGHT BROWN SILTY S medium dense, line to estimated 20 to 30% sil	coarse grained sand,	
1435.5	- _ 1 -						СН		AY moist, stiff to very stiff,	
	1.52	À	MC	16		w,uw,s	1	1.83	3	25% fines
1434.5	2 1.98						sc	LIGHT BROWN CLAYE	Y SAND moist, medium rained sand, moderate to	
1433.5	- 3 <u>3.05</u> - 3.51	В	MC	3		w,uw,s	-	Very loose, fine grained	sand, estimated 15 to	29% fines
	3,66		110]	25% fines		
1432.5	_ 4	С	MC	9			i I			
1402.0	4.11			l l	<u> </u>		-	4 27 Loose		
-	- 2						CL	BED BROWN SANDY O	LAY moist, very stiff, very	
-	4,57	D	MC	21		W,UW		fine grained sand, mod	erate plasticity fines.	
1431.5	_ 5 5.03						SM	4.88		
							SM	YELLOW BROWN SLIG medium dense, fine to estimated 5 to 12% fine		
1430.5		ļ					sc		4000	
1430.5	- ⁶ _{6.10}	Ε	MC	13		s.oc		LIGHT BROWN CLAYED dense, fine to coarse grand high plasticity fines.	ained sand, moderate to	15% fines
1429.5	6.55 _ 7									-
	7.62			-		1011111111111111		Pagemina gray and	non alaetic	
		F	MC	45		W,UW,S,PI		Becoming more sandy,	numpiasus,	17% fines
1428.5	- ⁸ 8.08									
[8 53		
1427.5	- 9 9.14						CL	LIGHT BROWN SILTY C trace of fine sand, mode fines.	LAY moist, very stiff, erate to high plasticity	
-	9.60	G	МС	22		W,UW,S,PI				Pl=32 62% fines
	3.001									
, by Kleinfe						E P		LOG OF B	BORING	PLATE
	Ī	V L	4875 LON	JFE GLEY LANE, D. NEVADA (702) 689-	SUITE 100 89502	ER		CARSON F		D-6

PROJECT NO. 30-1348-15.002

D-6

4/30/97

START DATE:

EXPLORATION LOG

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4875 LONGLEY LANE, SUITE 100 RENO, NEVADA 89502 Tel. (702) 689-7800

LOG OF BORING CARSON FREEWAY

CARSON CITY, NEVADA

PLATE

SHEET 2 OF 2

REMARKS

24% fines

25% fines

16% fines

Could not measure

water level due to use of mud rotary

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PROJECT NO. 30-1348-15.002

START DATE: 4/30/97

END DATE:

LOCATION

4/30/97

JOB DESCRIPTION CARSON FREEWAY

EAST STRUCTURE, NORTHGATE LANE

B\$-06

BORING

E.A. #

30-1348-15.002

GROUND ELEV 1436.52 m

HAMMER DROP SYSTEM MANUAL

EXPLORATION LOG

GROUNDWATER LEVEL

DATE DEPTH ELEV.

STA 431+00 STATION

15.24 M RIGHT OFFSET ENGINEER J. FORGA

EQUIPMENT _ CME 85

OPERATOR SPECTRUM

MUD ROTARY

SHEET 1 OF 2

BACKFILLED YES DATE 4/30/97

ELEV. (m)	DEPTH (m)	NO.	TYPE	AMPLE BLOWS/ 300mm	Recovery	LAB TESTS	Giod	MATERIAL DESCRIPTION	REMARKS
							SM	LIGHT BROWN SILTY SAND moist, loose to medium dense, fine to coarse grained sand, estimated 20 to 30% silt.	
435.5	_1						СН	LIGHT BROWN SLIGHTLY SANDY CLAY moist, medium stiff, fine to medium grained sand, high plasticity fines.	
	1.52	Α	МС	6			1		
434.5	2 1.98						SC	1.83	
	2.13	В	МС	3				LIGHT BROWN CLAYEY SAND moist, very loose, fine to medium grained sand, low to moderate plasticity fines, estimated 10 to 20% fines	
	-					oc	СН	1 ICHT PROMAISTICHT V CANDY CLAY	
433.5	_ 3 3.05	С	МС	7			SC	3.05 LIGHT BROWN SUGHTLY SANDY CLAY moist, medium stiff, fine grained sand, high plasticity fines.	
	3.51			1				LIGHT BROWN CLAYEY SAND moist, loose, fine to medium grained sand, low plasticity fines.	
132.5	3.96	D	MC	25				4 27	29% fines
	4.42						SP	LIGHT BROWN CLAYEY SAND moist, loose, fine	
31.5	_ 5 5.03	Ε	MC	31		W,UW,S	SM	than 5% silt. GRAY BROWN SILTY SAND moist, fine to medium grained sand.	19% fines
30.5	_ 6 _{6.10}							i i	
	6.55	F	мС	32		s,os			26% fines
29.5	7					i			
}	-				ĺ	ŀ	CL	7.32	
	7.62	G	MC	4				LIGHT BROWN SLIGHTLY SANDY CLAY moist, very loose to loose, fine grained sand, moderate to high plasticity fines.	
28.5	- 8 a 08							נט וויקוו אומטונטוגץ וווודט.	
	8.53		110	17		ALIM S CI			
27.5	9 8.99	Н	MC	17		W,UW,S,PI		8,99	PI=34 81% fines
3	9.14	1	мс	65		3	SM SP	GRAY BROWN SLIGHTLY SILTY SAND moist, very dense, fine to medium grained sand.	7% fines
-	9.60	-		<u> </u>					



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CARSON FREEWAY

LOG OF BORING

PLATE

PROJECT NO. 30-1348-15.002

CARSON CITY, NEVADA

L\1995

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KLEINFELDER

START DATE: _

4/30/97

4/30/97

JOB DESCRIPTION __CARSON FREEWAY

EAST STRUCTURE, NORTHGATE LANE

LOCATION **BORING**

E.A. #

END DATE:

30-1348-15.002

GROUND ELEV 1436.52 m

HAMMER DROP SYSTEM MANUAL

EXPLORATION LOG

GROUNOWATER LEVEL

DATE DEPTH ELEV.

STATION

SHEET 2 OF 2 STA 431+00

OFFSET

15.24 M RIGHT

ENGINEER

J. FORGA

EQUIPMENT _ CME 85

OPERATOR SPECTRUM

DRILLING METHOD

MUD ROTARY

						SYSTEM_N			BACKFILLED YES DATE 4/30)/97
ļ	ELEV.	OEPTH (m)	NO.	TYPE	AMPLE BLOWS/ 300mm	Recovery	LAB TESTS	USCS	MATERIAL DESCRIPTION REMA	RKS
	1425.5	- 10.67 - 11.13	J	МС	29		w,uw	SC	BED BROWN CLAYEY SAND moist, medium dense, fine to coarse grained sand, low to moderate plasticity fines, estimated 20 to 30% fines.	
	1424.5	12 12 12.19	i							
	1423.5	- 12.65 - 13	к	MC	24		WU,W		Estimated 10 to 20% fines, low plasticity fines.	
	1420.0	- "					ļ	SM	13.25	
		13.72						2141	GRAY WHITE SILTY SAND moist, very dense, fine to coarse grained sand.	
	1422.5	144.02	L	MC	50/140		S		14,02 Could not r	neasure
	1421.5	_ 15							use of mud	drotary
	1420.5	_ 16								35
	1419.5	_ 17								
1	418.5	. 18					ļ			:
1	417.5	. 19								

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CARSON FREEWAY

LOG OF BORING

CARSON CITY, NEVADA

PLATE

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PROJECT NO. 30-1348-15.002

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4875 LONGLEY LANE, SUITE 100 RENO, NEVADA 89502 Tel. (702) 689-7800 LOG OF BORING

CARSON FREEWAY

U

PLATE

PROJECT NO. 30-1348-15.002

CARSON CITY, NEVADA

CAD FILE: L'1999 TINC\301348\301348150020PLATES.DWG

.

START DATE: __5/1/97

5/1/97 END DATE:

JOB DESCRIPTION __CARSON FREEWAY

LOCATION BORING

WEST STRUCTURE, EMERSON DRIVE

BS-07

E.A. #

30-1348-15.002

GROUND ELEV_ 1433.17 m

HAMMER DROP SYSTEM MANUAL

EXPLORATION LOG

GROUNDWATER LEVEL

DATE | DEPTH | ELEV.

STATION

STA 406+00

SHEET 2 OF 2

OFFSET

15.24 M SOUTH

ENGINEER

_J. FORGA

EQUIPMENT _ CME 85 **OPERATOR**

_SPECTRUM

DRILLING METHOD

MUD ROTARY

BACKFILLED . YES DATE 5/1/97

				R DROP S				BACKFILLED YES DATE 5/1/97
ELEV.	DEPTH (m)	NO.	TYPE	AMPLE BLOWS/ 300mm	Recovery	LAB TESTS	USC	MATERIAL DESCRIPTION REMARKS
	-						SC	LIGHT BROWN CLAYEY SAND moist, dense, fine
	10.67		1					to medium grained sand, low to moderate plasticity fines, estimated 20 to 30% fines,
	-	l ï	MC	41				, , , , , , , , , , , , , , , , , , , ,
1422.2	_ 11	ļ						
	Ţ							
	-							
1421.2	12							
	12.19							
	}	J	МС	92		<u> </u>		_12.50
	12.65						SP SM	GRAY BROWN SLIGHTLY SILTY SAND moist,
1420.2	_ 13		i					dense to very dense, fine to coarse sand, estimated 5 to 12% silt.
	-							
			i					
	13.72	К	MC	38				
1419.2	_ 14 14.17!						SÇ	14.02
			,				30	LIGHT BROWN CLAYEY SAND moist, dense, fine
	-	ļ						to medium grained sand, low to moderate plasticity fines, estimated 20 to 30% fines.
1418.2	15					1		14 94
1410.2	15.24					. [SC CL	BROWN CLAYEY SAND moist, dense/hard, fine
	-	L	мс	32		W,UW		to coarse grained sand, moderate to high plasticity lines.
	15.70							presiony inico.
1417.2	_ 16	- 1		ŀ			ĺ	İ
	-							
Ţ	.					1		
416.2	_ 17		-					
	.	- 1		İ				
[.	ĺ		- 1		- 1		
				İ				17 98
415.2	- 18			- 1		ŀ	sc	
-	18.29	М	MC	45				AROWN CLAYEY SAND moist, dense, fine to medium grained sand, moderate plasticity fines. Could not measure the country of the c
-	18.75							estimated 15 to 25% fines. water level due to use of mud rotary
414.2 E	. 19							and the state of t
-								
-								
į								
Γ				- 3				

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CARSON FREEWAY

LOG OF BORING

PLATE

PROJECT NO. 30-1348-15.002

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PROJECT NO. 30-1348-15.002

4875 LONGLEY LANE, SUITE 100 RENO. NEVADA 89502 Tel. (702) 689-7800 CARSON FREEWAY

CARSON CITY, NEVADA

D-9

EXPLORATION LOG START DATE: ____5/1/97 SHEET 2 OF 2 5/1/97 END DATE: STA 405+00 **STATION** JOB DESCRIPTION CARSON FREEWAY 15.24 M RIGHT OFFSET EAST STRUCTURE, EMERSON DRIVE ENGINEER J. FORGA LOCATION **CME 85** BS-08 EQUIPMENT _ **BORING** KLEINFELDER **SPECTRUM OPERATOR** 30-1348-15.002 **GROUNDWATER LEVEL** E.A. # DRILLING METHOD DATE | DEPTH | ELEV. GROUND ELEV 1433.17 m MUD ROTARY HAMMER DROP SYSTEM MANUAL BACKFILLED YES DATE 5/1/97 ELEV. DEPTH BLOWS/ Recovery LAB TESTS MATERIAL DESCRIPTION REMARKS NO. TYPE 14692 (m) LIGHT BROWN CLAYEY SAND moist, medium 36% fines 10,21 dense, fine to medium grained sand, moderate plasticity fines. 10.67 MC 28 1422.2 1421.2 MC 43 SM SP 12.65 GRAY BROWN SLIGHTLY SILTY SAND moist, dense, fine to coarse grained sand, estimated 5 1420.2 13 to 12% silt. SC BROWN SLIGHTLY CLAYEY SAND moist, very dense, fine to medium grained sand, low MC 66 Could not measure 1419.2 14 water level due to plasticity fines. use of mud rotary 1418.2 _ 15 1417.2 _ 16 1416.2 17 1415.2 . 18 1414.2 19

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PROJECT NO. 30-1348-15.002

LOG OF BORING

CARSON FREEWAY

CARSON CITY, NEVADA

PLATE

D-9A

CAD PAGE 12\1995

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START DATE: _5/1/97

5/1/97

JOB DESCRIPTION CARSON FREEWAY

LOCATION

E.A. #

END DATE:

NORTH STRUCTURE, COLLEGE PARKWAY

BORING

BS-09

30-1348-15.003

GROUND ELEV_ 1429.51 m

HAMMER DROP SYSTEM MANUAL

EXPLORATION LOG

GROUNDWATER LEVEL

DATE DEPTH ELEV.

STATION

STA 384+50

SHEET 1 OF 2

OFFSET

15.24 M LEFT

ENGINEER

J. FORGA

EQUIPMENT _CME 85

OPERATOR SPECTRUM

MUD ROTARY

BACKFILLED YES DATE 5/1/97

<u> </u>					SYSTEM		BACKFILLED TES D	#15 311/31
ELEV.	DEPTH	NO.	S. L TYPE	BLOWS/	Recovery LAB TESTS	USC	MATERIAL DESCRIPTION	REMARKS
						SM	LIGHT BROWN SILTY SAND moist, medium dense, fine to coarse grained sand, estimated 20 to 30% silt.	
1428.5	1.52					SM SP	GRAY BROWN SUGHTLY SILTY SAND medium dense, fine to medium grained sand, estimated 5 to 12% silt.	
1427.5	2 1.98	A	МС	21	W,UW,S	SM	1 83 GRAY BROWN SILTY SAND moist, medium dense, fine grained sand.	19% lines
1426.5	- - -3 3.05					SM SP	GRAY SUGHTLY SILTY SAND moist, dense, fine	
	3.51	В	MC	36	w,uw,s		to medium grained sand.	11% fines
1425.5	_4 4,57					sc	4.42	
1424.5	5 5.03	С	МС	36	W,UW		LIGHT BROWN CLAYEY SAND moist, dense, fine grained sand, low to moderate plasticity fines, estimated 25 to 35% fines.	
1423.5	- ⁶ 6.10	D	МС	25				
422.5	6.55 - -7							
421.5	7.62 - 8 8,08	E	MC	59		SM SP	7 92	
						SP	RED BROWN SLIGHTLY SILTY SAND moist, very dense, fine to coarse grained sand and fine gravel, estimated 5 to 12% silt.	
420.5	9 9 14	F	MC	44	S	sc	9 14	19% lines
ļ	9.60					50	RED BROWN CLAYEY SAND moist, dense, fine grained sand, low plasticity fines.	

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PROJECT NO. 30-1348-15.002

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LOG OF BORING CARSON FREEWAY

CARSON CITY, NEVADA

PLATE

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KLEINFELDER

START DATE: _5/1/97

5/1/97 END DATE:

JOB DESCRIPTION CARSON FREEWAY

NORTH STRUCTURE, COLLEGE PARKWAY

LOCATION

BORING E.A. #

30-1348-15.003 GROUND ELEV_ 1429.51 m

GROUNDWATER LEVEL DATE | DEPTH | ELEV.

EXPLORATION LOG

SHEET 2 OF 2 STA 384+50 STATION

15.24 M LEFT

OFFSET J. FORGA ENGINEER

CME 85 EQUIPMENT SPECTRUM **OPERATOR**

DRILLING METHOD MUD ROTARY

	AAAAU IAI						MICHOD WISHING				
	HAMMER DROP SYSTEM MANUAL							BACKFILLED YES DATE 5/1/97			
ELEV. 14(70)5	DEPTH (m)	NO.	TYPE	MPLE BLOWS/ 300mm	Recovery	LAB TESTS	USCS	MATERIAL DESCRIPTION REMARKS			
1418.5	- 10.67 - 11 - 11,13	G	MC	44			SM SP	RED BROWN SLIGHTLY SILTY SAND moist, dense, fine to medium grained sand, estimated 5 to 12% fines.			
1417.5	_ 12 12.19 12.42	н	MC	50/64mm (GC	RED CLAYEY GRAVEL moist, very dense, fine to			
1416.5	_13						СН	coarse sand and gravel, moderate plasticity 12.65 fines, 62.5 mm maximum particle size. RED BROWN FAT CLAY moist, hard, fine grained sand, high plasticity fines.			
1415.5	13.72 14 14.17	1	МС	59				Could not measure water level due to use of mud rotary			
1414.5	15										
1413.5	16										
1412.5	17					À					
1411.5	18				4						
1410.5	- _ 19 -										
			ā			J.					

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LOG OF BORING CARSON FREEWAY

PLATE

PROJECT NO. 30-1348-15.002

START DATE: __5/2/97

5/2/97 END DATE:

JOB DESCRIPTION CARSON FREEWAY

LOCATION **BORING**

SOUTH STRUCTURE, COLLEGE PARKWAY

30-1348-15.002 E.A. #

GROUND ELEV. 1430.12 m HAMMER DROP SYSTEM MANUAL **EXPLORATION LOG**

GROUNDWATER LEVEL

DATE DEPTH ELEV.

STA 382+00 STATION

15.24 M RIGHT OFFSET

ENGINEER

J. FORGA EQUIPMENT _ CME 85

SHEET 1 OF 2

OPERATOR

_SPECTRUM MUD ROTARY

BACKFILLED

DATE

	HAMMER DROP SYSTEM_MANUAL							BACKFILLED YES DATE 5/2/97			
ELEV.	DEPTH	NO.	S. TYPE	AMPLE BLOWS/ 300mm	Recovery	LAB TESTS		MATERIAL DESCRIPTION REMARKS			
	-						SC	LIGHT BROWN VERY CLAYEY SAND slightly moist, dense to very dense, fine to medium grained sand, moderate plasticity fines.			
1429.1	1										
1428.1	1,52	A	МС	67		W,UW,S,PI		PI = 12 43% fines			
1427.1	3 3,05	8	NG			***************************************					
1426.1	3.51 - 4	В	MC	56		W,UW		Fine to coarse grained sand.			
1425.1	4.57 - 5 5.03	С	MC	38	į	w,uw		Becoming more sandy. 5.03 Slightly clayey, estimated 10 to 15% fines.			
							SM SP	GRAY BROWN SLIGHTLY SILTY SAND moist, dense, fine to coarse grained sand, estimated 5 to 12% silt.			
1424.1	_ 6 _{6,10}	D	мс	42				6.40			
1423.1	6.55 - - 7						SC	LIGHT BROWN CLAYEY SAND moist, dense, fine to medium grained sand, low plasticity fines, estimated 10 to 20% fines.			
	7.62	E	мс	31		W,UW,S,PI	sc	UGHT BROWN CLAYEY SAND moist, dense, fine grained sand, low plasticity fines. Pt = 10			
1422.1	_8 _{8.08}							30% fines			
1421.1	9 9 14	F	MC	30			SC	LIGHT BROWN CLAYEY SAND moist, dense, fine to medium grained sand, low to moderate			
	9.60	İ		-				plasticity fines, estimated 10 to 20% fines, some fine gravel.			

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CARSON FREEWAY

LOG OF BORING

PLATE

PROJECT NO. 30-1348-15.002

CARSON CITY, NEVADA

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4875 LONGLEY LANE, SUITE 100 RENO, NEVADA 89502 Tel. (702) 689-7800 LOG OF BORING
CARSON FREEWAY

CARSON CITY, NEVADA

PLATE

D-11A

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PROJECT NO. 30-1348-15.002

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PROJECT NO. 30-1348-15.002

KLEINFELDER

4875 LONGLEY LANE, SUITE 100 RENO, NEVADA 89502 Tel. (702) 689-7800 CARSON FREEWAY

LOG OF BORING

CARSON CITY, NEVADA

PLATE

D-12

		START DATE: _5/2/97 EXPLORATION LOG												
			_	END D		5/2/97						SHEET 2 OF 2		
2						ON CAR	SON FREE	WAY			STATION STA 332	2+00		
				LOCAT			STRUCTUR		MINE	ESTREET	ENGINEER _J. FORG	A		
	,	KLEINFELD	ER	BORING BS-11					. ,		EQUIPMENT _ CME 85			
1				E.A. # 30-1348-15.002						GROUNDWATER LEVEL	OPERATORSPECTE	MUF		
				GROUND ELEV 1417.32 m					. }	DATE DEPTH ELEV.	DRILLING METHOD <u>MUD RO</u>	DTARY		
				HAMMER DROP SYSTEM MANUAL					. [BACKFILLED YES	DATE 5/2/97		
		ELEV. DEPTH NO.			SAMPLE TYPE BLOWS/ Recovery			TS US	g	MATERIAL DE	SCRIPTION	REMARKS		
	Ration	-			300mm	1 (%)	 	10.0	7			I REMARKS		
		}							1					
	ľ	10.67	G	MC	23			_				1 1		
	1406.3	111113	"	IVIC	23			ſ						
		- 11.13				1		_						
		[Ι,	1.89				
	1405.3	12 12.19						SM						
		12.13	Н	MC	59		W,UW,S	-		GRAY BROWN SILTY S fine to medium grained	AND moist, very dense, sand.	21% fines		
	1	- 12.65												
	1404.3	13												
	1404.5	F"	- 1											
		- 1	- 1					-	1					
	ĺ	13.72		110	20			_		But a second				
	1403.3	14	+	MC	32				14	Red gray, fine to coarse 1.02 dense.	grained sand, medium			
		14.17	+					SC		LIGHT BROWN CLAYEY	SAND moist, medium	1		
										dense, fine grained sand plasticity fines, estimate	1 low to moderate			
		-			j	}		ĺ		productly area, estimate	u 20 to 30% tines.			
	1402.3	L 15												
		- 15.2 <u>4</u>	J	мС	29			-						
1		15.70						СН	15	54		- 1		
	1401.3	16	-	ĺ			_	7		LIGHT BROWN FAT CLA	Y moist, very stiff to and, high plasticity fines.			
		-		ĺ				1	l	yy grantos s	and, mgn plasticity filles.			
	} }	-			İ	[4		
		16.76										}		
	1400.3	_ 17	K	MC	63				ĺ	Fine grained sand.				
	1 }	17.22	-				<u>-</u>	-	ĺ					
				-										
1						-								
	1399.3	_ 18				İ		1 1						
		18.29	L	MC	58			1				Could not measure		
	1	18.75			1				18.	75		water level due to		
	1398.3	. 19										use of mud rotary		
	-			-				1 1						
© 199	i9, by Kleinfeld	er, Inc.	1	- !		10								
)		_	1 5	1	e e ·	Dr	D			LOG OF BO	RING	PLATE		
	KLEINFELDER 4875 LONGLEY LANE, SUITE 100										· 			
	RENO, NEVADA 88502 Tel. (702) 689–7800									CARSON FRE	EWAY	D-12A		

CARSON CITY, NEVADA

D-12A

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PROJECT NO. 30-1348-15.002

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4875 LONGLEY LANE, SUITE 100 RENO, NEVADA 89502 Tel. (702) 689-7800

PROJECT NO. 30-1348-15,002

LOG OF BORING CARSON FREEWAY

5/2/97

5/2/97

JOB DESCRIPTION _ CARSON FREEWAY

START DATE: _

END DATE:

EXPLORATION LOG

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CARSON FREEWAY

LOG OF BORING

PLATE

SHEET 2 OF 2

REMARKS

Could not measure

water level due to use of mud rotary

STA 331+00

0

STATION

OFFSET

PROJECT NO. 30-1348-15.002

EXPLORATION LOG

GROUNDWATER LEVEL

DATE DEPTH | ELEV. |

STATION

OFFSET

ENGINEER

DRILLING METHOD

MATERIAL DESCRIPTION

<u>LIGHT BROWN SILTY SAND</u> moist, dense, weakly cemented lenses, fine to coarse grained sand and fine gravel, non-plastic fines.

0.27 Approx. 10 cm of A.C. and 15 cm of A.B.

becoming medium dense

BACKFILLED YES

1/3/97 & 6/2/99

3/3/97 & 6/2/99

30-1348-15.003

CENTER STRUCTURE, COLLEGE PARKWAY

LAB TESTS USCS

S.UC

SM

JOB DESCRIPTION CARSON FREEWAY

HAMMER DROP SYSTEM MANUAL

START DATE:

END DATE:

LOCATION

GROUND ELEV_M

NO. TYPE | SLOWS/|Recovery

35

13

BORING

E.A. #

SPT

MC

KLEINFELDER

DEPTH

(m)

1 1.07

L:\1999\drp#ing\30134815\30134815003-LOC.DW6

KLEINFELDER

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CARSON FREEWAY

D-14

SHEET 1 OF 3

J. FORGAM. DOEHRING

__ DATE_ 5/3/97

REMARKS

26% lines

EQUIPMENT _CME85/MOB. DRILL, 880

MUD ROTARY

OPERATOR SPECTRUMNOOT

PROJECT NO. 30-1348-15,003

EXPLORATION LOG

GROUNDWATER LEVEL

DATE ! DEPTH : ELEV.

STATION

OFFSET

ENGINEER

DRILLING METHOD

MATERIAL DESCRIPTION

BACKFILLED YES

3/3/97 ± 6/2/99

5/3/97 1 6/2/99

30-1348-15.003

CENTER STRUCTURE, COLLEGE PARKWAY

JOB DESCRIPTION CARSON FREEWAY

HAMMER DROP SYSTEM MANUAL

NO. ! TYPE | BLOWS/ | Hecovery | LAB TESTS USCS

START DATE:

GROUND ELEV_M

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Tel. (702) 689-7800

PROJECT NO. 30-1348-15.003

END DATE:

BORING

EA. #

KLEINFELDER

DEPTH |

ELEV.

(m)

- Clisss (argining (Journated Sales and Judiuma

BORING LOG CARSON FREEWAY PLATE

SHEET 2 OF 3

J. FORGAM. DOEHRING

__ DATE <u>5/3/97</u>

REMARKS

EQUIPMENT _CMESS/MOS. ORILL SEC

MUD ROTARY

OPERATOR SPECTRUMNOOT

D-14A

			ENO O	ATE:	1/3/97 & 6/2		A.V.	EXPLORATION LOG	STATION		SHEET 3 OF 3
								EGE PARKWAY	OFFSET		
		_	LOCATI	-	BS-13A	STRUCTURE	. 000	EGE LYUKANI			AVM. DOEHRING
K	ENFELD	ER	BORING	_	30-1348-19	5.003		GROUNDWATER LEVEL			108. ORILL 380
			E.A. #	_		3.003	_	DATE DEPTH ELEV.		3FEC I H	UMVNOOT
				O ELEV_		4440444		(d (d	_	MUD RO	
					SYSTEM			(1 52	BACKFILLED .	YES	DATE_5/3/97
ELEV. (m)	DEPTH (m)	NO.	TYPE	BLOWS	/ Recovery	LAB TESTS	Group	MATERIAL DI	ESCRIPTION		REMARKS
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	<u> </u>							GRAY CLAYEY SANDY dense, medium to high p	lastem lines fin	e to	
	_23							coarse grained sand, hig gravel.	hly weathered as	ngular	
	-							Argaet.			
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BORING LOG CARSON FREEWAY

PLATE

D-14B

PROJECT NO. 30-1348-15.003

KLEINFELDER

4875 LONGLEY LANE, SUITE 100 RENO, NEVADA 89502 Tel. (702) 689-7800 LOG OF BORING

CARSON FREEWAY

U-13

PROJECT NO. 30-1348-15.002

CARSON CITY, NEVADA

CAD FILE: L-\1999 TING\301348\30134815002DPLATES.DWG

EXPLORATION LOG START DATE: __5/3/97 SHEET 2 OF 2 5/3/97 END DATE: STA 316+00 STATION JOB DESCRIPTION CARSON FREEWAY 32 M LEFT OFFSET U.S. 50 STRUCTURE LOCATION ENGINEER EQUIPMENT _ CME 85 **BS-14** BORING KLEINFELDER **SPECTRUM OPERATOR** 30-1348-15.002 **GROUNDWATER LEVEL** E.A. # DRILLING METHOD DATE | DEPTH | ELEV. GROUND ELEV 1411,22 m MUD ROTARY HAMMER DROP SYSTEM MANUAL BACKFILLED YES DATE 5/3/97 ELEV. DEPTH LAB TESTS MATERIAL DESCRIPTION REMARKS NQ. 14f(1)b (m) H MC 26 CH LIGHT GRAY FAT CLAYEY moist, very stiff, 1400.2 moderate to high plasticity fines. 1399.2 12 MC Brown slightly sandy, fine to medium grained 48 sand, hard. 1398.2 SC BROWN CLAYEY SAND moist, medium dense, MC 24 fine to coarse grained sand, moderate to high plasticity fines. 1397.2 14 1396.2 _ 15 к MC 90 Could not measure water level due to use of mud rotary 15.70 1395.2 16 1394.2 1393.2 . 18 1392.2 19 @1899, by Kleinfelder, Inc. LOG OF BORING

TNG\301348\301348150020PLATES.DWG

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PROJECT NO. 30-1348-15.002

CARSON FREEWAY

CARSON CITY, NEVADA

PLATE

MOISTURE CONTENT AND UNIT WEIGHT STRUCTURE BORINGS

	DEPTH	MOISTURE CONTENT	DRY UNIT WEIGHT
BORING	(meters)	(%)	(kN/cu m)
BS-1	3.35	9.0	17.28
BS-1	4.88	5.0	17.90
BS-1	6.40	9.9	16.96
BS-1	7.92	9.2	18.85
BS-1	9.45	9.8	19.00
BS-1	10.97	8.8	19.95
BS-1	12.34	7.9	19.48
BS-I	13.87	10.6	19.00
BS-2	3.35	8.4	17.43
BS-2	4.88	11.5	19.63
BS-2	6.40	11.4	19.00
BS-2	7.92	12.9	19.00
BS-2	9.45	6.1	18.85
BS-2	10.97	8.2	20.26
BS-3	1.83	10.7	17.43
BS-3	3.35	13.8	18.22
BS-3	4.88	13.7	18.22
BS-3	9.45	25.9	16.65
BS-3	10.97	17.2	16.96
BS-3	12,50	11.5	18.38
BS-3	14.02	15.0	18.53
BS-4	3.35	15.4	17.59
BS-4	4.88	14.0	18.38
BS-4	6.40	14.7	17.75
BS-4	7.92	17.6	17.12
BS-4	9.45	17.4	17.28
BS-4	10.97	15.1	17.75
BS-4	15.54	30.3	13.82
BS-5	1.83	16.7	17.12
BS-5	3.35	19.2	16.49
BS-5	4.88	20.8	15.86
BS-5	7.92	12.5	18.06
BS-5	9.45	26.9	15.08
BS-5	10.97	13.5	18.69
BS-5	12.34	18.2	17.12
BS-5	14.02	14.7	17.59
BS-5	15.54	14.3	18.06
BS-5	18.59	15.2	18.22
BS-6	4.88	20.9	16.49
BS-6	8.84	23.0	15.55
BS-6	10.97	15.2	18.22
BS-6	12.50	19.6	16.81
BS-7	3.35	15.7	17.43

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PROJECT NO. 30-1348-15.002

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MOISTURE / DENSITY TABLE CARSON FREEWAY

CARSON CITY, NEVADA

PLATE

D-16

MOISTURE CONTENT AND UNIT WEIGHT STRUCTURE BORINGS

BORING	DEPTH (meters)	MOISTURE CONTENT (%)	DRY UNIT WEIGHT (kN/cu.m)
BS-7	6.40	16.1	17.75
BS-7	15.54	11.5	18.69
BS-8	3.35	14.6	17.28
BS-8	4.88	13.2	17.59
BS-9	1.83	19.3	17.39
BS-9	3.35	14.4	18.22
BS-9	4.88	15.3	17.90
BS-10	1.83	18.7	
BS-10	3.35	11.8	16.96
BS-10	4.88	15.2	19.16
BS-10	7.92	14.2	17.59
BS-10	10.97	15.5	18.69
BS-10	15.54	33.0	18.22
BS-11	3.35	18.1	13.51
BS-11	4.88	21.2	16.65
BS-11	12.50	12.6	16.33
BS-12	1.83	15.0	18.22
BS-12	3.35	6.8	16.96
BS-12	6.40	18.4	17.12
BS-12	7.92	17.0	17.12
BS-13	3.35	18.0	17.28
BS-13	4.11	23.2	16.49
BS-13	6.40	18.0	16.21
BS-13	9.45	16.8	16.96
BS-13	10.21	22.6	17.43
BS-13	13.26	19.4	16.48
BS-13	19.33	19.8	17.01
BS-13	22.25	16.2	16.84
BS-14	3.385	18.8	16.08
BS-14	4.88	15.3	16.49
BS-14	6.40	12.7	16.96
BS-14	9.45	13.6	18.06
	J. T.	15.0	17.43

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PROJECT NO. 30-1348-15.003

MOISTURE / DENSITY TABLE

CARSON FREEWAY

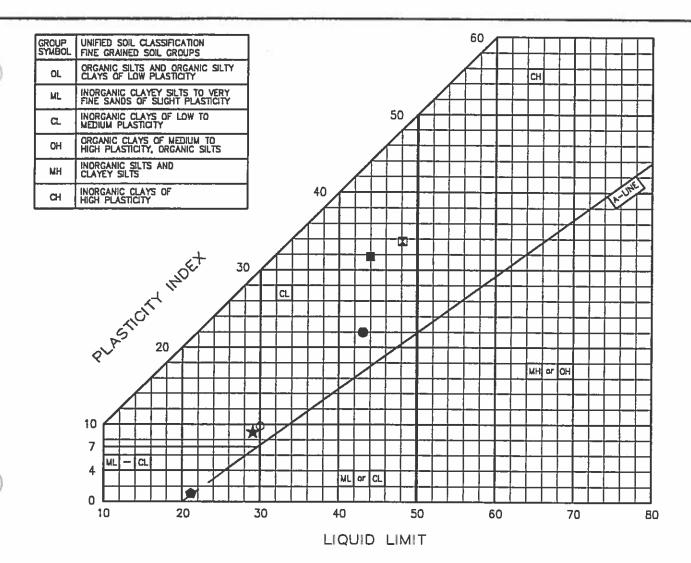
CARSON CITY, NEVADA

PLATE

D-16A



FILE



TEST SYMBOL	SAMPLE NO.	SAMPLE (DEPTH)	LIQUID LIMIT	PLASTICITY INDEX	CLASSIFICATION
*	BS-1	6.40	29	9	Yellow Brown Clayey Sand (SC) -200=32%
•	BS-3	9.45	21	1	Light Brown Clayey Sand (SC) —200=19%
•	BS-4	15.54	43	22	Olive Slighty Sandy Clay (CL) —200=62%
A	BS-5	7.92		NP	Light Brown Clayey Sand (SC) -200=17%
	BS-5	9.45	44	32	Light Brown Silty Clay (CL) -200=62%
M	BS-6	8.84	48	34	Light Brown Slightly Sandy Clay (CL) -200=81%
0	BS-7	5.33	30	10	Brown Clayey Sand (SC) -200=30%

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PLASTICITY CHART

PLATE

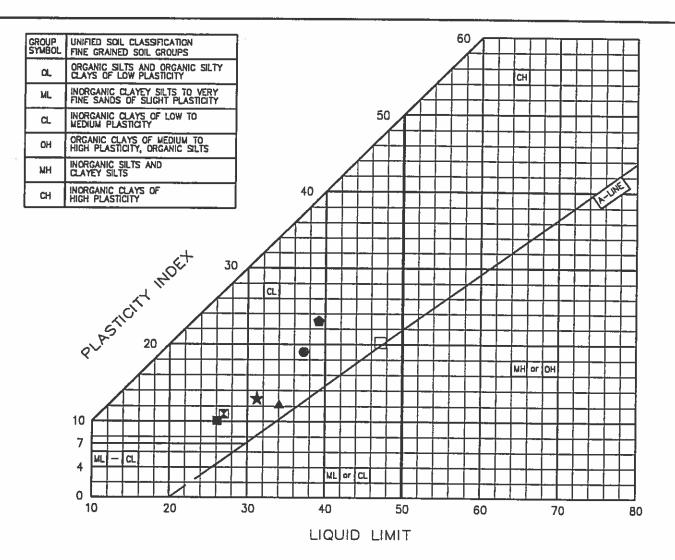
CARSON FREEWAY
CARSON CITY, NEVADA

PROJECT NO. 30-1348-15.002

D-17

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TEST SYMBOL	SAMPLE NO.	SAMPLE (DEPTH)	LIQUID LIMIT	PLASTICITY INDEX	CLASSIFICATION
*	BS-8	1.68	31	13	Light Brown Clayey Sand (SC) −200 =38%
•	BS-8	4.88	39	23	Light Brown Clayey Sand (SC) -200=22%
•	BS-8	9.91	37	19	Light Bown Clayey Sand (SC) -200=36%
A	BS-10	1.83	34	12	Light Brown Clayey Sand (SC) -200=43%
	BS-10	7.92	26	10	Light Brown Clayey Sand (SC) -200= 30%
I	BS-10	10.97	27	11	Light Brown Clayey Sand (SC) -200=34%
	8S-10	15.54	47	20	Red Sandy Clay (CL) -200=63%

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PLASTICITY CHART

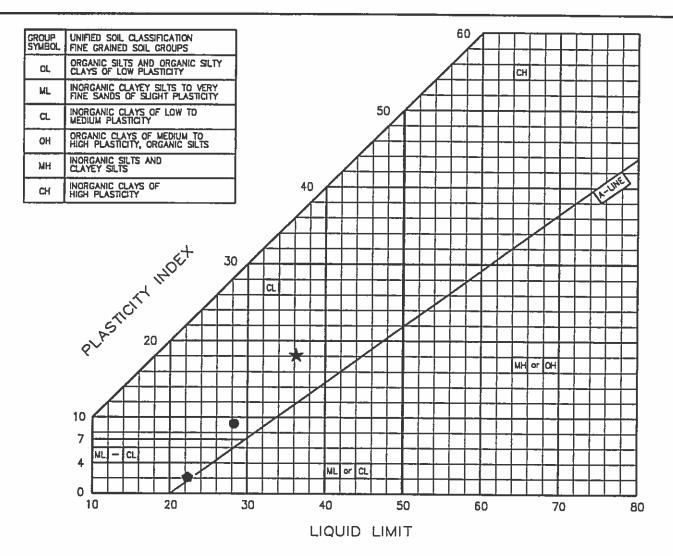
PLATE

CARSON FREEWAY CARSON CITY, NEVADA

PROJECT NO. 30-1348-15.002

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TEST SYMBOL	SAMPLE NO.	SAMPLE (DEPTH)	LIQUID LIMIT	PLASTICITY INDEX	CLASSIFICATION
*	BS-11	4.88	36	18	Light Brown Clayey Sand (SC) -200=38%
•	8S-13	3.35	22	2	Light Brown Clayey Sand (SC) -200=20%
•	BS-13	9.45	28	9	Light Brown Clayey Sand (SC) -200=39%
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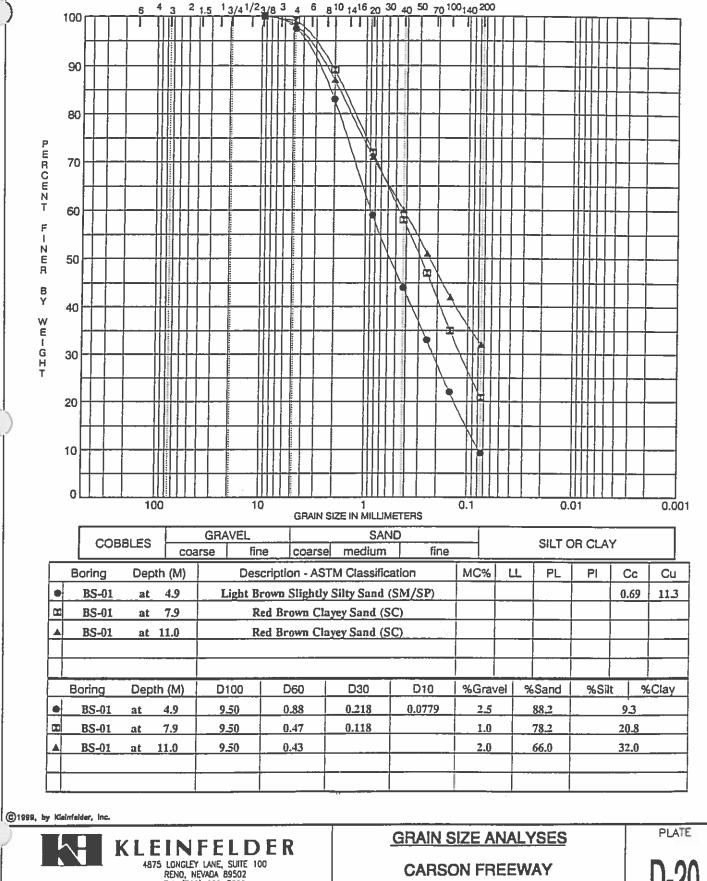
PLASTICITY CHART

D-1

PLATE

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U.S. SIEVE NUMBERS

HYDROMETER

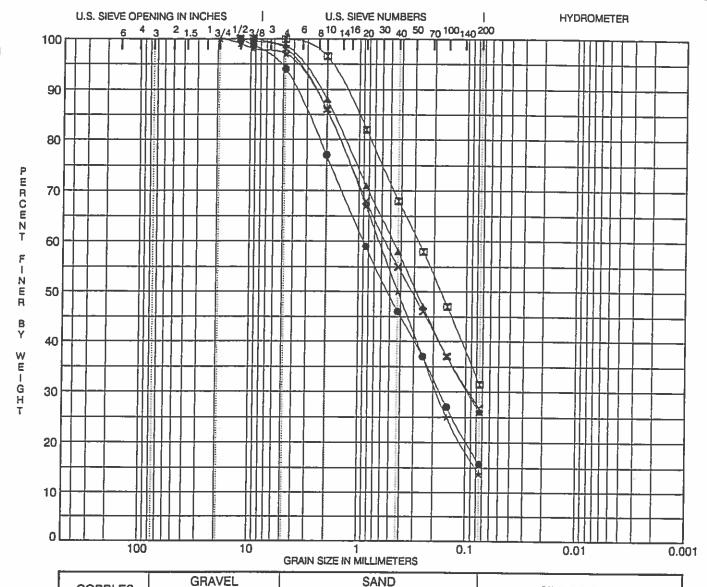
PROJECT NO. 30-1348-15.002

Tel. (702) 689-7800

U.S. SIEVE OPENING IN INCHES

CARSON CITY, NEVADA

CARSON FREEWAY



			BLES		010110		0/114				OUT			
			DLES	coa	arse fin	e coars	se medium	fine			SILI	OR CLAY		
	- 1	Boring	Dep	th (M)	Des	cription - A	STM Classifica	ation	MC%	Ц	- PL	PI	Сс	С
	•	BS-02	at	4.9	R	ed Brown C	layey Sand (S	C)						
	X	BS-02	at	6.4	R	ed Brown C	layey Sand (S	C)						
i	A	BS-02	at	7.9	R	ed Brown C	layey Sand (S	C)						
	*	BS-02	at	9.5	R	Red Brown Clayey Sand (SC)								
	×	BS-02	at	11.0	Re	ed Brown C	layey Sand (So	C)					ĺ	
[E	Boring	Dep	th (M)	D100	D60	D30	D10	%Gra	vel	%Sand	%Silt	%	Cla
į	•	BS-02	at	4.9	12.50	0.89	0.175		6.0		78.3		15.7	
Į		BS-02	at	6.4	9.50	0.28			0.1		68.5		31.4	
	A	BS-02	_at	7.9	9.50	0.47	0.097		1.3		72.7		26.0	
	*	BS-02	at	9.5	19.00	0.64_	0.186		3.0		83.2		13.8	
	×	BS-02	at	11.0	12.50	0.55	0.094		2.0		71.3		26.7	



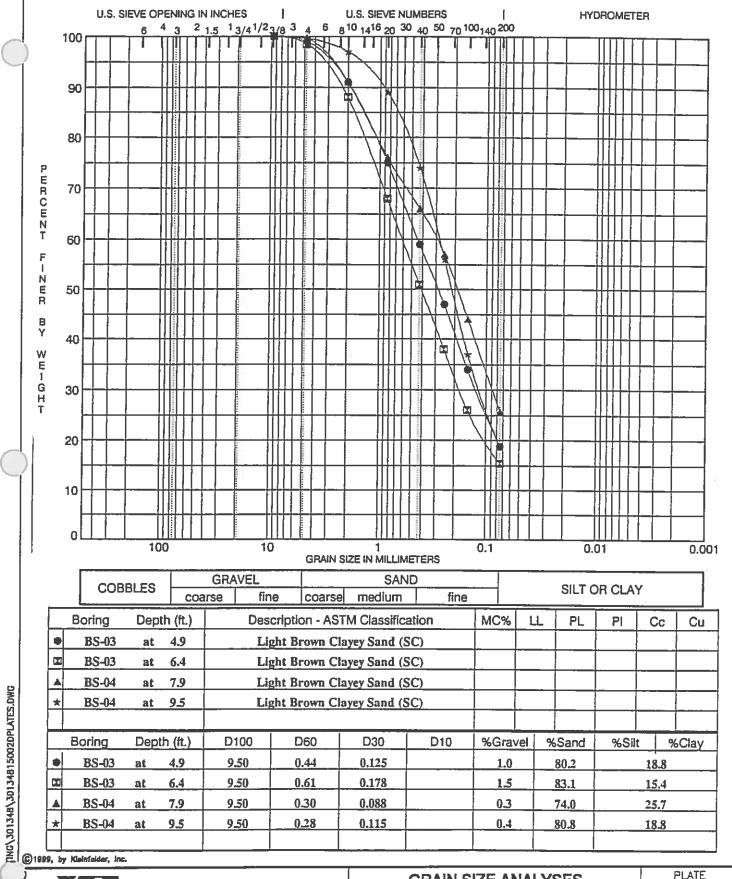
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GRAIN SIZE ANALYSES CARSON FREEWAY

PLATE

PROJECT NO. 30-1348-15.002



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PROJECT NO. 30-1348-15.002

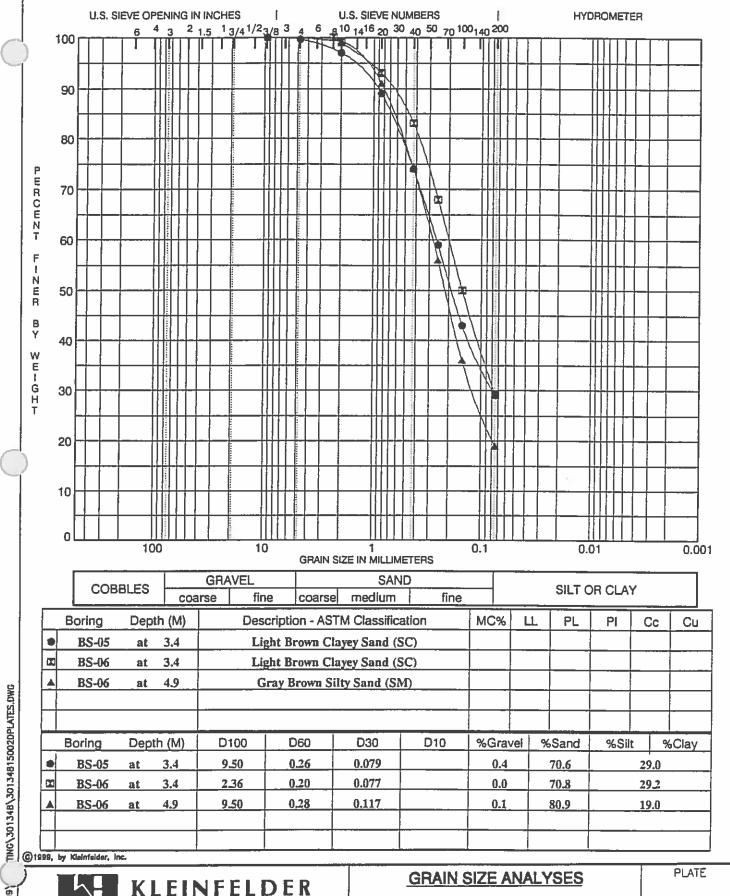
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GRAIN SIZE ANALYSES

CARSON FREEWAY



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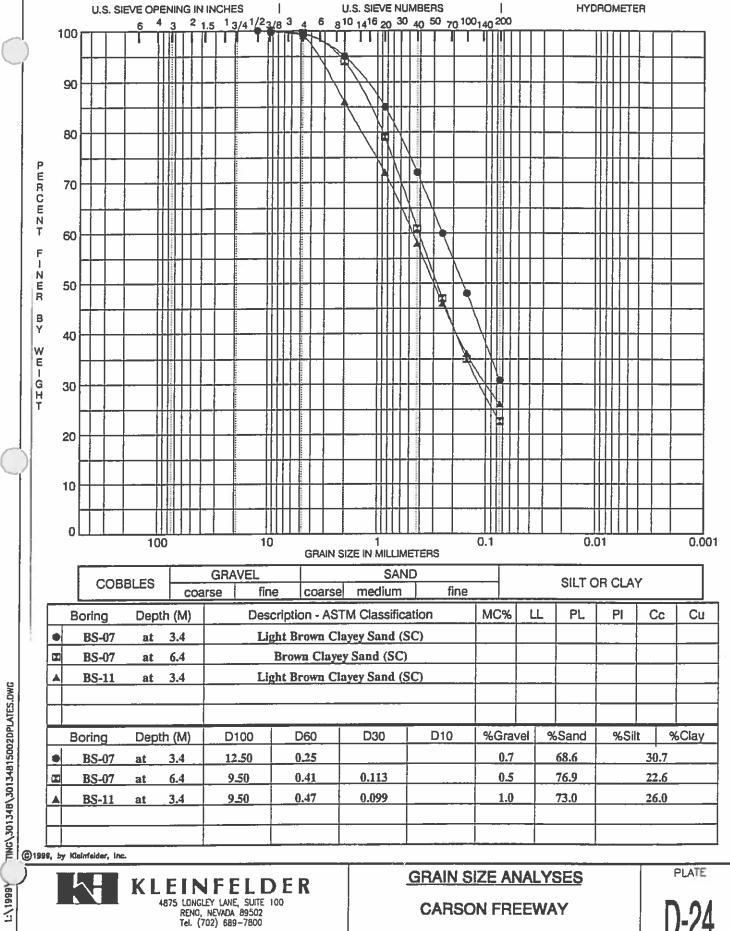
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CARSON CITY, NEVADA

CARSON FREEWAY

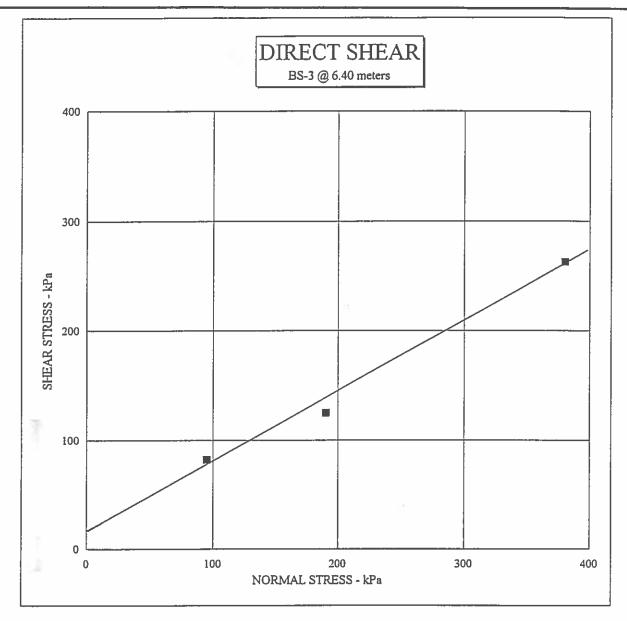


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PROJECT NO. 30-1348-15.002

CARSON FREEWAY





TEST TYPE:	CD/WET/STAGED
BORING NO:	BS-3
DEPTH:	6.40 meters
SOIL DESCRIPTION:	Lt. Brown Clayey Sand
RATE OF SHEAR:	0.0019 cm/sec

FRICTION ANGLE:	33
COHESION:	13 kPa

DRY DENSITY - kN/cu m	16.7		
INITIAL WATER CONTENT - %	45.1		
FINAL WATER CONTENT - %	19.2		
NORMAL STRESS - kPa	95	190	380
MAXIMUM STRESS - kPa	82	125	263

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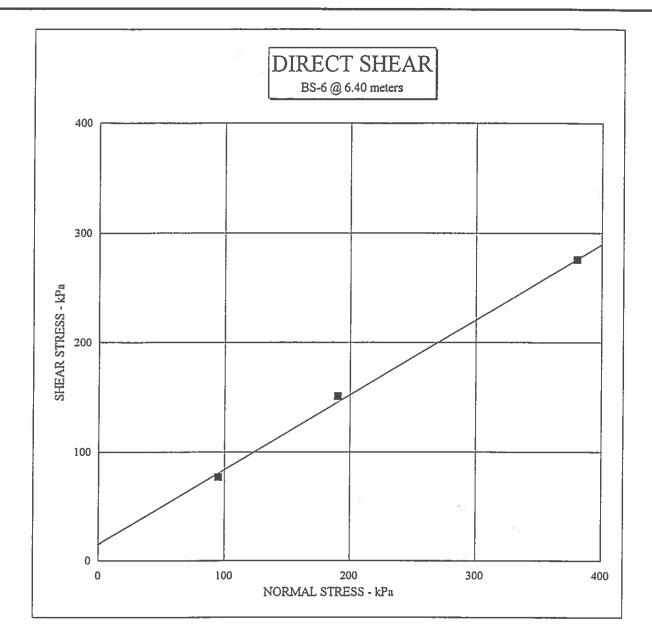
DIRECT SHEAR CARSON FREEWAY

CARSON CITY, NEVADA

PLATE

PROJECT NO. 30-1348-15.002

F\199



TEST TYPE:	CD/WET/STAGED
BORING NO:	BS-6
DEPTH:	6.40 meters
SOIL DESCRIPTION:	Gray Brown Silty Sand
RATE OF SHEAR:	0.0019 cm/sec

FRICTION ANGLE:	35
COHESION:	45 kPa

DRY DENSITY - kN/cu m	16.7		
INITIAL WATER CONTENT - %	15.9		
FINAL WATER CONTENT - %	20.4		
NORMAL STRESS - kPa	95	190	380
MAXIMUM STRESS - kPa	77	151	276

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CARSON FREEWAY

CARSON CITY, NEVADA

PLATE

D-26

PROJECT NO. 30-1348-15.002



SOO FILE



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CONSOLIDATION TEST

100

BORING NO.: BS-1	DEPTH:	1.83 m
SAMPLE DESCRIPTION	N: Red Brown S	ilty Sand
OVERBURDEN PRESS	URE, kPa	31

10

PRESSURE - kPa

	INITIAL	FINAL
DRY DENSITY - kN/cu m	16.37	17,23
WATER CONTENT - %	10.7	17.8
VOID RATIO	0.5393	0.4622
DEGREE OF SATURATION, %	51.00	99.00
SAMPLE HEIGHT - cm	1.6300	1.51

1000

CARSON FREEWAY

PLATE

PROJECT NO. 30-1348-15.002

0

-1

-2

-3

-4

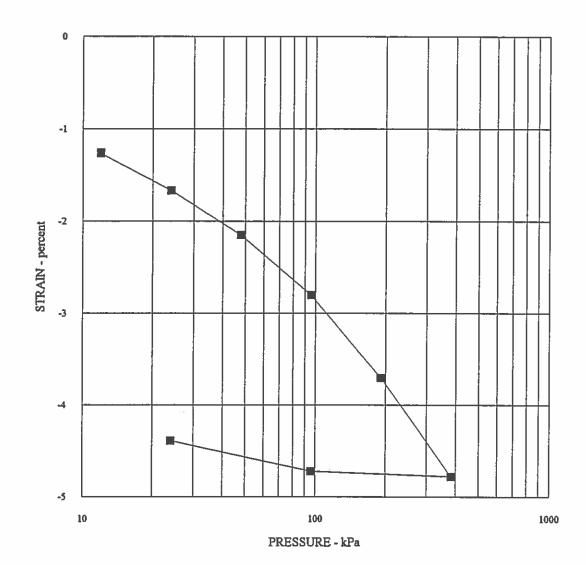
-5

-6

1

STRAIN - percent





BORING NO.: BS-4	DEPTH:	1.83 m
SAMPLE DESCRIPTION: Yellow Brown Silty Sand		
OVERBURDEN PRESSURE, kPa 32		
PRECONSOLIDATION PRESSURE, kPa:		48

	INITIAL	FINAL
DRY DENSITY - kN/cu m	17.39	18.22
WATER CONTENT - %	12.8	16.3
VOID RATIO	0.5131	0.4413
DEGREE OF SATURATION, %	67.00	99.00
SAMPLE HEIGHT - cm	2.54	2.43

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CONSOLIDATION TEST CARSON FREEWAY

PLATE

PROJECT NO. 30-1348-15.002





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CONSOLIDATION TEST CARSON FREEWAY

CARSON CITY, NEVADA

TAL 22 300

BORING NO.:	BS-2	DEPTH:	1.83 m
SAMPLE DESCR	IPTION:	Light Brown Sil	ty Sand
		_	•
OVERBURDEN I	PRESSUR	E, kPa	35

10

PRESSURE - kPa

1

0

-1

-2

-3

-4

-5

-6

1

STRAIN - percent

	INIT
DRY DENSITY - kN/cu m	15.3
WATER CONTENT - %	7.8
VOID RATIO	0.61
DEGREE OF SATURATION, %	32.0
SAMPLE HEIGHT - cm	1.63

100

PLATE

FINAL

16.19 20.5

0.5201 99.00

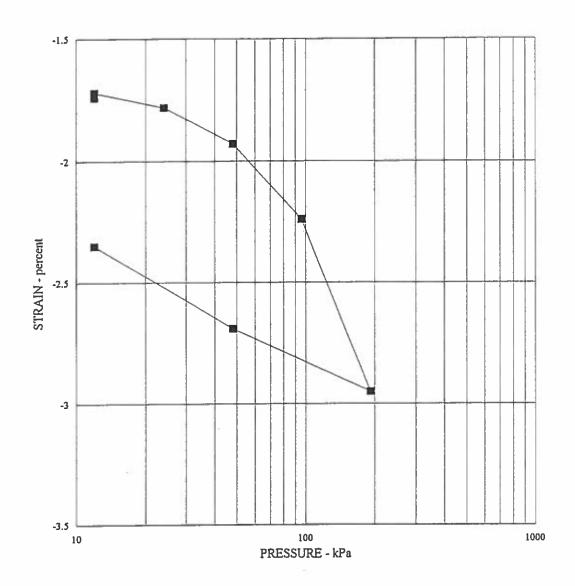
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PROJECT NO. 30-1348-15.002

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CAD FILE: LE\199



BORING NO.: BS-5	DEPTH:	6.4
SAMPLE DESCRIPTION	Light Brown Cla	yey Sand
OVERBURDEN PRESSU	JRE, kPa	112
PRECONSOLIDATION F		57

general and the second	INITIAL	FINAL
DRY DENSITY - kN/cu m	16.7	17.1
WATER CONTENT - %	20.0	20.0
VOID RATIO	0.5792	0.5431
DEGREE OF SATURATION, %	93.00	99.00
SAMPLE HEIGHT - cm	2.5400	2.48

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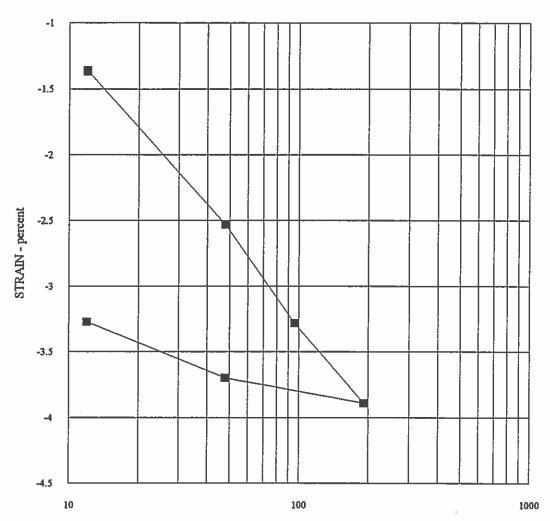
CONSOLIDATION TEST CARSON FREEWAY

PLATE

PROJECT NO. 30-1348-15.002



CAD FILE: LE\1991



PR	ESS	URE	- kPa

BORING NO.: BS-6	DEPTH:	2.44 m
SAMPLE DESCRIPTION: Light Brown Clayey San		
OVERBURDEN PRESSU	DE LDa	38
PRECONSOLIDATION PRESSURE, kPa :		7

	INITIAL	FINAL
DRY DENSITY - kN/cu m	16.32	16.85
WATER CONTENT - %	26.1	23.2
VOID RATIO	0.7309	0.6768
DEGREE OF SATURATION, %	100.00	96.10
SAMPLE HEIGHT - cm	2.54	2.46

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CONSOLIDATION TEST

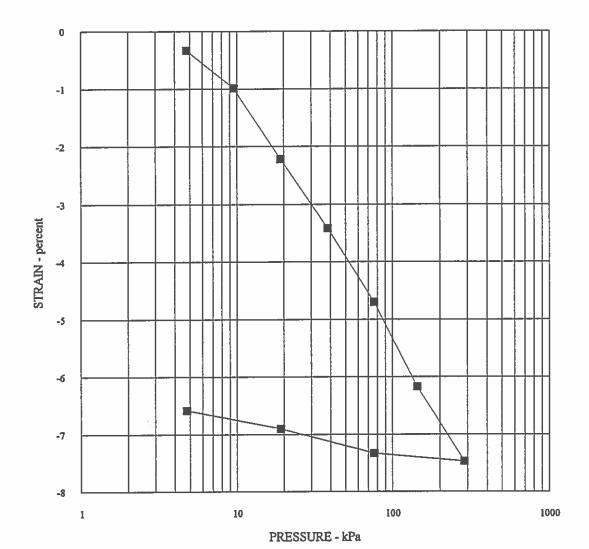
CARSON FREEWAY

PLATE

CARSON CITY, NEVADA

PROJECT NO. 30-1348-15.002





BORING NO.: BS-7	DEPTH:	1.83 m
SAMPLE DESCRIPTION:	Light Brown Cla	yey Sand
OVERBURDEN PRESSUR	E, kPa	32
PRECONSOLIDATION PRESSURE, kPa:		8

	INITIAL	FINAL
DRY DENSITY - kN/cu m	15.67	17.45
WATER CONTENT - %	20.8	22.4
VOID RATIO	0.8614	0.6746
DEGREE OF SATURATION, %	72.00	99.00
SAMPLE HEIGHT - cm	2.54	2.39

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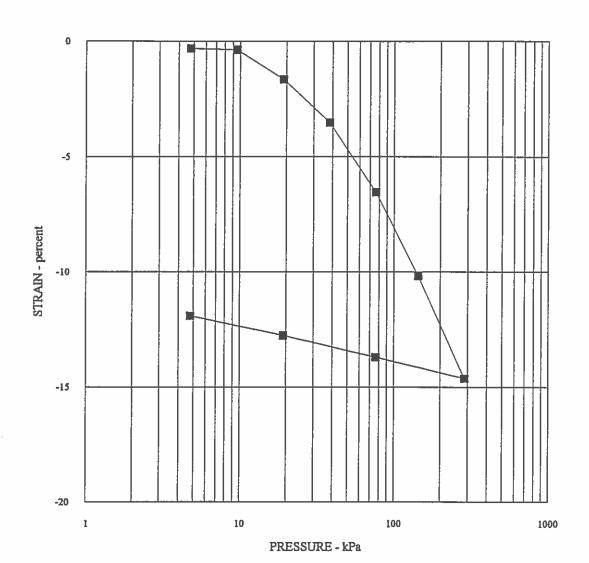
CONSOLIDATION TEST

CARSON FREEWAY

CARSON CITY, NEVADA

PLATE





BORING NO.: BS-7	DEPTH:	4.66 m		
SAMPLE DESCRIPTION: Light Brown Sl. Sandy Clay				
OVERBURDEN PRESSUR	68			

	INITIAL	FINAL
DRY DENSITY - kN/cu m	10.19	12.53
WATER CONTENT - %	48.1	45.3
VOID RATIO	1.9771	1.4100
DEGREE OF SATURATION, %	75.00	99.00
SAMPLE HEIGHT - cm	1.63	1.32

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CONSOLIDATION TEST

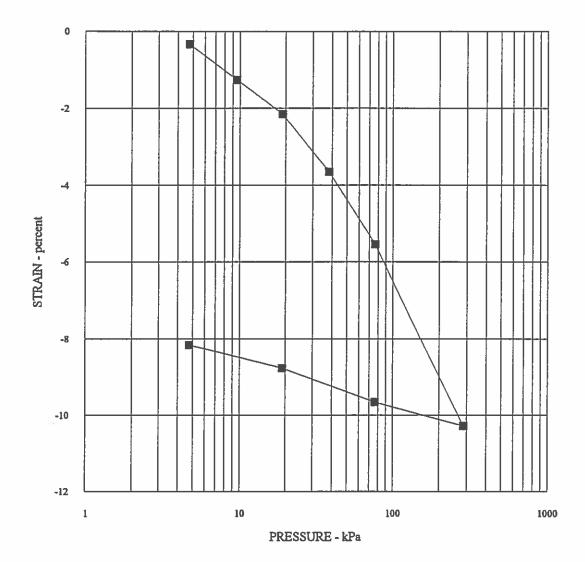
CARSON FREEWAY

CARSON CITY, NEVADA

PLATE

L\1996

CAD FILE:



BORING NO.:	BS-8	DEPTH:	9.45 m
SAMPLE DESC	RIPTION:	Light Brown Clay	
OVERBURDEN PRESSURE, kPa			166
PRECONSOLIDATION PRESSURE, kPa:		60	

	INITIAL	FINAL
DRY DENSITY - kN/cu m	12.91	14.26
WATER CONTENT - %	35.8	34.0
VOID RATIO	1.2108	0.9989
DEGREE OF SATURATION, %	86.00	99.00
SAMPLE HEIGHT - cm	2.54	2.39

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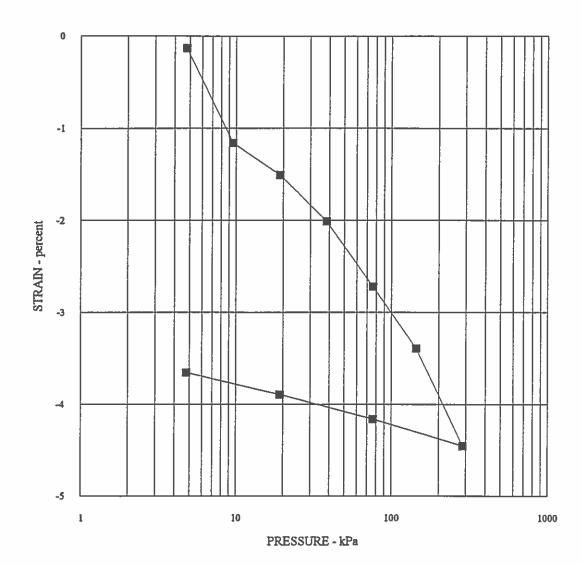
4875 LONGLEY LANE, SUITE 100 RENO, NEVADA 89502 Tel. (702) 689-7800 CONSOLIDATION TEST
CARSON FREEWAY

PLATE

D-34

PROJECT NO. 30-1348-15.002





X	BORING NO.: BS-13	DEPTH:	1.83 m
1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	SAMPLE DESCRIPTION:	Light Brown Clay	ey Sand
	OVERBURDEN PRESSUR	E, kPa	30
<u> </u>	PRECONSOLIDATION PR	ESSURE, kPa :	20

	INITIAL	FINAL
DRY DENSITY - kN/cu m	14.73	15.41
WATER CONTENT - %	8.1	25.6
VOID RATIO	0.7663	0.6850
DEGREE OF SATURATION, %	28.00	99.00
SAMPLE HEIGHT - cm	2.54	2.39



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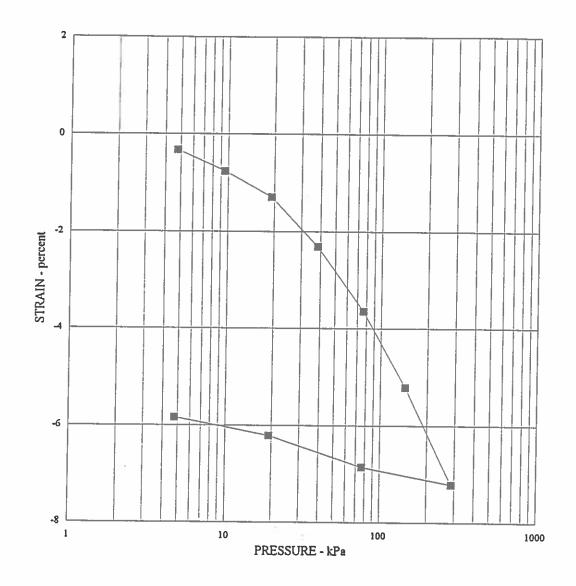
CONSOLIDATION TEST

CARSON FREEWAY

PLATE

PROJECT NO. 30-1348-15.002





BORING NO.: BS-14	DEPTH:	1.83 m		
SAMPLE DESCRIPTION Gray Silty Sand				
OVERBURDEN PRESS	SURE, kPa	30		

	INITIAL	FINAL
DRY DENSITY - kN/cu m	13.18	14.36
WATER CONTENT - %	34.4	34.0
VOID RATIO	1.1928	1.0122
DEGREE OF SATURATION, %	85.00	99.00
SAMPLE HEIGHT - cm	1.6300	1.48

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CONSOLIDATION TEST

CARSON FREEWAY

CARSON CITY, NEVADA

PLATE

TABLE 1: RECOMMENDATIONS FOR CONCRETE IN SULFATE ENVIRONMENTS *					
Soluble Sulfates in Soil %	Sulfates In Water P.P.M.	Cement Type	Maximum Water/Cement Ratio	Minimum Cement Content — Lbs.	
0-0.02	0-150	(Negligible	Sulfate	Reaction)	
0.02-0.10	150-1000	l or II	0.55	470	
0.10-0.20	1000-2000	II .	0.50	560	
0.20-1.50	2000-15,000	II	0.45	660	
		٧	0.50	560	
Over 1.50	Over 15,000	٧	0.45	660	

- * NOTE A. Concrete for piling and other concrete in sea water environments may contain Type II cement when the water—cement ratio is a maximum of 0.50 or the cement factor is a minimum of 560 pounds. The sulfate concentration in Table I should govern in all cases.
- * NOTE B. Sewage treatment facilities normally are constructed using Type II cement except in areas where high sulfate soils of waters exist (See Table I). In sewage, sulfides rather than where sulfates are formed. The sulfide combining with water in the presence of oxygen, can produce sulfuric acid to which no Portland cement is time resistant. Under these conditions, plastic liners, or coatings, are generally used. Closed tanks normally contain an atmosphere of methane rather than oxygen, so acid attack would not be likely to occur. Good quality concretes containing Type II cement with a maximum water cement ratio of 0.53 have provided excellent service in Los Angeles City and County sanitary treatment facilities.

Under special conditions, a concrete materials engineer should be consulted.

Reference: "Recommended Practice to Minimuze Attack on Concrete by Sulfate Soits and Water" by Cament Industry Technical Committee of California.

SAMPLE IDENTIFICATION	BS-01 0 1.68 M	85-03 o 1.58 M	85-05 0 1.68 M	BS-06 • 2.29 M
SAMPLE DESCRIPTION	SILTY SAND (SM)	SUGHTLY SILTY SAND (SM/SP)	FAT CLAY (CH)	CLAYEY SAND (SC)
SOLUBLE SULFATE (%)	0.0036	0.0029	<0.001	<0.001
SOLUBLE SULFATES (PPM)				<u>=</u>
COMMENTS	NEGLIGIBLE SULFATE REACTION	NEGLIGIBLE SULFATE REACTION	NEGLIGIBLE SULFATE REACTION	NEGLIGIBLE SULFATE REACTION

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PLATE

D-37

PROJECT NO. 30-1348-15.002

CARSON FREEWAY
CARSON CITY, NEVADA

POTENTIAL REACTIVITY OF SOLUBLE SULFATES IN SOIL OR GROUNDWATER WITH PORTLAND CEMENT CONCRETE

TABLE 1	: RECOMMENDATION	ONS FOR CONCRETE	IN SULFATE ENVIR	ONMENTS *
Soluble Sulfates In Soil X	Sulfates In Water P.P.M.	Cement Type	Maximum Water/Cament Ratio	Minimum Cament Content — Lbs.
0-0.02	0→150	(Negligible	Sulfate	Reaction)
0.02-0.10	150-1000	l or II	0.55	470
0.10-0.20	1000-2000	#	0.50	560
0.20-1.50	2000-15,000] n]	0.45	660
		· •	0.50	560
Over 1.50	Over 15,000	v	0.45	660

- NOTE A. Concrete for piling and other concrete in sea water environments may contain Type II cement when the water—cement ratio is a maximum of 0.50 or the cement factor is a minimum of 560 pounds. The sulfate concentration in Table I should govern in all cases.
- * NOTE 8. Sewage treatment facilities normally are constructed using Type II coment except in areas where high sulfate soils of waters exist (See Table I). In sewage, sulfides rather than where sulfates are formed. The sulfide combining with water in the presence of oxygen, can produce sulfuric acid to which no Portland coment is time resistant. Under these conditions, plastic liners, or coalings, are generally used. Closed tanks normally contain an atmosphere of methone rather than oxygen, so acid attack would not be likely to occur. Good quality concretes containing Type II cement with a maximum water coment ratio of 0.53 have provided excellent service in Los Angeles City and County sanitary treatment

Under special conditions, a concrete materials engineer should be consulted.

Reference: "Recommended Practice to Minimuze Attack on Concrete by Sulfate Solls and Water" by Cement Industry Technical Committee of California.

SAMPLE IDENTIFICATION	BS-07 9 1.68 M	85-10 0 1.68 M	BS-11 o 1.68 M	
SAMPLE DESCRIPTION	CLAYEY SAND (SC)	CLAYEY SAND (SC)	CLAYEY SAND (SC)	
SOLUBLE SULFATE (%)	0.0023	0,0047	0.02	
SOLUBLE SULFATES (PPM)				
COMMENTS	NEGLIGIBLE SULFATE REACTION	NEGLIGIBLE SULFATE REACTION	SEE ABOVE WATER/CEMENT RATIO AND MIN. CEMENT CONTENT	

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PROJECT NO. 30-1348-15.002

POTENTIAL REACTIVITY

CARSON FREEWAY

CARSON CITY, NEVADA

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PLATE

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Soluble Sulfates In Soli X	Sulfates In Water P.P.M.	Cement Type	Maximum Water/Cement Ratio	Minimum Cement Content — Lbs.
0-0.02	0-150	(Negligible	Sulfate	Reaction)
0.02-0.10	150-1000	l or II	0.55	470
0.10-0.20	1000-2000	II .	0.50	560
0.20-1.50	2000-15,000	H	0.45	660
		v	0.50	560
Over 1.50	Over 15,000	v	0.45	660

- * NOTE A. Concrete for piling and other concrete in sea water environments may contain Type II cement when the water—cement ratio is a maximum of 0.50 or the cement factor is a minimum of 560 pounds. The sulfate concentration in Table I should govern in all cases.
- * NOTE B. Sewage treatment facilities normally are constructed using Type il cement except in areas where high sulfate soils of waters exist (See Table I). In sewage, sulfides rather than where sulfates are formed. The sulfide combining with water in the presence of oxygen, can produce sulfuric acid to which no Portland cement is time resistant. Under these conditions, plastic liners, or coatings, are generally used. Closed tanks normally contain an atmosphere of methane rather than oxygen, so acid attack would not be likely to occur. Good quality concretes containing Type II cement with a maximum water cement ratio of 0.53 have provided excellent service in Los Angeles City and County sanitary treatment facilities.

Under special conditions, a concrete materials engineer should be consulted.

Reference: "Recommended Practice to Minimuze Attack on Concrete by Sulfate Soils and Water" by Cement Industry Technical Committee of California.

SAMPLE IDENTIFICATION	BS-12 0 4.57 M	BS-13 0 1.68 M	BS-14 0 1.68 M	
SAMPLE DESCRIPTION	CLAYEY SAND (SC)	CLAYEY SAND (SC)	SILTY SAND (SM)	
SOLUBLE SULFATE (%)	0.0076	0.0014	0.013	
SOLUBLE SULFATES (PPM)				-
COMMENTS	NEGLIGIBLE SULFATE REACTION	NEGLIGIBLE SULFATE REACTION	NEGLIGIBLE SULFATE REACTION	

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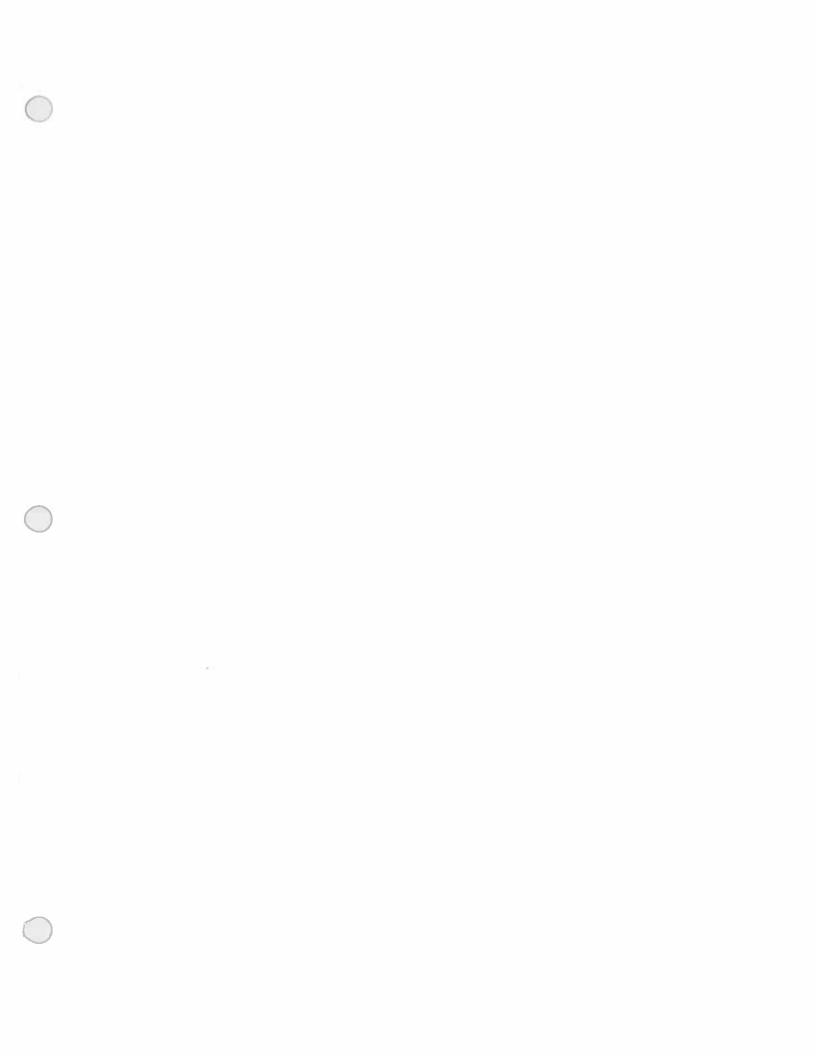
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PLATE

D-39

PROJECT NO. 30-1348-15.002

CARSON FREEWAY
CARSON CITY, NEVADA



STEEL CORROSION POTENTIAL OF SOILS*

<u>Aggressiveness</u>	<u>Resistivity</u> (<u>ohm-cm)</u>
Non-Corrosive	>10,000
Mildly Corrosive	5,000 to 10,000
Moderately Corrosive	2,000 to 5,000
Corrosive	700 to 2,000
Very Corrosive	<700

LABORATORY TEST RESULTS

Soil Type	Source	Resistivity (ohm-cm)	рН**
SILTY SAND (SM	BS-01 @ 1.68 M	2,080	7.25
SLIGHTLY SILTY SAND (SM/SP)	BS-03 @ 1.68 M	4,650	6.91
FAT CLAY (CH)	(BS-05 @ 1.68 M	1,910	8.53
CLAYEY SAND (SC)	(BS-06 @ 2.29 M	4,970	8.24
CLAYEY SAND (SC)	BS-07 @ 1.68 M	1,890	7.67
	(EMERSON)		

*Reference:

FHWA Publication No. FHWA-SA-96-072

**Note:

Allowable pH range is greater than 5 and less than 10

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PROJECT NO. 30-1348-15.002

STEEL CORROSION POTENTIAL OF SOIL

CARSON FREEWAY
CARSON CITY, NEVADA

PLATE

D-4(

STEEL CORROSION POTENTIAL OF SOILS*

<u>Aggressiveness</u>	<u>Resistivity</u> (ohm-cm)
Non-Corrosive	>10,000
Mildly Corrosive	5,000 to 10,000
Moderately Corrosive	2,000 to 5,000
Corrosive	700 to 2,000
Very Corrosive	<700

LABORATORY TEST RESULTS

		<u>Resistivity</u>	
Soil Type	<u>Source</u>	(ohm-cm)	**Ha
CLAYEY SAND (SC)	BS-10 @ 1.68 M	1,060	8.68
CLAYEY SAND (SC)	BS-11 @ 1.68 M	473	9.49
CLAYEY SAND (SC)	BS-12 @ 4.57 M	1,160	9.12
CLAYEY SAND (SC)	BS-13 @ 1.68 M	1,790	8.35
SILTY SAND (SM)	BS-14 @ 1.68 M	1,205	8.22

*Reference:

FHWA Publication No. FHWA-SA-96-072

**Note:

Allowable pH range is greater than 5 and less than 10

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STEEL CORROSION POTENTIAL OF SOIL

CARSON FREEWAY
CARSON CITY, NEVADA

PLATE

D-41

APPENDIX C

Line Borings and Laboratory Test Results **USCS GROUP**

GW

GP

GM

GC

SOIL	CEMENT.	ATION	CRITERIA

Description	<u>Criteria</u>	Description	Criteria
Dry	Absence of moisture, dusty, dry to touch.	Weak	Crumbles or breaks with handling or little finger pressure.
Moist	Damp, no visible water.	Moderate	Crumbles or breaks with considerable finger pressure,
Wet	Visible free water, usually below water table.	Strong	Will not crumble or break with finger pressure.

STANDARD PENETRATION CLASSIFICATION*						
GI	RANULAR SOIL	CLAYEY SOIL				
BLOWS/0.3m	DENSITY	BLOWS/0.3m	CONSISTENCY			
0 – 4	VERY LOOSE	0-1	VERY SOFT			
5-10	5-10 LOOSE		SOFT			
11 - 30	11 – 30 MEDIUM DENSE		MEDIUM STIFF			
31 – 50	31 – 50 DENSE		STIFF			
OVER 50 VERY DENSE		16 – 30	VERY STIFF			
*Standard Penetration Test (N) 63.5 Kg hammer		31 – 60	HARD			
760mm free fall on	50.8mm O.D. x 35mm I.D. sampler.	OVER 60	VERY HARD			

TYPICAL SOIL DESCRIPTION

Well graded gravels, gravel-sand mixtures, little or no fines

Silty gravels, poorly graded gravel-sand-silt mixtures

Clayey gravels, poorly graded gravel-sand-clay mixtures

Poorly graded gravels, gravel-sand mixtures, little or no fines

Blow counts on California Split Spoon (Nea) can be converted to N_{spt} by: $(N_{css})(0.563) = N_{spt}$

Blow counts from Automatic Hammer can be converted to Standard Nin by: $(N_{Automatic \, Hammer})(1.33) = N_{int}$

TEST	<u> ABBREVIATIONS</u>			<u>SAMI</u>	PLER NOTATION
CD	CONSOLIDATED DRAINED	0	ORGANIC CONTENT	CPT	CONE PENETRATION
CH	CHEMICAL (CORROSIVENESS)	OC	CONSOLIDATION	CS	CONTINUOUS SAMPLER ⁽¹⁾
CM	COMPACTION	PΙ	PLASTICITY INDEX	MC	MODIFIED CA SPLIT SPOON (2)
CU	CONSOLIDATED UNDRAINED	RQD	ROCK QUALITY DESIGNATION	P	PUSHED (NOT DRIVEN)
D	DISPERSIVE SOILS	RV	R- VALUE	PB	PTICHER BARREL
DS	DIRECT SHEAR	Ş	SIEVE ANALYSIS/-200 WASH	RC	ROCK CORE ⁽³⁾
E	EXPANSIVE SOIL	SL	SHRINKAGE LIMIT	SH	SHELBY TUBE (4)
G	SPECIFIC GRAVITY	U	UNCONFINED COMPRESSION	SPT	STANDARD PENETRATION TEST
H	HYDROMETER	UU	UNCONSOLIDATED UNDRAINED	TP	TEST PIT
HC	HYDRO-COLLAPSE	UW	UNIT WEIGHT). = 82mm with tube; 88.9mm w/o tube
K	PERMEABILITY	W	MOISTURE CONTENT). = 61.5mm VW
). = 73mm

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KEY TO SOIL CLASSIFICATION **AND TERMS**

CARSON FREEWAY

PLATE

PROJECT NO. 30-1348-15.002

CARSON CITY, NEVADA

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EXPLORATION LOG START DATE: 5/13/97 SHEET 1 OF 1 5/13/97 END DATE: STA 227+00 STATION JOB DESCRIPTION __CARSON FREEWAY 0 **OFFSET** 30.48 M EAST OF LOMPA LANE J. FORGA ENGINEER LOCATION EQUIPMENT _CME 55 BL-01 **BORING** KLEINFELDER SPECTRUM **OPERATOR** 30-1348-15.002 GROUNDWATER LEVEL E.A. # DATE | DEPTH | ELEV. GROUND ELEV. 1414.27 m HOLLOW STEM AUGER HAMMER DROP SYSTEM AUTOMATIC BACKFILLED YES __ DATE _5/13/97 SAMPLE TYPE | BLOWS/ Recovery LAB TESTS ELEV. DEPTH MATERIAL DESCRIPTION REMARKS NO. (m) 0.09 Approx. 6.35 cm of AC, 10.16 cm of AB SC BROWN CLAYEY SAND moist, medium dense, fine to medium grained sand, low to moderate plasticity fines, estimated 20 to 30% fines. 1413.3 SM LIGHT BROWN SILTY SAND moist, medium dense, fine to medium grained sand. W.UW,S MC 14 13% fines 2 1.98 1412.3 No free water encountered. 1411.3 ... 3 1410.3 1409.3 _ 5 1408.3 _ 6

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L\1999V

1407.3

1406.3

1405.3

_ 8

_ 9



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4875 LONGLEY LANE, SUITE 100

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PROJECT NO. 30-1348-15.002

LOG OF BORING

CARSON FREEWAY

CARSON CITY, NEVADA

PLATE

G-2

START DATE: 5/12/97 5/12/97

EXPLORATION LOG

SHEET : OF 1

LOCATION

END DATE:

JOB DESCRIPTION CARSON FREEWAY

STATION

STA 332+00

OFFSET

0

6.10 M NORTH OF CARMINE STREET

ENGINEER

J. FORGA

BORING

BL-02

EQUIPMENT _CME 55 OPERATOR

SPECTRUM

KLEINFELDER

30-1348-15.002 E.A. #

GROUND ELEV. 1417.32 m

GROUNDWATER LEVEL DATE | DEPTH | ELEV.

HOLLOW STEM AUGER

HAMMER DROP SYSTEM AUTOMATIC

BACKELLED YES

						RUTOMATIC	—	BACKFILLED YES D	ATE _5/12/97
ELEV.	DEPTH (m)	NO.	TYPE	AMPLE BLOWS/ 300mm	Recovery	LAB TESTS	USCS	MATERIAL DESCRIPTION	REMARKS
-	0.30	A	GRAB			RV,S	SM	LIGHT BROWN SILTY SAND slightly moist, medium dense, fine to medium grained sand.	31% fines
1416.3	_ 1								
	1.52	8	MC	21		w.uw.s		Becoming less silty, gray brown, moist, fine grained sand.	18% fines
1415.3	2 1 98							No free water encountered.	
						i			
1414.3	_ 3		i						
1413.3	_4				!				
1412.3	5					ļ			
						1			
1411.3	_6								
1410.3	7					ł			
409.3	- _ 8								
408.3	_ 9								
	7.								

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LOG OF BORING CARSON FREEWAY

PLATE

PROJECT NO. 30-1348-15.002

CARSON CITY, NEVADA

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EXPLORATION LOG START DATE: 5/12/97 SHEET 1 OF 5/12/97 END DATE: STA 337+00 **STATION** JOB DESCRIPTION __CARSON FREEWAY 0 **OFFSET** 76.2 M WEST OF LOMPA LANE **ENGINEER** J. FORGA LOCATION EQUIPMENT _ **CME 55 BORING** KLEINFELDER **OPERATOR** SPECTRUM 30-1348-15.002 **GROUNDWATER LEVEL** E.A. # DRILLING METHOD DATE | DEPTH | ELEV. GROUND ELEV 1417.32 m HOLLOW STEM AUGER HAMMER DROP SYSTEM AUTOMATIC BACKFILLED YES DATE 5/12/97 SAMPLE | BLOWS/ Recovery ELEV. DEPTH LAB TESTS MATERIAL DESCRIPTION REMARKS TYPE NO. (m) (m) SM LIGHT BROWN SILTY SAND slightly moist, medium dense, fine to coarse grained sand, estimated 25 to 35% fines. 1416.3 SM SP GRAY BROWN SLIGHTLY SILTY SAND wet, medium dense, fine grained sand. Encountered groundwater at 1,22 M. МÇ 20 W,UW,S 7% fines 1415.3 1.98 1414.3 3 3.05 MC Gray fine to medium grained sand. 27 W,UW,S 6% fines 3.51 1413.3 1412.3 _ 5 1411.3 _ 6 1410.3 1409.3 1408.3 ©1999, by Kleinfelder, Inc.

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LOG OF BORING CARSON FREEWAY

PLATE

L\1999\

TING\301348\30134815002CPLATES.DWG

PROJECT NO. 30-1348-15.002

START DATE: _5/12/97

END DATE:

5/12/97

JOB DESCRIPTION _ CARSON FREEWAY

152.4 M WEST OF LOMPA LANE

LOCATION BL-04

BORING 30-1348-15.002

E.A. # GROUND ELEV 1420.37 m

HAMMER DROP SYSTEM_AUTOMATIC

EXPLORATION LOG

GROUNDWATER LEVEL

DATE | DEPTH | ELEV.

1416.7

5/12/97 3.7

SHEET 1 OF 1

STA 342+00 STATION 0 OFFSET

ENGINEER J. FORGA

EQUIPMENT _CME 55

OPERATOR

<u>SPECTRUM</u>

HOLLOW STEM AUGER

BACKFILLED YES DATE _ 5/12/97

SM LIGHT E medium estimate	ATERIAL DESCRIPTION BROWN SILTY SAND slightly moist, in dense, fine to coarse grained sand, ed 25 to 35% fines. BROWN CLAYEY SAND moist, medium line grained sand, low plasticity fines. PI=4
1419.4 1 SC	BBOWN CLAYEY SAND moist, medium line grained sand, low plasticity fines.
1419.4	ine grained sand, low plasticity fines.
dense, f	PI = 4
A MC 28 PI,W,UW,S	
1418.4 2 1.98	14% fines
1417.4 _ 3 3,05 SM YELLOW medium	V BROWN SILTY SAND wet, fine to grained sand.
B MC 28 W,UW,S	14% fines
1416.4 _ 4	ered groundwater at 3,66 M.
4.57 C MC 20 W,UW,S SC GRAY CI medium	LAYEY SAND wet, medium dense, fine to grained sand, low plasticity fines. 33% fines
1415.4 5 5.03 5.03	
1414.4 _ 6	
1413.4 _ 7	
1412.4 8	
1411.4 - 9	

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LOG OF BORING CARSON FREEWAY

CARSON CITY, NEVADA

PLATE

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5/12/97 END DATE: STA 347+00 STATION JOB DESCRIPTION CARSON FREEWAY 0 OFFSET 244 M WEST OF LOMPA LANE J. FORGA LOCATION **ENGINEER CME 55** BL-05 EQUIPMENT **BORING** KLEINFELDER 30-1348-15.002 **OPERATOR** SPECTRUM **GROUNDWATER LEVEL** E.A. # DRILLING METHOD DATE ! DEPTH | ELEV. GROUND ELEV_1420.37 m **HOLLOW STEM AUGER** 5/12/971 1417.9 HAMMER DROP SYSTEM_AUTOMATIC BACKFILLED YES _ DATE <u>5/12/97</u> SAMPLE
TYPE | BLOWS/ Recovery ELEV. DEPTH USCS LAB TESTS MATERIAL DESCRIPTION REMARKS NO. (m) (m) SC LIGHT BROWN CLAYEY SAND slightly moist. medium dense, fine to coarse grained sand, low to moderate plasticity fines. 0.91 1419.4 SM SP GRAY BROWN SLIGHTLY SILTY SAND moist, medium dense, fine grained sand. 1.52 MC 23 W,UW,S 9% fines 1.98 1418.4 2.50 Encountered groundwater at 2.44 M. SC YELLOW BROWN CLAYEY SAND wet, medium dense, fine to medium grained sand, low 1417.4 3 3.05 MC 18 S,W,UW plasticity fines. B 32% fines 3.66 C MC 26 1416.4 Gray, becoming less clayey. O MC 35 W,UW,S 16% fines 1415.4 5 5.03 Lenses of clean sands Becoming more clayey, 1414.4 6 Ę MC 20 W.UW.S 39% fines 6.55 6.55 Low to moderate plasticity fines. 1413.4 1412.4 8 1411,4 ©1999, by Kleinfelder, Inc.

EXPLORATION LOG

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KLEINFELDER

PROJECT NO. 30-1348-15.002

4875 LONGLEY LANE, SUITE 100 RENO, NEVADA 89502 Tel. (702) 689-7800

START DATE: __5/12/97

CARSON FREEWAY

LOG OF BORING

CARSON CITY, NEVADA

PLATE

SHEET 1 OF

EXPLORATION LOG 5/12/97 START DATE: SHEET 1 OF 1 5/12/97 END DATE: STA 352+00 **STATION** JOB DESCRIPTION __CARSON FREEWAY 0 **OFFSET** 120 M WEST OF LOMPA LANE ENGINEER J. FORGA LOCATION BL-06 EQUIPMENT _CME 55 **BORING** KLEINFELDER **OPERATOR** SPECTRUM 30-1348-15.002 GROUNDWATER LEVEL E.A. # DRILLING DATE DEPTH ELEV GROUND ELEV 1420.37 m HOLLOW STEM AUGER 1 1419.3 HAMMER DROP SYSTEM AUTOMATIC BACKFILLED YES __ DATE _5/12/97 SAMPLE | BLOWS/ Recovery | 300mm (%) DEPTH ELEV. LAB TESTS MATERIAL DESCRIPTION REMARKS NO. TYPE (m) (m) SM LIGHT BROWN SILTY SAND slightly moist, loose to medium dense, fine to medium grained sand, estimated 25 to 35% fines. 1419.4 Encountered groundwater at 1.07 M. MC CD 12 Α 1.98 1 99 1418.4 SC В MC 14 RED BROWN CLAYEY SAND wet, medium dense, fine grained sand, low plasticity fines. 2.90 1417.4 3 3.05 SM GRAY BROWN SILTY SAND wet, medium dense, МÇ G 21 S.W 32% fines **Y**51 1416.4 SC LIGHT BROWN CLAYEY SAND wet, medium MC dense, fine to medium grained sand, low D 21 W.UW.S 47% fines plasticity fines. 1415.4 5 5.03 6 6.10 1414.4 E MC 21 WU.W 1413.4 1412.4 . 8 1411.4 L۹ @1999, by Kleinfelder, Inc.

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CARSON FREEWAY

G-/

PLATE

PROJECT NO. 30-1348-15.002

EQUIPMENT _CME 55 BORING KLEINFELDER **SPECTAUM OPERATOR** 30-1348-15.002 GROUNDWATER LEVEL E.A. # DRILLING METHOD DATE | DEPTH | ELEV. GROUND ELEV 1423.42 m HOLLOW STEM AUGER 5/12/97 1419.9 3.5 HAMMER DROP SYSTEM_AUTOMATIC BACKFILLED _YES DATE 5/12/97 MPLE BLOWS/ Recovery ELEV. DEPTH LAB TESTS MATERIAL DESCRIPTION NQ. TYPE (m)(m) SC LIGHT BROWN CLAYEY SAND slightly moist, dense, fine to medium grained sand, low to moderate plasticity fines. 1422.4 MC 15 W,UW,S Becoming more sandy. 1.98 1421.4 В МС 11 SM SP GRAY BROWN SLIGHTLY SILTY SAND moist, loose to medium dense, fine to coarse grained sand, estimated 5 to 12% fines. 1420.4 3 3.05 MÇ DS Encountered groundwater at 3.51 M. 0 МC 15 1419.4 SC LIGHT BROWN CLAYEY SAND wet, medium dense, fine to medium grained sand, low MC 16 W,UW,S plasticity fines. 1418.4 5.03 MC 5.64 SM SP 1417.4 GRAY BROWN SLIGHTLY SILTY SAND wet, fine G MC 26 W.UW.S to medium grained sand 6.55 6 55 1416.4 1415.4 8 1414.4 _ 9

EXPLORATION LOG

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KLEINFELDER

START DATE: __5/12/97

END DATE:

LOCATION

5/12/97

JOB DESCRIPTION _CARSON FREEWAY

SOUTHWEST OF RESEARCH WAY

4875 LONGLEY LANE, SLITE 100 RENO, NEVADA 89502 Tel. (702) 689-7800

CARSON FREEWAY

LOG OF BORING

PLATE

SHEET 1 OF 1

REMARKS

18% fines

25% fines

12% fines

STA 357+00

J. FORGA

0

STATION

OFFSET

ENGINEER

CARSON CITY, NEVADA

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PROJECT NO. 30-1348-15.002

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START DATE: 5/12/97

END DATE:

LOCATION

BORING

5/12/97

JOB DESCRIPTION CARSON FREEWAY

SOUTHWEST OF RESEARCH WAY

BL-08

30-1348-15.002 E.A. # GROUND ELEV 1424.94 m

HAMMER DROP SYSTEM AUTOMATIC

EXPLORATION LOG

GROUNDWATER LEVEL

DATE | DEPTH | ELEV.

STATION

STA 362+00

_0 OFFSET

ENGINEER

_J. FORGA

EQUIPMENT _CME 55 **OPERATOR**

SPECTRUM

DRILLING METHOD

HOLLOW STEM AUGER

SHEET 1 OF 1

			HAMME	R DROP S	SYSTEM_	AUTOMATIC	BACKFILLED YES D	ATE 5/12/97	
ELEV.	DEPTH (m)	NO.	TYPE	AMPLE BLOWS/ 300mm	Recovery	LAB TESTS		MATERIAL DESCRIPTION	REMARKS
	-						SC	BED BROWN CLAYEY SAND slightly moist, medium dense, fine to coarse grained sand, low plasticity fines, estimated 20 to 30% fines.	
1423.9	-1						SM	1.07	
	1.52	A	MC	38		W.UW,S	3141	YELLOW BROWN SILTY SAND slightly moist, fine to coarse grained sand and fine gravel.	
1422.9	2 1.98					1		Becoming more silty.	15% fines
1421.9	3 3,05	В	мс	20		W.UW		Becoming cleaner	
	3.51	- 4	D			-		3.51 Becoming wet.	
1420.9	4							No free water encountered.	
	.								
1419.9	_5								
1418.9	6								
1417.9	7								
1416.9	_8			ŀ					
1415.9	9								

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PROJECT NO. 30-1348-15.002

LOG OF BORING CARSON FREEWAY

CARSON CITY, NEVADA

PLATE

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START DATE: 5/12/97 END DATE:

LOCATION

5/12/97

JOB DESCRIPTION CARSON FREEWAY

WEST OF RESEARCH WAY

BORING

30-1348-15.002 E.A. # GROUND ELEV 1425.55 m

HAMMER DROP SYSTEM_AUTOMATIC

EXPLORATION LOG

GROUNDWATER LEVEL

DATE DEPTH ELEV.

STATION

STA 367+00 0

OFFSET ENGINEER

J. FORGA

EQUIPMENT CME 55 **OPERATOR**

SPECTRUM

DRILLING METHOD

HOLLOW STEM AUGER

SHEET 1 OF 1

BACKFILLED YES

ELEV. (m)	OEPTH (m)	NO.	TYPE	BLOWS/	Recovery (%)	LAB TESTS	SM	MATERIAL DESCRIPTION REMARK
	1						i sanii	
	1							BROWN SILTY SAND slightly moist, medium dense, fine to medium grained sand.
1423.5	Ţ					:		
	1.52	A	мс	11		CD		
	2.13	В	MC	34				Dense
1422.5	3 3.05	С	мс	36		w,uw,s		Light brown, some coarse grained sand. 32% fine:
1421.5	- 4							No free water encountered.
1420.5	5					ļ		
1419.5	_6					:		
1418.5	7					ļ		
1417.5	8							
1416.5	9							
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PROJECT NO. 30-1348-15.002

LOG OF BORING CARSON FREEWAY

CARSON CITY, NEVADA

EXPLORATION LOG 5/12/97 START DATE: _ SHEET 1 OF 1 5/12/97 END DATE: STA 372+00 **STATION** JOB DESCRIPTION __CARSON FREEWAY 0 **OFFSET** SOUTH OF COLLEGE PARKWAY J. FORGA LOCATION ENGINEER EQUIPMENT _ CME 55 **BORING** KLEINFELDER **OPERATOR SPECTRUM** 30-1348-15.002 GROUNDWATER LEVEL E.A. # DRILLING METHOD DATE | DEPTH ELEV. GROUND ELEV_ 1435.00 m HOLLOW STEM AUGER 5/12/97 1432.0 HAMMER DROP SYSTEM_AUTOMATIC BACKFILLED YES DATE 5/12/97 SAMPLE BLOWS/ Recovery 300mm (%) ELEV. DEPTH USCS LAB TESTS MATERIAL DESCRIPTION REMARKS NO. TYPE (m)(m) SM LIGHT BROWN SILTY SAND slightly moist, medium dense, fine to coarse grained sand, some fine surface gravel, estimated 20 to 30% 1434.0 SC BROWN CLAYEY SAND moist, medium dense, fine to medium grained sand, low plasticity fines. MC 19 W,UW,S,PI PI = 8 41% fines 2 1.98 1433.0 2 44 GC GRAY CLAYEY GRAVEL wet, very dense, fine to coarse sand and gravel, estimated 10 to 20% 1432.0 fines, low to moderate plasticity fines. В MC 50/4" Encountered groundwater at 3.04 M. 1431.0 C GRAB Refusal at 4.27 Meters. 1430.0 . 5 1429.0 L 6 1428.0 _ 7 1427.0 8 1426.0 9 @1999, by Kleinfelder, Inc.

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CARSON FREEWAY

PLATE

PROJECT NO. 30-1348-15.002

SHEET 1 OF 1 5/9/97 END DATE: STA 377+00 **STATION** JOB DESCRIPTION CARSON FREEWAY 0 **OFFSET** SOUTH OF COLLEGE PARKWAY LOCATION **ENGINEER** J. FORGA _ CME 55 **BORING** EQUIPMENT KLEINFELDER **SPECTRUM** 30-1348-15,002 **GROUNDWATER LEVEL OPERATOR** E.A. # DRILLING METHOD DATE | DEPTH | ELEV. GROUND ELEV 1432.56 m **HOLLOW STEM AUGER** 5/9/97 1 3.0 1429.5 HAMMER DROP SYSTEM AUTOMATIC BACKFILLED YES __ DATE <u>5/9/97</u> SAMPLE | BLOWS/ Recovery | 300mm (%) ELEV. DEPTH LAB TESTS USCS MATERIAL DESCRIPTION REMARKS NO. TYPE SM LIGHT BROWN SILTY SAND slightly moist, dense to very dense, fine to medium grained sand, slight plasticity fines. 1431.6 MC W,UW,S 57 21% fines 2 1.98 1430.6 SM YELLOW BROWN SLIGHTLY SILTY SAND moist, medium dense, fine to medium grained sand, 3 3,05 1429.6 estimated 5 to 12% fines MC 23 W,UW,S 16% fines Encountered groundwater @ 3.2M 3.51 1428.6 CL BROWN SANDY CLAY wet, stiff to very stiff, fine grained sand, low to moderate plasticity fines. C MC 16 W,UW,S,PI PI=11 38% fines 1427.6 5.03 5.18 MC 8 5.64 5 79 CL 1426.6 6 LIGHT BROWN SLIGHTLY SANDY CLAY wet, 6.10 stiff, fine grained sand, moderate to high plasticity fines E MC 11 S,CD 72% fines 6.71 Red brown, becoming more sandy. MC 24 1425.6 7.16 1424.6 8 1423.6 . 9

5/9/97

START DATE: _

EXPLORATION LOG

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LOG OF BORING CARSON FREEWAY

PLATE

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PROJECT NO. 30-1348-15.002

EXPLORATION LOG SHEET 1 OF 5/9/97 END DATE: STA 382+00 STATION JOB DESCRIPTION __CARSON FREEWAY 0 OFFSET SOUTH OF COLLEGE PARKWAY LOCATION **ENGINEER** J. FORGA _CME 55 EQUIPMENT BORING KLEINFELDER SPECTRUM 30-1348-15.002 GROUNDWATER LEVEL **OPERATOR** E.A. # DRILLING METHOD DATE | DEPTH | ELEV. GROUND ELEV_ 1431,34 m **HOLLOW STEM AUGER** 5/9/97 1428.1 HAMMER DROP SYSTEM AUTOMATIC BACKFILLED YES DATE 5/9/97 SAMPLE BLOWS/Recovery 300mm (%) ELEV. DEPTH LAB TESTS USCS MATERIAL DESCRIPTION REMARKS NO. TYPE (m) (m) SM SP LIGHT BROWN SUGHTLY SILTY SAND slightly moist, medium dense, fine to coarse grained sand, fine gravel. 1430.3 MC 20 W,UW,S Α 9% fines 1429.3 1.98 SC LIGHT BROWN CLAYEY SAND slightly moist, medium dense, fine to medium grained sand, 3 3.05 1428.3 low plasticity fines, estimated 15 to 25% fines. В MC 20 W,UW,S 17% fines Encountered groundwater at 3.20 M. 1427.3 4.57 МC W,UW,S С 20 14% fines 1426.3 Lenses of clean sands SM GRAY BROWN SILTY SAND wet, medium dense, fine to medium grained sand 1425.3 MC 25 W,UW,S O 14% fines 6 55 1424.3 1423.3 8 1422.3 @1999, by Kleinfelder, Inc. PLATE **LOG OF BORING**

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PROJECT NO. 30-1348-15.002

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CARSON FREEWAY

EXPLORATION LOG 5/9/97 START DATE: SHEET 1 OF 1 5/9/97 END DATE: STA 387+00 **STATION** JOB DESCRIPTION CARSON FREEWAY 0 OFFSET NORTH OF COLLEGE PARKWAY **ENGINEER** _J, FORGA LOCATION BL-13 EQUIPMENT _ CME 55 BORING KLEINFELDER OPERATOR SPECTRUM GROUNDWATER LEVEL 30-1348-15.002 E.A. # DRILLING METHOD DATE | DEPTH | ELEV. GROUND ELEV_1431.34 m HOLLOW STEM AUGER 5/9/97 1.8 1429.5 HAMMER DROP SYSTEM AUTOMATIC BACKFILLED YES DATE 5/9/97 SAMPLE
TYPE | BLOWS/ Recovery | 300mm (%) ELEV. (m) DEPTH USCS LAB TESTS MATERIAL DESCRIPTION REMARKS NO. (m) SC LIGHT BROWN CLAYEY SAND slightly moist, medium dense, fine grained sand, low plasticity fines, estimated 20 to 30% fines. 1430.3 SM LIGHT BROWN SILTY SAND moist, medium dense, fine grained sand. MC 20 W,UW,S A 25% fines Encountered groundwater at 1,83 M. 1429.3 SC BED BROWN CLAYEY SAND wet, medium 1428.3 3 3.05 dense, fine to medium grained sand, low plasticity fines. В MC 16 W.UW.S 20% fines Lenses of cleaner sand MC 26 1427.3 D MC 27 W,UW 1426.3 5 5.03 SM SP GRAY BROWN SLIGHTLY SILTY SAND wet, dense, fine to medium grained sand, estimated 5 - ⁶ <u>6.10</u> 1425.3 to 12% fines. MC WUW. 1424.3 1423.3 . 8 1422.3 @1999, by Kielmfeider, Inc.

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CARSON FREEWAY

LOG OF BORING

PLATE

PROJECT NO. 30-1348-15.002

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; ;	1431
	1430
33	1429
	1428
	1427
	1426
	1425

									EXPLORATION LOG		
				START	DATE:	5/9/97			EXPLORATION LOG		SHEET 1 OF 2
	90	3 3 3 3		END D		5/9/97				STATION STA 393+0	00
				JOB DE	SCRIPT		SON FREEW			OFFSET 0	
	18			LOCAT	ION .		HOT SPRIN	GS RO		ENGINEERJ. FORGA	
	K	LEINFELD	ER	BORIN	G.	BL-14	<u> </u>			EQUIPMENT CME 55	
1				E.A. #		30-1348-1	-		GROUNDWATER LEVEL DATE DEPTH ELEV.	OPERATOR SPECTRUM	<u>a</u>
						1432.56 m			DATE DEPTH ELEV. 5/9/97 1.5 1431.0		TEM AUGER
							AUTOMATIC		1	BACKFILLED YES D	ATE 5/9/97
	ELÉV. (m)	DEPTH (m)	NO.	TYPE	BLOW:	S/ Recovery	LAB TESTS	Group	MATERIAL DES	SCRIPTION	REMARKS
1								CL	LIGHT_BROWN_SANDY_ fine to medium grained plasticity fines.	CLAY slightly moist, stiff, sand, low to moderate	
14	431.6	-1						SC	0.91 LIGHT BROWN CLAYEY	SAND moist medium	
		7.52		!		1			dense, fine grained sand	d, low plasticity fines.	
			А	МС	12		W,UW,S		Encountered groundwat	er at 1.52 M.	43% fines
14	430.6	2 1.98 2.13						SM	1.98		
		-	В	MC	29			SM SP	LIGHT BROWN SLIGHTI grained sand, estimated		
		2.59						-			
14	129.6	<u> 3 3,05</u>	С	MC	20		W,UW,S	-		3	5% fines
		3.51							Fine to medium grained	sand.	
			15		ļ				3 96		
14	28.6	- 4						SC		wet, medium dense, fine	
		4.57				j j			to medium grained sand estimated 10 to 20% fine	l, low plasticity fines.	
		7.07	D	MC	13		S,DS			-	32% fines
14	27.6	_ 5 5,03 5,18						-			
			Ε	MC	18			1			
1		5.64			i						
143	26.6	- -6 _{6.10}									
		-	F	MC	40		W,UW,S	1			18% fines
		6.55							Dense		
	I										
144	25.6	_ 7 -									
	Ì	7.62	G	MC	33		W,UW,S		Lenses of clean sand, fine	e grained.	26% fines
142	24.6	- 8 8.08									
		_		1							
	-	-								ľ	
142	23.6	- _9									
		9.14	Н	MC	16		W,UW,S,PI				Pl = 14
1	İ	9,60									29% fines
	ļ	9.75		MC	8				Lenses of soft fat clay		
 ! 1000 !	i bu Mate	sfelder, Inc.	· !	0		<u> </u>			<u>. </u>		
1	J7 NIBIT	micer, Uic.	1/ *		. 1 = -				LOG OF B	ORING	PLATI
		V	ΚĹ			ELD E, SUITE 100					
•				REN	O. NEVADA	N 89502	,		CARSON FF	REEWAY	C-1
				161.	(702) 68	3-1000					U-1
			474	8-15.0					CARSON CITY,	NEVADA	

EXPLORATION LOG START DATE: 5/9/97 SHEET 2 OF 2 5/9/97 END DATE: STA 393+00 **STATION** JOB DESCRIPTION __CARSON FREEWAY 0 WEST OF HOT SPRINGS ROAD **ENGINEER** J. FORGA LOCATION _CME 55 **EQUIPMENT** BORING KLEINFELDER SPECTRUM 30-1348-15.002 GROUNDWATER LEVEL OPERATOR E.A. # DATE | DEPTH | ELEV. GROUND ELEV_ 1432.56 m HOLLOW STEM AUGER 5/9/97 1 1431.0 HAMMER DROP SYSTEM AUTOMATIC BACKFILLED YES DATE 5/9/97 MPLE BLOWS/ Recovery 300mm (%) ELEV. DEPTH LAB TESTS USCS MATERIAL DESCRIPTION REMARKS NO. TYPE (m) 1400 G 10,21 10.67 W,UW МС 15 1421.6 LIGHT BROWN FAT CLAY wet, very stiff, no sand, high plasticity fines. CH 11.13 1420.6 1419.6 _ 13 1418.6 1417.6 1416.6 1415.6 1414.6 __ 18 1413.6 @1999, by Klainfalder, Inc.

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CARSON FREEWAY

LOG OF BORING

PLATE

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PROJECT NO. 30-1348-15.002

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			START		5/10/97	_		EXPLORATION LOG	SHEET 1
		188	END D	3164 -	5/10/97			STATION STA 39	7+00
			JOB DE	SCRIPTI		SON FREEW		OFFSET 0	
		2300	LOCAT	ION _		EMERSON (PRIVE	ENGINEER J. FOR	
K	LEINFELD	ER	BORING	3 _	BL-15			EQUIPMENT _CME 5	
			E.A. #	_	30-1348-1	5.002		GROUNDWATER LEVEL OPERATOR SPECT	RUM
1			GROUN	ID ELEV.	1432.56 m	1		DATE DEPTH ELEV. DRILLING HOLLO	W STEM AUGER
			HAMME	R DROP	SYSTEM	AUTOMATIC		BACKFILLED YES	
ELEV.	DEPTH		S	AMPLE_		4	Luca		1
(m)	(m)	NO.	TYPE	300mm	Recovery	LAB TESTS	Grou	MATERIAL DESCRIPTION	REMAR
							SC	LIGHT BROWN CLAYEY SAND slightly moist, fine grained sand, low to moderate plasticity fines.	
1431.6	- 1 -						CL	1 22	
	1.52	A	MC	9		S,PI,CD	- ~ -	YELLOW BROWN VERY SANDY CLAY moist, stiff, fine grained sand, moderate plasticity fines	05
	_ ₹	_ ^	IVIC	a	14	3,71,00		Encountered groundwater at 1,68 M.	Pl= 17 51% fine
1430.6	2 1.98 2.13						-		
		В	MC	20	1			2 29	
	2 59						SM SP	GRAY BROWN SLIGHTLY SILTY SAND wet, fine	
	-						l .	to coarse grained sand, fine gravel, estimated 5	
1429.6	_ 3 3.05						SC		- i
		С	MC	14		W,UW,S,PI		YELLOW BROWN VERY CLAYEY SAND wet, medium dense, very fine grained sand, moderate	PI = 18 e 49% fine
	3.51					-	-	plasticity fines.	
ļ	-	D	MC	22		-			
1428.6	- ⁴ 4.11				<u>i i</u>			4 11 Becoming more sandy.	
İ	-	ĺ					SM SP	YELLOW BROWN SLIGHTLY SILTY SAND wet,	1
	4.57	E	140			14118416	35	fine to medium grained sand, estimated 5 to 129 fines.	
}	-	-	MC	8		w,uw,s		mes.	6% fines
1427.6	<u>5 5.03</u>								
ĺ		F	MC	18				Yellow brown, fine grained sand.	
[5.64				- 4				1
	-		ĺ						į
1426.6 L	6 6.10							6.10	_i
	.	G	МС	33		w,uw,s	SC	BBOWN CLAYEY SAND wet, dense, fine to	28% fine:
-	6,55				i i			medium grained sand, moderate to high plasticity fines, estimated 20 to 30% fines.	
1425.6	7								
-	7.62	Н	MC	10		w.uw			
1424.6	8	-	1410	,,		147.7			
	8 8.08 8.23		1	84	<i>2</i>				
F			MC	16	41	N,UW			
E	8.69							Medium dense	1
1423.6	.9								
- 0.63	9 14		146 1		- 1		212	9 14	1
		J	MC	2			SM	GRAY BROWN SLIGHTLY SILTY SAND wet, very	į
-	9.60	1						loose, fine to medium grained sand, estimated 5 to 12% fines.	
-	- 1							4 of heaving sands.	4

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CARSON FREEWAY

LOG OF BORING

C-16

PLATE

PROJECT NO. 30-1348-15.002

EXPLORATION LOG 5/10/97 START DATE: _ 5/10/97 END DATE: STATION JOB DESCRIPTION CARSON FREEWAY **OFFSET** EAST OF EMERSON DRIVE LOCATION **ENGINEER BL-15 BORING** EQUIPMENT KLEINFELDER 30-1348-15.002 GROUNDWATER LEVEL **OPERATOR** E.A. # DATE | DEPTH | ELEV. GROUND ELEV 1432.56 m 5/10/97 1.7 1430.9 HAMAGE DOOD SYSTEM AUTOMATIC

			HAMME	R DROP S	SYSTEM_A	UTOMATIC	BACKFILLED YES	DATE _5/10/97	
ELEV.	DEPTH (m)	NO.	TYPE	AMPLE BLOWS/	Recovery	LAB TESTS	USCS	MATERIAL DESCRIPTION	REMARKS
	-								
	10.67								
1421.5	-	K	мс	38					
1421.0	- 111.13			0				11.13	
	-								
	-								1
1420.6	_ 12	ĺ							
	-								
1419.6	L 13								
				1	į		ł		
1418.6	_ 14					1			
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	-		i			1			
417.5	_ 15								
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416.6	_ 16	İ				ĺ			
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ļ									
415.6	_ 17							7 9	
-								Ì	
						1		1	
414.6	_ 18							1	
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ļ									
413.6	_ 19			31	!				
-								ii ii	
Ł						}			
F									

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KLEINFELDER

4875 LONGLEY LANE, SUITE 100 RENO. NEVADA 89502 Tel. (702) 689-7800 CARSON FREEWAY

LOG OF BORING

G-16A

PLATE

PROJECT NO. 30-1348-15.002

CARSON CITY, NEVADA

CAD FILE LY 1999'

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PROJECT NO. 30-1348-15.002

			_		E /0 /07			EXPLORATION LOG		
			START		5/9/97 5/9/97					SHEET 1 OF 2
100	9 28	100	END DA				/A.V		STATION STA 402	-00
						RSON FREEV FEMERSON			OFFSET 0	
.00	M ANY	- :	LOCAT		BL-16	F EINERSON	DHIVE		ENGINEER J. FORGA	\
KI	LEINFELDI	ER	BORING	G _	30-1348-	15.000	——	GROUNDWATER LEVEL	EQUIPMENT CME 55 OPERATOR SPECTRUM	IM
			E.A. #		*			DATE DEPTH ELEV.		
					1432.56			5/9/97 1.7 1430.9		STEM AUGER
						AUTOMATIC			BACKFILLED YES	DATE _5/9/97
ELEV. (m)	DEPTH (m)	NO.	TYPE	AMPLE BLOWS 300mm	Aecover	LAB TEST	Gro		SCRIPTION	REMARKS
							SC	I LIGHT BROWN CLAYS	EY_SANQ moist, loose, fine w to moderate plasticity 30% fines.	
1431.6	-				!					
	1.52	Α	MC	8		CD	_			
1430.6	2 1.98							Encountered groundw	ater at 1.68 M.	
1400.0	2.13	В	MC	13			- SM SP	RED SLIGHTLY SILTY	SAND wet, medium dense,	
	2.59						sc		sand, estimated 5 to 12%	-
				1	İ			RED BROWN CLAYEY dense, fine grained sar		
1429.6	3 3,05	С	MC	26		W.UW.S	_	plasticity fines, estimat	ed 20 to 30% fines.	
	3.51		IVIC	20		vv,uvv,5	SP	.GRAY_BROWN_CLEAN		4% fines
								dense, fine to medium	grained sand.	
1428.6	- 4									
	4.57						SC	4.42		
1427.6	5 5,03	D	MC	16		W,UW,S,PI	- 30	BED BROWN CLAYEY dense, fine grained san fines.	S&NO wet, medium id, moderate plasticity	PI=23 44% fines
	5,18	E	MC	31	4			7.1		
	5.64							5.54 Lenses of clean sand a	nd fat clay	
426.6	- - 6 _{6.10}						SM	WHITE SILTY SAND we medium grained sand.	t, medium dense, fine to	
	-	F	мс	23		w.uw.s		6 40		23% fines
į	6.55						sc			
	-	ŀ						to medium grained san) wet, medium dense, fine d, moderate to high	
425.6	_7							plasticity fines.	-	
	_					1	SM SP	LIGHT BROWN SUGHT	TV SILTV SAND	
	7.62						155	dense, fine grained san	d, estimated 5 to 12%	
404 -	.	G	MC	39		WU.W		fines,		
424.6	-8 8.08								j	
				ļ						
423.6	_ 9 _ 9.14	11	140					Heaving sands at 9 M.:		
	9.60	н	MC	5			sc	9.45		
F	9.75	1 1	MC	16		W,UW,S	-	LIGHT BROWN CLAYEY medium dense, fine to n		24% fines
19. hv 141-	sinfelder, Inc	<u>!</u>								
				NIF) E D		LOG OF	BORING	PLATE
		K			ELC					_
			RE	INGLEY LA INO, NEVAL I. (702) 6		idŲ		CARSON	FREEWAY	C-17

5/9/97 START DATE:

5/9/97 END DATE:

JOB DESCRIPTION CARSON FREEWAY

EAST OF EMERSON DRIVE LOCATION

BL-16 BORING

E.A. #

30-1348-15.002

GROUND ELEV 1432,56 m HAMMER DROP SYSTEM_AUTOMATIC

GROUNDWATER LEVEL

DATE DEPTH ELEV.

5/9/97

1430.9

EXPLORATION LOG

STATION OFFSET

SHEET 2 OF 2 STA 402+00

J. FORGA ENGINEER

0

EQUIPMENT _CME 55 OPERATOR

SPECTRUM **HOLLOW STEM AUGER**

BACKFILLED YES DATE 5/9/97

]						OTOMATIC		BACKFILLED YES DA	TE <u>5/9/97</u>
ELEV.	DEPTH (m)	NO.	TYPE	AMPLE BLOWS/ 300mm	Recovery	LAB TESTS	USCS	MATERIAL DESCRIPTION	REMARKS
	10,21							moderate plasticity fines, estimated 30 to 40% to 36 fines.	
	10.67	J	мс	37			SM SP	GRAY BROWN SUGHTLY SILTY SAND wet, dense, fine grained sand, estimated 5 to 12% silt.	
1421,6	111.13		MC	37				11.13	
1420.6	12								
	- - -								
1419.6	13			:					
						1			
1418.6	_ 14								
1417.6	15								
1417.5	- 13					ļ			
1416.6	- - _ 16								
1415.6	_ 17						Ì		
1414.6	_ 18			31					
ļ							İ		
1413.6	_ 19	!						-	
	-								

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PROJECT NO. 30-1348-15.002

LOG OF BORING CARSON FREEWAY

CARSON CITY, NEVADA

PLATE

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PROJECT NO. 30-1348-15.002

4875 LONGLEY LANE, SUITE 100

RENO, NEVADA 89502 Tel. (702) 689-7800 CARSON FREEWAY

′ | C-1

5/8/97 END DATE: STA 424+00 STATION JOB DESCRIPTION __CARSON FREEWAY 0 OFFSET EAST OF NORTHGATE LANE J. FORGA LOCATION ENGINEER BL-18 EQUIPMENT _CME 55 BORING KLEINFELDER OPERATOR _SPECTRUM 30-1348-15.002 GROUNDWATER LEVEL E.A. # DRILLING DATE | DEPTH | ELEV. GROUND ELEV 1435.91 m HOLLOW STEM AUGER 5/8/97 1434.2 HAMMER DROP SYSTEM AUTOMATIC BACKFILLED YES DATE 5/8/97 SAMPLE TYPE BLOWS/ Recovery 300mm (%) ELEV. DEPTH USCS LAB TESTS MATERIAL DESCRIPTION NO. REMARKS (m) SC LIGHT_BROWN_CLAYEY_SAND moist, loose to medium dense, fine to medium grained sand, low to moderate plasticity fines. 1434.9 CH LIGHT BROWN SLIGHTLY SANDY CLAY moist, stiff to very stiff, line grained sand, moderate to high plasticity fines. MC 20 W.UW Encountered groundwater at 1.68 M. Becoming more sand. SC 1433.9 GRAY BROWN CLAYEY SAND wet, medium dense, fine grained sand, low plasticity fines, estimated 10 to 20% fines. 1432.9 ₿ MC 20 W,UW,S 34% fines 3.5 Fine to coarse grained sand. CH LIGHT BROWN SLIGHT SANDY CLAY wet, stiff to 1431.9 very stiff, fine grained sand, moderate to high plasticity fines. MC C 24 W,UW,S 16% fines SC 1430.9 LIGHT BROWN CLAYEY SAND wet, medium 5 5 03 dense, fine to medium grained sand, low to moderate plasticity fines. SM SP GRAY BROWN SLIGHTLY SILTY SAND wet, medium dense, fine to medium grained sand, 1429.9 6 estimated 5 to 12% fines. D MC 21 W,UW,S 29% fines 6.55 SC LIGHT BROWN CLAYEY SAND wet, medium dense, fine to medium grained sand, low to SM SP moderate plasticity fines, estimated 15 to 25% 1428.9 fines GRAY BROWN SLIGHTLY SILTY SAND wet, medium dense, fine to medium grained sand, estimated 5 to 12% fines. W.UW Ε MC 32 1427.9 8 8.08 SC 8.08 LIGHT BROWN CLAYEY SAND wet, dense, fine to medium grained sand, low to moderate plasticity fines. 1426.9 _ 9

EXPLORATION LOG

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KLEINFELDER

START DATE: 5/8/97

4875 LONGLEY LANE, SUITE 100 RENO, NEVADA 89502 Tel. (702) 689-7800 LOG OF BORING

CARSON FREEWAY

6-18

PLATE

SHEET 1 OF 1

PROJECT NO. 30-1348-15.002

CARSON CITY, NEVADA

CAD FILE: L:\1999/ TING\301348\30134815002CPLATES.DWG

_									
	HAMMER	E: CRIPTIC N N ELEV_ I DROP S	EAST OF BL-19 30-1348-15			GROUNDWATER LEVEL DATE DEPTH ELEV. 5/8/97 1.4 1434.2	EQUIPMENT . OPERATOR . DRILLING	STA 429+0 0 J. FORGA CME 55 SPECTRUM HOLLOW S	1 TEM AUGER
— Э.	TVDE	MPLE BLOWS/ 300mm	Recovery	LAB TESTS	USCS USCS	MATERIAL DE	SCRIPTION		REMARKS
					sc	LIGHT BROWN CLAYE loose to medium dens to moderate plasticity	e, fine grained s		

	1	·	SA	AMPLE		1	I		
ELEV. (m)	DEPTH (m)	NO.	TYPE	BLOWS/ 300mm	Recovery	LAB TESTS	Group	MATERIAL DESCRIPTION	REMARKS
	}		i	ĺ			SC	LIGHT BROWN CLAYEY SAND slightly moist,	
	}						İ	loose to medium dense, fine grained sand, low to moderate plasticity fines.	
								is with the same production of the same same same same same same same sam	
434.6									
			li						
	T.52							Encountered groundwater at 1.37 M.	
	-	Α	МС	7		W,UW,S			13% fines
433.6	2 1.98							198	
,00.0	2.13	В	MC	2		PI	SC	LIGHT BROWN CLAYEY SAND wet, very loose,	550 fi
	- 1		IVIC	-		Fi		fine grained sand, non-plastic.	22% fines
	2.59	С	SHELBY			(8)			
432.6	L ₃			İ					
73E.0									
	3.35			- !				į	
	}			İ				ļ	
	3.96							İ	
431.6	- 4 0.50	D	MC	14					
	4.42					ĺ		4.42	
	4.57	Е	MC I	7		S.DS	SM	GRAY BROWN SILTY SAND wet, loose, fine to	
	- 1	-	NIC	_ ′		3,03		medium grained sand, some thin lenses of more	19% fines
430.6	_ 5 5.03				1			silty material,	
	3.18	F	МС	3				Very loose	
	5.64	ļ					SC	5.49	
	_						30	LIGHT BROWN CLAYEY SAND wet, very loose,	
429.6	- 6 _{6.10}							very fine grained sand, non-plastic.	
	-	G	MC	3	1	S,DS	ļ		29% fines
	6.55		-					Becoming more sandy.	
	6.71	H	мс	2	<u> </u>	w.uw			
428.6	7	*		-				Assessed	
	7.16	l I		-	<u>:</u>		1	Less sandy.	
	[-	İ			¥	
1	7.52	T	MC	29		w.uw			
127.6	8 8.08	50		1		10	SC	9 08 LIGHT BROWN CLAYEY SAND was madium	
	- 6.08			1	i		SUL	dense fine to medium grained sand moderate	
	-			i				to high plasticity fines, estimated 20 to 30% fines	
	-				9 10 9				
26.6	_ 9		i	ļ	[
-20.0	- "			1					
[i		ĺ]	
	.				Ī		1		
	. !								

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PROJECT NO. 30-1348-15.002

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LOG OF BORING CARSON FREEWAY

CARSON CITY, NEVADA

PLATE

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PROJECT NO. 30-1348-15.002

	ا ا	1	START I	TE: _	5/14/97 5/14/97				07.47.0	STA 439+0	SHEET 1 OF 1
		₹.			3/14/9/				OTATION	CTA 420 LO	
	\sim				CAR	CON EDEEM	IAV		STATION		
1214						SON FREEW			OFFSET	0	
	L ANY	- 7.	LOCATI		BL-20	NONTINGA	IE LAN		ENGINEER .	J. FORGA	
KLE	EINFELDE	Я	BORING	3 -				COCUMOWATER LEVEL		SPECTRUM	
			E.A. #	-	30-1348-1			DATE DEPTH ELEV.	· · · · ·		
				~	1439.27 п			5/14/97 1.5 1437.7	DRILLING METHOD	HOLLOW S	
						AUTOMATIC		<u> </u>	BACKFILLED .	YES DA	TE _5/14/97
ELEV. (m)	DEPTH (m)	NO.	TYPE	AMPLE BLOWS 300mm	Recovery	LAB TEST	Gibu	MATERIAL DE	SCRIPTION		REMARKS
-							SM	LIGHT BROWN SILTY medium dense, fine to estimated 20 to 30% fi	coarse grained	oist, sand,	
1438.3	_1	- 1				1					
ŀ		ı					SM	1 22			
-	7.52						SM SP	LIGHT BROWN SLIGHT dense, fine to coarse o) maist,	
[.	_	Α	MC	32		W,UW,S		Encountered groundwa			6% fines
1437.3	2 1 98					1	SC	1 98			-
			i					LIGHT BROWN CLAYE grained sand, low plass	Y SAND wet, loc ticity fines.	ose, fine	
1436.3	3 3.05	- 1				1					
F		В	МС	7		S	-				15% fines
Ł	3,51						_				
-	3.66	-	MC	5		ICD	-				
435.3	. 4	١		,							ļ
-	4.11						-				
-	4.57										
-	4.37	ō	MC	13	İ	w.uw.s	-				23% fines
434.3 L	5 5.03		- 1				1	5 03 Becoming more sandy.			
	5.18	_	140	24		t .	SM	GRAY BROWN SLIGHT			İ
-		E	MC	34			SP	5 40 dense, fine to medium	grained sand, es	stimated 5	ļ
-	5.64						_ sc	to 12% fines. LIGHT BROWN CLAYE	V SAND wet .me	dium	
433.3 L	.60	İ	į					dense, very fine grained	d sand, estimate	d 15 to	
F	6.10	F	мс	28		W.UW.S	SM	6.10 25% fines, low plasticity	fines.		7% fines
-	6.55	- 1					SP	GRAY BROWN SLIGHT medium dense, fine to	LY SILTY SAND	wet,	
F	0.557	i					1	mediam dense, inte to	mediam gramec	- Saliu	
432.3 L	7		Į								
	'										
-					1						
-	7.62	G	MC I	10	71		-l i	Becoming less sandy:			
431.3	8 2 02	٦		10	8			7.92			
- E	8.23						CL	LIGHT BROWN VERY S		, stiff, fine	
-		Н	MC	5		S,PI,CO		grained sand, low plasti	city fines.		PI= 12
<u> </u>	8,69						.				54% fines
430.3	9 9 14							Becoming more sandy,	moderate plasti	city fines.	
	9,60	1	МС	30			SM SP	LIGHT BROWN SLIGHT 9.60 medium dense to dense sand, estimated 5 to 12	fine to mediun		
					1						
i, by Klainf	felder, inc.										
								10005	PODING		PLATE
		KI	EI	NF	ELD	ER		LOG OF	DURING		

EXPLORATION LOG

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CARSON FREEWAY

LOG OF BORING

CARSON CITY, NEVADA

PLATE

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PROJECT NO. 30-1348-15.002

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333	$\Delta \Gamma$	100
100	469	12.35
15465	ALC:	17405

START DATE: __ END DATE:

5/8/97

5/8/97

JOB DESCRIPTION CARSON FREEWAY

LOCATION

WEST OF NORTHGATE LANE

BORING

E.A. #

30-1348-15.002

GROUND ELEV 1435.61 m

GROUNDWATER LEVEL DATE DEPTH ELEV. 1434.1 5/8/97 1.5

EXPLORATION LOG

STATION

SHEET 2 OF 2

STA 434+00 OFFSET

0

ENGINEER

J. FORGA

EQUIPMENT **OPERATOR**

CME 55 **SPECTRUM**

			HAMME		YSTEM_A	UTOMATIC	_	5/8/97 1.5 1434.1 BACKFILLED YES DAT	
ELEV.	OEPTH (m)	NO.	TYPE	MPLE BLOWS/ 300mm	Recovery	LAB TESTS	USCS	MATERIAL DESCRIPTION	REMARKS
	10,21							10.21 Becoming more sandy, fine to medium grained sand.	
1424.6	_ 11						ļ		
1423.6	_ 12								
1422.6	13								
1421.6	_ 14				1 1 1 1 1 1 1 1 1				
1420.6	15								
1419.6	16 								
1418.6	- - - 17					ļ			
1417.6	- 18								
1416.6	_ 19						:		

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CARSON CITY, NEVADA

LOG OF BORING **CARSON FREEWAY**

PLATE

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PROJECT NO. 30-1348-15.002

					5/8/97			EXPLORATION LOG		
			START	····	<u>3/0/9/</u> 5/8/97					SHEET 1 OF
			END DA			SON FREEWA	v		STATION STA 444+0	0
						F BROADLE			OFF3E1	
_10	IL TILL		LOCATI		BL-22	A SHOULE	AL PAIAI	·	ENGINEER J. FORGA EQUIPMENT CME 55	
K	LEINFELD	ER	BORING	-	30-1348-1	E 002		GROUNDWATER LEVEL	OPERATOR SPECTRUM	
			E.A. #	_				DATE DEPTH ELEV.	DRILLING	
					1437.13 m			5/8/97 1.4 1435.8		TEM AUGER
						AUTOMATIC			BACKFILLED YES DA	ATE 5/8/97
ELEV. (m)	DEPTH (m)	NO:	TYPE	AMPLE BLOWS/ 300mm	Recovery	LAB TESTS	GIOUH	MATERIAL DE	SCRIPTION	REMARKS
	-						SM	LIGHT BROWN SILTY medium dense, fine to estimated 20 to 30% fi	coarse grained sand.	
1436.1	<u></u>						}	4.07		
	. <u>▼</u> <u>T.</u> 52				-	12.	ML	1.37		
	2 1.98	A	MC	7		S,CD		Encountered groundway LIGHT BROWN SANO) grained sand	ater at 1,37 M. (SILT wet, stiff, fine	64% fines
1435.1	2.13	<u> </u>	146	12			1	* arco 16 11		
		В	MC	13						
	2.59									
1434.1	3 3,05									
	-	С	MC	12		W,UW,S,PI		3.35		P! = 5 18% fines
	3.51					-	SC) wet, medium dense, fine	
	-	D	МС	14				grained sand, low plast Lenses of cleaner sand	ticity fines.	
1433.1	- 4 4.11							Editor of olderer serio		
	4.57	E	MC	10						
1432.1	5 5,03									
	5.18	F	MC	18	- 2					
	-	[(VIÇ	10	i i					
	5.64							5.79		
1431.1	- 6 _{6.10}						SM	GRAY BROWN SUGHT	LY SILTY SAND wet, very	
	-	G	MC	54		W,UW,S	-	dense, fine to medium	grained sand.	11% fines
	6.55							6.55		i.
	-									
1430.1	7									
	-				ĺ					
1430 4										
1429.1	_8									
428.1	_9									
				Ì						
	-									
by Klei	nfelder, inc.				0-2-3				2 2 2	
		17.1	EIN	NEE	ID	ED	0.	LOG OF E	BORING	PLA"

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CARSON FREEWAY

PROJECT NO. 30-1348-15.002

EXPLORATION LOG

5/7/97

START DATE:

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PROJECT NO. 30-1348-15.002

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CARSON FREEWAY

(;-24

PLATE

SHEET 1 OF 2

START DATE: __5/7/97

EXPLORATION LOG

SHEET 2 OF 2

5/7/97 END DATE:

JOB DESCRIPTION CARSON FREEWAY

STATION OFFSET STA 449+00

LOCATION

WEST OF BROADLEAF LANE

ENGINEER

_J. FORGA

BORING KLEINFELDER

BL-23

EQUIPMENT _CME 55

OPERATOR SPECTRUM

E.A. #

30-1348-15.002 GROUND ELEV 1440.48 m

GROUNDWATER LEVEL DATE | DEPTH | ELEV.

HOLLOW STEM AUGER

HAMMER DROP SYSTEM AUTOMATIC

1438.7

BACKFILLED YES DATE 5/7/97

ELEV.	OEPTH (m)	NO.	TYPE	AMPLE BLOWS/ 300mm	Recovery (%)	LAB TESTS	Group	MATERIAL DESCRIPTION	REMARKS
	10,21						_	10 21	
	ļ								
429.5	_ 11								ĺ
	-								
428.5	- _ 12	ł		:					
	.								
	-								
427.5	- _ 13								
	-								
	-					i			
1426.5	_ 14	ľ							
	.								
ŀ	:			1					
425.5	_ 15								
ļ	.								
424.5	_ 15						-		
-						ļ]
						Ì			
423.5	_ 17								
-	İ								
-									
422.5	. 18	ı							
-									
ļ									
421.5	. 19	-							
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CARSON FREEWAY

LOG OF BORING

PLATE

PROJECT NO. 30-1348-15.002

CARSON CITY, NEVADA

L\1999\1 TING\301348\30134815002CPLATES.DWG

START DATE: 5/7/97

5/7/97

JOB DESCRIPTION __CARSON FREEWAY

STATION **OFFSET** SHEET 1 OF 1

LOCATION

END DATE:

NORTH OF BROADLEAF LANE

1440.0

0 _J. FORGA ENGINEER

KLEINFELDER

BL-24 **BORING** 30-1348-15.002

GROUNDWATER LEVEL

EQUIPMENT _CME 55 **OPERATOR**

SPECTRUM

STA 454+00

E.A. # GROUND ELEV 1441.70 m

DATE DEPTH ELEV.

EXPLORATION LOG

HOLLOW STEM AUGER

HAMMER DROP SYSTEM AUTOMATIC

BACKFILLED YES DATE 5/7/97

	-			AMOI E		T		BACKFILLED TES	AIE _3/.//31
ELEV	DEPTH	NO.	TYPE	BLOWS,	Aecovery	LAB TESTS		MATERIAL DESCRIPTION	REMARKS
1440.7	1						SM	LIGHT BROWN SILTY SAND slightly moist, loose to medium dense, fine to coarse grained sand, estimated 20 to 30% fines.	
1439.7	1,52	A	МС	26			sc	LIGHT BROWN CLAYEY SAND wet, medium dense, fine to medium grained sand, low plasticity fines. Encountered groundwater at 1.68 M.	
							614	2 59	
1438.7	3 3.05	8	мс	21		w.uw.s	SM	GRAY BROWN SILTY SAND wet, medium dense, fine to medium grained sand.	18% fines
1437.7	3.51								
1436.7	4.57 _ 5 5.03	С	MC	10		DS	SC	4.88	:
:	5.18 - 5.64	D	МС	13		s		LIGHT BROWN CLAYEY SAND loose to medium dense, fine grained sand, low plasticity fines. Fine to medium grained sand.	27% fines
1435.7	6 6.10	E	MC	21		s,w.uw	į		29% fines
1434.7	7							6.55 Low to moderate plasticity fines.	8.0
1433.7	8								
1432.7	9				4				

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CARSON CITY, NEVADA

LOG OF BORING CARSON FREEWAY

PLATE

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PROJECT NO. 30-1348-15.002

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GROUND ELEV 1443.23 m

END DATE:

LOCATION

BORING

E.A. #

KLEINFELDER

5/7/97

BL-25

30-1348-15.002

JOB DESCRIPTION CARSON FREEWAY

SOUTH OF ARROWHEAD DRIVE

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LOG OF BORING CARSON FREEWAY

SHEET 1 OF 1

STA 458+00 **STATION** 0 OFFSET

EXPLORATION LOG

GROUNDWATER LEVEL

DATE | DEPTH | ELEV.

ENGINEER J. FORGA EQUIPMENT _CME 55

OPERATOR SPECTRUM

HOLLOW STEM AUGER BACKFILLED YES

					SYSTEM A	AUTOMATIC		5/7/97 1.7 1441.6 METHOD HOLLOW STEM AUGER BACKFILLED YES DATE 5/7/97
							·-··	SACKTEED TO DATE STATE
ELEV.	OEPTH (m)	NO.	TYPE	BLOWS/	Recovery (%)	LAB TESTS	Group	
1442.2							SM	LIGHT BEOWN SILTY SAND slightly moist, loose to medium dense, fine to coarse grained sand, estimated 20 to 30% fines.
1442.2	1.52	А	MC	7		IDS	SM SP	Encountered groundwater at 1.68 M. YELLOW BROWN SLIGHTLY SILTY SAND toose,
1441.2	2 1.98 2.13	8	MC	9		s		fine to coarse grained sand, estimated 5 to 12% fines. 8% fines
1440.2	2.59 3 3.05							
	3.51	С	мС	33				Light brown, medium dense to dense, some line gravel.
1439.2	- _ 4 -							
	4.57	D	MC	20		W,UW,S	SC	4.57 25% fines
1438.2	_ 5 5.03			20			30	BROWN CLAYEY SAND wet, medium dense, fine grained sand, low to moderate plasticity fines.
1437.2	- 6 6.10	E	MC	27			SM SP	GRAY BROWN SLIGHTLY SILTY SAND wet,
	6.55						0,	medium dense, fine to medium grained sand, estimated 5 to 12% fines. Fine sand.
1436.2	7.62						SC	7.32 LIGHT_BROWN_SLIGHTLY_CLAYEY_SAND
1435.2	- 8 _{8.08}	F	мс	20				medium dense, fine grained sand, low plasticity fines, estimated 10 to 20% fines.
1434.2	9							
	-							

PLATE

PROJECT NO. 30-1348-15.002

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CARSON FREEWAY

LOG OF BORING

PLATE

PROJECT NO. 30-1348-15.002

EXPLORATION LOG

DATE | DEPTH | ELEV.

SHEET 1 OF 1

5/7/97 END DATE:

GROUND ELEV 1447.80 m

START DATE: _

LOCATION

BORING

E.A. #

KLEINFELDER

5/7/97

30-1348-15.002

JOB DESCRIPTION CARSON FREEWAY

NORTH OF ARROWHEAD DRIVE

STA 467+00 STATION OFFSET

ENGINEER

_J. FORGA

GROUNDWATER LEVEL

EQUIPMENT _CME 55 SPECTRUM **OPERATOR**

HOLLOW STEM AUGER

					SYSTEM A	UTOMATIC	5/7/97 3.5 1444.3 METHOD HOLLOW STEM AUGER BACKFILLED YES DATE 5/7/97			
	-									
ELEV.	DEPTH (m)	NO.	TYPE	BLOWS/	Recovery	LAB TESTS		MATERIAL DESCRIPTION REMARKS		
1446.8	_1						SM SP	LIGHT BEOWN SLIGHTLY SILTY SAND slightly moist, loose, fine to medium grained sand, trace of fine surface gravel, estimated 5 to 12% fines.		
1445.8	1.52 2 1.98 2 1.3	A	MC MC	7 26		S,CD	SM	9% fines 1 98 LIGHT BROWN SILTY SAND moist, medium		
1444.8	2.59	C	мс	41		w.uw,s		dense, fine to medium grained sand. Becoming wet, red brown. 14% fines		
1443.8	y 51							Encountered groundwater at 3.5 M.		
1442.8	4.57 - 5 5.03	D	MC	38		w,ūw,s		17% fines		
1441.8	6.55 6.71	E	МС	17			SC	6.55		
1440.8	7 7.16 7.62	F	MC	15			30	BROWN CLAYEY SAND moist, medium dense, fine to medium grained sand, low plasticity fines.		
1439.8	8 8.08 8,23	н	MC MC	25		W,UW,S,PI		Pt=9 31% fines Becoming more dense.		
1438.8	9									

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CARSON FREEWAY

LOG OF BORING

CARSON CITY, NEVADA

PLATE

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START DATE: __5/7/97

LOCATION

BORING

E.A. #

5/7/97

END DATE: JOB DESCRIPTION _ CARSON FREEWAY

30-1348-15.002

STA 474+00 STATION

0

SOUTH OF BONANAZA DRIVE

OFFSET

ENGINEER __J. FORGA

EQUIPMENT CME 55 OPERATOR SPECTRUM

GROUNDWATER LEVEL DATE DEPTH ELEV.

EXPLORATION LOG

HOLLOW STEM AUGER

SHEET 1 OF 1

GROUND ELEV_1452.68 m HAMMER DROP SYSTEM AUTOMATIC

BL-28

BACKFILLED YES DATE 5/7/97

						UTOMATIC			BACKFILLED YES D	ATE <u>5/7/97</u>
ELEV.	DEPTH	NO.	S. TYPE	AMPLE BLOWS/	Recovery	LAB TESTS	USCS		MATERIAL DESCRIPTION	REMARKS
							SM		BBOWN SILTY SAND slightly moist, loose to medium dense, fine to coarse grained sand and fine surface gravel, estimated 20 to 30% fines.	
1451.7	1.52							j 	Encountered coarse gravel at 2 M.	
1450.7	2 1.98 2.13	A	MC	12		CD		<u> </u> 	Elicodificied coalse graver at 2 M.	
	2.59	8	MC	25		w,uw,s		274	Low plasticity fines.	21% fines
1449.7	_3 3.05	С	МС	62	-	w.uw.s	SP		YELLOW BROWN CLEAN SAND slightly moist, very dense, fine to coarse grained sand, trace of fine gravel.	4% fines
1448.7	3.51		**!							
1447,7	4.57	D	мс	44		w,uw,s			*	4% fines
1447.7	<u> 5 5,03 </u>							5.64		
1446.7	- ⁶ 6.10	E	MC	25		w,uw,s	SM SP		RED BROWN SLIGHTLY SILTY SAND moist, medium dense, fine grained sand.	12% fines
1445.7	7									
1444.7	8 _{8.08}	F	мс	32		w.uw.s	;		Encountered groundwater @ 7.62M Wet, fine to coarse grained sand.	10% fines
1443.7	- - 9 - 9 14	G	мс	36				9.45		
	9.60						SC	9 60	LIGHT BROWN CLAYEY SAND wet, medium dense to dense, fine to coarse grained sand, moderate to high plasticity fines.	

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LOG OF BORING

CARSON FREEWAY

PLATE

PROJECT NO. 30-1348-15.002

		ELE (m	y.
		1454.	7
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)	1451.	7 -
		1450.3	,
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		1448.7	, , , , , , , , , , , , , , , , , , , ,
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8\3013481500		1446.7	
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8		1+				F BONANZA			DC4
,80	H ANY 1		LOCATI		BL-29	J DONAND	COMIVE	ENGINEER J. FO EQUIPMENT CME	
K	LEINFELDE	R	BORING	. –	30-1348-1	5.002			
			E.A. #	_					47
					1455.72 m			5/7/97 9.4 1446.3 METHOD HOLL	OW STEM AUGER
						AUTOMATIC		BACKFILLED YES	DATE <u>5/7/97</u>
ELEV. (m)	OEPTH (m)	NO.	TYPE	AMPLE BLOWS/ 300mm	Recovery	LAB TEST	Group	MATERIAL DESCRIPTION	REMARKS
							SM	BROWN SILTY SAND slightly moist, fine to coarse grained sand, some fine surface grave	ı.
154.7	1,52								
53.7	2 1.98 2.13	A	MC	11		CD	-	Becoming more sandy.	
	2.59	В	MC	10		S	- - -	Trace of coarse gravet.	20% fines
452.7	_ 3 _{3.05}	С	мс	28		W,UW,S			20% fines
	3.51						-	Medium dense, fine grained sand.	
151.7	- 4		i					4.42	
150.7	4.57 _ 5 5.03	D	МС	52		w,uw,s	SM	BED BROWN SUGHTLY SILTY SAND moist, ve dense, fine grained sand, estimated 5 to 12% fines, some fine gravel.	8% fines
49.7	- - 6 _{6.10}	-	MC	20		AN LUM E	634	6.10	
	6.55	E	MC	38		w,uw,s	SM	LIGHT BROWN SILTY SAND moist, medium dense, fine grained sand, estimated 15 to 25% fines, low plasticity fines.	16% fines
48.7	_ 7 -								
47.7	7.62 - 8 8.08	F	мс	23	1	w,uw			
									•
46.7	9 9 14	G	мс	26		v,uw	sc	LIGHT BROWN CLAYEY SAND wet, medium dense, fine to coarse grained sand, moderate to	
	y 9. 60							high plasticity fines, 9.60 Encountered groundwater at 9.45 M.	



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LOG OF BORING **CARSON FREEWAY**

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9.50 Very moist, very dense, fine gravel.
No free water encountered.

CARSON FREEWAY

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PLATE

PROJECT NO. 30-1348-15.002

CARSON CITY, NEVADA

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			STADT	DATE: .	5/6/97			EXPLORATION LOG		
		257.00	END D		5/6/97					SHEET 1 OF
	_					SON FREEW	ΑY			89+00
			LOCAT			F U.S. 395			OFFSET _U ENGINEER _J.FOF	264
	C EINEEL E		BORIN		BL-31	-0.1			EQUIPMENT CMES	
"	KLEINFELD	ÆΗ	E.A. #	-	30-1348-15	5.002		GROUNDWATER LEVEL	OPERATOR _SPEC	
				un ei ev	1469.14 m			DATE DEPTH ELEV.	DRILLING	
						UTOMATIC				DW STEM AUGER
	1	ì		AMPLE	01014112				BACKFILLED YES	DATE <u>5/6/97</u>
ELEV.	DEPTH (m)	NO.	TYPE	BLOWS 300mg	Recovery	LAB TESTS	RSCA	MATERIAL DE	SCRIPTION	REMARKS
	1						SM	BOOWN SILTY CAND -	Colonius — class de la colonius	
	-	1						BROWN SILTY SAND s medium dense, fine to	coarse grained sand	
	†						!	some fine gravel, estim	ated 15 to 25% fines.	
1468.1	L,	1								
] [
l	1.52									
	-	A	МС	6	i i		1			
1467.1	2.1.98									1
	2.13	В	MC	11	-		1			
	}		""	``			1 1			
	2.59			 						
1466.1	3 3.05									
	-	С	MC	12		W,UW		Becoming more sandy,	estimated 10 to 20%	
	3.51							fines. Lenses of clean sand, e	stimated 5 to 12% fines	. [
1465.1	<u>L</u> 4							-		
	-		i					Fine to coarse grained s piece of coarse gravel 3	and, encountered a cm in diameter.	
	4,57									
		D	MC	30	1	WU,UW				
1464.1	5 5,03									is a
	-									
	- 1							5.49		ļ
	Ì I						SC	BROWN CLAYEY SAND	moist, medium dense	
1463.1	- 6 _{6.10}	-					}	fine to medium grained : plasticity fines, estimate:	sand low to moderate	
	- 0.10	E	мС	28	l v	v,uw		presucity lines, estimated	2 10 10 25% lines,	
	6.55					ļ	ŀ			
į										
1462.1	7			ļ	ŀ	}				
}	-							7_32		İ
	-					<u> </u>	SM		AID	1
	7.62	F	MC	30	!v	v,uw	Ì	LIGHT BROWN SILTY SA medium grained sand, e.	ND moist, dense, line to stimated 10 to 20%	· [
1461.1	_ 8 _{8.08}		_		ľ			fines.		
	- 9.08	i			i					ļ
-	.					H	sc	9.38		1
								BROWN CLAYEY SAND to medium grained sand.	ery moist, dense, fine	1
1460.1	_ 9			1				plasticity fines, estimated	15 to 25% fines.	
-	9.14	G	MC	43	- Iw	/.UW				1
- 1		-	1110	70	1 41	1411	- 1			30

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LOG OF BORING CARSON FREEWAY

9.60 Becoming more sandy. No free water encountered.

PLATE

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PROJECT NO. 30-1348-15.002

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CARSON FREEWAY

C - 33

PLATE

CARSON CITY, NEVADA

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LOG OF BORING CARSON FREEWAY

CARSON CITY, NEVADA

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PROJECT NO. 30-1348-15.002

PLATE

START	DATE:	5/12/97
SIMUL	DATE:	-7, -, -, -

END DATE: 5/12/97

JOB DESCRIPTION CARSON FREEWAY

LOCATION NORTH OF CARMINE

BORING BL-34

E.A. # 30-1348-15.002 GROUND ELEV 1417.32 m

HAMMER DROP SYSTEM_AUTOMATIC

EXPLORATION LOG

GROUNDWATER LEVEL

DATE | DEPTH | ELEV.

5/12/97 | 1.5 | 1415.8

STATION STA 339+00

OFFSET 0

ENGINEER J. FORGA
EOUIPMENT CME 55

OPERATOR SPECTRUM

PRILLING HOLLOW STEM AUGER

SHEET 1 OF 1

BACKFILLED YES DATE 5/12/97

1	HAMMER DROP SYSTEM AUTOMATIC						BACKFILLED YES DATE 5/12/97	
ELEV.	DEPTH	\vdash		AMPLE	Aecovery (%)	LAB TESTS	LISC	S MATERIAL DECORPORATION
(m)	(m)	NO	TYPE	300mm	(%)	CAB TESTS	Gion	
	-						SM	LIGHT BROWN SILTY SAND slightly moist, loose ti medium dense, fine to coarse grained sand, estimated 25 to 35% lines.
1416.3	- 1					İ		1.07
		A	MC	37		W,UW,S,PI	SM	YELLOW BROWN SILTY SAND moist, fine to medium grained sand. Encountered groundwater at 1.52 M.
1415.3	2 1.98							Non-plastic.
1415.3								274
1414.3	3 3.05						SC	YELLOW BROWN CLAYEY SAND wet, loose to
	3.51	В	МС	11				medium dense, fine grained sand, estimated 25 to 35% fines, low to moderate plasticity fines.
1	3.55	С	MC	15				
1413.3	_ 4 4.11		IVIC	13				
	[ŀ	-		E E		SM	4 27
	4.57	D	MC	34	-	w.uw	SM SP	GRAY SLIGHTLY SILTY SAND wet, dense, fine to medium grained sand, estimated 5 to 12% fines,
1412.3	_ 5 5.03	1		"	1	,		
1412.3	5 5.03							5.03
1411.3	_ 6 - -							
1410.3	7					;		
1409.3	- - 8							
1408.3	9							

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CARSON FREEWAY

CARSON CITY, NEVADA

PLATE

C-35

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CARSON FREEWAY

CARSON CITY, NEVADA

PLATE

C-36

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START DATE: ____5/8/97

GROUND ELEV. 1435.61 m

LOCATION

BORING

E.A. #

5/8/97

END DATE: JOB DESCRIPTION __CARSON FREEWAY

BL-36

EAST OF EMERSON

30-1348-15.002

STATION 0

EXPLORATION LOG

GROUNDWATER LEVEL

DATE DEPTH ELEV.

STA 404+00

OFFSET ENGINEER __J. FORGA

EQUIPMENT _CME 55 OPERATOR SPECTRUM

DRILLING METHOD

HOLLOW STEM AUGER

SHEET 1 OF 1

i				in erev		UTOMATIC		5/8/97 1.7 1433.9 METHOD HOLLOWS	
								BACKFILLED YES DA	ATE 5/8/97
ELEV.	DEPTH (m)	NO.	TYPE	AMPLE BLOWS/ 300mm	Aecovery	LAB TESTS		MATERIAL DESCRIPTION	REMARKS
	-						SC	LIGHT BROWN CLAYEY SAND slightly moist, medium dense, fine to coarse grained sand, low to moderate plasticity fines.	
1434.6	_ 1								
1433.6	1.52 2 1.98 2.13	A	МС	17		w,uw,s		Encountered groundwater at 1.68 M., fine to medium grained sand.	12% fines
	2.59	В	МС	18				Becoming more clayey.	
1432.6	3 3.05	С	мс	18		w.uw,s			35% fines
1431.6	3.51 - 3.66 - 4 - 4.11	D	MC	5				Loose	
	4.57	E	MC	11		CD,S			39% fines
1430.6	_ 5 5.03 5.18	F	МС	19					
1429.6	5.64 - 6 6.10							Fine grained sand, becoming more sandy.	
***	6.55	G	МС	32		w.uw.s		Lenses of clean sand, fine to medium grained sand.	24% fines
1428.6	- _ 7 -								
1427.6	- - _ 8				1 1 1 4				
1425.6	- - - - 9								
					!				

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LOG OF BORING CARSON FREEWAY

CARSON CITY, NEVADA

PLATE

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CARSON FREEWAY

LOG OF BORING

CARSON CITY, NEVADA

PLATE

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EXPLORATION LOG 5/13/97 START DATE: SHEET 1 OF 2 5/13/97 END DATE: STA 503 + 00 STATION JOB DESCRIPTION __CARSON FREEWAY 121.9 M NORTH OFFSET NORTH OF U.S. 395 LOCATION ENGINEER _CME 55 BL-38 **EQUIPMENT** BORING KLEINFELDER SPECTRUM **OPERATOR** 30-1348-15.002 GROUNDWATER LEVEL E.A. # DRILLING METHOD DATE | DEPTH | ELEV. GROUND ELEV 1514.25 m HOLLOW STEM AUGER HAMMER DROP SYSTEM AUTOMATIC BACKFILLED _ YES DATE _5/13/97 SAMPLE BLOWS/ Recovery 300mm (%) ELEV. DEPTH LAB TESTS MATERIAL DESCRIPTION REMARKS NO. TYPE | (m) SM YELLOW BROWN SILTY SAND slightly moist, dense, fine to coarse grained sand, surface gravel, cobbles and boulders up to 36 cm Decomposed granite 1513.2 MC 50/127 W.UW.S Very dense. 16% fines 1512.2 1511.2 3.20 B MC 1 50/140 W.S 18% fines 1510.2 4.57 MC 50/127 1509.2 1508.2 6 5.10 6.25 D MC 50/76 1507.2 7.62 MC 50/114 1506.2 1505.2 9.301 F MC 50/76 ©1999, by Kleinfelder, Inc. PLATE **LOG OF BORING**

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PROJECT NO. 30-1348-15.002

CARSON FREEWAY

KLEINFELDER

START DATE: 5/13/97

5/13/97 END DATE:

JOB DESCRIPTION __CARSON FREEWAY

LOCATION

NORTH OF U.S. 395

BORING

30-1348-15.002

E.A. # GROUND ELEV 1514.25 m

HAMMER DROP SYSTEM AUTOMATIC

EXPLORATION LOG

GROUNDWATER LEVEL

DATE | DEPTH

ELEV.

SHEET 2 OF 2

STA 503+00 STATION 121.9 M NORTH

ENGINEER _J. FORGA

EQUIPMENT _ CME 55

SPECTRUM OPERATOR

HOLLOW STEM AUGER BACKFILLED YES DATE 5/13/97

					- INC			DAONFILLED	
ELEV.	OEPTH (m)	NO.	TYPE	BLOWS/	Recovery	LAB TESTS	USCS	MATERIAL DESCRIPTION	REMARKS
15090	1 (111)			300mimu	1-01				
	ŀ								
	10.67								
Ì	10.82	G	MC	50/127					
1503.2	_ 11								
	-								
	}								
	-								
1502.2	_ 12								
	12.19							821	
	12,34	H	MC	50/114	-			12.34	1
	-							No free water encountered,	
1501.2	_ 13				l i				
					-				
1500.2	_ 14								
	-								
	-								
1499.2	_ 15						Ī		
1499.4	_ 13								
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1498.2	_ 16		i						
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				-					
1497,2	_ 17		-		30				
777	-								
	-		İ						
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1495.2	_ 18				1		-		
1430.4	- 19			į.					
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1495.2	_ 19 💿						- 1		
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	-								
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PROJECT NO. 30-1348-15.002

LOG OF BORING CARSON FREEWAY

CARSON CITY, NEVADA

PLATE

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EXPLORATION LOG START DATE: _5/13/97 SHEET 1 OF 5/13/97 END DATE: STA 511+00 STATION JOB DESCRIPTION CARSON FREEWAY 91 M NORTH **OFFSET** NORTH OF U.S. 395 LOCATION J. FORGA ENGINEER BORING EQUIPMENT CME 55 KLEINFELDER **OPERATOR SPECTRUM** 30-1348-15,002 GROUNDWATER LEVEL E.A. # DRILLING METHOD DATE | DEPTH | ELEV. GROUND ELEV_ 1530.10 m **HOLLOW STEM AUGER** HAMMER DROP SYSTEM AUTOMATIC BACKFILLED YES DATE _5/13/97 AMPLE | BLOWS/ Recovery | 300mm (%) ELEV. (m) DEPTH (m) USCS LAB TESTS MATERIAL DESCRIPTION REMARKS NO. TYPE SM SP LIGHT BROWN SLIGHTLY SILTY SAND slightly moist, dense, fine to coarse grained sand, some fine to coarse surface gravel. Decomposed granite. 1529.1 MC Very dense, 50/76 !W.S 9% fines 1528.1 SM LIGHT BROWN SILTY SAND slightly moist, very dense, fine to coarse grained sand and some fine to coarse gravel. Decomposed granite. 1527.1 В MC lw.s 3,20 50/76 13% fines 1526.1 4.57 MC 50/51 4.72 1525.1 1524.1 6.10 6.25 D MC 50/51 1523.1 Encountered more gravelly soils at 7 M. 50/51 MC 1522.1 1521.1 9.30 F MC 50/51 Refusal at 10 M: No free water encountered. © 1999, by Kleinfelder, Inc. PLATE **LOG OF BORING** KLEINFELDER 4875 LONGLEY LANE, SUITE 100

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PROJECT NO. 30-1348-15.002

RENO, NEVADA 89502 Tel. (702) 689-7800

CARSON FREEWAY

EXPLORATION LOG START DATE: __5/12/97 SHEET 1 OF 2 5/12/97 END DATE: STA 521+00 **STATION** JOB DESCRIPTION CARSON FREEWAY 15 M NORTH OFFSET NORTH OF U.S. 395 LOCATION **ENGINEER** _J. FORGA EQUIPMENT _ CME 55 **BORING** KLEINFELDER _SPECTRUM **OPERATOR** 30-1348-15.002 **GROUNDWATER LEVEL** E.A. # **DRILLING**METHOD DATE | DEPTH | ELEV. GROUND ELEV 1527.66 m HOLLOW STEM AUGER HAMMER DROP SYSTEM AUTOMATIC BACKFILLED YES __ DATE __5/12/97 SAMPLE | BLOWS/ Recovery | 300mm (%) ELEV. DEPTH USCS LAB TESTS MATERIAL DESCRIPTION REMARKS NO. TYPE (m) SM LIGHT BROWN SILTY SAND slightly moist, loose to medium dense, fine to coarse grained sand, SC estimated 10 to 20% fines. DARK BROWN CLAYEY SAND moist, medium dense, fine to medium grained sand, low to 1526.7 moderate plasticity fines, estimated 20 to 30% MC 29 W,UW,S 13% fines Light brown, some fine to coarse gravel. 1.98 1525.7 SC 3 3.05 1524.7 YELLOW_CLAYEY_SAND moist, very stiff, fine В MC 13 W,UW,S grained sand, high plasticity fines. Pt=33 48% fines 3.66 С MC 13 1523.7 SM RED BROWN SILTY SAND slightly moist, very dense, fine to coarse grained sand, estimated 10 to 20% fines (decomposed granite). D MC 50/127 WU,W 1522.7 1521.7 6.10 E MC 50/76 W.UW.S 28% fines 1520.7 FÍ MC 50/25 1519.7 я 1518.7 9 Some fine gravel, granite becoming stronger. 9.30 G MC 50/76 @1999, by Kleinfelder, inc. PLATE **LOG OF BORING** KLEINFELDER

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PROJECT NO. 30-1348-15.002

4875 LONGLEY LANE, SUITE 100

RENO, NEVADA 89502 Tel. (702) 689-7800 **CARSON FREEWAY**

EXPLORATION LOG

SHEET 2 OF 2

5/12/97 END DATE:

JOB DESCRIPTION __CARSON FREEWAY

STA 521+00 STATION 15 M NORTH **OFFSET**

KLEINFELDER

NORTH OF U.S. 395

ENGINEER

_ J. FORGA

BORING

BL-40

EQUIPMENT _ CME 55 OPERATOR SPECTRUM

E.A. #

30-1348-15.002

GROUND ELEV 1527.66 m

LOCATION

GROUNDWATER LEVEL DATE DEPTH ELEV.

DRILLING METHOD

HOLLOW STEM AUGER

HAMMER DROP SYSTEM AUTOMATIC

BACKFILLED YES DATE 5/12/97

E) EV	DERTH		S	AMPLE	5		liecs	142770141 000000000000000000000000000000000	
ELEV. LSfm)7	DEPTH (m)	NO.	TYPE	300mm	Hecovery	LAB TESTS	Group	MATERIAL DESCRIPTION	REMARKS
	- 10.67 - 10.82	_H_	MC	50/51					
516.7	_ 11					,			
	-								
515.7									
313.7	_ 12 12.19 12.34		MC	_50/51_				12.34	
	1534		WL	30/23.1				2.365	
514.7	- _ 13								
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508.7	_ 19								
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PROJECT NO. 30-1348-15.002

LOG OF BORING **CARSON FREEWAY**

CARSON CITY, NEVADA

PLATE

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EXPLORATION LOG START DATE: __5/12/97 SHEET 2 OF 2 5/12/97 END DATE: STA 521+00 **STATION** JOB DESCRIPTION CARSON FREEWAY 15 M NORTH OFFSET NORTH OF U.S. 395 ENGINEER J. FORGA LOCATION EQUIPMENT _CME 55 **BORING** KLEINFELDER OPERATOR _SPECTRUM 30-1348-15.002 GROUNDWATER LEVEL E.A. # DATE DEPTH ELEV. GROUND ELEV_ 1527.66 m HOLLOW STEM AUGER HAMMER DROP SYSTEM AUTOMATIC BACKFILLED YES DATE 5/12/97 MPLE BLOWS/ Recovery 300mm (%) ELEV. **DEPTH** LAB TESTS MATERIAL DESCRIPTION REMARKS NO. TYPE 15(63)7 10.82 H MC 50/51 1516.7 - 11 1515.7 12 12,19 MC 50/51 12.34 1514.7 _ 13 1513.7 _ 14 1512.7 15 1511.7 _ 16 1510.7 _ 17 1509.7 <u>|</u> 18 1508.7 _ 19

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PROJECT NO. 30-1348-15.002

LOG OF BORING CARSON FREEWAY

PLATE

CARSON CITY, NEVADA

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MOISTURE CONTENT AND UNIT WEIGHT LINE BORINGS

	DEPTH	MOISTURE CONTENT	DRY UNIT WEIGHT
BORING	(meters)	(%)	(kN/cu m)
BL-I	1.83	12.2	mang
BL-2	1.83	22.2	15.86
BL-3	1.83	13.0	16.33
BL-3	3,35	18.8	16.65
BL-4	1.83	13.0	16.33
BL-4	3.35	17.5	16.18
BL-4	4.88	20.7	15.71
BL-5	1.83	15.6	16.96
BL-5	3.35	25.5	15.08
BL-5	4.88	16.9	17.12
BL-5	6.40	17.8	17.43
BL-6	3.35	19.5	15.71
BL-6	4.88	19.9	16.18
BL-6	6.40	17.2	16.65
BL-7	1.83	13.2	16.65
BL-7	4.88	14.6	14.76
BL-7	6.40	19.0	14.14
BL-8	1.83	10.7	15.08
BL-8	3.35	19.0	13.51
BL-9	3.35	16.1	10.52
BL-10	1.83	16.9	14.61
BL-11	1.83	7.6	18.06
BL-11	3.35	10.5	15.55
BL-II	4.88	15.0	16.02
BL-12	1.83	15.3	13.19
BL-12	3.35	15.4	12.88
BL-12	4.88	19.1	13.66
BL-12	6.40	14.5	11.62
BL-13	1.83	12.2	15.86
BL-13	3.35	12.1	15.55
BL-13	4.88	3.3	15.55
BL-13	6.40	7.5	18.53
BL-14	1.83	17.0	16.49
BL-14	3.35	17.8	15.71
BL-14	6.40	10.2	18.38
BL-14	7.92	12.5	19.00
BL-14	9.45	16.1	17.28
BL-14	10.97	20.5	16.02
BL-15	3.35	22.8	15.39
BL-15	5.49	11.0	18.38
BL-15	6.40	23.2	14.14
BL-15	7.92	19.4	16.65
BL-15	10.97	15.7	17.12
BL-16	3.35	7.0	17.75

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PROJECT NO. 30-1348-15.002

MOISTURE / DENSITY TABLE

CARSON FREEWAY

CARSON CITY, NEVADA

PLATE

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	DEPTH	MOISTURE CONTENT	DRY UNIT WEIGHT
BORING	(meters)	(%)	(kN/cu m)
BL-16	4,88	22.5	16.02
BL-16	6.40	15.5	17.28
BL-16	7.92	17.8	16.81
BL-16	10.05	20.0	16.18
BL-17	1.83	18.5	16.02
BL-17	3.35	2.0	15.86
BL-17	6.40	17.3	15.86
BL-17 BL-18	1.83	10.7	16.65
BL-18	3.35	18.5	15.39
BL-18	4.88	12.7	18.22
BL-18	6.40	14.7	18.38
BL-18	7.92	12.2	18.06
BL-19	1.83	13.8	18.06
BL-19	7.01	34.8	12.56
BL-19	7.92	8.3	17.90
BL-19	1.83	12.2	17.90
BL-20	4,88	22.2	14.92
BL-20	6.40	10.0	18.38
BL-21	3.35	12.0	18.38
BL-21	4.88	11.9	18.06
BL-21	9.45	21.4	15.55
BL-22	3.35	14.6	17.58
BL-22	6.40	9.2	18.53
BL-23	4.88	13.0	16.64
BL-23	7.92	31.1	13.35
BL-24	3.35	19.6	16.02
BL-24	6.40	14.9	15.55
BL-25	4.88	14.2	17.59
BL-26	6.40	10.1	17.90
BL-26	9.45	15.8	17.12
BL-27	3.35	12.4	17.75
BL-27	4.88	9.8	18.69
BL-27	7.92	15.3	16.18
BL-28	2.44	8.7	16.81
BL-28	3.35	6.4	16.49
BL-28	4.88	6.4	16.96
BL-28	6.40	9.9	17.59
BL-28	7.92	9.4	18.38
BL-29	3.35	10.3	15.86
BL-29	4.88	10.3	17.43
BL-29	6.40	7.8	17.12
BL-29	7.92	15.3	16.96
BL-29	9.45	22.1	15.86
BL-30	3.35	16.5	16.18

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MOISTURE / DENSITY TABLE

CARSON FREEWAY

CARSON CITY, NEVADA

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	DEPTH	MOISTURE CONTENT	DRY UNIT WEIGHT
BORING	(meters)	(%)	(kN/cu m)
BL-30	4.88	6.9	16.65
BL-30	6.40	6.3	16.65
BL-30	7.92	8.5	16.65
BL-30	9.45	5.9	18.53
BL-31	3.35	3.6	14.45
BL-31	4.88	5.8	14.76
BL-31	6.40	7.3	14.92
BL-31	7.92	9.0	15.08
BL-31	9.45	7.9	17.28
BL-32	2.74	9.4	15.55
BL-32	3.35	10.2	15.39
BL-32	4.88	6.9	15.39
BL-32	7.92	7.8	16.18
BL-33	1.83	9.3	15.71
BL-33	3.35	10.9	15.86
BL-33	4.88	5.8	14.76
BL-33	6.40	5.9	18.06
BL-33	9.45	8.3	18.06
BL-34	1.83	15.2	14.45
BL-34	4.88	16.9	15.39
BL-35	1.83	19.3	13.19
BL-35	3.35	22.4	12.41
BL-35	4.88	21.0	13.35
BL-36	1.83	13.3	15.08
BL-36	3.35	19.0	13.66
BL-36	6.40	10.3	15.39
BL-37	1.83	11.8	15.71
BL-37	3.35	11.2	17.37
BL-37	6.40	14.1	14.76
BL-38	1.52	3.0	14.76
BL-38	3.05	8.0	****
BL-39	1.52	0.5	****
BL-39	3.05	0.8	
BL-40	1.83	8.7	16.81
BL-40	3.35	23.2	
BL-40	4.72	8.9	***
BL-40	6.10	5.6	***

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MOISTURE / DENSITY TABLE

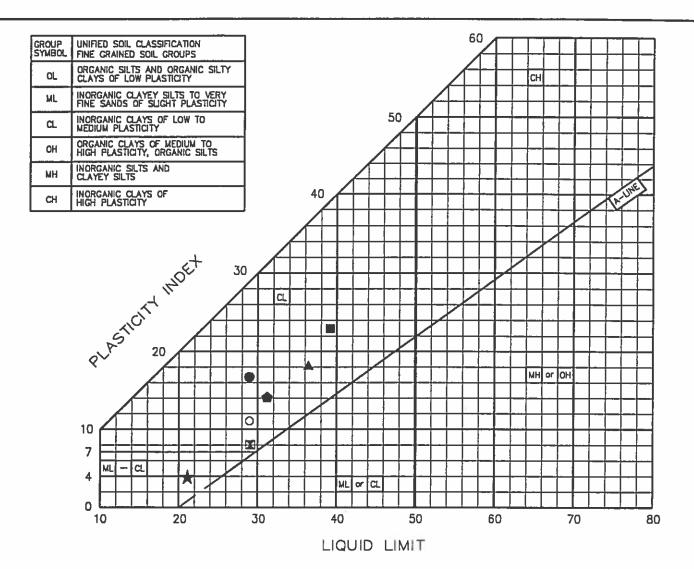
CARSON FREEWAY

CARSON CITY, NEVADA

PLATE

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TEST SYMBOL	SAMPLE NO.	SAMPLE (DEPTH)	LIQUID LIMIT	PLASTICITY INDEX	CLASSIFICATION
*	BL-4	1.83	21	4	Light Brown Clayey Sand (SC) -200=14%
	BL-10	1.83	29	8	Brown Clayey Sand (SC) -200=41%
0	BL-11	4.88	29	11	Brown Clayey Sand (SC) -200=38%
•	BL-14	9.45	31	14	Brown Clayey Sand (SC) -200=29%
•	BL-15	1.68	29	17	Yellow Brown Very Sandy Clay (CL) -200=51%
A	BL-15	3.35	36	18	Yellow Brown Very Clayey Sand (SC) -200=49%
	BL-16	4.88	39	23	Red Brown Clayey Sand (SC) -200=44%

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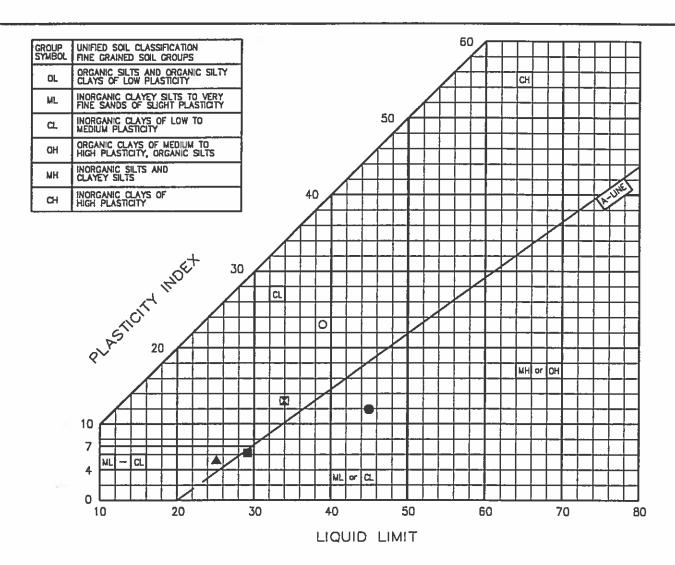
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PLASTICITY CHART

CARSON FREEWAY
CARSON CITY, NEVADA

PLATE

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TEST SYMBOL	SAMPLE NO.	SAMPLE (DEPTH)	LIQUID LIMIT	PLASTICITY INDEX	CLASSIFICATION
I	BL17	1.83	34	13	Light Brown Clayey Sand (SC) -200=28%
0	BL-17	4.72	39	23	Light Brown Clayey Sand (SC) -200=22%
*	BL19	2.29		NP	Light Brown Clayey Sand (SC) -200 =22%
•	BL-19	6.25		NP	Light Brown Clayey (SC) -200=29%
•	BL-20	8.38	46	12	Light Brown Very Sandy Clay (CL) −200=54%
A	BL-22	3.35	25	5	Brown Clayey Sand (SC) -200=18%
	BL-23	1,83	29	6	Light Brown Clayey Sand (SC) —200= 11%

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PLASTICITY CHART

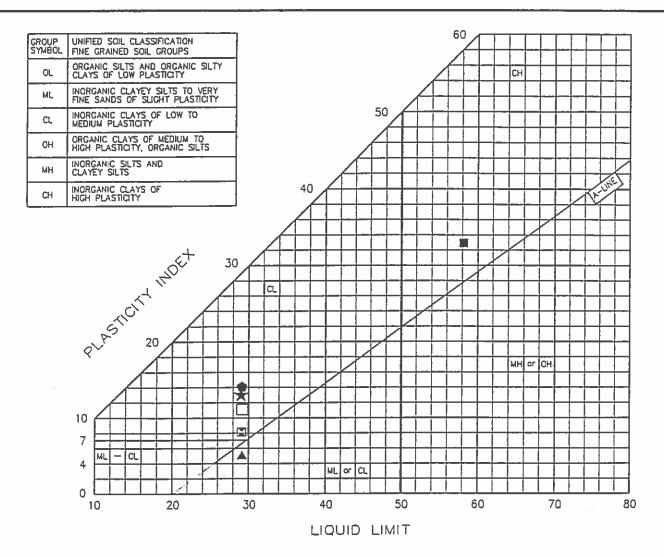
CARSON FREEWAY
CARSON CITY, NEVADA

PLATE

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TEST SYMBOL	SAMPLE NO.	SAMPLE (DEPTH)	LIQUID	PLASTICITY INDEX	CLASSIFICATION
*	BL-32	2.74	29	13	Red Brown Clayey Sand (SC) -200=15%
•	BL-33	1.83	29	14	Brown Clayey Sand (SC) -200=14%
•	BL-34	1.83		NP	Yellow Brown Silty Sand (SM) -200=16%
A	BL-35	4.88	29	5	Brown Clayey Sand (SC) -200=29%
=	BL-40	3.35	58	33	Red Yellow Clayey Sand (SC) —200=48%
M	BL-10	1.83	29	8	Brown Clayey Sand (SC) -200=41%
	BL-11	4.88	29	11	Brown Clayey Sand (SC) -200=38%

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PLASTICITY CHART CARSON FREEWAY

CARSON CITY, NEVADA

PLATE

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Г	Boring	Dep	th (M)	Desc	ription - AS	TM Classific	MC%	LL PL	PI	Cc Cu	
•	BL-02	at	0.6	Li	ght Brown S	ilty Sand (S					
1	BL-03	at	3.4	Yel	low Brown S	Silty Sand (S				1.07 7.0	
A	BL-04	at	4.9		Gray Claye	y Sand (SC)			<u> </u>		
							<u>.</u>				
	Boring	Dep	th (M)	D100	D60	D30	D10	%Gravel	%Sand	%Silt	%Clay
	BL-02	at	0.6	9.50_	0.22			1.0	67.6] :	31,4
2	BL-03	at	3.4	9.50	0.75	0.295	0.1078	0.3	94.1	5.6	
	2 2 00										
A		at	4.9	9.50	0.19			0.1	67.3		32.6
A	BL-04			9.50	0.19			0.1	67.3		32.6

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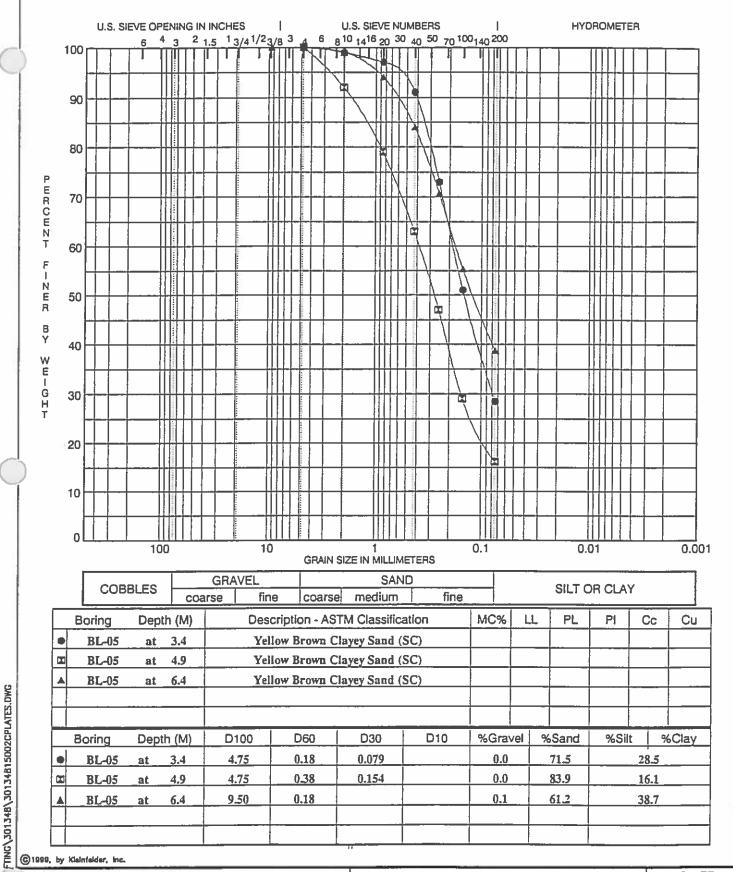
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GRAIN SIZE ANALYSES CARSON FREEWAY

PLATE

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PROJECT NO. 30-1348-15.002



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PROJECT NO. 30-1348-15.002

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CARSON FREEWAY

GRAIN SIZE ANALYSES

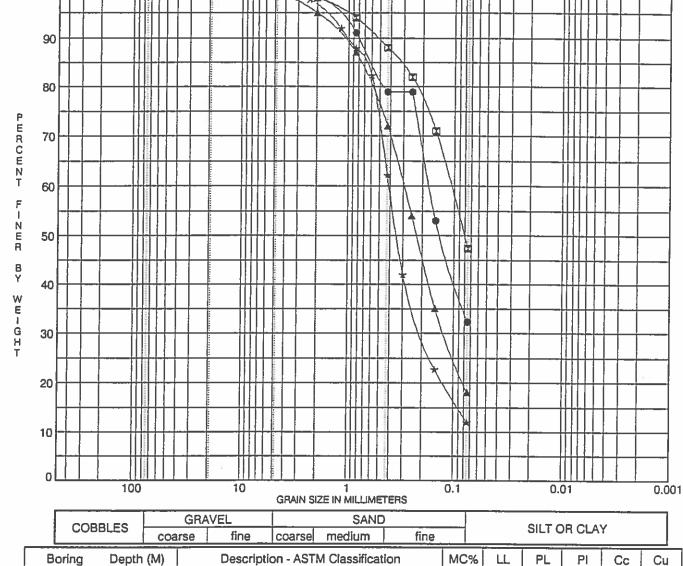
CARSON CITY, NEVADA

PLATE

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3



U.S. SIEVE NUMBERS

6 8 10 14 16 20 30 40 50 70 100 140 200

	Boring	Dep	th (M)	Desc	cription - AS	STM Classific	ation	MC%	LL PL	PI	Сс	Cu
	BL-06	at	3.4	G	ray Brown S	Silty Sand (Si	M)					
Œ	BL-06	at	4.9	Lig	ht Brown C	layey Sand (SC)					
	BL-07	at	1.8	Lig	ht Brown C	layey Sand (S	SC)					
*	BL-07	at	4.9	Lig	ht Brown C	layey Sand (S	SC)				1.33	5.8
	Boring	Dep	th (M)	D100	D60	D30	D10	%Gravel	%Sand	%Sil	t %	Clay
	BL-06	at	3.4	4.75	0.17			0.0	67.7		32.3	
×	BL-06	at	4.9	9.50	0.11			0.2	52.4		47.4	
	BL-07	at	1.8	9.50	9.50 0.30 0.122			1.0	81.0		18.0	
*	BL-07	at	4.9	9.50	0.41	0.2	87.8		12.0			
Т												

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U.S. SIEVE OPENING IN INCHES

100

4 3 2 1.5 1 3/4 1/2 3/8 3

4875 LONGLEY LANE, SUITE 100 RENO, NEVADA 89502 Tel. (702) 689-7800 CARSON FREEWAY

GRAIN SIZE ANALYSES

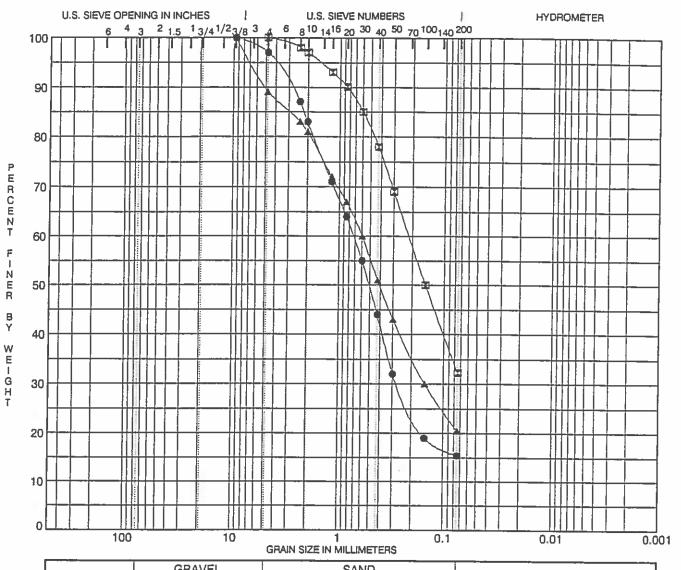
HYDROMETER

C-50

PLATE

PROJECT NO. 30-1348-15.002

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	RRIFC		GRA	VEL		SAND				SITORCIAV					
	3000	coa	arse	fine	coarse	medium	fine			SILI	OR CLA	1			
Boring	Dept	h (M)		Descripti	on - AST	M Classifica	tion	МС	% L	L PL	PI	Cc	Си		
BL-08	at	1.8		Yellow	Brown Si	Ity Sand (SN	1)								
BL-09	at	3.4		Bro	wn Silty	Sand (SM)									
BL-11	at	1.8		Light F	Brown Sil	lty Sand (SM	0						·		
										,0					
Boring	Deptl	1 (M)_	D1	00 [060	D30	D10	%G	ravel	%Sand	%Sil	t %	Clay		
BL-08	at	1.8	9.	50 0).73	0.270		3.0		81.6		15.4	15.4		
BL-09	at	3.4	4.	75 0	.22				0.0 67.7			32.3			
BL-11	at	1.8	9.	50 0	.60	0.150		1	1.0	68.5					
	Boring BL-09 BL-11 Boring BL-08 BL-09	BL-08 at BL-09 at BL-11 at Boring Depth BL-08 at BL-09 at	Boring Depth (M) BL-08 at 1.8 BL-09 at 3.4 BL-11 at 1.8 Boring Depth (M) BL-08 at 1.8 BL-09 at 3.4	COBBLES coarse	Coarse fine	COBBLES Coarse fine Coarse	COBBLES Coarse fine Coarse medium	COBBLES Coarse fine Coarse medium fine	COBBLES Coarse fine Coarse medium fine	COBBLES Coarse fine Coarse medium fine	COBBLES	COBBLES Coarse fine Coarse medium fine SILT OR CLA	COBBLES		

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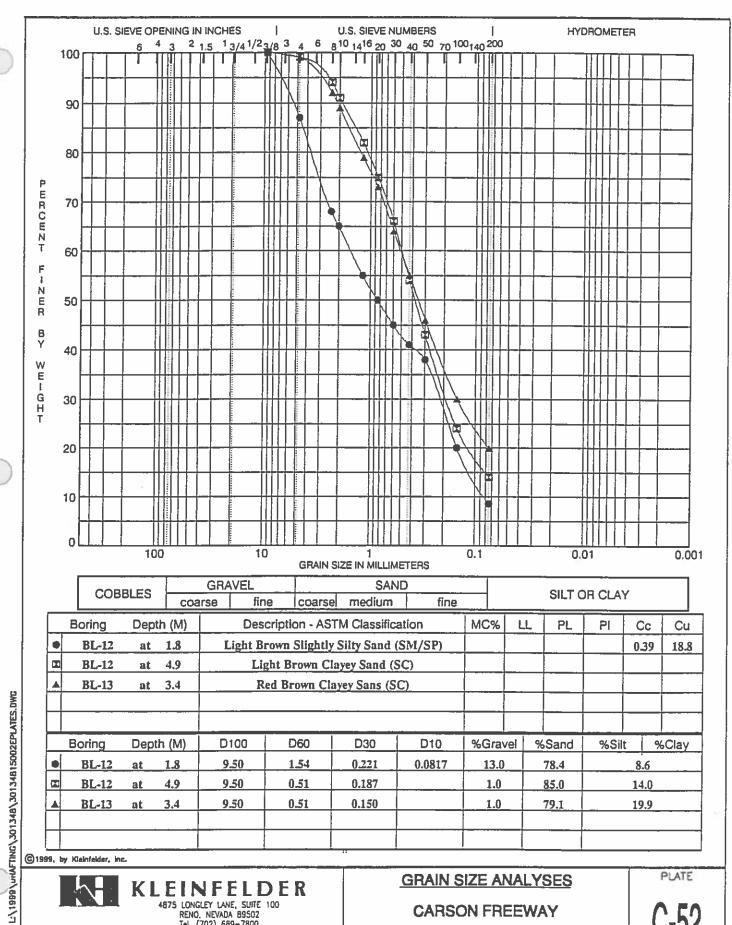
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GRAIN SIZE ANALYSES CARSON FREEWAY

6-5]

PLATE

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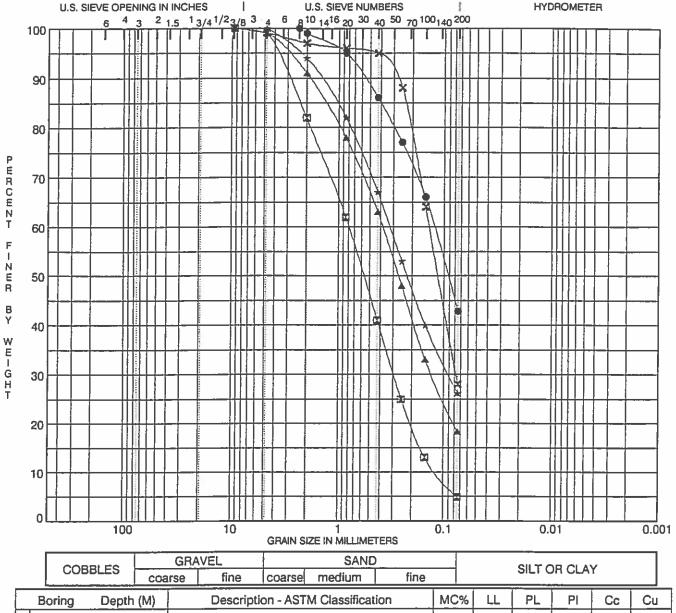
GRAIN SIZE ANALYSES

CARSON FREEWAY

CARSON CITY, NEVADA

PLATE

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			coa	ırse fin	e fine coarse medium fine								
	Boring	Depi	th (M)	Des	Description - ASTM Classification				LL I	PL	PI	Cc	Cu
	BL-14	at	1.8	Li	ght Brown C	C)							
	BL-14	at	3.4	Light B	rown Slightl	SM/SP)					0.95	6.9	
A	BL-14	at	6.4		Brown Clayey Sand (SC)								
*	BL-14	at	7.9		Brown Clay								
×	BL-15	at	6.4		Brown Clayey Sand (SC)								
	Boring	Dept	th (M)	D100	D60	D30	D10	%Grave	l %Sa	nd	%Sili	t %	Clay
	BL-14	at	1.8	2.36	0.13			0.0	57.3	2		42.8	
(2)	BL-14	at	3.4	9.50	0.80	0.295	0.1157	1.0	94.	<u> </u>		5.0	
	BL-14	at	6.4	9.50	0.38	0.130		1.0	80.	7		18.3	
*	BL-14	at	7.9	4.75	0.33	0.091		0.0	73.	3		26.2	
×	BL-15	at	6.4	9,50	0.14	0.078		1.0	71.0	<u>, </u>		28.0	

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GRAIN SIZE ANALYSES

CARSON FREEWAY

C.53

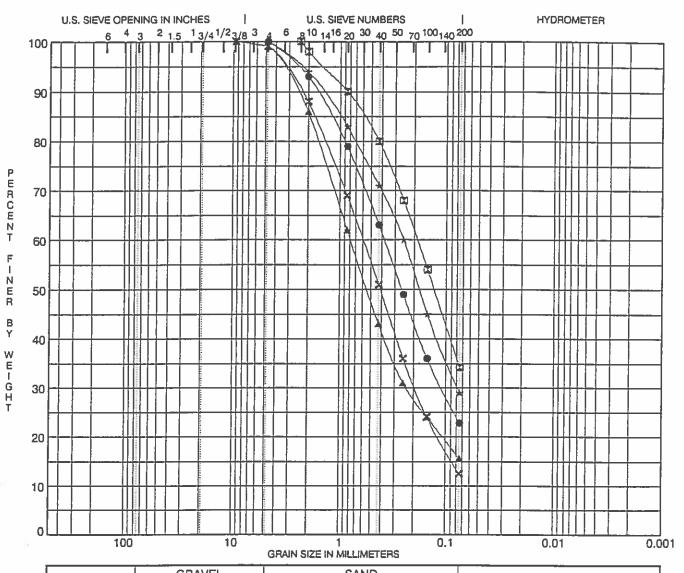
PLATE

PROJECT NO. 30-1348-15.002

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	COB	BLES		GRAV	/EL		SAND)		SILT OR CLAY		,		
	LCOB	DLES	coa	rse	fine	coarse	medium	fine			SILI C	IN CLAT		
	Boring	Depth	ı (M)		Description	on - AST	M Classificat	tion	мс%	LĻ	PL	PI	Сс	Cu
	BL-16	at	6.4		White Silty Sand (SM)							j		
X	BL-18	at	3.4		Gray Brown Clayey Sand (SC)									
A	BL-18	at	4.9		Light Brown Clayey Sand (SC)									
*	BL-18	at	6.4		Light Brown Clayey Sand (SC)									
×	BL-19	at	1.8		Light Brown Clayey Sand (SC)			C)						
	Boring	Depth	(M)	D10	00 E	060	D30	D10	%Grav	el	%Sand	%Silt	9	%Clay
•	BL-16	at	6.4	4.7	5 0	38	0.110		0.0		77.2		22.8	
X	BL-18	at	3.4	2.3	6 0	.19			0.0		65.8		34.2	
A	BL-18	at	4.9	9.5	0 0	.79	0.232		1.0	\perp	83.4		15.6	
*	BL-18	at	6.4	4.7	5 0	25	0.078		0.0		71.0		29.0	
X	BL-19	at	1.8	9.5	0 0	.60	0.194		1.0		86.5		12.5	

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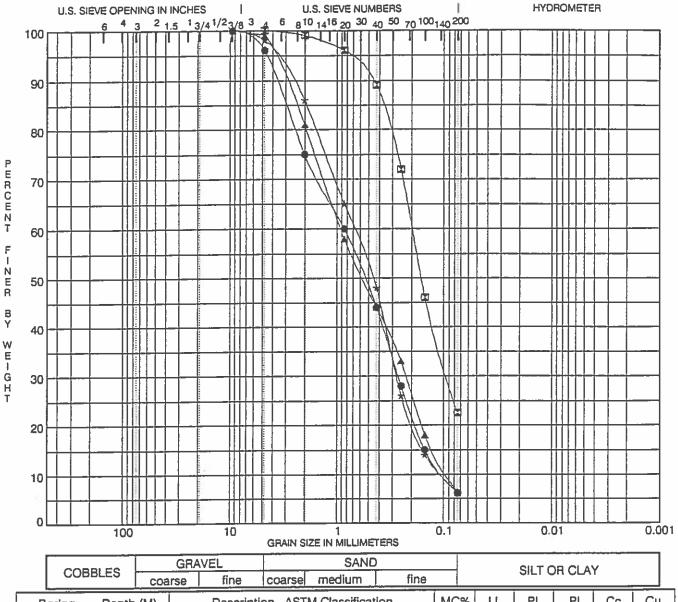
CARSON FREEWAY

PLATE

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PROJECT NO. 30-1348-15.002

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	СОВ	BLES	coa	rse	fine	coarse	medium	fine			SILIC	OH CLA	T	
	Boring	Depti	л (M)		Description - ASTM Classification				MC%	LL	PL	PI	Сс	Сц
•	BL-20	at	1.8	L	Light Brown Slightly Silty Sand (SM/SP)								0.83_	8.4
M	BL-20	at	4.9		Light Brown Clayey Sand (SC)									
	BL-21	at	3.4		Brown Slightly Slity Sand (SM/SP)							0.60	9.9	
*	BL-21	at	4.9		Brown S	lightly Sili	ty Sand (SM	(/SP)					1.05	6.7
	Boring	Depti	ո (M)	D.	100	D60	D30	D10	%Grav	/el 9	6Sand	%Sil	t 9	Clay
•	BL-20	at _	1.8	9.	50	0.85	0.267	0.1012	4.0		89.8		6.2	
Œ	BL-20	at	4.9	4.	75	0.20	0,094	 	0.0		77.5		22.5	
lack	BL-21	at	3.4	9.	50	0,92	0.226	0.0926	1.0		92.5		6.5	
*	BL-21_	at	4.9	9.	50	0.69	0.275	0.1041	2.0		91.6		6.4	

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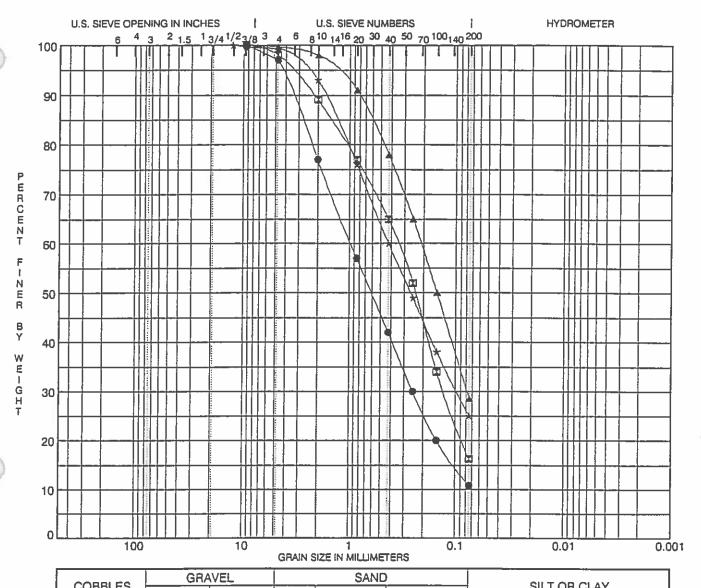
CARSON CITY, NEVADA

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PLATE

C-55

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	COE	BLES	coa	rse fin	e fine coarse medium fine SILTOR C				JH CLA	Υ 			
	Boring	Depti	n (M)	Des	Description - ASTM Classification				LL	. PL	PI	Сс	Cu
	BL-22	at	6.4	Gray Bı	Gray Brown Slightly Silty Sand (SM/SP)							0.92	13.8
	BL-23	at	4.9	Li	Light Brown Clayey Sand (SC)								
	BL-24	at	6.4	Lig	Light Brown Clayey Sand (SC)								
*	BL-25	at	4.9										
	Boring	Depti	1 (M)	D100	D60	D30	D10	%Grav	/el	%Sand	%Sil	t %	Clay
•	BL-22	at	6.4	9.50	0.97	0.250		3.0		86.2		10.8	
X	BL-23	at	4.9	9.50	0.35	0.128		2.0		81.7		16.3	
	BL-24	at	6.4	9.50	0.21	0.079		0.4		71.0		28.6	
*	BL-25	at	4.9	12.50	0.43	0.098		0.7		743		25.0	

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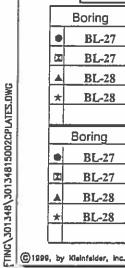
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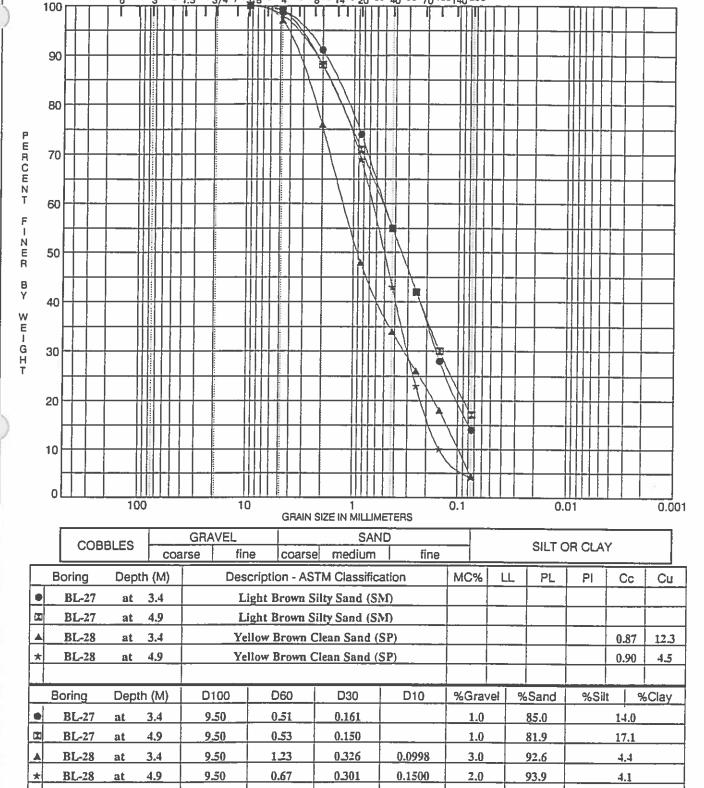
C-56

PLATE

PROJECT NO. 30-1348-15.002



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U.S. SIEVE NUMBERS

810 1416 20 30 40 50 70 100 140 200

HYDROMETER

PROJECT NO. 30-1348-15.002

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U.S. SIEVE OPENING IN INCHES

1 3/4 1/2 3/8 3

6

GRAIN SIZE ANALYSES CARSON FREEWAY

CARSON CITY, NEVADA

PLATE

U.S. SIEVE OPENING IN INCHES

U.S. SIEVE NUMBERS

HYDROMETER

	Boring	Dep	th (M)	Des	cription - AS	TM Classific	ation	MC%	LL	PL	Pl	Сс	
•	BL-28	at	6.4	Red Br	own Slightly	Silty Sand (SM/SP)					0.74	Ī
×	BL-28	at	7.9	Red Br	own Slightly	Silty Sand (SM/SP)					0.54	Ī
A	BL-29	at	2.6		Brown Silt	Sand (SM)							
*	BL-29	at	3.4		Brown Silty	Sand (SM)							
×	BL-29	at	4.9	Red Br	own Slightly	Silty Sand (SM/SP)					1.20	
	Boring	Dep	th (M)	D100	D60	D30	D10	%Grav	/el	%Sand	%Sil	t %	6
•	BL-28	at	6.4	19.00	0.72	0.194		3.0		85.3		11.7	_
I	BL-28	at	7.9	19.00	1.28	0.230	0.0770	11.0		79,4		9.6	_
A	BL-29	at	2.6	9.50	0.68	0.172		6.0		74.4		19.6	_
*	BL-29_	at	3.4	9.50	0.43	0.105		1.0		78.6		20.4	_
×	BL-29	at	4.9	12.50	0.61	0.258	0.0903	4.0	<u>i_</u>	88.2		7.8	_
	y Klainfaldar, ir					1							



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GRAIN SIZE ANALYSES CARSON FREEWAY

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CARSON FREEWAY

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PLATE

PROJECT NO. 30-1348-15.002

CARSON CITY, NEVADA

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TEST TYPE:	CD/WET/STAGED
BORING NO:	BL-7
DEPTH:	3.35 Meters
SOIL DESCRIPTION:	Brown Silty Sand w/some Clay
RATE OF SHEAR:	0.0019 cm/sec

100	FRICTION ANGLE:	34 deg.
	COHESION:	0 kPa

(<u> </u>					DIDECT CL	
色	@1999	, by Kleinfelder, Inc.					_
[C]		MAXIMUM STRESS .	- kPa	46	115	241	
013		NORMAL STRESS - k	Pa	95	190	380	
48\2		FINAL WATER CONT	TENT - %	20.8			
5013		INITIAL WATER CON	VTENT - %	17.2			
4815		DRY DENSITY - kN/c	u m	14.92			
NG\301348\30134815002CPL		RATE OF SHEAR:	0.0013	/ CHI/SEC			
		RATE OF SHEAR:		cm/sec		97	
ATES.DWG		SOIL DESCRIPTION:	ind w/some Clay	COHESION:			
DWC		DEPTH:	3.35	Meters		FRICTION A	NG
		BORING NO:	В	L-7			



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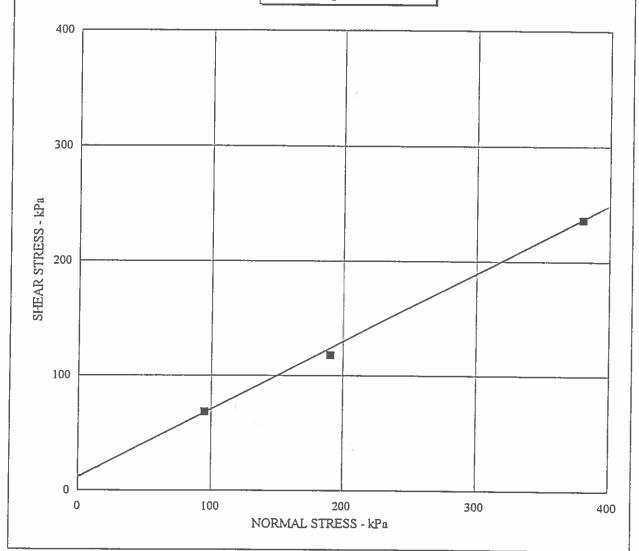
PROJECT NO. 30-1348-15.002

DIRECT SHEAR CARSON FREEWAY

CARSON CITY, NEVADA

PLATE





TEST TYPE:	CD/WET/STAGED
BORING NO:	BL-14
DEPTH:	4.88 meters
SOIL DESCRIPTION:	Brown Clayey Sand
RATE OF SHEAR:	0.0019 cm/sec

FRICTION ANGLE:	31
COHESION:	9 kPa

DRY DENSITY - kN/cu m	16.18		
INITIAL WATER CONTENT - %	18.1		
FINAL WATER CONTENT - %	18.6		
NORMAL STRESS - kPa	95	190	380
MAXIMUM STRESS - kPa	68	118	236

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DIRECT SHEAR CARSON FREEWAY

6-6

PLATE

PROJECT NO. 30-1348-15.002

200

NORMAL STRESS - kPa

TEST TYPE:	CD/WET/STAGED
BORING NO:	BL-19
DEPTH:	4.88 meters
SOIL DESCRIPTION:	Gr. Brown Silty Sand
RATE OF SHEAR:	0.0019 cm/sec

100

FRICTION ANGLE:	40
COHESION:	0 kPa

300

DRY DENSITY - kN/cu m	16.02		
INITIAL WATER CONTENT - %	18.9		
FINAL WATER CONTENT - %	19.7	7.6	
NORMAL STRESS - kPa	95	190	380
MAXIMUM STRESS - kPa	52	130	294

DEPTH:
SOIL DESCR
RATE OF SH

DRY DENSI:
INITIAL WATE
NORMAL ST
MAXIMUM ST
MAXIMUM ST
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DIRECT SHEAR

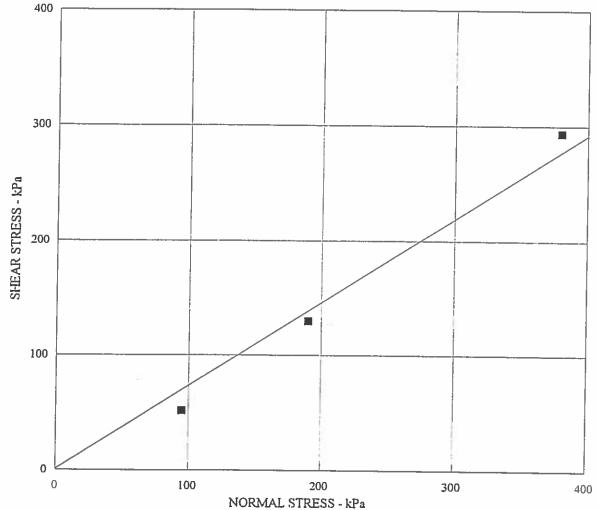
CARSON FREEWAY

CARSON CITY, NEVADA

PLATE

400

C-62



TEST TYPE:	CD/WET/STAGED
BORING NO:	BL-19
DEPTH:	4.88 meters
SOIL DESCRIPTION:	Gr. Brown Silty Sand
RATE OF SHEAR:	0.0019 cm/sec

FRICTION ANGLE:	40
COHESION:	0 kPa

16.02		
18.9	_	
19.7		
95	190	380
52	130	294
	18.9 19.7 95	18.9 19.7 95 190



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DIRECT SHEAR CARSON FREEWAY

CARSON CITY, NEVADA

PLATE

TEST TYPE:	CD/WET/STAGED
BORING NO:	BL-24
DEPTH:	4.88 meters
SOIL DESCRIPTION:	Lt. Brown Clayey Sand
RATE OF SHEAR:	0.0019 cm/sec

FRICTION ANGLE:	33
COHESION:	4 kPa

DRY DENSITY - kN/cu m	15.71]	
INITIAL WATER CONTENT - %	20.7		
FINAL WATER CONTENT - %	20.0		
NORMAL STRESS - kPa	95	380	
MAXIMUM STRESS - kPa	67	256	

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DIRECT SHEAR

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200

NORMAL STRESS - kPa

TEST TYPE:	CD/WET/STAGED
BORING NO:	BL-25
DEPTH:	1.83 meters
SOIL DESCRIPTION:	Yellow Brown Sl. Silty Sand
RATE OF SHEAR:	0.0019 cm/sec

FRICTION ANGLE:	26
COHESION:	23 kPa

300

DRY DENSITY - kN/cu m	16.33		
INITIAL WATER CONTENT - %	16.5		
FINAL WATER CONTENT - %	18.7		
NORMAL STRESS - kPa	95	190	380
MAXIMUM STRESS - kPa	65	126	211

100

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DIRECT SHEAR CARSON FREEWAY

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0 L

TEST TYPE:	CD/WET/STAGED
BORING NO:	BL-26
DEPTH:	3.35 meters
SOIL DESCRIPTION:	Red Brown Sl. Silty Sand
RATE OF SHEAR:	0.0019 cm/sec

FRICTION ANGLE:	ANGLE: 37	
COHESION:	6 kPa	

DRY DENSITY - kN/cu m	15.71		
INITIAL WATER CONTENT - %	15.7		
FINAL WATER CONTENT - %	19.1		
NORMAL STRESS - kPa	95	190	380
MAXIMUM STRESS - kPa	80	145	294

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<u>DIRECT SHEAR</u> CARSON FREEWAY

CARSON CITY, NEVADA

PLATE

C-66

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TEST TYPE:	CD/WET/STAGED
BORING NO:	BL-32
DEPTH:	6.40 meters
SOIL DESCRIPTION:	Red Brown Silty Sand
RATE OF SHEAR:	0.0019 cm/sec

FRICTION ANGLE:	8
COHESION:	64 kPa

DRY DENSITY - kN/cu m	13.04		1
INITIAL WATER CONTENT - %	11.3		
FINAL WATER CONTENT - %	20.4		
NORMAL STRESS - kPa	95	190	380
MAXIMUM STRESS - kPa	38	148	95

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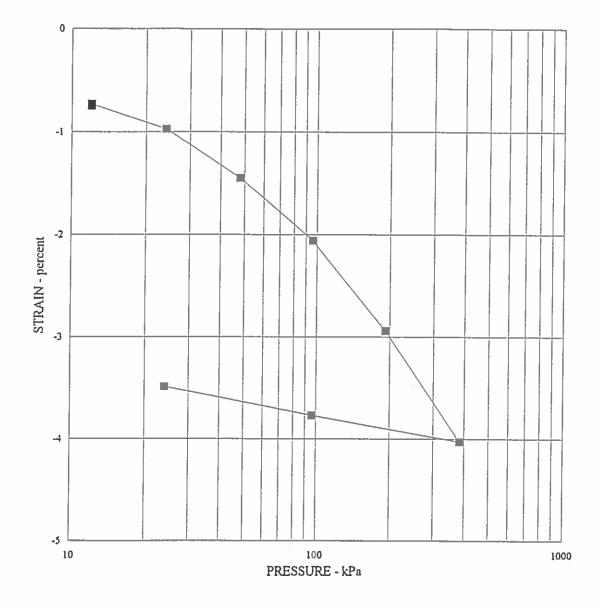
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DIRECT SHEAR CARSON FREEWAY

CARSON CITY, NEVADA

PLATE

400



BORING NO.: BL-6	DEPTH:	1.83 m
SAMPLE DESCRIPTION Light Brown Silty Sand		
OVERBURDEN PRESSURE, kPa		33
PRECONSOLIDATION P	RESSURE, kPa	58

	INITIAL	FINAL
DRY DENSITY - kN/cu m	16.43	17.03
WATER CONTENT - %	20.8	20.0
VOID RATIO	0.5983	0.5393
DEGREE OF SATURATION, %	93.00	99.00
SAMPLE HEIGHT - cm	2.5400	2.45

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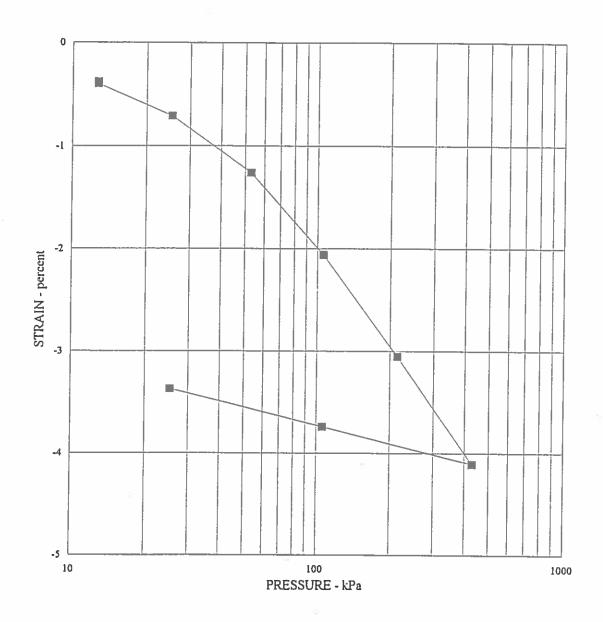
CONSOLIDATION TEST

CARSON FREEWAY

CARSON CITY, NEVADA

PLATE

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BORING NO.: BL-9	DEPTH:	1:83 m
SAMPLE DESCRIPTION	Brown Silty Sand	i
OVERBURDEN PRESSU	RE, kPa	33
PRECONSOLIDATION P	RESSURE, kPa	43

	INITIAL	FINAL
DRY DENSITY - kN/eu m	18.38	20.17
WATER CONTENT - %	8.0	15.3
VOID RATIO	0.4197	0.3810
DEGREE OF SATURATION, %	47.00	99.00
SAMPLE HEIGHT - cm	2.5400	2.46

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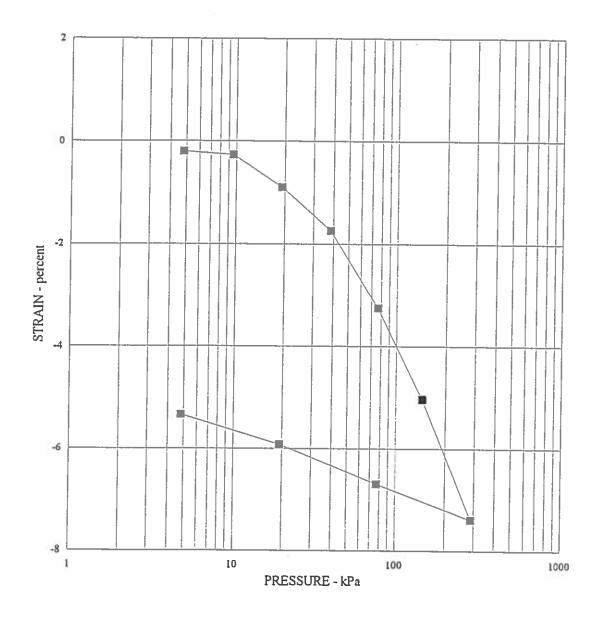
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PROJECT NO. 30-1348-15.002

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BORING NO.: BL-11	DEPTH:	6.40 m
SAMPLE DESCRIPTION	Lt Brown Sl. Sa	ndy Clay
OVERBURDEN PRESSU	RE, kPa	100
PRECONSOLIDATION P.	RESSURE, kPa	38

	INITIAL	FINAL
DRY DENSITY - kN/cu m	15.06	16.26
WATER CONTENT - %	24.2	27.7
VOID RATIO	1.0121	0.8660
DEGREE OF SATURATION, %	74.00	99.00
SAMPLE HEIGHT - cm	1.6300	1.49

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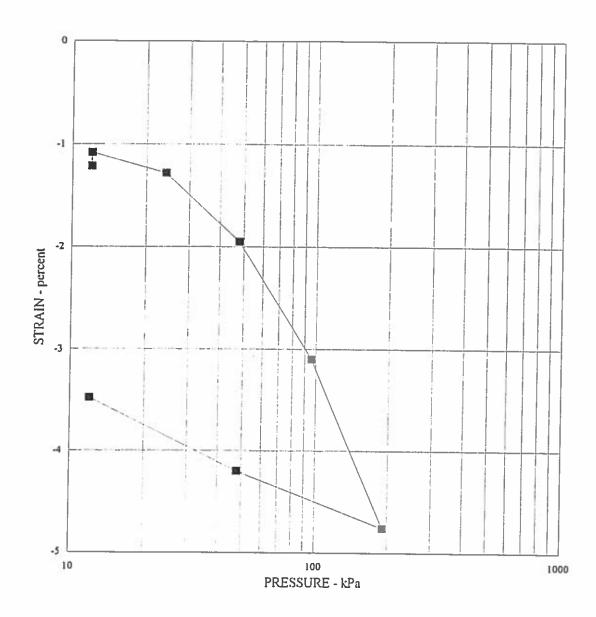
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CONSOLIDATION TEST

CARSON FREEWAY

CARSON CITY, NEVADA



BORING NO.: BL-15 DEPTH:	1.83 m
SAMPLE DESCRIPTION Yellow Brown	Sandy Clay
OVERBURDEN PRESSURE, kPa	28
PRECONSOLIDATION PRESSURE, kP	a 38

	INITIAL	FINAL
DRY DENSITY - kN/cu m	14.84	15.38
WATER CONTENT - %	24.7	26.7
VOID RATIO	0.7990	0.7343
DEGREE OF SATURATION, %	84.00	99.00
SAMPLE HEIGHT - cm	2.5400	2.45

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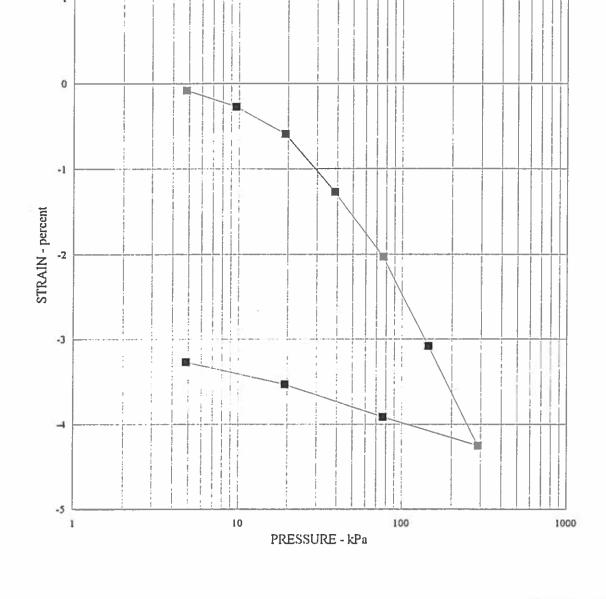
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CONSOLIDATION TEST

CARSON FREEWAY

CARSON CITY, NEVADA



BORING NO.: BL-16	DEPTH:	1.83 m
SAMPLE DESCRIPTION	Lt. Brown Claye	y Sand
	_	-
OVERBURDEN PRESSU	RE, kPa	33
PRECONSOLIDATION P	RESSURE, kPa	36

	INITIAL	FINAL
DRY DENSITY - kN/cu m	19.02	17.84
WATER CONTENT - %	5.6	21.5
VOID RATIO	1.0671	0.8691
DEGREE OF SATURATION, %	21.00	99.00
SAMPLE HEIGHT - cm	1.6300	1.54

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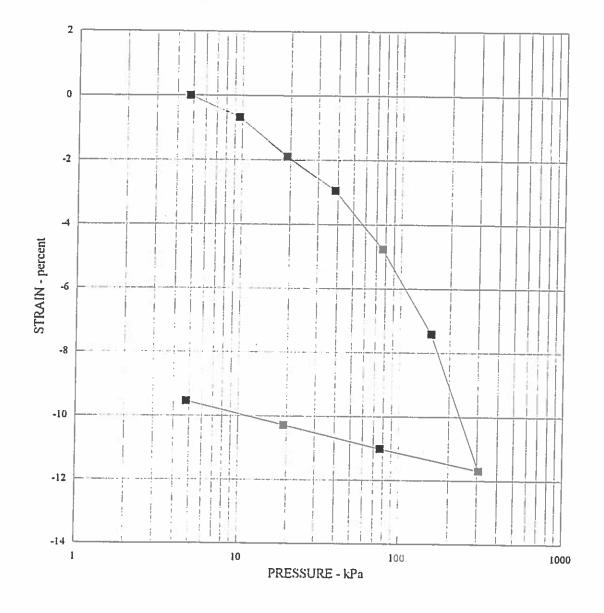
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PLATE

PROJECT NO. 30-1348-15.002

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TH: 4.88
own Clayey Sand
Pa 77
URE, kPa 50

	INITIAL	FINAL
DRY DENSITY - kN/cu m	13.63	15.44
WATER CONTENT - %	30.0	30.2
VOID RATIO	1.1816	0.9251
DEGREE OF SATURATION, %	77.00	99.00
SAMPLE HEIGHT - cm	2.5400	2.45

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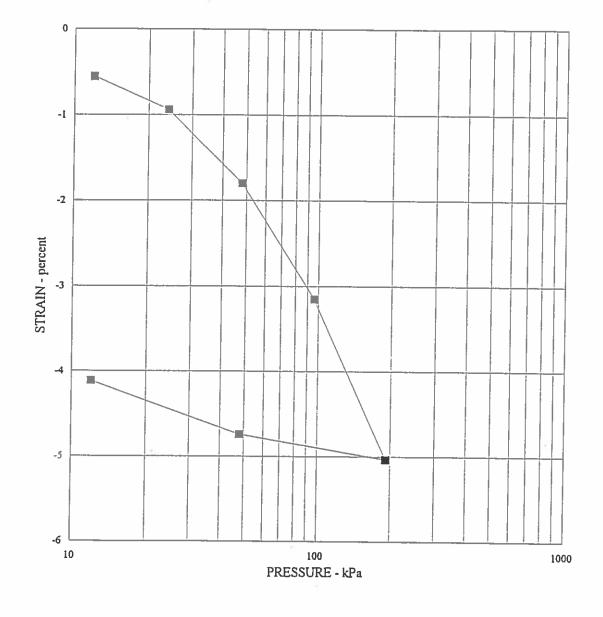
CARSON FREEWAY

CARSON CITY, NEVADA

PLATE

C-73

CAD FILE LEVIE



BORING NO.: BL-19 D	EPTH:	6.4
SAMPLE DESCRIPTION L	. Brown Cla	yey Sand
	•	
OVERBURDEN PRESSURE	E, kPa	116
PRECONSOLIDATION PRE	COULDE ID	a 41

	INITIAL	FINAL
DRY DENSITY - kN/cu m	12.56	13.11
WATER CONTENT - %	39.6	37.3
VOID RATIO	1.1024	1.0182
DEGREE OF SATURATION, %	97.00	99.00
SAMPLE HEIGHT - cm	2.5400	2.44

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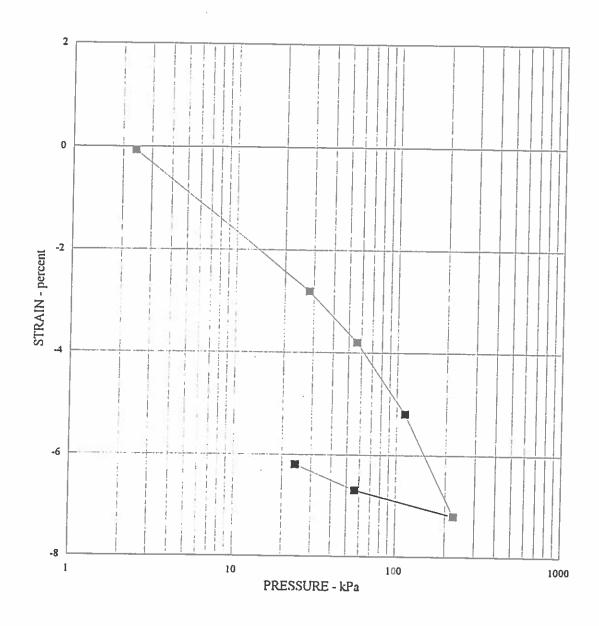
CARSON FREEWAY

CARSON CITY, NEVADA

PLATE

C-74

CAD FILE: LE\16



BORING NO.: BL-20	DEPTH:	3.96 m
SAMPLE DESCRIPTION	N Lt. Brown Claye	y Sand
	,	•
OVERDIMDENIARECO	LIDE LD	
OVERBURDEN PRESS	UKE, KPa I	59

	INITIAL	FINAL
DRY DENSITY - kN/cu m	9.27	NA
WATER CONTENT - %	20.0	NA
VOID RATIO	NA	NA
DEGREE OF SATURATION, %	NA	NA
SAMPLE HEIGHT - cm	2.5400	2.45

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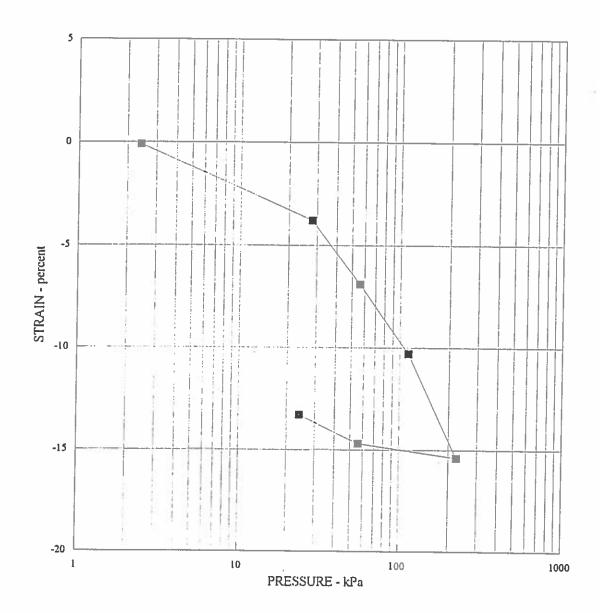
CONSOLIDATION TEST

CARSON FREEWAY

CARSON CITY, NEVADA

L/19

3E 85



BORING NO.: BL-20A DEPTH:	8.53 m
SAMPLE DESCRIPTION Lt. Brown	n Very Sandy Clay
OVERBURDEN PRESSURE, kPa	160
PRECONSOLIDATION PRESSUR	E kPa 67

	INITIAL	FINAL
DRY DENSITY - kN/cu m	6.44	NA
WATER CONTENT - %	41.0	NA
VOID RATIO	NA	NA
DEGREE OF SATURATION, %	NA	NA
SAMPLE HEIGHT - cm	2.5400	2.45

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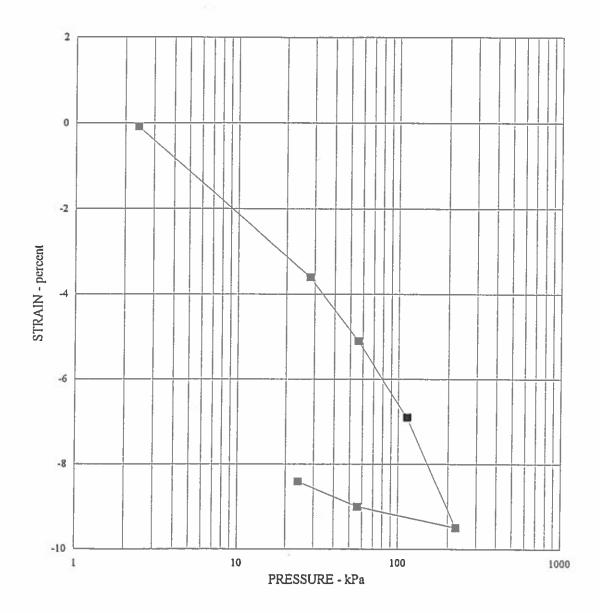
CARSON FREEWAY

CARSON CITY, NEVADA

PLATE

C-76

פיט צופי ה\ום



DEPTH:	6,40 m
Lt. Brown Sand	v Clav
9	,
IRE kPa	105
	DEPTH: Lt. Brown Sand RE, kPa RESSURE, kPa

	INITIAL	FINAL
DRY DENSITY - kN/cu m	8.95	NA
WATER CONTENT - %	20.0	NA
VOID RATIO	NA	NA
DEGREE OF SATURATION, %	NA	NA
SAMPLE HEIGHT - cm	2.5400	2.45

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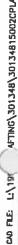
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CARSON FREEWAY

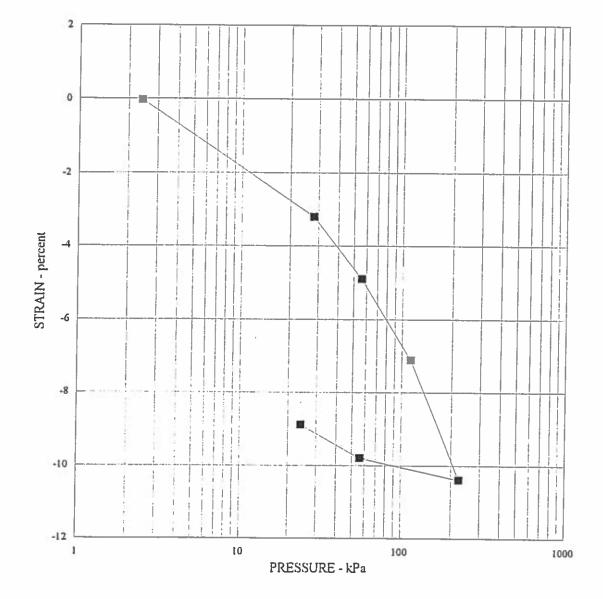
PLATE

C-77

PROJECT NO. 30-1348-15.002







BORING NO.: BL-21A DEPTH:	8,53 m
SAMPLE DESCRIPTION Lt. Brown Sand	y Clay
OVERBURDEN PRESSURE, kPa	145
PRECONSOLIDATION PRESSURE, kPa	72

	INITIAL	FINAL
DRY DENSITY - kN/cu m	6.13	NA
WATER CONTENT - %	41.0	NA
VOID RATIO	NA	NA
DEGREE OF SATURATION, %	NA	NA
SAMPLE HEIGHT - cm	2.5400	2.45

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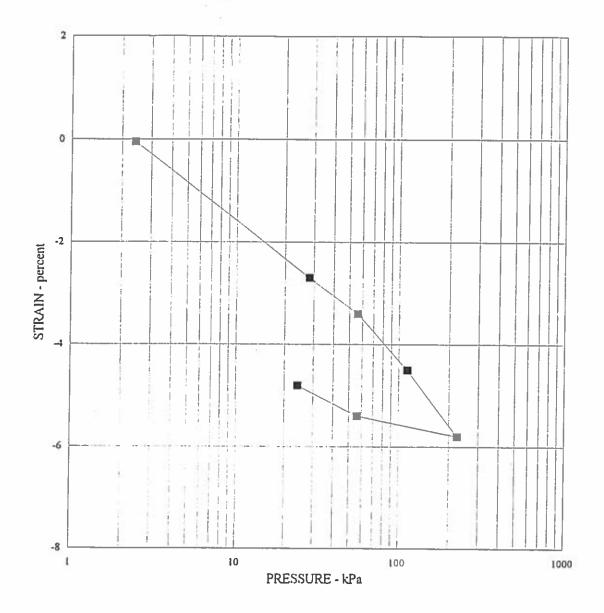
CONSOLIDATION TEST

CARSON FREEWAY

CARSON CITY, NEVADA

EVI.

CAD FILE



BORING NO.: BL-22	DEPTH:	1.83
SAMPLE DESCRIPTION	Lt. Brown Sandy	/ Silt
OVERBURDEN PRESSU	RE, kPa	29
PRECONSOLIDATION P		24

	_ INITIAL	FINAL
DRY DENSITY - kN/cu m	9.89	NA
WATER CONTENT - %	7.0	NA
VOID RATIO	NA	NA
DEGREE OF SATURATION, %	NA	NA
SAMPLE HEIGHT - cm	2.5400	2.45

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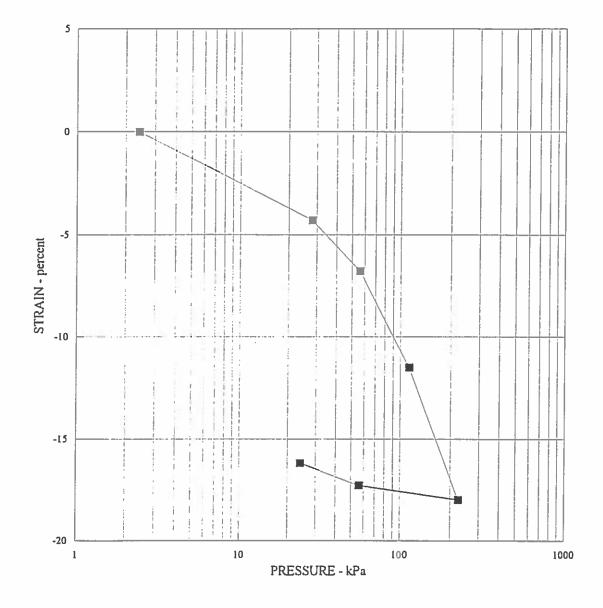
PROJECT NO. 30-1348-15.002

CONSOLIDATION TEST

CARSON FREEWAY

CARSON CITY, NEVADA

PE 다시9



BORING NO.: BL-23	DEPTH:	8,84 m
SAMPLE DESCRIPTION	Lt. Brown Claye	y Sand
OVERBURDEN PRESSU	RE, kPa	132
PRECONSOLIDATION F	RESSURE, kPa	72

	INITIAL	FINAL
DRY DENSITY - kN/cu m	5.97	NA
WATER CONTENT - %	48.0	NA
VOID RATIO	NA	NA
DEGREE OF SATURATION, %	NA	NA
SAMPLE HEIGHT - cm	2.5400	2.45

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CONSOLIDATION TEST

CARSON FREEWAY

CARSON CITY, NEVADA

PLATE

C-80

BORING NO.: BL-27	DEPTH:	1.83 m
SAMPLE DESCRIPTION	Lt. Brown Sl. S	ilty Sand
OVERBURDEN PRESSU	JRE, kPa	32
PRECONSOLIDATION I	RESSURE, kPa	29

	INITIAL	FINAL
DRY DENSITY - kN/cu m	15.5	17.12
WATER CONTENT - %	4.6	19.2
VOID RATIO	0.6717	0.5137
DEGREE OF SATURATION, %	18.00	99.00
SAMPLE HEIGHT - cm	2.5400	2.3

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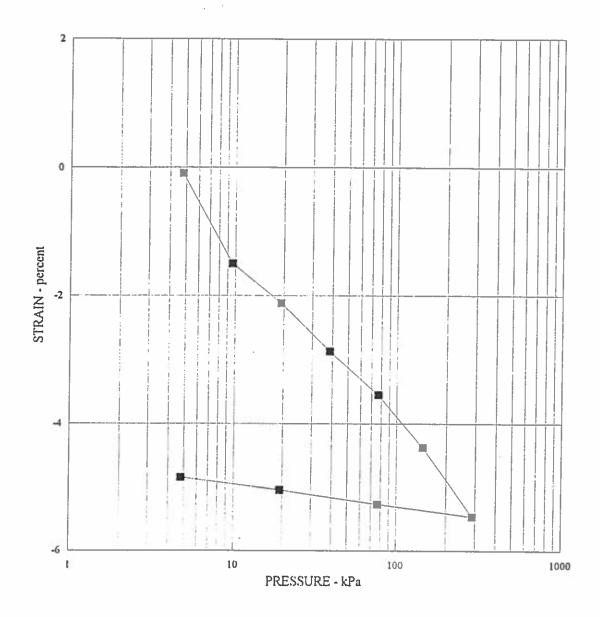
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CONSOLIDATION TEST

CARSON FREEWAY

CARSON CITY, NEVADA

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BORING NO.: BL-28 DE	PTH: 1.83 m
SAMPLE DESCRIPTION Brown Silty Sand	
OVERBURDEN PRESSURE,	kPa 31
PRECONSOLIDATION PRES	SURE, kPa 12

	INITIAL	FINAL
DRY DENSITY - kN/ cu m	15.17	16.08
WATER CONTENT - %	7.2	17.7
VOID RATIO	0.5067	0.4152
DEGREE OF SATURATION, %	33.00	99.00
SAMPLE HEIGHT - cm	1.6300	1.5

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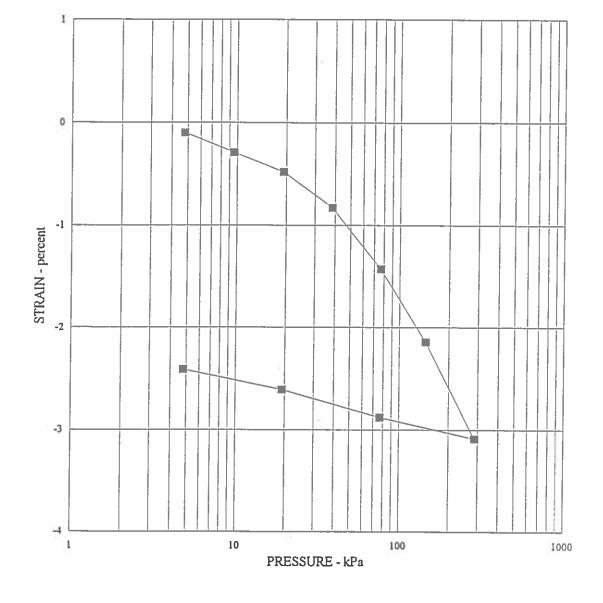
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CONSOLIDATION TEST CARSON FREEWAY

CARSON CITY, NEVADA

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BORING NO.: BL-29	DEPTH:	1.83 m
SAMPLE DESCRIPTION	Brown Silty San	d
	•	
OVERBURDEN PRESSU	RE, kPa	29
PRECONSOLIDATION P	RESSURE, kPa	48

	INITIAL	FINAL
DRY DENSITY - kN/cu m	16.44	16.9
WATER CONTENT - %	11.9	17.7
VOID RATIO	0.4863	0.4457
DEGREE OF SATURATION, %	61.00	99.00
SAMPLE HEIGHT - cm	1.6300	1.56

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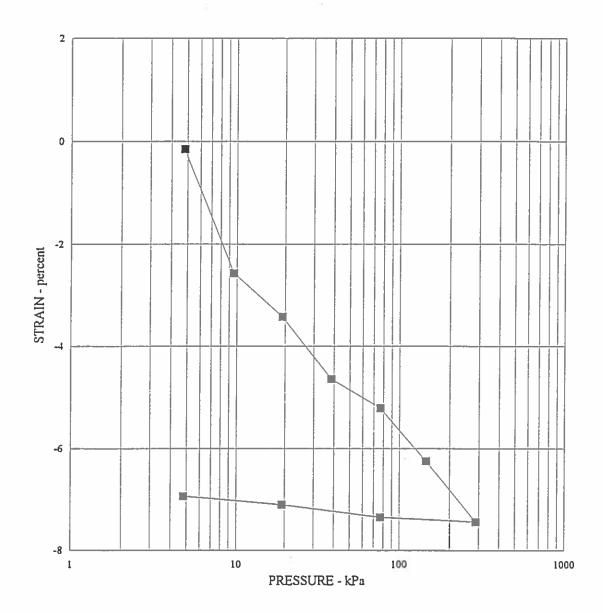
PROJECT NO. 30-1348-15.002

CONSOLIDATION TEST

CARSON FREEWAY

CARSON CITY, NEVADA

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BORING NO.: BL-30	DEPTH:	1.83 m
SAMPLE DESCRIPTION	Light Brown Si	lty Sand
OVERBURDEN PRESSUR	RE, kPa	30
PRECONSOLIDATION PR	ESSURE, kPa	5

	INITIAL	FINAL
DRY DENSITY - kN/cu m	14.39	15.71
WATER CONTENT - %	6.4	19.2
VOID RATIO	0.5953	0.4510
DEGREE OF SATURATION, %	25.00	99.00
SAMPLE HEIGHT - cm	1.6300	1.45

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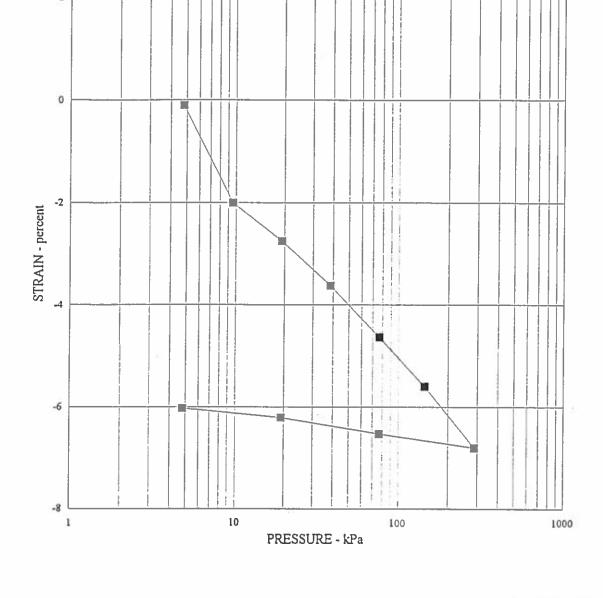
CARSON FREEWAY

CARSON CITY, NEVADA

PLATE

C-84

L\19



1.83 m
ıd
26
7

	INITIAL	FINAL
DRY DENSITY - kN/cu m	14.42	15.17
WATER CONTENT - %	5.8	19.8
VOID RATIO	0.5202	0.4485
DEGREE OF SATURATION, %	25.00	99.00
SAMPLE HEIGHT - cm	1.6300	1.51

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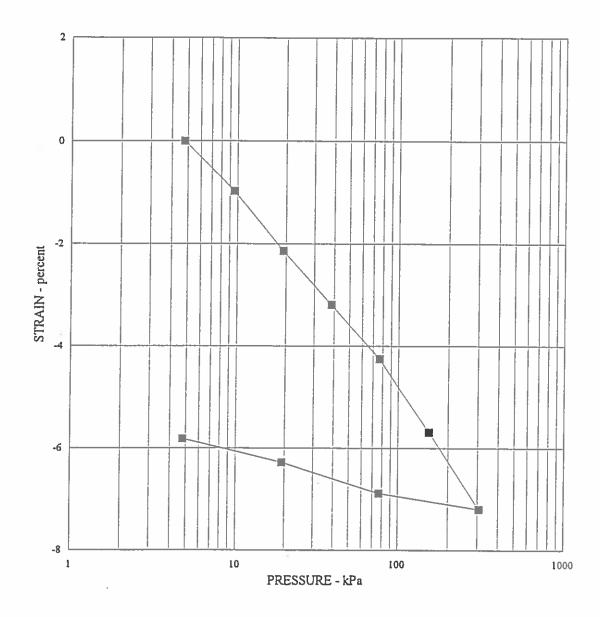
CARSON FREEWAY

CARSON CITY, NEVADA

PLATE

C-85

CAD FILE 12/194



BORING NO.: BL-36	DEPTH:	4.88 m
SAMPLE DESCRIPTION	Lt. Brown Claye	y Sand
1	-	-
OVERBURDEN PRESSUI	RE, kPa	72
PRECONSOLIDATION PI	RESSURE, kPa	10

	INITIAL	FINAL
DRY DENSITY - kN/cu m	18.38	19.81
WATER CONTENT - %	12.0	13.6
VOID RATIO	0.4937	0.3843
DEGREE OF SATURATION, %	68.00	99.00
SAMPLE HEIGHT - cm	2.5400	2.3

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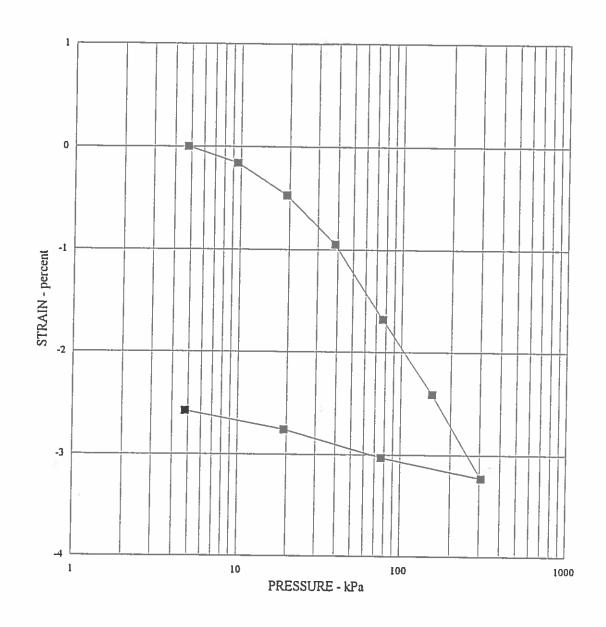
CONSOLIDATION TEST

CARSON FREEWAY

CARSON CITY, NEVADA



11/1



	DEPTH:	4.88 m
SAMPLE DESCRIPTION I	t. Brown Sl. Si	lty Sand
OVERBURDEN PRESSUR	E, kPa	76
PRECONSOLIDATION PR	ESSURE, kPa	12

	INITIAL	FINAL	
DRY DENSITY - kN/cu m	18.36	18.97	
WATER CONTENT - %	15.0	14.6	
VOID RATIO	0.4466	0.3996	
DEGREE OF SATURATION, %	91.00	99.00	
SAMPLE HEIGHT - cm	2.5400	2.45	

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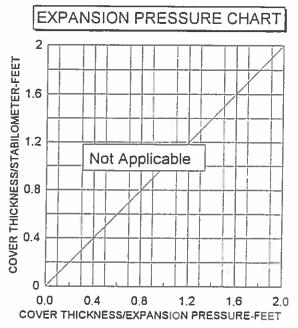
CONSOLIDATION TEST

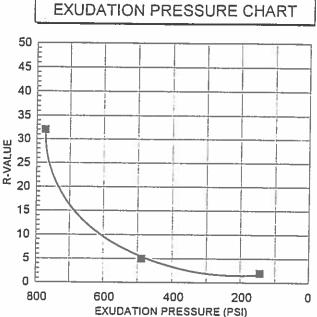
CARSON FREEWAY

CARSON CITY, NEVADA

Sample Source: BL-2 @ .30-.61 meters

Sample Description: Light Brown Silty Sand





Specimen	A	В	С
Exudation Pressure, psi	774	490	144
Expansion Dial (.0001")	0.0000	0.0000	0.0000
Expansion Pressure, psf	0	0	0
Resistance Value, R	32	5	2
% Moisture at Test	11.7	13.9	16.1
Dry Density at Test, pcf	123.2	117.8	112.0
R Value by Expansion Pressure (TI=)	Not Apr	licable	
R Value at 300 psi Exudation Pressure	3		

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CARSON FREEWAY

PLATE

C-88

PROJECT NO. 30-1348-15.002

APPENDIX E

Drainage Borings and Laboratory Test Results

USCS GROUP	TYPICAL SOIL DESCRIPTION
GW	Well graded gravels, gravel-sand mixtures, little or no fines
GP	Poorly graded gravels, gravel-sand mixtures, little or no fines
GM	Silty gravels, poorly graded gravel-sand-silt mixtures
] GC	Clayey gravels, poorly graded gravel-sand-clay mixtures
SW	Well graded sands, gravelly sands, little or no fines
SP	Poorly graded sands, gravelly sands, little or no fines
SM	Silty sands, poorly graded sand-silt mixtures
SC	Clayey sands, poorly graded sand-clay mixtures
ML	Inorganic silts and very fine sands, rock flour, silty or clayey fine sands with slight plasticity
CL	Inorganic clays of low to medium plasticity, gravelly clays, sandy clays, silty clays, lean clays
OL	Organic silts and organic silt-clays of low plasticity
МН	Inorganic silts, micaceous or diatomaceous fine sandy or silty soils, elastic silts
СН	Inorganic clays of high plasticity, fat clays
ОН	Organic clays of medium to high plasticity
CE	Caliche
PT	Peat and other highly organic soils

MOISTURE CONDITION CRITERIA

SOIL CEMENTATION CRITERIA

MOIDIONE	CONDITION CRITCHIA	SOID CLINE I	TATION CRITERIA
Description	<u>Criteria</u>	Description	<u>Criteria</u>
Dry	Absence of moisture, dusty, dry to	Weak	Crumbles or breaks with handling or
	touch.		little finger pressure.
Moist	Damp, no visible water.	Moderate	Crumbles or breaks with considerable
			finger pressure.
Wet	Visible free water, usually below	Strong	Will not crumble or break with finger
	water table.		pressure.

	STANDARD PENETRATION	CLASSIFICA	TION*
GF	RANULAR SOIL	CL	AYEY SOIL
BLOWS/0.3m	DENSITY	BLOWS/0.3m	CONSISTENCY
0-4	VERY LOOSE	0 – 1	VERY SOFT
5 10	LOOSE	2-4	SOFT
11 - 30	MEDIUM DENSE	5-8	MEDIUM STIFF
31 - 50	DENSE	9 – 15	STIFF
OVER 50	VERY DENSE	16 – 30	VERY STIFF
*Standard Penetrat	ion Test (N) 63.5 Kg hammer	31 60	HARD
760mm free fall on :	50.8mm O.D. x 35mm I.D. sampler.	OVER 60	VERY HARD

Blow counts on California Split Spoon (Ncu) can be converted to N_{spt} by: $(N_{ess})(0.563) = N_{spt}$

Blow counts from Automatic Hammer can be converted to Standard N_{iot} by: $(N_{Automatic Hammer})(1.33) = N_{spt}$

TEST	ABBREVIATIONS			SAMPLER NOTATION
CD CH CM CU D DS E G	CONSOLIDATED DRAINED CHEMICAL (CORROSIVENESS) COMPACTION CONSOLIDATED UNDRAINED DISPERSIVE SOILS DIRECT SHEAR EXPANSIVE SOIL SPECIFIC GRAVITY HYDROMETER HYDRO-COLLAPSE	O OC PI RQD RV S SL U UU UV	ORGANIC CONTENT CONSOLIDATION PLASTICITY INDEX ROCK QUALITY DESIGNATION R- VALUE SIEVE ANALYSIS/-200 WASH SHRINKAGE LIMIT UNCONFINED COMPRESSION UNCONSOLIDATED UNDRAINED UNIT WEIGHT	CPT CONE PENETRATION CS CONTINUOUS SAMPLER(1) MC MODIFIED CA SPLIT SPOON (2) P PUSHED (NOT DRIVEN) PB PTICHER BARREL RC ROCK CORE(3) SH SHELBY TUBE (4) SPT STANDARD PENETRATION TEST TP TEST PIT (1) I.D. = 82mm with tube; 98.9mm w/o tube
к	PERMEABILITY	w	MOISTURE CONTENT	(2) L.D. = 61.5mm (3) N XW (4) L.D. = 73mm

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AND TERMS

CARSON FREEWAY

CARSON CITY, NEVADA

KEY TO SOIL CLASSIFICATION

PLATE

PROJECT NO. 30-1348-15.002

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EXPLORATION LOG

7/15/97 START DATE: _

7/15/97 END DATE:

JOB DESCRIPTION _ CARSON FREEWAY

15.24 M SOUTH OF KOONTZ LANE LOCATION BORING

KLEINFELDER

30-1348-15.002 E.A. # GROUND ELEV_ 1444,75 m

GROUNDWATER LEVEL

DATE DEPTH ELEV.

STATION OFFSET 15.24 M RIGHT

STA 144+00

ENGINEER J. FORGA

EQUIPMENT _CME 55

OPERATOR SPECTRUM

HOLLOW STEM AUGER DATE 7/15/97

SHEET 1 OF 2

REMARK	MATERIAL DESCRIPTION	USCS	LAB TESTS	Recovery	AMPLE	S/		DEPTH	ELEV.
REWARK	MATERIAL DESCRIPTION	SM	S,RV	(%)	300mm	TYPE	NO.	(m) I 0.00	(m)
	RED BROWN SILTY SAND slightly moist, very dense, fine to medium dense, trace fine grave).	2141	JINV			QI XD		- 0.00	
	Sense, and to medical dense, trace and graves.							0.61	
	10			1	83	MC	В	- 1	
26% fine:	Weakly cemented.							- 1 1.07	1443.8
40% IIIIe:	5.9			1			İ		
					50/64	MC	-	1,52	
	1.98					2		1.83	4440.0
	YELLOW BROWN SUGHTLY SILTY SAND	SM SP			24	110		2 2 13	1442.8
8% fines	slightly moist, very dense, line to medium grained sand.	SP	s 		64	MC	D	2 59	
	***			İ			ĺ		
	400		DS		60	MC	E	3 3.05	1441.8
			U3		00	1410			
							\dashv	3.51	
	T- 0-output	1						}	1440.0
	411	CIA		ŀ				-4	1440.8
	YELLOW BROWN SILTY SAND moist, very	SM						[
	dense, fine grained sand.				61	MC	F	4.57	
								5 5.03	1439.8
		-			1		i	-	
							}	-	
	5.79						Ì		
	YELLOW BROWN CLAYEY SAND moist, very	sc						6 6.10	1438.8
	dense, fine grained sand, low plasticity fines, estimated 20 to 30% fines.				96/254	MC	G	-	
	-30111000 00 10 00 10 1111001							6.55	
	, , , , , , , , , , , , , , , , , , ,						1	_ 7	1437.8
					Ī				
	1							7.62	
				į	50/127	МС	Н	7.92	
			Ì					_ 8	1436.8
							-	-	
								-	1435.8
C1 42			C 01			110		9 9 14	1433.0
Pl = 13 31% fines		- [S,PI		50	MC			
	Becoming low to moderate plasticity fines.							9.60	

LOG OF BORING

CARSON FREEWAY

PLATE

PROJECT NO. 30-1348-15.002

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E L\1999)	H	4875 LC RE	NFEL DINGLEY LANE, SL NO, NEVADA 895 I. (702) 689-78	JITE 100 502
CAD FILE	PROJ	ECT NO. 30	0-1348-15.	.002	

		700		
EXPL	OR/	ATIC)N L	.00

GROUNDWATER LEVEL

DATE | DEPTH | ELEV.

START DATE: 7/15/97
END DATE: 7/15/97

LOCATION

BORING

KLEINFELDER

JOB DESCRIPTION CARSON FREEWAY

IPTION CARSONTHEEWAT

15.24 M SOUTH OF KOONTZ LANE BD-01

E.A. # 30-1348-15.002 GROUND ELEV 1444.75 m

HAMMER DROP SYSTEM AUTOMATIC

SHEET 2 OF 2

STATION STA 144+00

OFFSET 15.24 M RIGHT

ENGINEER J. FORGA
EQUIPMENT CME 55

OPERATOR SPECTRUM

DRILLING
METHOD HOLLOW STEM AUGER

						UTOMATIC		BACKFILLED YES DATE 7/15/97
ELEV. 14kg/a	DEPTH (m)	NO	TYPE	BLOWS, 300mm	Recovery	LAB TESTS	Rec	MATERIAL DESCRIPTION REMARKS
1433.8	- 10.57 - 11 - 11.13	J	MC	35			SM	Becoming less clayey. 11 29 BED YELLOW SILTY SAND moist, very dense,
1432.8	12 12.19	К	MC	50/140				fine grained sand, estimated 35 to 45% fines.
1431.8	13							
430.8	13.72	Ļ	MC	50/114			SC	HED BROWN CLAYEY SAND moist, very dense, fine to coarse grained sand, some fine gravel, estimated 20 to 30% fines.
429.8	_ 15 _ 15.24	М	MC	80/279			SM	LIGHT BROWN SILTY SAND moist, very dense, fine grained sand, estimated 30 to 40% fines. Encountered trace of coarse gravel.
428.8	15.70 16							No free water encountered
427.8	_ 17							
426.8	_ 18						ļ	
:25.8	_ 19					7.7		
F								

LOG OF BORING

CARSON FREEWAY

CARSON CITY, NEVADA

PLATE

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SOOZEPLAT	
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END DATE:

7/16/97

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CARSON FREEWAY

CARSON CITY, NEVADA

LOG OF BORING

SHEET 1 OF 2

PROJECT NO. 30-1348-15.002

L\1999\

PLATE

STA 138+50 STATION JOB DESCRIPTION CARSON FREEWAY 15.24 M RIGHT OFFSET NORTH OF VALLEY VIEW DRIVE LOCATION ENGINEER _J.FORGA EQUIPMENT CME 55 **BORING** KLEINFELDER OPERATOR . SPECTRUM 30-1348-15.002 GROUNDWATER LEVEL E.A. # DATE | DEPTH | ELEV. GROUND ELEV 1444.75 m HOLLOW STEM AUGER HAMMER DROP SYSTEM_AUTOMATIC BACKFILLED YES DATE 7/16/97 SAMPLE
TYPE BLOWS/ Recovery LAB TESTS ELEV. DEPTH USCS MATERIAL DESCRIPTION REMARKS NO. (m) 0.00 **GRAB** S,RV SM 21% fines LIGHT BROWN SILTY SAND slightly moist, medium dense, fine to medium grained sand, some surface gravel. 0.61 MC 18 W.UW,S 17% fines 1443.8 1 1.07 MC W.UW С 15 1442.8 MC Red brown, dense, fine to coarse grained sand, 46 WUW slightly plastic. Becoming more silty. Fine grained sand, very dense. 1441.8 3.05 E MC 50/140 1440.8 4.57 F (MC 50/127 IW.UW.S 21% fines 1439.8 5 1438.8 6 6.10 Yellow brown, becoming more sandy: MÇ 50/127 G 6.40 1437.8 MC 94/279 Becoming more sitry. Encountered some fine gravel. 1436.8 1435.8 9 MC 50/102 9.45

EXPLORATION LOG

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CARSON FREEWAY

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CARSON FREEWAY

EXPLORATION LOG START DATE: 7/16/97 SHEET 2 OF 2 7/16/97 END DATE: STA 135+00 STATION JOB DESCRIPTION CARSON FREEWAY __15.24 M RIGHT OFFSET SOUTH OF VALLEY VIEW DRIVE J. FORGA ENGINEER LOCATION BD-03 EQUIPMENT _CME 55 BORING **KLEINFELDER** SPECTRUM **OPERATOR** 30-1348-15.002 GROUNDWATER LEVEL E.A. # DATE | DEPTH ELEV. GROUND ELEV_ 1444.14 m **HOLLOW STEM AUGER** HAMMER DROP SYSTEM AUTOMATIC BACKFILLED YES DATE 7/16/97 SAMPLE BLOWS/ Recovery 300mm (%) ELEV. DEPTH LAB TESTS USCS MATERIAL DESCRIPTION REMARKS NO. TYPE (m) MÇ 69 1433.1 Some fine gravel. 1432.1 _ 12 К MÇ 50/89 1431.1 L 13 MC 50/127 1430.1 1429.1 L 15 MC 48 No free water encountered. 1428.1 1427.1 1426.1 _ 18 1425.1 L 19 @1989, by Kleinfelder, Inc.

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PROJECT NO. 30-1348-15.002

KLEINFELDER

4875 LONGLEY LANE, SUITE 100 RENO, NEVADA 89502 Tel. (702) 689-7800 LOG OF BORING
CARSON FREEWAY

CARSON CITY, NEVADA

PLATE

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ì.				STADI	DATE	7/16/97			EXPLORATION LOG	-		
	- 1 -		_	END D		7/16/97						SHEET 1 OF 2
		關州		_	-		RSON FREEV	NAY		STATION	STA 130	
			7.	LOCAT			F SOUTH EC		S DRIVE	OFFSET ENGINEER	30.48 M F J. FORGA	
	1 1	KLEINFELI	DER	BORIN	-	BD-04				EQUIPMENT		
	1	MENALERI	DEN	E.A. #	_	30-1348-	15.002		GROUNDWATER LEVEL	OPERATOR		JM
					ND ELEV.	1444.75 r	n		DATE DEPTH ELEV.	DRILLING METHOD		
	ļ						AUTOMATIC	:				STEM AUGER DATE 7/16/97
	ELE	V. DEPTH	,					_			163	DATE
	(m)	(m)	I NO.	TYPE	BLOWS	Recover	y LAB TEST	Grou	MATERIAL DE	SCRIPTION		REMARKS
		0.00	0 A	GRAB			S,RV	SM	BED BROWN SILTY Somedium dense, fine to	AND slightly moi	st,	27% fines
		0.6							medium dense, fine to	coarse grained	sand.	
	1443.8	,	В	MC	15	1	w.uw					
	1443.6	3 - 1 1.07	7	<u> </u>		-	 					
		1.52	,						a.]
	ŀ	- 1.34	C	MC	88/229	<u> </u>		-	Fine grained sand, very	/ dense,		
	1442.8	1.9			ļ	<u> </u>	<u> </u>	_				
		2.13	D	MC	40			-				
		2.59			"				Danasii			i
	- 1				 			-	Dense			
	1441.8	3 3,05									121	
		}	E	MC	36		w,uw,s					54% fines
		3,51	-					_	Lenses of sandy silt.			
	İ	-				i						
	1440.8	- 4										
	- 1	t						1 1				1
	ļ	4.57	F	MC	42		1471044	.				
	1439.8	}		IVIC	43		w.uw		4 98			
	1439.8	_5 5.03						sc	BED BROWN CLAYEY	AND maist dea	se fine	
	- 1	[i	- 1			1 1	grained sand, low plastic	city fines.	3C, INIC	i
	ļ	- 1			- 1			1 1				1
	1438.8	6 6.10		-		[]]				
		- 8.10	G	MC	88/279	,	W,UW,S	1]				30% fines
	ĺ	6.55							Low plasticity fines			50 % liftes
	1							1	parametry integral			
	1437.8	_7		ŀ	- 1	İ						
		†]		SM	7 16			j
	ĺ	7.62							BED BROWN SILTY SAN fine grained sand.	D maist, very de	nse,	
		+	Н	MC	84	V	V,UW					
	1436.8	8 8.08										
		[1	-					
		} [
	1435.8	_9					[
		9.14	1	MC !	50/140	I I A	v.uw.s					
		9.45	100	1410	30/140	1,	v,U1V,S					21% fines
											1	
	<u>i</u>											1
@18	99, by Kleinfe	ilder, inc.										
			/1 [: I N	EC	LDE	D		LOG OF BO	ORING		PLATE
		T L			Y LANE, S		n					
		_	107	RENO, I	NEVADA 89	502			CARSON FR	EEWAY		F_5
				Tel. (70	12) 689-71	800		1				- \frac{1}{2}

PROJECT NO. 30-1348-15.002

EXPLORATION LOG START DATE: 7/16/97 SHEET 2 OF 2 7/16/97 END DATE: STA 130+00 STATION JOB DESCRIPTION CARSON FREEWAY 30.48 M RIGHT OFFSET WEST OF SOUTH EDMONDS DRIVE J. FORGA LOCATION ENGINEER EQUIPMENT _CME 55 BORING KLEINFELDER **SPECTRUM** GROUNDWATER LEVEL OPERATOR 30-1348-15,002 E.A. # DATE DEPTH ELEV. GROUND ELEV_1444.75 m **HOLLOW STEM AUGER** HAMMER DROP SYSTEM_AUTOMATIC BACKFILLED YES DATE 7/16/97 TYPE | BLOWS/ Recovery LAB TESTS USCS ELEV. DEPTH MATERIAL DESCRIPTION NO. REMARKS 14(0)R (m) 10.67 MC 50/140 W,UW 10.97 1433.8 SC 1432.8 . 12 BED BROWN CLAYEY SAND moist, very dense, fine to medium grained sand, low plasticity fines. MC 92/279 W.UW.S 22% fines 12.50 Occasional coarse gravel. 1431.8 13 MC 50/140 W,UW 1430.8 144.02 Less plastic. 1429.8 15 MC 50/140 Encountered some fine gravel. No free water encountered, 1428.8 . 16 1427.8 L 17 1426.8 ∟ 18 1425.8 @1999, by Kleinfelder, Inc. **LOG OF BORING**

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PROJECT NO. 30-1348-15.002

KLEINFELDER

4875 LONGLEY LANE, SUITE 100 RENO, NEVADA 89502 Tel. (702) 689-7800

CARSON FREEWAY

CARSON CITY, NEVADA

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			CTART	DATE	7/16/97			EXPLORATION LOG		
-			END D		7/16/97					SHEET 1 OF 2
		.y.			ON CAR	SON FREEW	'AY		STATION STA 125+ OFFSET 15.24 M L	
			LOCAT			NORTH OF C		IEW DRIVE	ENGINEER J. FORGA	
F	KLEINFELD	ER	BORIN	G _	BD-05				EQUIPMENT _CME 55	
			E.A. #	_	30-1348-1	5.002		GROUNDWATER LEVEL	OPERATORSPECTRU	М
				ND ELEV_				DATE DEPTH ELEV.	DRILLING METHOD HOLLOW	STEM AUGER
ĺ						AUTOMATIC			BACKFILLED YES	
ELEV.		NG	7405	BLOWS BLOWS 300mm	/ Recover	LAB TESTS	usc:	MATERIAL DE		
(m)	(m) 0.00	NO.	TYPE GRAB	30000	(94)	IS,RV	SM	WATERIAL DE	SCRIPTION	REMARKS
ĺ	Ţ				1		0.41	YELLOW BROWN SILT	Y_SAND slightly moist, se grained sand, coarse	28% fines
ĺ	0.61	В	MC	80/222			-	gravel	se grameu sano, coarse	
1443.1	1 1.07	l -	MC	80/229		1				
	- 1.07						-			
	1.52						1	No gravel.		
	1.83	С	МС	50/127				<u> </u>		
1442,1	- 2 2.13						1			
	2.29	_D_	MC	50/140	-					
	[]								3	İ
1441.1	_ 3 3.05	E	MC	50/140			-[
	3.35						1 1			
	<u> </u>									
1440.1	4						ÍΙ			
	} [- 1								
	4.57									20
	[F	MC	50/140	İ					
1439.1	5 5.03	- 1								ĺ
					ŀ					
	-						- 1			}
1438.1	6	-				İ				
	- 9.10	G	MC	50/127			- 1	Sands becoming cleaner		
1	6.40	\dashv								
ļ										
1437.1	_7			ĺ		ľ	- [
						}			-	
	7.62		110	04/222						
1436.1	. 8	H	MC	94/279						
	8 8 08	-		-			1	8.23		ļ
-	. [Ī	sc	RED BROWN CLAYEY SA	ND moist year dage	
t								fine to medium grained sa estimated 20 to 30% fines	and, low plasticity fines.	
1435.1	9 9.14							assumered to re 30 to Illies	•	
F		1	МС	50/127			- }			
Ē	9.45	S .								
-										
, by Kleinfe	elder, Inc.	- 1		ı	<u> </u>					
1 h			FIN	1FE	IDI	- R		LOG OF B	ORING	PLATE
-		48	375 LONG	LEY LANE, I NEVADA 8	SUITE 100	- 11			-	
			RENO, Tel. (1	NEVADA 8 702) 689-1	9502 7800			CARSON FR	ILEWAY	F-6

CARSON CITY, NEVADA

E-6

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PROJECT NO. 30-1348-15.002

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EXPLORATION LOG START DATE: __7/16/97 SHEET 2 OF 2 7/16/97 END DATE: STA 125+00 STATION JOB DESCRIPTION CARSON FREEWAY 15.24 M LEFT OFFSET _ 15,24 M NORTH OF CLEARVIEW DRIVE LOCATION ENGINEER __J. FORGA BD-05 BORING EQUIPMENT _ CME 55 KLEINFELDER 30-1348-15.002 OPERATOR _ SPECTRUM E.A. # GROUNDWATER LEVEL DATE | DEPTH | ELEV. GROUND ELEV 1444.14 m HOLLOW STEM AUGER HAMMER DROP SYSTEM AUTOMATIC BACKFILLED YES __ DATE__7/16/97 SAMPLE
TYPE BLOWS/ Recovery LAB TESTS Group DEPTH ELEV. MATERIAL DESCRIPTION NO. REMARKS 1 क्ष्या) 1 (m)10.57 MÇ 50/140 110.97 1433.1 Low to moderate plasticity fines. 1432.1 _ 12 12.19 MC 50 12.65 Encountered some fine gravel. 1431.1 _ 13 13.26 SM YELLOW BROWN SILTY SAND moist, very dense, fine to medium grained sand, estimated 15 to 25% fines. 13.72 MC 72 1430.1 _ 14 1429.1 _ 15 MC 46 15.70 15.70 No free water encountered. 1428.1 _ 16 1427.1 L 17 1426.1 L 18 1425.1 _ 19 @1999, by Kleinfelder, Inc.

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PROJECT NO. 30-1348-15.002

LOG OF BORING
CARSON FREEWAY

ON TOOM THEE WAT

CARSON CITY, NEVADA

PLATE

E-6A

EXPLORATION LOG START DATE: 9/24/97 SHEET 1 OF 1 9/24/97 END DATE: STA 254+00 STATION JOB DESCRIPTION CARSON FREEWAY 457 M RIGHT OFFSET PRISON PROPERTY _J. FORGA LOCATION **ENGINEER** EQUIPMENT _CME 55 BORING KLEINFELDER SPECTRUM 30-1348-15.002 GROUNDWATER LEVEL OPERATOR _ E.A. # DATE DEPTH ELEV DRILLING METHOD GROUND ELEV 1409.70 m HOLLOW STEM AUGER 1408.2 HAMMER DROP SYSTEM_STANDARD BACKFILLED YES DATE 9/24/97 SAMPLE
TYPE | BLOWS/ Recovery | 300mm (%) LAB TESTS USCS ELEV. DEPTH MATERIAL DESCRIPTION NO. REMARKS SM LIGHT BROWN SILTY SAND (SM) moist, medium dense, very fine to medium grained sand. 0.61 MC 31 W,UW,S 32% fines 1408.7 Dense В MC 42 K 1.68 Groundwater encountered at 1.5 M SP GRAY_CLEAN_SAND_(SP) wet, dense, fine to coarse sand and fine gravel. 1407.7 1406.7 3 3.05 MC 17 S.K 3% fines Heaving sands. Medium dense 1405.7 D MC 50/76 SC 1404.7 DARK GRAY CLAYEY SAND (SC) wet, very dense, fine to medium grained sand, low to moderate plasticity fines, estimated 15 to 25% 1403.7 6 6.10 E MC 50/102 6 40 1402.7 CL DARK BROWN SANDY CLAY (CL) wet, hard, fine grained sand, low to moderate plasticity fines. MC 95/279 1401.7 8 8.08 SC GRAY CLAYEY SAND (SC) wet, very dense, fine to coarse sand, low plasticity fines. 1400.7 MC 50/102 G @1999, by Kleinfelder, inc. PLATE **LOG OF BORING** KLEINFELDER

CARSON FREEWAY

CARSON CITY, NEVADA

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PROJECT NO. 30-1348-15.002

EXPLORATION LOG START DATE: __9/24/97 SHEET 1 OF 1 9/24/97 END DATE: STA 253+75 STATION JOB DESCRIPTION CARSON FREEWAY 457 M RIGHT **OFFSET** PRISON PROPERTY _J. FORGA LOCATION ENGINEER BD-07 EQUIPMENT _CME 55 **BORING** KLEINFELDER SPECTRUM 30-1348-15.002 **OPERATOR GROUNDWATER LEVEL** E.A. # DRILLING METHOD DATE | DEPTH | ELEV. GROUND ELEV. 1409.70 m HOLLOW STEM AUGER 9/24/97 HAMMER DROP SYSTEM STANDARD BACKFILLED YES __ DATE <u>9/24/97</u> BLOWS/ Recovery LAB TESTS USCS ELEV. DEPTH MATERIAL DESCRIPTION NQ. TYPE REMARKS (m) SM LIGHT BROWN SILTY SAND (SM) moist, medium dense, fine to coarse grained sand, estimated 15 0.61 MC 19 WU,UW 1408.7 1 1.07 SM SP GBAY SUGHTLY SILTY SAND (SM/SP) wet, MC medium dense, fine to coarse grained sand. Groundwater encountered at 1.5 M. B 25 W,UW,S 5% fines 1407.7 SC GBAY CLAYEY SAND (SC) wet, medium dense, fine to medium grained sand, low to moderate 1406.7 3 3,05 С MC 17 W,UW,S 16% fines plasticity fines. 1405.7 Becoming more clayey. D MC 41 W,UW,S 38% fines 1404.7 CL GRAY SANDY CLAY (CL) wet, hard, fine grained sand, low to moderate plasticity fines, estimated 60 to 70% fines, 1403.7 MC 87/279 1402.7 SC DARK GRAY CLAYEY SAND (SC) wet, very dense, fine to medium grained sand, low to moderate plasticity fines. F MC 50/140 1401.7 1400.7 G 50/127 9.45 @1999, by Kleinfelder, Inc.

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PROJECT NO. 30-1348-15.002

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CARSON FREEWAY

CARSON CITY, NEVADA

PLATE

E-8

LOMPA RANCH J. FORGA ENGINEER LOCATION BD-08 EQUIPMENT _CME 55 **BORING** KLEINFELDER 30-1348-15.002 **GROUNDWATER LEVEL** OPERATOR _ SPECTRUM E.A. # DRILLING METHOD DATE | DEPTH | ELEV. GROUND ELEV_1409.70 m HOLLOW STEM AUGER 9/24/97 1.2 1408.5 HAMMER DROP SYSTEM STANDARD BACKFILLED YES SAMPLE BLOWS/ Recovery ELEV. DEPTH LAB TESTS MATERIAL DESCRIPTION NQ. TYPE CL DARK BROWN SANDY CLAY (CL) moist, stiff, fine grained sand, moderate to high plasticity 0.61 W,UW,S,PI MC 8 1408.7 1.07 Groundwater encountered at 1.2 M. В MC 10 K Wet Yellow brown 1.98 1407.7 3 3.05 W,UW Becoming less clayey MC 22 1405.7 SM DARK BROWN SILTY SAND (SM) wet, dense, fine to medium grained sand, low plasticity fines. D MC W,UW,S 1404.7 5.03 SM SP 1403.7 6 GRAY SLIGHTLY SILTY SAND (SM/SP) wet, very dense, fine to coarse grained sand, estimated 5 MÇ W,UW 65 to 12% fines. 1402.7 SC OUVE CLAYEY SAND (SC) wet, dense, fine to medium grained sand, low to moderate plasticity fines, estimated 20 to 30% fines. MC 35 1401.7 8 1400.7

EXPLORATION LOG

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PROJECT NO. 30-1348-15.002

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KLEINFELDER

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START DATE: __9/24/97

END DATE:

9/24/97

JOB DESCRIPTION _CARSON FREEWAY

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LOG OF BORING

9 50 Very dense.

CARSON FREEWAY

CARSON CITY, NEVADA

PLATE

SHEET 1 OF

DATE 9/24/97

REMARKS

Pi=32

63% fines

28% fines

STA 254+00

137 M RIGHT

STATION

OFFSET

END DATE:

7/14/97

JOB DESCRIPTION CARSON FREEWAY

91 M WEST OF LOMPA RANCH

BORING E.A. #

30-1348-15.002

GROUND ELEV 1417.32 m

HAMMER DROP SYSTEM AUTOMATIC

EXPLORATION LOG

GROUNDWATER LEVEL

DATE | DEPTH | ELEV

7/14/97 1.4 1415.9

SHEET 1 OF 2

STATION OFFSET

STA 343+86 84 M RIGHT

J. FORGA

ENGINEER

EQUIPMENT _CME 55

OPERATOR _ SPECTRUM

DRILLING METHOD

HOLLOW STEM AUGER

BACKELLED YES

						NOTOWATIC		BACKFILLED YES	DATE <u>7/14/97</u>
ELEV. (m)	DEPTH (m)	NO.	TYPE	AMPLE BLOWS/ 300mm	Recovery (%)	LAB TESTS	Grou		REMARKS
	- - -						SC	YELLOW BROWN CLAYEY SAND slightly moist, medium dense, fine to coarse grained sand, low to moderate plasticity fines.	
1416.3	1								
1415.3	1.52 2 1.98	A	MC	31		S,K	-	Fine to medium grained sand. Encountered groundwater at 1,37 M.	19% fines
1414.3	3 3.05 - 3.51	В	MC	35	:	S,K		Becoming more clayey, fine grained.	19% fines
413.3	- 4							421	
	4,57	С	MC	27			SC	GRAY CLAYEY SAND moist, medium dense, fine grained sand, low to moderate plasticity fines. Lenses of sandy clay (CL) low to moderate plasticity fines.	
412.3	<u>5 5.03</u>								
411.3	- 6 _{6.10}	0	MC I	25					
	6.55								
110.3	_ 7 - - - 7.62							Becoming less plastic.	
:09.3	- 8 _{8,08}	Н	MC	23					
							SM	8 69 GRAY SILTY SAND wet, medium dense, fine to	
08.3	9,14	1	МС	29	[ŝ	:	coarse grained sand.	24% fines

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LOG OF BORING

CARSON FREEWAY

CARSON CITY, NEVADA

PLATE

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EXPLORATION LOG SHEET 2 OF 2 7/14/97 END DATE: STA 343+86 **STATION** JOB DESCRIPTION CARSON FREEWAY 84 M RIGHT OFFSET 91 M WEST OF LOMPA RANCH ENGINEER _J. FORGA BD-09 EQUIPMENT _CME 55 BORING KLEINFELDER GROUNDWATER LEVEL OPERATOR SPECTRUM 30-1348-15.002 E.A. # DATE | DEPTH | ELEV. GROUND ELEV_ 1417.32 m HOLLOW STEM AUGER 7/14/97 1415.9 HAMMER DROP SYSTEM AUTOMATIC BACKFILLED YES __ DATE __ 7/14/97 TYPE BLOWS/ Recovery ELEV. DEPTH (m) LAB TESTS USCS MATERIAL DESCRIPTION NO. REMARKS 14(17) 10.67 MC 43 11 11.13 10.07 Heaving sands at 11 M. 1406.3 SP GRAY CLEAN SAND wet, dense, medium to coarse grained sand, less than 5% silt. 1405.3 12 12.19 24 MC SC 12.65 LIGHT BROWN CLAYEY SAND wet, medium dense, fine to medium grained sand, low to 12.65 moderate plasticity fines. 1404.3 _ 13 No free water encountered. 1403.3 L 14 1402.3 _ 15 1401.3 _ 16 1400.3 _ 17 1399.3 _ 18 1398.3 ... 19

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PROJECT NO. 30-1348-15.002

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CARSON FREEWAY

CARSON CITY, NEVADA

PLATE

E-10A

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EXPLORATION LOG SHEET 1 OF 2 7/15/97 END DATE: STA 344+60 STATION JOB DESCRIPTION CARSON FREEWAY 198 M RIGHT OFFSET 46 M WEST OF LOMPA LANE LOCATION ENGINEER EQUIPMENT _CME 55 **BORING** KLEINFELDER OPERATOR _SPECTRUM 30-1348-15.002 GROUNDWATER LEVEL E.A. # DATE DEPTH ELEV. DRILLING METHOD GROUND ELEV_ 1417.62 m HOLLOW STEM AUGER 7/15/97 1.2 HAMMER DROP SYSTEM AUTOMATIC BACKFILLED YES _ DATE _7/15/97 BLOWS/ Recovery LAB TESTS ELEV. DEPTH (m) USCS MATERIAL DESCRIPTION NO. TYPE REMARKS SC BROWN CLAYEY SAND moist, medium dense, fine grained sand, moderate to high plasticity fines. 1416.6 Encountered groundwater at 1.2 M. MC W,UW,S 20 28% fines Becoming more sandy, low plasticity fines. 1415.6 1414.6 3 3.05 MC 8 24 W,UW,S 27% fines Fine to medium grained sand. 3.51 1413.6 SM SP LIGHT BROWN SUGHTLY SILTY SAND wet, fine to coarse grained sand, estimated 5 to 12% fines. C MC 1412.6 5 5.03 SC GRAY CLAYEY SAND wet, medium dense, fine to medium grained sand, low to moderate plasticity 1411.6 6 D MC 20 1410,6 Н MC 19 Becoming more sandy, low plasticity fines. 1409.6 SM SP GRAY SLIGHTLY SILTY SAND wet, medium dense, line to medium grained sand. 1408.6 MC W,UW,S 28 9% fines 9,60 @1999, by Kleinfelder, Inc.

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CARSON FREEWAY

LOG OF BORING

F-11

PLATE

PROJECT NO. 30-1348-15.002

EXPLORATION LOG SHEET 2 OF 2 7/15/97 END DATE: STA 344+60 STATION JOB DESCRIPTION CARSON FREEWAY 198 M RIGHT **OFFSET** 46 M WEST OF LOMPA LANE ENGINEER __J.FORGA LOCATION 80-10 EQUIPMENT _CME 55 **BORING** KLEINFELDER OPERATOR _SPECTRUM 30-1348-15.002 GROUNDWATER LEVEL E.A. # DRILLING METHOD DATE DEPTH ELEV. GROUND ELEV 1417.62 m HOLLOW STEM AUGER HAMMER DROP SYSTEM_AUTOMATIC BACKFILLED YES DATE 7/15/97 SAMPLE
TYPE | BLOWS/ Recovery (%) ELEV. DEPTH LAB TESTS MATERIAL DESCRIPTION REMARKS NO. 14(12)6 (m) SC MC GRAY CLAYEY SAND wet, medium dense, fine to medium grained sand, low to moderate plasticity 1406.6 SM SP 1405.6 12 GRAY SUGHTLY SILTY SAND wet, dense, fine to medium grained sand, estimated 5 to 12% fines. MC 43 12.65 No free water encountered. 1404.6 _ 13 1403.6

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1402.6

1401.6

1400.6

1399.6

1398.6

L 16

L 17

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_ 19



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PROJECT NO. 30-1348-15.002

LOG OF BORING

CARSON FREEWAY

CARSON CITY, NEVADA

PLATE

E-11A

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7/15/97 END DATE: STA 349+83 STATION JOB DESCRIPTION CARSON FREEWAY __107 M RIGHT OFFSET WEST OF LOMPA LANE LOCATION ENGINEER J. FORGA EQUIPMENT _ CME 55 BORING KLEINFELDER 30-1348-15.002 GROUNDWATER LEVEL OPERATOR SPECTRUM E.A. # DATE | DEPTH | ELEV. GROUND ELEV_ 1417.62 m DRILLING METHOD HOLLOW STEM AUGER 7/15/97 HAMMER DROP SYSTEM_AUTOMATIC BACKFILLED YES DATE 7/15/97 DEPTH NO. BLOWS/ Recovery ELEV. LAB TESTS MATERIAL DESCRIPTION TYPE (m) SC GRAY BROWN CLAYEY SAND slightly moist, loose to medium dense, fine grained sand, low plasticity fines. 1416.6 1.07 SM SP YELLOW BROWN SLIGHTLY SILTY SAND wet. loose to medium dense, fine to coarse grained MC 12 K sand, estimated 5 to 12% fines. Encountered groundwater at 1.83 M. 1415.6 MC 20 B Medium dense 1414.6 3 3.05 MC K SC 1413.6 GRAY CLAYEY SAND wet, medium dense, fine to medium grained sand, moderate to high plasticity fines. MC 28 D 1412.6 1411.6 - 6 MC 21 Lenses of sandy clay (CL), moderate plasticity fines. 1410.6

EXPLORATION LOG

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1409.6

1408.6

8 8.08

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KLEINFELDER

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MC

MC

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LOG OF BORING

CARSON FREEWAY

PLATE

SHEET 1 OF 2

REMARKS

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PROJECT NO. 30-1348-15.002

KLEINFELDER

7/15/97

END DATE: JOB DESCRIPTION ___CARSON FREEWAY

LOCATION

WEST OF LOMPA LANE

BORING

30-1348-15.002 E.A. # GROUND ELEV__1417.62 m

HAMMER DROP SYSTEM_AUTOMATIC

EXPLORATION LOG

GROUNDWATER LEVEL

DATE | DEPTH | ELEV.

7/15/97 1.8 1415.8

SHEET 2 OF 2

STATION OFFSET

STA 349+83 107 M RIGHT

ENGINEER

_J, FORGA

EQUIPMENT _ CME 55

OPERATOR

SPECTRUM

DRILLING METHOD

HOLLOW STEM AUGER

BACKFILLED YES DATE 7/15/97

ELEV.	DEPTH (m)	NO.	TYPE	BLOWS/ 300mm	Recovery (%)	LAB TESTS	Group	MATERIAL DESCRIPTION	REMARKS
406.6	- 10.57 - 11 - 11,13	К	мс	22	,			Lenses of slightly clayey sands.	
405.6	_ 12 12_19	L	MC	51			SM SP	GRAY SLIGHTLY SILTY SAND wet, very dense, line to coarse grained sand, estimated 5 to 12% fines.	
404.6	- 12.65 - _ 13 -							12 65	
403.6	- _ 14 -					,			
402.5	- _ 15							12	
401.6	_ 16						ļ		
100.6	_ 17								
399.6	_ 18								
398.6	_ 19								

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PROJECT NO. 30-1348-15.002

KLEINFELDER

4875 LONGLEY LANE, SLITTE 100
RENO, NEVADA 89502
Tel. (702) 689-7800

LOG OF BORING

CARSON FREEWAY

CARSON CITY, NEVADA

PLATE

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EXPLORATION LOG SHEET 1 OF 2 7/15/97 END DATE: STA 350+89 STATION JOB DESCRIPTION CARSON FREEWAY 139 M RIGHT OFFSET WEST OF LOMPA LANE _J. FORGA LOCATION **ENGINEER** BD-12 EQUIPMENT _CME 55 **BORING** KLEINFELDER OPERATOR SPECTRUM 30-1348-15 002 GROUNDWATER LEVEL E.A. # DRILLING METHOD DATE DEPTH ELEV. GROUND ELEV_ 1417.62 m HOLLOW STEM AUGER HAMMER DROP SYSTEM AUTOMATIC BACKFILLED YES __ DATE _7/15/97 MPLE BLOWS/ Recovery 300mm (%) ELEV. DEPTH LAB TESTS USCS MATERIAL DESCRIPTION NO. TYPE REMARKS <u>(m)</u> SC LIGHT BROWN CLAYEY SAND moist, medium dense, fine grained sand, low plasticity fines. 1416.6 MC 22 Encountered groundwater at 1.52 M. 1415.6 1414.6 3 3.05 MC 23 Gray, becoming more sandy. Yellow brown. 1413.6 SM SP GRAY SLIGHTLY SILTY SAND wet, medium dense, fine to coarse grained sand, estimated 5 to 12% fines. С MC 29 1412.6

SC

GRAY CLAYEY SAND wet, dense, fine to coarse grained sand, low to moderate plasticity fines.

Becoming more clayey, fine grained sand.

Medium dense, becoming more sandy, fine to medium grained sand.

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1410.5

1409.6

1408.6

8 8.08

. 9



6.10

Н

MC

MC

38

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24

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PROJECT NO. 30-1348-15.002

LOG OF BORING

CARSON FREEWAY

CARSON CITY, NEVADA

PLATE

E-13

EXPLORATION LOG SHEET 2 OF 2 END DATE: 7/15/97 STA 350+89 STATION JOB DESCRIPTION CARSON FREEWAY 139 M RIGHT OFFSET WEST OF LOMPA LANE LOCATION ENGINEER J. FORGA BD-12 EQUIPMENT _CME 55 BORING OPERATOR _SPECTRUM 30-1348-15.002 GROUNDWATER LEVEL E.A. # DATE | DEPTH | ELEV. GROUND ELEV 1417,62 m HOLLOW STEM AUGER HAMMER DROP SYSTEM_AUTOMATIC BACKFILLED YES DATE 7/15/97 SAMPLE
TYPE BLOWS/ Recovery ELEV. DEPTH LAB TESTS USCS MATERIAL DESCRIPTION NO. REMARKS 10.67 MC 56 Very dense, moderate to high plasticity fines. 1406.6 1405.6 12 MC 23 1404.6 13 1403.6 1402.6 1401.6 L 16 1400.6 17 1399.6 ∟ 18 1398.6 19 @1999, by Kleinfelder, Inc.

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CARSON FREEWAY

E-13A

PLATE

PROJECT NO. 30-1348-15.002

EXPLORATION LOG SHEET 1 OF 2 7/14/97 END DATE: STA 337+00 STATION JOB DESCRIPTION __CARSON FREEWAY 35 M RIGHT **OFFSET** WEST OF LOMPA LANE LOCATION ENGINEER _J. FORGA BD-13 BORING EQUIPMENT _CME 55 KLEINFELDER GROUNDWATER LEVEL OPERATOR _SPECTRUM 30-1348-15.002 E.A. # DATE | DEPTH | ELEV. GROUND ELEV. 1417.02 m HOLLOW STEM AUGER 7/14/97 1415.3 HAMMER DROP SYSTEM AUTOMATIC BACKFILLED YES DATE 7/14/97 SAMPLE
TYPE BLOWS/ Recovery LAB TESTS USCS Group ELEV. DEPTH MATERIAL DESCRIPTION NO. REMARKS (m) (m) SC LIGHT BROWN CLAYEY SAND slightly moist, moderately dense, fine to coarse grained sand, low plasticity fines. 1416.0 L 1 МÇ 19 S,K Fine grained sand, becoming less clayey. 22% fines Encountered groundwater at 1.68 M. 1415.0 2 1.98 SM SP LIGHT BROWN SLIGHTLY SILTY SAND wet, medium dense to dense, line to coarse grained 4% fines 1414.0 3 3,05 sand. MC 30 S SC OLIVE CLAYEY SAND wet, medium dense, fine to medium grained sand, low, to moderate 1413.0 plasticity fines. SM SP LIGHT BROWN SLIGHTLY SILTY SAND wet, fine to medium grained sand, estimated 5 to 12% C MC 29 fines. 1412.0 5 5.03 SC LIGHT BROWN CLAYEY SAND wet, dense, fine to medium grained sand, low plasticity fines. 6 6.10 1411.0 MC Becoming more clayey, low to moderate plasticity fines, red brown. 37 1410.0 Н MC 23 1409.0 8.08 Heaving sands at 7.6 M. Lenses of clean sands. 1408.0 MC 36 Fine grained sands. 9.50

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PROJECT NO. 30-1348-15.002

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CARSON FREEWAY

CARSON CITY, NEVADA

PLATE

E-14

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EXPLORATION LOG START DATE: 7/14/97 SHEET 2 OF 2 7/14/97 END DATE: STA 337+00 STATION JOB DESCRIPTION CARSON FREEWAY 35 M RIGHT OFFSET WEST OF LOMPA LANE **ENGINEER** J. FORGA BORING EQUIPMENT _CME 55 KLEINFELDER 30-1348-15.002 E.A. # GROUNDWATER LEVEL OPERATOR _SPECTRUM DATE | DEPTH | ELEV. DRILLING METHOD GROUND ELEV 1417.02 m HOLLOW STEM AUGER 7/14/97 HAMMER DROP SYSTEM_AUTOMATIC BACKFILLED YES DATE 7/14/97 TYPE BLOWS/ Recovery LAB TESTS USCS DEPTH (m) ELEV. NO. MATERIAL DESCRIPTION 14/11h **REMARKS** 10.67 MC 49 Becoming less plastic. 1406.0 1405.0 12 MC 28 12.65 1404.0 _ 13 1403.0 _ 14 1402.0 _ 15 1401.0 _ 16 1400.0 _ 17 1399.0 . 18 1398.0 _ 19

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PROJECT NO. 30-1348-15.002

LOG OF BORING CARSON FREEWAY

CARSON CITY, NEVADA

PLATE

KLEINFELDER

END DATE:

7/14/97

JOB DESCRIPTION CARSON FREEWAY

WEST OF LOMPA LANE

BORING 30-1348-15.002

LOCATION

E.A. # GROUND ELEV 1417.32 m

HAMMER DROP SYSTEM AUTOMATIC

EXPLORATION LOG

GROUNDWATER LEVEL

DATE | DEPTH | ELEV.

SHEET 1 OF 2

STA 337 + 90 STATION 35 M LEFT OFFSET

ENGINEER __J. FORGA

EQUIPMENT CME 55 OPERATOR SPECTRUM

DRILLING METHOD

HOLLOW STEM AUGER BACKFILLED YES DATE 7/14/97

ELEV.	DEPTH (m)	NO.	TYPE	BLOWS/	Recovery	LAB TESTS		MATERIAL DESCRIPTION	REMARKS
							SC	LIGHT BROWN CLAYEY SAND slightly moist, medium dense, fine to coarse grained sand, low to moderate plasticity fines.	
416.3	_ 1							122	
	1,52	A	MC	27		W.UW.S	SM	LIGHT BROWN SILTY SAND wet, medium dense, fine to medium grained sand,	ent t
415.3	2 1.98			2,		11,011,0		Encountered groundwater at 1,52 M.	20% fines
	-							2.59	
414.3	_ 3 3.05		ĺ				SC	Fine grained sand, low plasticity fines.	
	3.51	В	МС	11		w.uw,s			26% fines
412.2	4	Ì							
413.3	- "		:					Gray, lenses of clean fine to coarse sand.	
	4,57	С	MC	19		W,UW,S			20% fines
412.3	_ 5 5.03 -							Red brown, low to moderate plasticity fines.	
11.3	6 6.10	D	MC	37		W.UW.S			27% fines
	6.55	_		-					27 70 111105
10.3	_ 7 _ 7.16	Н	MC	15					
ĺ	7.52								
09.3	- 8 _{8.08}	1	MC	25	1	w.uw,s	SM	7.92	16% fines
-								RED BROWN SILTY SAND wet, medium dense, fine to coarse grained sand.	
08.3	_ 9								
-	9.14	J	МС	19	V	v,uw,s			25% fines
-	9.60	-						Becoming more clayey, fine grained sand.	

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LOG OF BORING CARSON FREEWAY

CARSON CITY, NEVADA

PLATE

E-15

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START DATE: __7/14/97

7/14/97 END DATE:

JOB DESCRIPTION CARSON FREEWAY

LOCATION

WEST OF LOMPA LANE

BORING

E.A. #

30-1348-15.002

GROUND ELEV 1417.32 m

HAMMER DROP SYSTEM_AUTOMATIC

EXPLORATION LOG

GROUNDWATER LEVEL

7/14/97 1.5 | 1415.8

DATE DEPTH ELEV.

SHEET 2 OF 2

STA 337+90 STATION

35 M LEFT OFFSET

ENGINEER J. FORGA

EQUIPMENT _CME 55 OPERATOR _SPECTRUM

HOLLOW STEM AUGER BACKFILLED YES DATE 7/14/97

	-			44401 C				DAOMINEED TEG DA	(IE <u>-7719/3)</u>
ELEV. 14(ចេង	DEPTH (m)	NO.	TYPE	BLOWS	Recovery	LAB TESTS	USC:	MATERIAL DESCRIPTION	REMARKS
	-								
	10.67								
1406,3		К	MC	N/A				Gray, becoming less clayey,	
	- ¹ 1 _{1.13}							Approx. 60 cm of heaving sand at 11 M.	
1405.3	_ 12					1			
	12.19	L	MC	N/A					
	- 12.65		100	17/2				12.65	
1404.3	_ 13								
1707.3	- '3								
						i			
1403.3	14								
1402.3	_ 15					ľ			
in	- "								
	-					İ			
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1400.3	- _ 17			Ì		ĺ	ł		
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CARSON CITY, NEVADA

LOG OF BORING

CARSON FREEWAY

PLATE

E-15A

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EXPLORATION LOG START DATE: __9/24/97 SHEET 1 OF 1 9/24/97 END DATE: STA 254 + 75 STATION JOB DESCRIPTION _ CARSON FREEWAY 137 M RIGHT OFFSET **LOMPA RANCH** J. FORGA LOCATION ENGINEER BD-15 EQUIPMENT _CME 55 BORING KLEINFELDER **OPERATOR** ANDRESEN 30-1348-15.002 GROUNDWATER LEVEL E.A. # DRILLING METHOD DATE | DEPTH | ELEV. GROUND ELEV 1410.00 m HOLLOW STEM AUGER 9/24/97 HAMMER DROP SYSTEM_STANDARD BACKFILLED YES __ DATE _9/24/97 SAMPLE BLOWS/ Recovery DEPTH ELEV. LAB TESTS MATERIAL DESCRIPTION NO. TYPE REMARKS (m) (m) SC LIGHT BROWN CLAYEY SAND (SC) moist, medium dense, fine to medium grained sand, low to moderate plasticity fines. MC 20 1409.0 1 1.07 Groundwater encountered at 1.22 M. В MC 18 W,UW,S Yellow brown. 20% fines 1408.0 2 1.98 SM GRAY SUGHTLY SILTY SAND (SM/SP) wet, dense, fine to coarse grained sand 1407.0 3 3,05 Ç MC 31 1406.0 D MC W,UW,S 61 Very dense 7% fines 1405.0 5 5,03 1404,0 6.10 MC W,UW 1403.0 MC 19 1402.0 Becoming more clayey. 8.08 8 23 SC DARK GRAY CLAYRY SAND (SC) wet, medium dense, fine to medium grained sand, low plasticity fines, estimated 15 to 25% fines. 1401.0 . 9 MC 29 9.60 @1999, by Kleinfelder, Inc.

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PROJECT NO. 30-1348-15.002

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4875 LONGLEY LANE, SUITE 100 RENO, NEVADA 89502 Tel. (702) 689-7800 CARSON FREEWAY

LOG OF BORING

PLATE

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START DATE: __9/25/97

9/25/97 END DATE:

JOB DESCRIPTION _ CARSON FREEWAY

LOMPA RANCH

BQ-16 BORING

LOCATION

30-1348-15.002 E.A. # GROUND ELEV 1410.31 m

HAMMER DROP SYSTEM STANDARD

EXPLORATION LOG

GROUNDWATER LEVEL

DATE | DEPTH | ELEV.

9/25/97 2.4 1407.9

SHEET 1 OF 1

STATION STA 262+50 30 M LEFT OFFSET

ENGINEER __J. FORGA

EQUIPMENT _CME 55 OPERATOR _ANDRESEN

HOLLOW STEM AUGER

						TANDARD		BACKFILLED YES DA	TE <u>9/25/97</u>
ELEV. (m)	DEPTH (m)	NO.	TYPE	AMPLE BLOWS/	Recovery	LAB TESTS	USC	MATERIAL DESCRIPTION	REMARKS
_	0.61						CL	LIGHT BROWN SANDY CLAY/CLAYEY SAND (CL/SC) moist, very stiff/medium dense, very fine grained sand, low plasticity fines.	
1409.3	- - 1 1.07	^	MC	24		PI, W,UW,S			Pt= 10 50% fines
	1.52	В	MC	22		w.uw.s	SM	REQ YELLOW SILTY SAND (SM) moist, medium	
1408.3	2 1.98		IVIC	22		vv,0vv,5	-	dense, fine to medium grained sand, low plasticity fines.	46% fines
	-							Groundwater encountered at 2.4 M,	
1407.3	- _3 3.05						SM	290	
	3.51	С	MC	43		K	SM SP	GRAY BROWN SUGHTLY SILTY SAND (SM/SP) moist, dense, fine to coarse grained sand, some fine gravel, estimated 5 to 12% fines.	
1406.3	- - 4								
	4.57	D	MC	24				Medium dense	
405.3	5 5 0 3					-		More gravelly	
							SM	YELLOW BROWN SILTY SAND (SM) wet, medium dense, fine grained sand, estimated 20	
404.3	6 6,10	н	MC I	19				to 30% fines.	
-	6.55							Low plasticity fines.	
403.3	. 7					ļ		7.15	
-	7.62		MC	46			SM SP	GRAY SUIGHTLY SILTY SAND (SM/SP) wet, dense, fine to coarse grained sand, estimated 5 to 12% fines, some fine gravel.	
402.3	. 8 _{8.08}	<u> </u>	INIO	70				Salar mile graver,	
ŀ								8 84	
±01.3 -	9,14	J	мс	30			SM	LIGHT BROWN SILTY SAND (SM) wet, dense, fine grained sand, estimated 15 to 25% fines,	
	9,60						\dashv	9.60	
1	F				İ		ſ	159	

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LOG OF BORING

CARSON FREEWAY

PLATE

PROJECT NO. 30-1348-15.002

KLEINFELDER

START DATE: 9/25/97

END DATE:

JOB DESCRIPTION __CARSON FREEWAY

LOMPA RANCH LOCATION

BD-17 **BORING**

30-1348-15.002

E.A. # GROUND ELEV 1410.31 m

HAMMER DROP SYSTEM STANDARD

EXPLORATION LOG

GROUNDWATER LEVEL

DATE DEPTH ELEV.

2.1 | 1408.2

SHEET 1 OF 1

STA 262 + 50 STATION

0 OFFSET

__J. FORGA ENGINEER

EQUIPMENT _CME 55

OPERATOR

ANDRESEN

DRILLING METHOD

HOLLOW STEM AUGER

BACKFILLED YES DATE 9/25/97

				H DHOP S				BACKFILLED YES D	A16 3/49/31
ELEV. (m)	DEPTH (m)	NO.	TYPE	BLOWS/ 300mm	Recovery (%)	LAB TESTS	Grou	MATERIAL DESCRIPTION	REMARKS
	0.61	A	MC	19		W,UW,S,PI	sc	LIGHT.BROWN_CLAYEY_SAND_(SC) moist, very stiff/medium dense, very fine grained sand, low plasticity fines.	PI= 12
1409.3	1 1.07				'	7/6	-	1 22	46% fines
	1.52	В	мс	33		K	SC	LIGHT BROWN CLAYEY SAND (SC) moist, dense, fine to medium grained sand, moderate plasticity fines, estimated 20 to 30% fines.	
1408.3								Groundwater encountered at 2.3 M.	
1407.3						į		2.90	
1407.3	_ 3_3.05 - 3.51	С	МС	52		W.UW,S	SM SP	GRAY SLIGHTLY SILTY SAND (SM/SP) wet, very dense, fine to coarse grained sand, estimated 5 to 12% fines.	5% fines
1406.3	4								
	4.57	D	мс	10		w,uw,s	sc	GRAY VERY CLAYEY SAND (SC) wet, loose, fine to medium grained sand, low to moderate plasticity fines.	50% fines
1405.3	_ 5 5,03							presitory intes,	
404.3					:	ĺ			
404.3	- ⁶ 6.10	Н	MC	28					
	6.55	+						Medium dense, fine to coarse sand.	
403.3	-7								
400 -	7.62	1	MC	48				Less clayey, estimated 20 to 30% fines.	
402.3	- 8 _{8.08}								
401.3	-9							Lenses of slightly silty sand (SM/SP), fine to coarse grained sand.	
-	9 14	J	МС	42					
-	9.60							9.60	

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LOG OF BORING

CARSON FREEWAY

CARSON CITY, NEVADA

PLATE

E-18

EXPLORATION LOG START DATE: 9/25/97 SHEET 1 OF 1 9/25/97 END DATE: STA 277 + 50 STATION JOB DESCRIPTION _ CARSON FREEWAY OFFSET Ð LOMPA RANCH LOCATION J. FORGA ENGINEER EQUIPMENT _CME 55 **BORING** KLEINFELDER E.A. # 30-1348-15.002 GROUNDWATER LEVEL OPERATOR ANDRESEN DATE | DEPTH | ELEV. DAILLING METHOD GROUND ELEV 1410.61 m HOLLOW STEM AUGER 9/25/971 1.8 1408.8 HAMMER DROP SYSTEM STANDARD BACKFILLED YES _ DATE _9/25/97 SAMPLE
TYPE BLOWS/ Recovery ELEV. DEPTH (m) LAB TESTS MATERIAL DESCRIPTION NO. REMARKS (m) SC LIGHT BROWN CLAYEY SAND (SC) moist, medium dense, fine to medium grained sand, 0.61 moderate plasticity fines. MC 23 W.UW.S.PI PI=17 1409.6 43% fines 1.07 ₿ MC 32 Groundwater encountered at 1.83 M, dense. 1408.6 1407.6 3 3.05 С MC 42 W,UW,S 15% fines Becoming more sandy. 1406.6 SM LIGHT BROWN SLIGHTLY SILTY SAND (SM/SP) wet, dense, fine to medium grained sand, estimated 5 to 12% fines. D MC 39 K 1405.6 6 6.10 1404.6 MC 35 6.55 1403.6 SC LIGHT EROWN CLAYEY SAND (SC) wet, dense, fine to medium grained sand, low to moderate plasticity fines, estimated 20 to 30% fines. MC 43 1402.6 8 1401.6 _ 9 MC 65 9.60 9.60 @1999, by Kleinfelder, Inc. KLEINFELDER

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LOG OF BORING

CARSON FREEWAY

CARSON CITY, NEVADA

PLATE

EXPLORATION LOG START DATE: 9/25/97 SHEET 1 OF 1 9/25/97 END DATE: STA 227 + 50 STATION JOB DESCRIPTION _ CARSON FREEWAY 32 M RIGHT OFFSET LOMPA RANCH LOCATION ENGINEER __J. FORGA BD-19 EQUIPMENT _CME 55 BORING KLEINFELDER 30-1348-15.002 GROUNDWATER LEVEL OPERATOR _ANDRESEN E.A. # DATE | DEPTH | ELEV. DRILLING METHOD GROUND ELEV 1410.61 m HOLLOW STEM AUGER 9/25/97 1.8 1408 8 HAMMER DROP SYSTEM_STANDARD BACKFILLED YES _ DATE_<u>9/25/97</u> MELE BLOWS/ Recovery 300mm (%) LAB TESTS USCS ELEV. (m) DEPTH TYPE MATERIAL DESCRIPTION NO. REMARKS (m) SC LIGHT BROWN CLAYEY SAND/SANDY CLAY (SC/CL) moist, medium dense/very stiff, fine grained sand, low to moderate plasticity fines. MC 19 Α 1409.6 1 1.07 SÇ YELLOW BROWN CLAYEY SAND (SC) moist, medium dense, fine to medium grained sand, В МC 19 moderate plasticity lines. Groundwater encountered at 1,83 M. 1408.6 1.98 1407.6 3 3.05 MÇ C 26 1406.6 Lenses of clean sand. D MC 1405.6 1404.6 Н MC 19 Encountered a piece of fine gravel. 6.55 1403.6 MÇ SM YELLOW BROWN SLIGHTLY SAND (SM/SP) wet, dense, line to medium grained sand, estimated 5 to 12% lines. - 9 MC 37 9,60 @1999, by Kleinfelder, Inc.

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LOG OF BORING CARSON FREEWAY

PLATE

PROJECT NO. 30-1348-15.002

EXPLORATION LOG START DATE: 9/25/97 SHEET 1 OF 1 9/25/97 END DATE: STA 282+50 STATION JOB DESCRIPTION CARSON FREEWAY 50 M LEFT OFFSET LOMPA RANCH LOCATION ENGINEER __J. FORGA BORING EQUIPMENT _CME 55 KLEINFELDER 30-1348-15.002 GROUNDWATER LEVEL OPERATOR ANDRESEN E.A. # DATE | DEPTH | ELEV. DRILLING GROUND ELEV_1410.61 m HOLLOW STEM AUGER 9/25/97! 1.5 1409.1 HAMMER DROP SYSTEM STANDARD BACKFILLED YES __ DATE _9/25/97 ELEV. DEPTH USCS BLOWS/ Recovery LAB TESTS MATERIAL DESCRIPTION NO. TYPE (m) REMARKS (m) SC LIGHT BROWN CLAYEY SAND (SC) moist, medium dense, fine grained sand, low plasticity fines. Α MC 17 W,UW,S 28% fines 1409.6 - ¹ 1.07 MC В 55 W.UW Groundwater encountered at 1.5 M. 1408.6 1407.6 3 3.05 Ç MC W,UW,S 16% fines Dense, becoming more sandy. 1406.6 D MC 35 WU,W 1405.6 - 6 <u>6.10</u>

Very dense.

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1404.6

1403.6

1402.6

1401.6

6.55

8 8,08

9

9.60



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MC

MC

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PROJECT NO. 30-1348-15.002

EXPLORATION LOG START DATE: 9/26/97 SHEET 1 OF 1 9/26/97 END DATE: STA 303+50 STATION JOB DESCRIPTION _ CARSON FREEWAY 91 M LEFT OFFSET SOUTH OF U.S. 50 LOCATION J. FORGA ENGINEER BD-21 BORING EQUIPMENT _ CME 55 KLEINFELDER 30-1348-15.002 GROUNDWATER LEVEL **OPERATOR** _ANDRESEN E.A. # DATE DEPTH ELEV. GROUND ELEV 1410.61 m HOLLOW STEM AUGER 1409.4 HAMMER DROP SYSTEM_STANDARD BACKFILLED YES _ DATE 9/25/97 BLOWS/ Recovery ELEV. (m) DEPTH LAB TESTS USCS MATERIAL DESCRIPTION NO. TYPE REMARKS (m)SÇ LIGHT BROWN CLAYEY SAND (SC) moist, dense, line to medium grained sand, low to moderate plasticity fines. MC 34 W,UW,S 32% fines 1409.6 1 1.07 Groundwater encountered at 1.2 M. 1.52 В MÇ 29 Wet 1408.6 1.98 SC 1407.6 GRAY CLAYEY SAND (SC) wet, loose, fine MC 11 grained sand, estimated 20 to 30% fines. К 1406.6 SC GRAY CLAYEY SAND (SC) wet, very dense, fine to medium grained sand, moderate plasticity MC 72 W,UW,\$ 16% fines 1405.6 1404.6 MC W,UW,S,PI Becoming more clayey, Pl= 17 33% fines 6.55 1403.6 MC 50/127 7.92 1402.6 1401.6 9 MC 37 9,60 9.50 @1999, by Kleinfalder, inc. **LOG OF BORING** PLATE KLEINFELDER

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PROJECT NO. 30-1348-15.002

4875 LONGLEY LANE, SUITE 100

RENO, NEVADA 89502 Tel. (702) 589-7800

CARSON FREEWAY

EXPLORATION LOG START DATE: _ 9/26/97 SHEET 1 OF 1 END DATE: 9/26/97 STA 308 + 50 STATION JOB DESCRIPTION _ CARSON FREEWAY OFFSET 118 M LEFT SOUTH OF U.S. 50 LOCATION ENGINEER J. FORGA BORING KLEINFELDER EQUIPMENT _CME 55 30-1348-15.002 GROUNDWATER LEVEL OPERATOR ANDRESEN DATE | DEPTH | ELEV. DRILLING METHOD GROUND ELEV_1410.61 m HOLLOW STEM AUGER 1 1409.4 9/26/97 HAMMER DROP SYSTEM STANDARD BACKFILLED _ YES DATE 9/26/97 AMPLE BLOWS/ Recovery 300mm (%) DEPTH ELEV. LAB TESTS USCS MATERIAL DESCRIPTION (m) (m) REMARKS SC LIGHT BROWN CLAYEY SAND (SC) moist, medium dense, fine to medium grained sand, low to moderate plasticity fines. 25 1409.6 1 1.07 1 22 Groundwater encountered at 1.2 M. SC LIGHT BROWN/GRAY CLAYEY SAND/SANDY CLAY (SC/CL) moist, loose/stiff, fine to medium grained sand, moderate to high plasticity. В MC 13 1408.6 estimated 40 to 60% fines. SC 1407.6 3 3.05 GRAY CLAYEY SANQ (SC) wet, dense, fine to MC 43 medium grained sand, low plasticity fines, estimated 20 to 30% fines. 1406.6 D MC 85/279 1405.6 5 5.03 Very dense, more sandy, estimated 15 to 25% fines. 1404.6 6 MC 41 Dense, more sandy, estimated 15 to 25% fines. 1403.6 SM GBAY SLIGHTLY SILTY SAND (SM/SP) wet, very dense, fine to medium grained sand, estimated 5 MC 50/140 to 12% fines. 1402.6 1401.6 . 9 MC @1999, by Kleinfelder, Inc. PLATE KLEINFELDER

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PROJECT NO. 30-1348-15.002

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LOG OF BORING

CARSON FREEWAY

EXPLORATION LOG START DATE: 9/26/97 SHEET 1 OF 1 9/26/97 END DATE: STATION STA 312+80 JOB DESCRIPTION _CARSON FREEWAY 15 M RIGHT OFFSET SOUTH OF U.S. 50 LOCATION ENGINEER J. FORGA BD-23 BORING EQUIPMENT _ CME 55 KLEINFELDER 30-1348-15,002 **GROUNDWATER LEVEL** OPERATOR ANDRESEN E.A. # DRILLING METHOD GROUND ELEV_ 1411.22 m DATE | DEPTH | ELEV. HOLLOW STEM AUGER 1.8 1409,4 HAMMER DROP SYSTEM STANDARD BACKFILLED YES __ DATE <u>9/26/97</u> SAMPLE
TYPE BLOWS/ Recovery
300mm (%) ELEV. DEPTH LAB TESTS USCS NO. MATERIAL DESCRIPTION (m) (m) REMARKS SC LIGHT BROWN CLAYEY SAND (SC) moist, medium dense, fine to medium grained sand, low to moderate plasticity fines, estimated 20 to 0.61 MC 17 30% fines. Lenses of clean sand 1410.2 16 1409.2 1.98 Groundwater encountered at 1,83 M. 1408.2 3 3.05 MC 49 Gray, dense, more sandy. 1407.2 SM GRAY SUGHTLY SILTY SAND (SM) wet, very dense, fine to medium grained sand, estimated 5 D MC 65/254 1406.2 5 5 03 Lenses of clayey sand. 6 6.10 1405.2 Н MC 47 Some heaving. 6.55 Becoming clayey. 1404.2 MC 42 1403.2 8 8.08 SM GRAY BROWN SILTY SAND (SM) wet, very dense, line grained sand, estimated 10 to 20% 1402.2 . 9 MC 50/140 J 9.45 @1999, by Kleinfelder, Inc.

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CARSON FREEWAY

LOG OF BORING

CARSON CITY, NEVADA

PLATE

E-24

MOISTURE CONTENT AND UNIT WEIGHT DRAINAGE BORINGS

	DEPTH	MOISTURE CONTENT	DRY UNIT WEIGHT
BORING	(meters)	(%)	(kN/cu m)
BD-2	.091	7.7	17.12
BD-2	3.35	5.6	17.12
BD-2	2,44	7.1	18.22
BD-2	4.57	3.5	76mm
BD-2	10.97	9.8	17.43
BD-2	13.72	7.6	19.16
BD-4	0.91	4.0	16.96
BD-4	3.35	16.8	15.86
BD-4	4.88	5.7	16.65
BD-4	6.25	2.3	
BD-4	6.40	7.2	18.22
BD-4	7.92	7.3	18.06
BD-4	9.14	3.8	***
BD-4	9.30	8.0	18.22
BD-4	10.97	8.5	18.85
BD-4	12.19	6.5	18.69
BD-4	12.34	7.5	17.90
BD-4	13.87	7.6	18.69
BD-5	0.91	4.7	17.59
BD-5	3.20	5.6	16.96
BD-5	6.25	4.4	16.96
BD-6	0.91	11.3	14.29
BD-7	0.91	7.6	16.96
BD-7	1.83	13.1	18.22
BD-7	3.35	17.5	17.59
BD-7	4.88	17.6	17.90
BD-8	0.91	29.9	13.98
BD-8	3.35	17.8	16.96
BD-8	4.88	19.8	16.96
BD-8	6.40	12.6	19.00
BD-15	1.83	16.4	17.43
BD-15	4.88	12.2	20.10
BD-15	6.40	14.7	17.12
BD-16	0.91	9.2	16.02
BD-16	1.83	17.7	15.71
BD-17	0.91	10.5	16.02
BD-17	3.35	17.5	17.59
BD-17	4.88	24.3	15.55
BD-18	0.91	17.6	16.18
BD-18	3.35	14.8	18.38
BD-20	0.91	18.6	16.96
BD-20	1.83	13.6	18.69
BD-20	3.35	15.5	17.90

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PROJECT NO. 30-1348-15.002

MOISTURE / DENSITY TABLE

CARSON FREEWAY

CARSON CITY, NEVADA

PLATE

E-25

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	DEPTH	MOISTURE CONTENT	DRY UNIT WEIGHT
BORING	(meters)	(%)	(kN/cu m)
BD-20	4.88	14.2	18.85
BD-21	0.91	16.5	17.59
BD-21	4.88	15.0	19.16
BD-21	6.40	15.9	17.90
BD-10	3.35	21.9	15.71
BD-10	4.88	17.6	17.12
BD-10	9.45	17.2	17.59
BD-14	1.83	14.3	16.96
BD-14	3.35	18.1	15.23
BD-14	4.88	19.2	16.65
BD-14	6.40	20.0	16.02
BD-14	7.92	17.9	16.96
BD-14	9.45	18.3	17.12

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4875 LONGLEY LANE, SUITE 100 RENO, NEVADA 89502 Tel. (702) 689-7800 MOISTURE / DENSITY TABLE
CARSON FREEWAY

L-20

PLATE

CARSON CITY, NEVADA

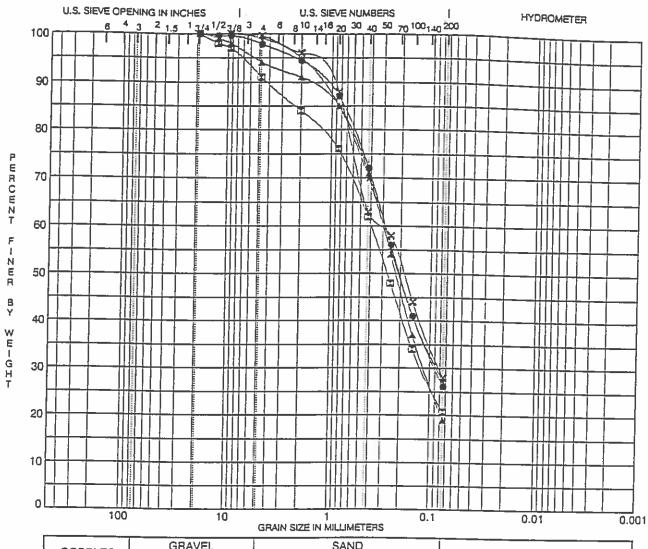
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PROJECT NO. 30-1348-15.002

BEON CITY NEWARA





	COR	BLES		GRAVEL	RAVEL SAND_				SILT OR CLAY						
		0110	coar	se fin	ne co	coarse medium fine SILT OR C					R CLAY	ſ			
	Boring	Depth	(M)	Description - ASTM Classification					МС	% L	L F	PL	PI	Cc	Cu
•	BD-01	at	0.6	R	Red Brow	vn Silt	y Sand (SM)							
<u>E</u>	BD-02	at	0.6	Ļi	Light Brown Silty Sand (SM)										
	BD-03	at	0.6	Ľ	Light Brown Silty Sand (SM)										
*	BD-04	at	0.6	R	Red Brown Silty Sand (SM)										
×	BD-05	at	0.6	Yellow Brown Silty Sand (SM)											
	Boring	Depth	(M)	D100	D60		D30	D10	1 %0	ravel	%Sar	nd	%Silt	1 %	Clay
	BD-01	at	0.6	19.00	0.29		0.090		1 :	2.2	71.7			26.1	
œ	BD-02	_at	0.6	19.00	0.39		0.122		-	2.0	70_3			20.7	
	BD-03	at	0.6	19.00	0.30		0.115				19.0				
*	BD-04	at	0.6	9.50	0.30		0.086	36		.0	71.6			27.4	
x	BD-05	at	0.6	9.50	0.31	0.082 0,4).4	71.6			28.0		

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GRAIN SIZE ANALYSES

CARSON FREEWAY

PLATE

E-26

PROJECT NO. 30-1348-15.002

60

TEST SYMBOL	SAMPLE NO.	SAMPLE (DEPTH)	LIQUID LIMIT	PLASTICITY INDEX	CLASSIFICATION
*	80-1	9.45	31	13	Yellow Brown Clayey Sand (SC) -200=31%
•	BD-8	0.91	47	32	Dark Brown Sandy Clay (CL) -200=63%
•	80-16	0.91	29	10	Light Brown Sandy Clay (CL) -200=50%
A	80-17	0.91	32	12	Light Brown Clayey Sand (SC) -200=46%
	BD-18	0.91	33	17	Light Brown Clayey Sand (SC) -200=43%
X	80-21	6.40	32	17	Gray Clayey Sand (SC) -200=33%

LIQUID LIMIT

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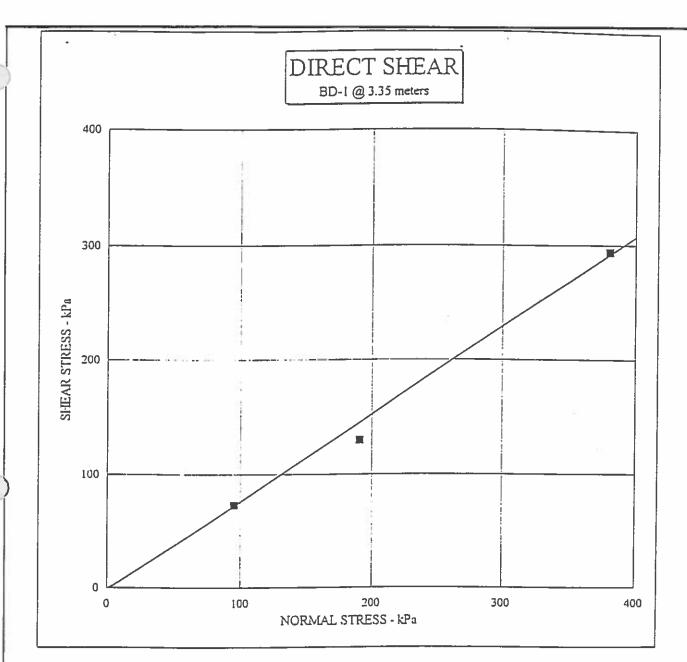
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PLASTICITY CHART CARSON FREEWAY

PLATE

PROJECT NO. 30-1348-15.002



TEST TYPE:	CD/WET/STAGED				
BORING NO:	BD-1				
DEPTH:	3.35 meters				
SOIL DESCRIPTION:	Yellow Brown Sl.Silty Sand				
RATE OF SHEAR:	0.0019 cm/sec				

FRICTION ANGLE:	38
COHESION:	0 kPa

DRY DENSITY - kN/cu m	13.5		
INITIAL WATER CONTENT - %	3.8		
FINAL WATER CONTENT - %	24.9		
NORMAL STRESS - kPa	95	190	380
MAXIMUM STRESS - kPa	73	130	294

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CARSON FREEWAY

CARSON CITY, NEVADA

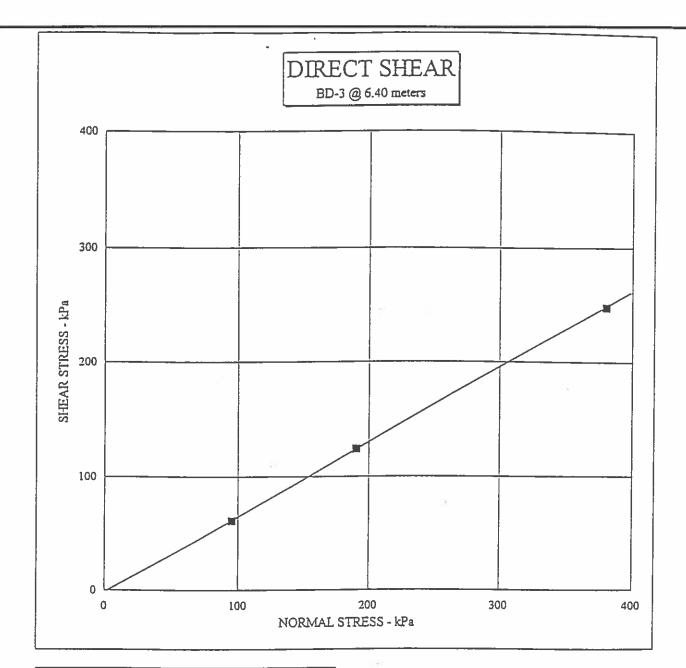
DIRECT SHEAR

PLATE

E-28

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TEST TYPE:	CD/WET/STAGED
BORING NO:	BD-3
DEPTH:	6.40 meters
SOIL DESCRIPTION:	Lt. Brown Silty Sand
RATE OF SHEAR:	0.0019 cm/sec

FRICTION ANGLE:	33
COHESION:	0 kPa

		BORING NO:	Bi	D-3	1	=	
)#G		DEPTH:	6.40 meters			FRICTION AL	ŊĢ
NTES.DWG		SOIL DESCRIPTION:	Lt. Brown Silty Sand			COHESION:	
~		RATE OF SHEAR:	0.0019 cm/sec]		
48\30134815002EPL							
FB15		DRY DENSITY - kN/c	u m	16.51			
0134		INITIAL WATER CONTENT - %		15.5			
18\3				18.8			
5134	NORMAL STRESS - kPa		95	190	380		
TNG\3013		MAXIMUM STRESS -	kPa	61	124	248]
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DIRECT SHEAR

CARSON FREEWAY

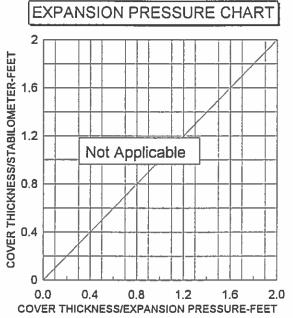
CARSON CITY, NEVADA

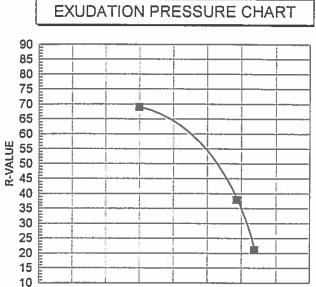
PLATE

PROJECT NO. 30-1348-15.002

Sample Source: BD-1 @ 0-.61 meters

Sample Description: Red Brown Silty Sand





400

EXUDATION PRESSURE (PSI)

200

0

Specimen A		В	С
Exudation Pressure, psi	502	215	164
Expansion Dial (.0001")	0.0000	0.0000	0.0000
Expansion Pressure, psf	0	0	0
Resistance Value, R	69	38	21
% Moisture at Test	8.9	10.6	11.7
Dry Density at Test, pcf	128.4	125.2	119.4
R Value by Expansion Pressure (TI=)	Not App	licable	
R Value at 300 psi Exudation Pressure	54	1	

800

600

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CARSON FREEWAY

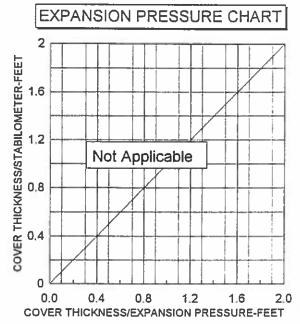
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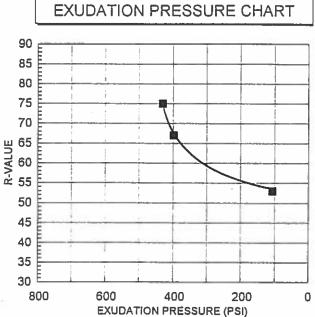
PLATE

PROJECT NO. 30-1348-15.002

Sample Source: BD-2 @ 0-.61 Meters

Sample Description: Light Brown Silty Sand





Specimen	A	В	С
Exudation Pressure, psi	431	399	108
Expansion Dial (.0001")	0.0000	0.0000	0.0000
Expansion Pressure, psf	0	0	0
Resistance Value, R	75	67	53
% Moisture at Test	8.7	9.8	10.2
Dry Density at Test, pcf	125.3	128.9	128.9
R Value by Expansion Pressure (TI=)		Not App	olicable
R Value at 300 psi Exudation Pressure	59	9	

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CARSON FREEWAY

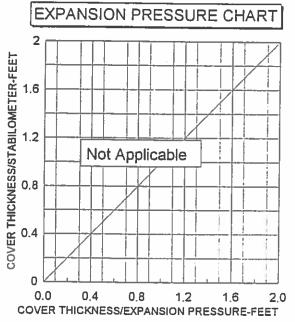
E-31

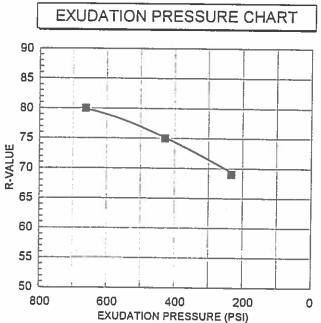
PLATE

PROJECT NO. 30-1348-15.002

Sample Source: BD-3 @ 0-.61Meters

Sample Description: Light Brown Silty Sand





Specimen	A	В	C
Exudation Pressure, psi	662	430	234
Expansion Dial (.0001")	0.0001	0.0000	0.0000
Expansion Pressure, psf	4	0	0
Resistance Value, R	80	75	69
% Moisture at Test	9.3	9.8	10.4
Dry Density at Test, pcf	124.4	122.5	124.8
R Value by Expansion Pressure (TI=)	Not Ap	plicable	
R Value at 300 psi Exudation Pressure 72			2

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CARSON FREEWAY

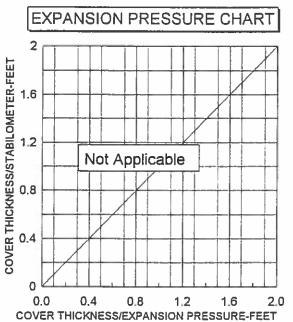
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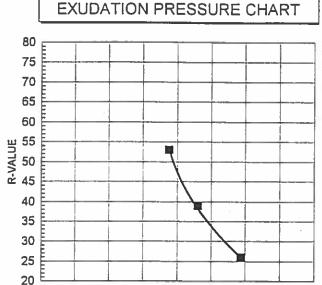
E-32

PROJECT NO. 30-1348-15.002

Sample Source: BD-4 @ 0 - 0.61 meters

Sample Description: Red Brown Silty Sand





400

EXUDATION PRESSURE (PSI)

200

0

Specimen	A	В	С
Exudation Pressure, psi	426	340	214
Expansion Dial (.0001")	0.0000	0.0000	0.0000
Expansion Pressure, psf	0	0	0
Resistance Value, R 53		39	26
% Moisture at Test	10.3	10.9	11.4
Dry Density at Test, pcf	127.1	126.8	124.5
R Value by Expansion Pressure (TI=)	Not App	licable	
R Value at 300 psi Exudation Pressure	34	4	

800

600

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CARSON FREEWAY

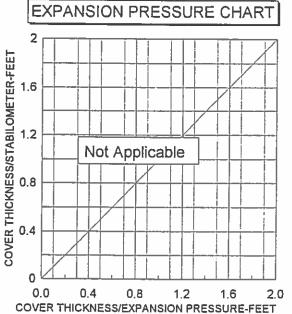
E-33

PLATE

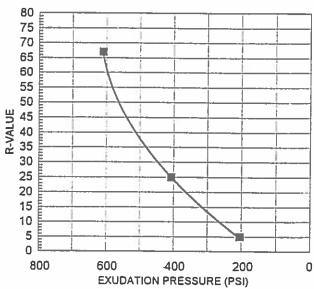
PROJECT NO. 30-1348-15.002

Sample Source: BD-5 at 0 - .61 meters

Sample Description: Yellow Brown Silty Fine Sand



EXUDATION PRESSURE CHART 80 75



Specimen	A	В	С
Exudation Pressure, psi	205	407	611
Expansion Dial (.0001")	0.0000	0.0000	0.0000
Expansion Pressure, psf	0	0	0
Resistance Value, R	5	25	67
% Moisture at Test	11.9	10.8	9.7
Dry Density at Test, pcf	127.2	129.0	
R Value by Expansion Pressure (TI=)	Not App	licable	
R Value at 300 psi Exudation Pressure	12	2	

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CARSON FREEWAY

R-VALUE

PLATE

PROJECT NO. 30-1348-15.002

APPENDIX F

Ramp and Realignment Borings
And
Laboratory Test Results

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USCS GROUP	TYPICAL SOIL DESCRIPTION			
GW	Well graded gravels, gravel-sand mixtures, little or no fines			
GP	Poorly graded gravels, gravel-sand mixtures, little or no fines			
GM	Silty gravels, poorly graded gravel-sand-silt mixtures			
GC	Clayey gravels, poorly graded gravel-sand-clay mixtures			
SW	Well graded sands, gravelly sands, little or no fines			
SP	Poorly graded sands, gravelly sands, little or no fines			
SM	Silty sands, poorly graded sand-silt mixtures			
SC	Clayey sands, poorly graded sand-clay mixtures			
ML	Inorganic silts and very fine sands, rock flour, silty or clayey fine sands with slight plasticity			
CL	Inorganic clays of low to medium plasticity, gravelly clays, sandy clays, silty clays, lean clays			
OL	Organic silts and organic silt-clays of low plasticity			
MH	Inorganic silts, micaceous or diatomaceous fine sandy or silty soils, elastic silts			
CH	Inorganic clays of high plasticity, fat clays			
ОН	Organic clays of medium to high plasticity			
CE	Caliche			
PT	Peat and other highly organic soils			

MOISTURE CONDITION CRITERIA

SOIL CEMENTATION CRITERIA

Description <u>Criteria</u>

Wet

Dry Absence of moisture, dusty, dry to

touch.

Damp, no visible water. Moist

Description Criteria

Crumbles or breaks with handling or Weak

little finger pressure.

Crumbles or breaks with considerable

finger pressure.

Visible free water, usually below Strong Will not crumble or break with finger

Moderate

water table.

pressure.

STANDARD PENETRATION CLASSIFICATION*					
GF	RANULAR SOIL	CLAYEY SOIL			
BLOWS/0.3m	DENSITY	BLOWS/0.3m	CONSISTENCY		
0-4	VERY LOOSE	0-1	VERY SOFT		
5-10 LOOSE		2-4	SOFT		
11 - 30	MEDIUM DENSE	5-8	MEDIUM STIFF		
31 - 50 DENSE		9 - 15	STIFF		
OVER 50 VERY DENSE		16 - 30	VERY STIFF		
*Standard Penetration Test (N) 63.5 Kg hammer		31 – 60	HARD		
760mm free fall on t	50.8mm O.D. x 35mm I.D. sampler.	OVER 60	VERY HARD		

Blow counts on California Split Spoon (Ness) can be converted to N_{spt} by:

 $(N_{csi})(0.563) = N_{spi}$

Blow counts from Automatic Hammer can be converted to

Standard N_{spt} by:

 $(N_{Automatic\ Hammer})(1.33) = N_{spt}$

CD CONSOLIDATED DRAINED O ORGANIC CONTENT CH CHEMICAL (CORROSIVENESS) OC CONSOLIDATION CM COMPACTION PI PLASTICITY INDEX CU CONSOLIDATED UNDRAINED RQD ROCK QUALITY DESIGNATION D DISPERSIVE SOILS RV R-VALUE DS DIRECT SHEAR S SIEVE ANALYSIS/-200 WASH E EXPANSIVE SOIL SL SHRINKAGE LIMIT G SPECIFIC GRAVITY U UNCONFINED COMPRESSION H HYDROMETER UU UNCONSOLIDATED UNDRAINED HC HYDRO-COLLAPSE UW UNIT WEIGHT K PERMEABILITY CONE PENETRATION CS CONTINUOUS SAMPLER(1) MC MODIFIED CA SPLIT SPOON (2) MC MODIFIED CA SPLIT SPOON (2) P PUSHED (NOT DRIVEN) PB PTICHER BARREL RC ROCK CORE(3) SH SHELBY TUBE (4) SPT STANDARD PENETRATION TEST TF TEST PIT (1) I.D. = 82mm with tube; 88.9mm w/s Tube (3) N.XW (3) N.XW	TEST	TABBREVIATIONS			SAMPLER NOTATION
	CD CH CM CU D DS E G H	CONSOLIDATED DRAINED CHEMICAL (CORROSIVENESS) COMPACTION CONSOLIDATED UNDRAINED DISPERSIVE SOILS DIRECT SHEAR EXPANSIVE SOIL SPECIFIC GRAVITY HYDROMETER HYDRO-COLLAPSE	OC PI RQD RV S SL U UU UW	CONSOLIDATION PLASTICITY INDEX ROCK QUALITY DESIGNATION R- VALUE SIEVE ANALYSIS/-200 WASH SHRINKAGE LIMIT UNCONFINED COMPRESSION UNCONSOLIDATED UNDRAINED UNIT WEIGHT	CPT CONE PENETRATION CS CONTINUOUS SAMPLER(1) MC MODIFIED CA SPLIT SPOON (2) P PUSHED (NOT DRIVEN) PB PTICHER BARREL RC ROCK CORE(3) SH SHELBY TUBE (4) SPT STANDARD PENETRATION TEST TP TEST PIT (1) I.D. = 82mm with tube; 88.9mm w/m tube (2) I.D. = 61.5mm

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KEY TO SOIL CLASSIFICATION **AND TERMS**

CARSON FREEWAY

CARSON CITY, NEVADA

PLATE

				CTART	DATE.	5/13/97			EXPLORATION LOG			
	START DATE: 5/13/97									SHEET 1 OF 1		
	END DATE: 5/13/97 JOB DESCRIPTION CARSON FREEWAY						STATION STA 18+0 OFFSET 0					
- [Y							
	LOCATION							ENGINEER _	J. FORGA			
									EQUIPMENT _			
						5.002		GROUNDWATER LEVEL	OPERATOR SPECTRU			
						•		DATE DEPTH ELEV.				
- 1										HOLLOW ST		
	HAMMER DROP SYSTEM_AUTOMATIC						NUTUMATIC			YES DA	TE 5/13/97	
ı	ELEV. DEPTH NO. TYPE BLOWS/Recovery LAB TESTS							ucco				
	(m)	(m)	NO.	TYPE	BLOWS,	Recovery	LAB TESTS	Group	MATERIAL DE	SCRIPTION		REMARKS
ſ						1 1111	Ì	SC				
		[1			LIGHT BROWN CLAYE dense, fine to coarse sa	Y SAND slightly r	noist,	
- 1				1		1	i		surface cobbles, estima	ated 20 to 30% fir	nes.	
ı		L			1	Į		1 1				
	1496.2	Li					1				ļ	
- 1		-	1		1		1					
		1.52			İ			611	1.37			
- 1		- 1002	A	MC	47		W,UW,S	SM	LIGHT BROWN SILTY S	SAND moist, dens	e, fine to	14% fines
-		-	-] -					coarse sand and gravel	decomposed gra	anite.	1470 111162
	1495.2	2 1.98	-	 								
		}		l	İ .							
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		}										
		.										
ı	1494.2	<u>- 3 3.05</u>	В	MC_	50/76				3.20 Very dense.			
		0.50	1	IVIO	30//0						- 5%	
-		i l		l i	,				No free water encounte	rea.		
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									LOGOFE	OPING		PLATE

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KLEINFELDER 4875 LONGLEY LANE, SUITE 100 RENO, NEVADA 89502 Tel. (702) 689-7800

PROJECT NO. 30-1348-15.002

CARSON FREEWAY

SM SP BROWN SLIGHTLY SILTY SAND slightly moist, loose fine to coarse grained sand. 1475.5 _ 1 1474.5 _ 2 1.98	ELEV.	DEPTH		S	AMPLE	/ Hecovery	LAB TESTS	USCS		MATERIAL DESCRIPTION	REMARKS
1475.5	(m)	(m)	NO.	TYPE	300000	(%)	5.5 12515				REWARK.
1474.5	1475.5	1								loose line to coarse grained sand.	
473.5 3 3.05 SP YELLOW BROWN CLEAN SAND moist, loose, fine to medium grained sand. 3% fines 3.51 No free water encountered. 471.5 -5 -6 469.5 -7 468.5 -8	474.5		Α	MC	7		w.uw,s			Fine to medium grained sand.	10% fines
A72.5	473.5	3 3.05	В	MC_	9		s	SP	,	YELLOW BROWN CLEAN SAND moist, loose line to medium grained sand.	3% fines
470.5	472.5	4								No free water encountered.	
469.5 _ 7 _ 8 _ 8 _ 8 _ 8 _ 8 _ 8 _ 8 _ 8 _ 8	471.5	_5					:				
\$68.5 _ 8	470.5	- - 6 -			:		·				
	¥69.5	- - - 7									
67.5 _ 9	¥68.5	- _ 8					:				
	467.5	- _9 -									

EXPLORATION LOG

GROUNDWATER LEVEL

DATE | DEPTH | ELEV.

STA 8+00

BACKFILLED YES DATE 5/15/97

HOLLOW STEM AUGER

PLATE

0

ENGINEER J. FORGA

OPERATOR SPECTRUM

EQUIPMENT __CME 55

STATION

DRILLING METHOD

LOG OF BORING

CARSON FREEWAY

CARSON CITY, NEVADA

OFFSET

START DATE: __5/15/97

GROUND ELEV 1476.45 m

END DATE:

LOCATION

BORING

E.A. #

KLEINFELDER

5/15/97

BR-02

HAMMER DROP SYSTEM AUTOMATIC

JOB DESCRIPTION CARSON FREEWAY

EAST RAMP U.S. 395

30-1348-15.002

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PROJECT NO. 30-1348-15.002

KLEINFELDER 4875 LONGLEY LANE, SUITE 100 RENO, NEVADA 89502 Tel. (702) 689-7800

						= 14= 10=			EXPLORATION LOG			
Ì				START DATE: 5/15/97 END DATE: 5/15/97 JOB DESCRIPTION CARSON FREEWAY LOCATION EAST RAMP U.S. 395								SHEET 1 OF 1
	100	100	335							STATION _	STA 3+00	
			4					AY	71	OFFSET 0		
	_ 100		SHE							ENGINEER _ EQUIPMENT _	J. FORGA	
	KI	LEINFELDI	ER	BORING BR-03				—	COOLINGUATED LEVEL	SPECTRUM		
				E.A. # 30-1348-15.002					DATE DEPTH ELEV.			
				GROUND ELEV_1469.14 m							HOLLOW S	
				HAMMER DROP SYSTEM_AUTOMATIC					BACKFILLED YES (TE_5/15/97
	ELEV.	DEPŢH	NO.	TYPE BLOWS/ Recovery LAB TESTS US				USCS	MATERIAL DE	SCRIPTION		REMARKS
	(m)	(m)	1.10.	1	300mm	(0%)	10	SM		i i		
		0.30	Α	GRAB		40 9	ISRV	-	RED BROWN SILTY SA coarse grained sand a	ND moist, loose, fines to		27% fines
		0.91		GIV.5					3,2,,,,,	81	27 To liftes	
ľ	1468.1	- 1				1						
		1.52	В	MC	12		W.UW,S	1				11% fines
	1467.1	2 1.98	98				1 08 Fine to medium graine	d sand, trace of f	V205			
	1407.1			8					gravel. No free water encountered.			
		}						i l	No nee water encounts	reu.	į	
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	1466.1	_3		- 0								
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	1464.1	_5										
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	}	- [1						
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	1460.1	_9				- 1						
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199	and of results and								1000	DOBING.		PLATE
)	KLEINFELDER						ER		LOG OF	BUHING		FORTE
		Ų.							CARSON F	DEEMAY		
	4875 LONGLEY LANE, SUITE 100 REMO, NEVADA 89502 Tel. (702) 689-7800								OAI 10014 (-4		

CAD FILE: L:\1999\pRAFTING\3D1348\30134815002FPLATES.DWG

PROJECT NO. 30-1348-15.002

	1470.2	<u> 5 5.03</u>	-					\dashv	becoming more sandy, medium dense
	1469.2	- 6 6.10	F	MC	19		w.uw.s		Fine to coarse grained sand.
	1468.2	7						sc	BROWN CLAYEY SAND moist, fine to m grained sand, low plasticity fines.
PLATES.DWG	1467.2	7.62 - 8 8.08	G	MC	24		w.uw.s		8.08 No free water encountered.
99) ARAFING\301348\30134815002FPLATES.DWG	1466.2	_ _ _ 9					132		
AFTING\30134	by Kleinfeld	der, Inc.			- 23		_		
00	7	T I	(L	EIN	IFE	LD	ER		LOG OF BORING

START DATE: ____5/13/97

GROUND ELEV_ 1475.23 m

SAMPLE
TYPE | BLOWS/ Recovery

6

4

9

21

END DATE:

LOCATION

BORING

E.A. #

MC

MC

MC

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MC

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В

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1,98

3 3.05

3.66

KLEINFELDER

DEPTH

ELEV.

1474.2

1473.2

1472.2

1471.2

5/13/97

BR-04

HAMMER DROP SYSTEM AUTOMATIC

30-1348-15.002

JOB DESCRIPTION CARSON FREEWAY

WEST RAMP U.S. 395

LAB TESTS USCS

W,UW,S

S.PI

W,UW

SM

SM SP

DAT	E DEPTH ELEV	DRILLING METHOD	HOLLOW	STEM AUGER
		BACKFILLED	YES	DATE 5/13/97
	MATERIAL DE	SCRIPTION		REMARKS
2.91	BROWN SILTY SAND in medium grained sand, sand, estimated 20 to 3	some surface of	e to coarse	
	BROWN SUGHTLY SIL fine to medium grained	TY SAND mois	i, laose,	8% fines
	Non-plastic		12	10% fines
	Becoming more sandy,	medium dense	3.	
	Fine to coarse grained s	sand.		6% fines
86	BROWN CLAYEY SAND grained sand, low plasti	moist, fine to r	nedium	

SHEET 1 OF 1

STA 15+00

__J. FORGA

Q

EQUIPMENT CME 55

OPERATOR SPECTRUM

STATION

OFFSET

ENGINEER

EXPLORATION LOG

GROUNDWATER LEVEL

4875 LONGLEY LANE, SUITE 100 RENO, NEVADA 89502 Tel. (702) 689-7800

CARSON FREEWAY

CARSON CITY, NEVADA

PLATE

14% fines

L:\1999) FILE

START DATE: 5/13/97

5/13/97 END DATE:

JOB DESCRIPTION __CARSON FREEWAY

LOCATION

WEST RAMP U.S. 395

BORING 30-1348-15.002 E.A. #

GROUND ELEV 1470.36 m

HAMMER DROP SYSTEM AUTOMATIC

EXPLORATION LOG

GROUNDWATER LEVEL

DATE DEPTH ELEV.

SHEET 1 OF 1 STA 10+00

STATION 0 OFFSET _

ENGINEER J. FORGA

EQUIPMENT _CME 55

OPERATOR _ SPECTRUM

HOLLOW STEM AUGER

BACKFILLED YES DATE 5/13/97

	<u>.</u>				YSTEM			_	BACKFILLED TES DA	(16 07 19:57
ELEV.	DEPTH (m)	NO.	TYPE	AMPLE BLOWS/ 300mm	Recovery	LAB TESTS			MATERIAL DESCRIPTION	REMARKS
							SM		LIGHT BROWN SILTY SAND slightly moist, loose, fine to medium grained sand, estimated 15 to 25% fines, some fine surface gravel.	
1469.4	- 1									
	1.52	A	MÇ	7		oc			**	
1468.4	2 1.98 2.13	В	мс	6				i	İ	
	2.59						SC	274	PROUNT OF AVEN AND AND AND AND AND AND AND AND AND AN	
1467.4	3 3,05	С	МС	15		W,UW,S,PI		3.51	RROWN CLAYEY SAND moist, medium dense, fine to medium grained sand, low plasticity fines.	PI=3 22% fines
1466.4	4								No free water encountered,	
1465.4	_5					į				
1464.4	_6									
									†	
1463.4	_7								1 1 1	
1462.4	_8					İ	ĺ			
1461.4	9									
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CARSON CITY, NEVADA

LOG OF BORING CARSON FREEWAY

PLATE

EXPLORATION LOG START DATE: 5/13/97 SHEET 1 OF 1 5/13/97 END DATE: STA 6+50 STATION JOB DESCRIPTION __CARSON FREEWAY 0 **OFFSET** WEST RAMP U.S. 395 J. FORGA LOCATION ENGINEER **BR-06** EQUIPMENT _CME 55 **BORING** KLEINFELDER OPERATOR SPECTRUM 30-1348-15.002 GROUNDWATER LEVEL E.A. # DRILLING METHOD DATE | DEPTH | ELEV. GROUND ELEV 1466.09 m HOLLOW STEM AUGER HAMMER DROP SYSTEM AUTOMATIC BACKFILLED YES DATE 5/13/97 SAMPLE BLOWS/ Recovery LAB TESTS ELEV. DEPTH MATERIAL DESCRIPTION NO. TYPE REMARKS (m) (m) SM LIGHT BROWN SILTY SAND slightly moist, loose, fine to coarse grained sand. GRAB S,RV 22% fines 1465.1 SP LIGHT BROWN CLEAN SAND moist, loose, fine to coarse grained sand. MC 8 W,UW,S 4% fines 1464.1 No free water encountered. 1463.1 L3 1462.1 L 4 1461.1 _ 5 1460.1 L 6 1459.1 L 7 1458.1 _ 8

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1457.1 L 9

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CARSON FREEWAY

LOG OF BORING

CARSON CITY, NEVADA

PLATE

START DATE: __5/14/97

END DATE:

LOCATION

BORING

E.A. #

30-1348-15.002

5/14/97

JOB DESCRIPTION CARSON FREEWAY

NORTH END IMUS REALIGNMENT

EXPLORATION LOG

GROUNDWATER LEVEL

DATE | DEPTH | ELEV. |

STATION

STA 22+00

SHEET 1 OF 1

OFFSET

ENGINEER J. FORGA

EQUIPMENT _CME 55

OPERATOR SPECTRUM

HOLLOW STEM AUGER

GROUND ELEV 1466.09 m

HAMMER DROP SYSTEM AUTOMATIC

BACKFILLED YES DATE 5/14/97

							OTOMATIO		BACKFILLED YES D	ATE 5/14/97
E	LEV. (m)	DEPTH (m)	NO.	TYPE	AMPLE BLOWS/ 300mm	Recovery	LAB TESTS	USC:	MATERIAL DESCRIPTION	REMARKS
		-						SC	0.21 Approx. 8 cm of AC, 15 cm of Aggregate Base BROWN CLAYEY SAND moist, medium dense, fine to coarse grained sand, estimated 20 to 30% fines, low plasticity fines.	
14	65.1	1 1_52_	A	МС	22		w.uw.s	SM	BROWN SILTY SAND moist, medium dense, fine to medium grained sand.	13% fines
14	64.1	2 1.98							2.74 Becoming bore sandy	
146	53.1	3 3.05 - 3.51	8	MC	13		w.uw.s	SM SP	BROWN SLIGHTLY SILTY SAND moist, medium dense, fine to medium grained sand.	10% fines
146	52.1	_ 4							No free water encountered.	
146	51.1	_5				A		:		
146	50.1	- 6 - 6				To the second se				
145	i9.1	- 7 				15				
145	8.1	_ 8			O California	i				
145	7.1	_ 9			99 47					
	- 1					1				

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LOG OF BORING CARSON FREEWAY

PLATE

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PROJECT NO. 30-1348-15.002

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								EXPLORATION LOG			
			START		5/13/97						SHEET 1 OF
			END D	~ -	5/13/97				STATION	STA 18+0	0
			JOB DE	ESCRIPTI		SON FREEWA	AY .		OFFSET	0	 -
			LOCAT	ION _		ALIGNMENT				_J. FORGA	
к	LEINFELD	ER	BORIN	G _	BR-08				EQUIPMENT		
			E.A. #	-	30-1348-1			DATE DEPTH ELEV.	OPERATOR .	<u> </u>	ENI
1					1458.77 ก			DATE DEPTH ECEV.	DRILLING METHOD .		STEM AUGER
						AUTOMATIC			BACKFILLED .	YES	DATE <u>5/13/97</u>
ELEV.	DEPTH (m)	NO.	TYPE	AMPLE BLOWS 300mm	Recover	LAB TESTS	Groot	MATERIAL DE	SCRIPTION		REMARKS
	0.30	1	-				SM	BROWN SILTY SAND	noist, loose, fine	to coarse	1
	-	Α	GRAB	1		RV,S	1	grained sand, some fir estimated 15 to 25% fir	ie surface gravel	l,	20% fines
	į.	1						estimated 15 to 25 e in	103		
1457.8	0.91						-				
	-		[1 22 Becoming more sandy	8		
	1.52				<u> </u>		SM	BROWN SLIGHTLY SIL	TY_SAND moist	loose to	
	Į	В	MC	B		w.uw,s		medium dense, fine to	coarse grained	sand.	10% fines
1456.8	2 1.98			ļ	1						
	}		1		İ						
	ŀ										
1455.8	3 3.05						}				
	-	С	MC	14		W,UW,S		Occasional coarse grav	rel_		6% lines
	3.51					!		3.51			
Ì	[l				No free water encounte	red.		
1454.8	4	i			!						
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1453.8	_ 5										
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1451.8	-7	-					ĺ			Ī	
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4875 LONGLEY LANE, SUITE 100 RENO, NEVADA 89502 Tel. (702) 689-7800 LOG OF BORING
CARSON FREEWAY

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PLATE

CARSON CITY, NEVADA

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				DATE: _		_					SHEET 1 OF 1
			END DA		5/14/97				01/11/01/	A 15+00	
	V=		JOB DE	SCRIPTIC	/·· —	ON FREEWA	\Y		OFFSET 0		
		•	LOCATI			LIGNMENT				FORGA	
K	LEINFELDE	ΞA	BORING	· _	BR-09				EQUIPMENT _CM		
			E.A. #	-	30-1348-1			GROUNDWATER LEVEL	OPERATOR SPE	<u> CTRUM</u>	
			GROUN	D ELEV_	1456.33 m			DATE DEPTH ELEV	DRILLING METHOD <u>HO</u>	LLOW ST	EM AUGER
			HAMME	R DROP	SYSTEM	AUTOMATIC			BACKFILLED YES	3 DA	TE 5/14/97
ELEV.	DEPTH	_	S	AMPLE IRLOWS	Recovery	LAB TESTS	Juscs	MATERIAL DE	SCRIPTION		PEMARKS
(m)	(m)	NO.	TYPE	300mm	(m)	1	SM	141/11/21/1/202		1	11247/1113
İ	0.30					0.51	31111	BROWN SILTY SAND r to coarse grained sand	moist, medium dense	t, fine	
	ŀ	Α	GRAB	į		S,RV		coarse surface gravel.		Some	20% fines
	[<u> </u>							
1455.3	0.91			 			1				
	-				-						
	1.52							(2)			1.0
		В	MC	14		w.uw,s		Becoming more sandy	, with gravel.		15% fines
1454.3	_2_1.98			ļ	<u> </u>			2.13		i	
							SM SP				
	}			1			SP	LIGHT BROWN SUGH medium dense, fine gr		St.	
	İ			-	-			28.1			
1453.3	3 3.05									-	
	-	C	MC	14		w,uw,s					8% fines
	3,51							3.51			
							ĺ	No free water encounte	ered		
1452.3	_4										
	-						i				
	-										
	<u> </u>										
1451.3	_5										
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1770.3											
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RENO, NEVADA 89502
Tel. (702) 689-7800

LOG OF BORING CARSON FREEWAY

CARSON CITY, NEVADA

PROJECT NO. 30-1348-15.002

F-10

PLATE

START DATE: 3/14/3/	START	DATE:	5/14/97
---------------------	-------	-------	---------

END DATE: 5/14/97

JOB DESCRIPTION __CARSON FREEWAY

IMUS REALIGNMENT, NEAR ARROWHEAD

BORING BR-10

LOCATION

E.A. # 30-1348-15.002 GROUND ELEV 1447.80 m

HARAMAED DEOD SYSTEM AUTOMATIC

EXPLORATION LOG

GROUNDWATER LEVEL

DATE | DEPTH | ELEV.

STATION STA5+50

TATION SI OFFSET 0

ENGINEER J. FORGA

EQUIPMENT _CME 55

OPERATOR SPECTRUM

SHEET 1 OF 1

DRILLING METHOD

HOLLOW STEM AUGER

					STEM_AUTOMATIC		BACKFILLED YES DATE 5/14/97
ELEV.	DEPTH (m)	NO.	TYPE	AMPLE BLOWS/ Re 300mm	covery LAB TESTS	Giogle	MATERIAL DESCRIPTION REMARKS
	0.30	A	GRAB		S,PI,RV	sc	### BROWN CLAYEY SAND moist, medium dense, fine to coarse grained sand, fine surface gravel, estimated 20 to 30% fines, low plasticity fines. 31% fines
1446.8	-					CM	1 22
	1.52	В	мс	13	w.uw.s	SM SP	YELLOW BROWN SLIGHTLY SILTY SAND moist, medium dense, fine to coarse grained sand. 8% fines
1445.8	1.98						
1444.8	3 3.05	С	MC	48	W,UW,S,PI		Light brown, non-plastic. 8% fines
	3.51						No free water encountered.
1443.8	_ 4						
			450				
1442.8	_5						
U.	-						
1441.8	- 6						
1441.0	-						
1440.8	_ 7						
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1439.8	8						
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CARSON FREEWAY

LOG OF BORING

PLATE

F-11

PROJECT NO. 30-1348-15.002

START DATE: __5/14/97

5/14/97 END DATE:

JOB DESCRIPTION __CARSON FREEWAY

NE RAMP COLLEGE PARKWAY LOCATION

BORING

BR-11

E.A. #

30-1348-15.002

GROUND ELEV 1429.51 m

HAMMER DROP SYSTEM_AUTOMATIC

EXPLORATION LOG

GROUNDWATER LEVEL

5-14-97 1.7 1427 8

DATE DEPTH ELEV.

SHEET 1 OF 1

HOLLOW STEM AUGER

STA 3+00 STATION

0 OFFSET

ENGINEER J. FORGA

EQUIPMENT _ CME 55

OPERATOR

SPECTRUM

DRILLING METHOD

BACKFILLED YES DATE 5/14/97

						0701174110		BACKFILLED YES DA	
ELEV. (m)	DEPTH (m)	NO.	TYPE	BLOWS/ 300mm	Recovery	LAB TESTS	USCS	MATERIAL DESCRIPTION	REMARKS
							SC	BROWN CLAYEY SAND moist, medium dense, fine to medium grained sand, low to moderate plasticity fines.	
1428.5	- 1 -								
1427.5	1.52 2 1.98	A	мс	19		w,uw,s		Yellow brown Encountered groundwater at 1.7 M. Becoming more sandy.	20% fines
							SP	YELLOW BROWN CLEAN SAND wet, dense, fine to coarse grained sand.	
1426.5	_ 3 3.05	В	MC	31		w.uw,s			3% fines
1425.5	3.51		i					3.51	
1424.5	_5								
1423.5	- 6								
1422.5	_7					į			
1421.5	- - - 8								
1420.5	9								

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PROJECT NO. 30-1348-15.002

LOG OF BORING CARSON FREEWAY

CARSON CITY, NEVADA

PLATE

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COLON SELECT	START DATE:	5/14/97 5/14/97		EXPLORATION LOG			SHEET 1 OF
KLEINFELDER	END DATE: JOB DESCRIPT LOCATION BORING	ION CARSON	I FREEWAY DLLEGE PARKV	VAY	OFFSET _C	J. FORGA	
REENTELDER	E.A. # GROUND ELEV HAMMER DROI			GROUNDWATER LEVEL	DRILLING	SPECTRUM HOLLOW STE PES DATI	M AUGER 5/14/97
LEV. DEPTH NO.	SAMPLE TYPE BLOW	S/Recovery U	AB TESTS USC	MATERIAL DE	ESCRIPTION		REMARKS
			sc	LIGHT BROWN CLAY			

	l	i	S	AMPLE		<u> </u>	Lucas		
ELEV. (m)	DEPTH (m)	NO.	TYPE	AMPLE BLOWS/ 300mm	Recovery (%)	LAB TESTS		MATERIAL DESCRIPTION REMARK	S
							SC	LIGHT BROWN CLAYEY SAND slightly moist, medium dense, fine to coarse grained sand, low plasticity fines.	
1430.0	_ 1								
1429.0	1,52 - 2 1,98	A	мс	22		W,UW,S,PI		Moist, fine to medium grained sand. 14% fine Encountered groundwater at 1,82 M. Non-plastic.	s
	-								
1428.0	_ 3 3,05	В	MC	7					
	3.51 - 3.56							Loose.	
1427.0	- 4 - 4 4.11	С	MC	21		oc		Fine grained sand.	
							SM SP	YELLOW BROWN SLIGHTLY SILTY SAND wet.	
1400.0	4.57	D	мс	32		w,uw,s	25	dense, fine to medium grained sand, estimated 5 to 12% fines.	
1426.0	<u>5 5,03</u>			-					
1425.0	- - 6 _{6.10}				į				
		E	MC	32		W.UW	SC	6.25 BROWN CLAYEY SAND wet, dense, fine to	
	6.55							fines, low to moderate plasticity fines.	
1424.0	7		!						
1423.0	_ 8					į			
25.5									
1422.0	_ 9							; ;	
-									

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LOG OF BORING CARSON FREEWAY

CARSON CITY, NEVADA

PLATE

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START DATE: __5/14/97

5/14/97 END DATE:

JOB DESCRIPTION CARSON FREEWAY

NW RAMP COLLEGE PARKWAY LOCATION

BA-13 BORING

30-1348-15.002 E.A. # GROUND ELEV 1430.43 m

HAMMER DROP SYSTEM AUTOMATIC

EXPLORATION LOG

GROUNDWATER LEVEL

DATE | DEPTH | ELEV.

SHEET 1 OF 1

STATION

STA 1+00 60 M EAST

OFFSET ENGINEER

J. FORGA

EQUIPMENT CME 55 OPERATOR SPECTRUM

DRILLING METHOD

HOLLOW STEM AUGER

BACKFILLED YES DATE 5/14/97

							UTOMATIC		BACKFILLED YES DA	TE 5/14/97
Ε	LEV.	DEPTH (m)	NO.	TYPE	AMPLE BLOWS/ 300mm	Recovery	LAB TESTS		MATERIAL DESCRIPTION	REMARKS
		0,30	A	GRAB			S,RV	sc	LIGHT BROWN VERY CLAYEY SAND moist, medium dense, fine grained sand, low plasticity fines.	46% fines
143	29.4	0.91								
14;	28.4	1.52	8	МС	15		W,UW,S,PI		Becoming more sandy. Non-plastic.	20% fines
		-							No free water encountered.	
143	27.4	_3								
142	26.4	- _ 4								
142	25.4	- - _5								28
		-								
142	24.4	_ 6 -								
142	3.4	- - - 7				9				
142	2.4	_ 8								
142	1.4	9								

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LOG OF BORING CARSON FREEWAY

CARSON CITY, NEVADA

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	Δ					SON FREEWA		Y		OFFSET 0 ENGINEER J.I	FORGA		
	. 4	375	LOCATI		BR-14	COLLEGE F/	THE VAN			EQUIPMENT _CA			
K	LEINFELDI	EA	BORING	· –	30-1348-1	5.002		GROUNDWATER	LEVEL	OPERATOR SP			
			E.A. #	- 	1430.43 гг			DATE DEPTH		DRILLING		MAUGER	
			HAMME	E DRUB	SYSTEM /	AUTOMATIC				BACKFILLED _YE			
							<u> </u>			571074 16465 2222			
ELEV.	DEPTH (m)	NO.	TYPE	BLOWS 300mm	Recovery	LAB TESTS	Group	MATE	RIAL DE	SCRIPTION		REMARKS	
	0.30						SM	LIGHT BROY	WN SILTY	SAND slightly moist,			
	- 0.30	A	GRAB			S,RV	1	dense, fine t	o coarse g	rained sand.		30% fines	
	t		1										
1429.4	0.91					-	1				-		
0.0	}										1		
	1.52	В	140	75	<u> </u>	W.UW.S	.	Moist very d	lense fine	to medium grained:	sand	109 6	
	[.	-	MC	75	1	VV,UVV,S		weakly ceme	ented.	E (3)	Janu,	10% fines	
1428.4	2 1.98						1	Cellaes of Cit	saner same	4.	Ì		
	ţ							2.44					
	-						SC		VN CLAYE	Y SAND moist, medi	um		
1407.4	-				ļ				a medium	grained sand, low	22		
1427.4	3 3.05	С	MC	20		S,PI		plasticity fine	73.			Pi=3	
	3.51					***		3.51			1	14% fines	
			1				П	No free wate	r encount	ered.			
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1425.4	5												
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by Meinteld													
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	_ '`		75 LONGLE	Y LANE, !	SUITE 100			CAD	CON T	DEEMAY			
			RENO.	NEVADA 89 12) 689-7	3502			CAR	SUN i	REEWAY			
			imi (vi				1						

EXPLORATION LOG

START DATE: __5/14/97

PROJECT NO. 30-1348-15.002

CARSON CITY, NEVADA

SHEET 1 OF 1

START DATE: __5/14/97

5/14/97 END DATE:

JOB DESCRIPTION __CARSON FREEWAY

SW RAMP COLLEGE PARKWAY

LOCATION BR-15 **BORING**

30-1348-15.002 E.A. #

GROUND ELEV 1431.65 m HAMMED DOOD EVETEN AUTOMATIC **EXPLORATION LOG**

GROUNDWATER LEVEL

DATE DEPTH ELEV.

STA 12+00 STATION

SHEET 1 OF 1

OFFSET

0 ENGINEER J. FORGA

EQUIPMENT _CME 55

OPERATOR _SPECTRUM

DRILLING METHOD

HOLLOW STEM AUGER

-				HAMME	R DROP S	SYSTEM_A	UTOMATIC		BACKFILLED YES D	ATE 5/14/97
Ì	ELEV.	DEPTH (m)	NO.	TYPE	AMPLE BLOWS/	Recovery	LAB TESTS	USCS	MATERIAL DESCRIPTION	REMARKS
	1430.6	_ 1						SM	LIGHT BROWN SILTY SAND slightly moist, very dense, fine to coarse sand and gravel, cobbles up to 30 cm.	
	1429.6	1.52	A	MC	50/127		w.uw.s		Gray	15% fines
\$	1428.6	3 3.05 3,20	В	MC	50/51					
	1427.6	4,57		8				:		
	1426.6	5	С	MC_	50/51		w.uw.s		No free water encountered.	22% fines
	425.6	- 6								
	424.6	7								
1	423.6	8				s dame on command only distribute comme case and				
1	422.6	_9				- think the				

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LOG OF BORING CARSON FREEWAY

PLATE

PROJECT NO. 30-1348-15.002

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LOG OF BORING **CARSON FREEWAY**

CARSON CITY, NEVADA

PLATE

START DATE: 5/14/97

END DATE: 5/14/97

JOB DESCRIPTION CARSON FREEWAY

LOCATION

SW RAMP COLLEGE PARKWAY

BORING

BR-17

E.A. # 30-1348-15.002 GROUND ELEV 1430.12 m

HAMMER DROP SYSTEM AUTOMATIC

EXPLORATION LOG

GROUNDWATER LEVEL

DATE | DEPTH | ELEV.

SHEET 1 OF 1

STATION STATION OFFSET 0

ENGINEER J. FORGA

EOUIPMENT CME 55

OPERATOR __SPECTRUM

PILLING

HOLLOW STEM AUGER

BACKFILLED YES DATE 5/14/97

- 1							UTOMATIC		BACKFILLED YES DATE 5/14/9	7
	ELEV.	DEPTH (m)	NO.	TYPE	MPLE BLOWS/ 300mm	Recovery	LAB TESTS	USCS	MATERIAL DESCRIPTION REMARK	s
	1429.1	1						SM	LIGHT BROWN SILTY SAND slightly moist, medium dense, fine to coarse grained sand.	
	1428.1	1,52	А	MC	28		W.UW,S		19% fine	s
	1427.1	3_3.05								
	1426.1	3.51	В	MC	14		w,üw,s		Yellow brown, fine to medium grained sand. 18% fine 3.51 No free water encountered.	s
	1425.1	5							Ā	
ŀ		-								
	1424.1	_ 6 - -				1	1			
		7								
	1422.1	_8				4				
	1421.1	_9			· · · · · · · · · · · · · · · · · · ·	epi erek e rente sama per dikin				
L		<u> </u>								

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LOG OF BORING

CARSON CITY, NEVADA

PLATE

F-18

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START DATE: __5/15/97

5/15/97

END DATE:

JOB DESCRIPTION __CARSON FREEWAY

ARROWHEAD REALIGNMENT LOCATION

BORING

30-1348-15.002 E.A. #

GROUND ELEV 1447.19 m HAMMER DROP SYSTEM AUTOMATIC **EXPLORATION LOG**

GROUNDWATER LEVEL

DATE | DEPTH | ELEV.

5-14-97 1.5 1445,7

STA 5+00 STATION

0

J. FORGA **ENGINEER**

EQUIPMENT _ CME 55

OPERATOR _SPECTRUM

DRILLING METHOD

OFFSET

HOLLOW STEM AUGER

SHEET 1 OF 1

BACKFILLED YES DATE 5/15/97

ELEV. (m)	DEPTH (m)	NO.	TYPE	BLOWS/	Recovery	LAB TESTS		MATERIAL DESCRIPTION REMARKS
1.111	0.30	А	GRAB			S,RV	SC	BROWN CLAYEY SAND slightly moist, medium dense, fine to medium grained sand, low 28% fines
446.2	0.91							plasticity fines.
	1.52							
145.2		В	мС	15		W,UW,S,PI		Encountered groundwater at 1.5 M. 11% fines Becoming more sandy, moist. Non-plastic. Lenses of cleaner sand.
								Red brown, fine to coarse grained sand.
144.2	_ 3 3,05 - 3.51	С	мС	27		w,uw,s		13% fines
43.2	4							No free water encountered.
					1			
42.2	_ 5 -				2 A A A A A A A A A A A A A A A A A A A			
141.2	_6			,	A G C C C C C C C C C C C C C C C C C C			
40.2	- - - 7							
39.2	- - - 8				9			
					1	1		
38.2	_9							
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CARSON FREEWAY

LOG OF BORING

CARSON CITY, NEVADA

PLATE

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START DATE: __5/15/97

5/15/97 END DATE:

JOB DESCRIPTION CARSON FREEWAY ARROWHEAD REALIGNMENT

LOCATION BORING

30-1348-15.002 E.A. # GROUND ELEV_ 1446.58 m

DOD EVETEM AUTOMATIC

EXPLORATION LOG

GROUNDWATER LEVEL

DATE | DEPTH | ELEV.

5-15-97 1.2 1445.4

STA 2+00 STATION

0 OFFSET

ENGINEER J. FORGA

EQUIPMENT _ CME 55 OPERATOR _SPECTRUM

HOLLOW STEM AUGER

SHEET 1 OF 1

			S	AMPLE			ucon		
ELEV. (m)	DEPTH (m)	NO.	TYPE	BLOWS/	Recovery	LAB TESTS	Groot	MATERIAL DESCRIPTION	REMARKS
			9			33 W	SC	BROWN CLAYEY SAND moist, medium dense	
	0.30	Α	GRAB			S.RV		BROWN CLAYEY SAND moist, medium dense, fine to medium grained sand, low plasticity fines.	21% fines
						053			
1445.6	0.91								
1443.0								ĺ	
						Č.		Encountered groundwater at 1.2 M.	
	1.52	В	MC	24		W,UW,S			19% fines
1444.6	2 1.98							X.	
1444.0			1000				3		
1	-							1	
						i			
1443.6	3 3,05	c	MC	23		W.UW.S	2	Becoming more sandy	19% fines
[3.51							3.51	
	- 3.51				Ži.				
	. 1				- 8			.	
1442.6	- 4				- 1			**	
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1441.5	_ 5				, i			1	
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1439.6	-7								
420.0	,				- 1	}	1		
1438.6	- 8			ĺ		İ			
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LOG OF BORING **CARSON FREEWAY**

CARSON CITY, NEVADA

PLATE

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5/15/97

END DATE:

JOB DESCRIPTION CARSON FREEWAY

LOMPA REALIGNMENT LOCATION BR-20

BORING E.A. #

30-1348-15,002

EXPLORATION LOG

GROUNDWATER LEVEL

STA 20+00 STATION

0 OFFSET

ENGINEER J. FORGA EQUIPMENT _CME 55

OPERATOR SPECTRUM DRILLING METHOD

SHEET 1 OF 1

			E.A. #	*****	30-1348-15		GROUNDWATER LEVEL OPERATOR SPECIA	NUM	
					1417,32 m			DATE DEPTH ELEV. DRILLING HOLLO	W STEM AUGER
			HAMME	R DROP S	SYSTEM_A	UTOMATIC		BACKFILLED YES	DATE 5/15/97
ELEV.	DEPTH		S	MPLE	Recovery	LAB TESTS	USCS	MATERIAL DESCRIPTION	REMARKS
(m)	(m)	NO.	TYPE	300mm	19(1)	CAD 12313	Group		HEMARKS
	-						SC	0.21 Approx. 9 cm AC., 15 cm Aggregate Base.	_
					,			LIGHT BROWN CLAYEY SAND moist, loose to medium dense, fine to medium grained sand,	
	-							moderate plasticity lines.	
1416.3	<u> 1</u>								
	†								
	1.52	A	MC	12		W,UW,S,PI		Wet	P1= 14
	2 1.98	''				11,011,011		Encountered groundwater at 1.68 M.	29% fines
1415.3	2 1.98								1.2
]								
	-								
1414.3	3 3,05				,				
1.414.3	- 3 4,05	В	MC	11	-	W,UW,S,PI		Gray, fine to medium grained sand. High plasticity fines.	PI=41
	3.51							righ plasticity lines.	28% fines
	-			Ì					-
1413.3	L ₄	İ							
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1412.3	_5		i						
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LOG OF BORING CARSON FREEWAY

PLATE

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PROJECT NO. 30-1348-15.002

5/15/97 START DATE: _

5/15/97

END DATE: JOB DESCRIPTION __CARSON FREEWAY

LOCATION

LOMPA REALIGNMENT

BORING E.A. #

30-1348-15.002

GROUND ELEV 1414.27 m HAMMER DROP SYSTEM AUTOMATIC **EXPLORATION LOG**

GROUNDWATER LEVEL

DATE | DEPTH | ELEV.

1412.7

SHEET 1 OF 1

STA 6+00 **STATION** 0 OFFSET

J. FORGA ENGINEER

EQUIPMENT _ CME 55

OPERATOR SPECTRUM

HOLLOW STEM AUGER

CI CI	Leeru					UTOMATIC	liece	BACKFILLED YES DA	
ELEV. (m)	DEPTH (m)	NO.	TYPE	BLOWS/	Recovery	LAB TESTS		MATERIAL DESCRIPTION	REMARKS
	- 0.30	A	GRAB			S,RV	SM	LIGHT BROWN SILTY SAND slightly moist, fine to medium grained sand.	33% fines
413.3	- 0.91 - 1					-			
	1,52	8	MC	28		W,UW,S	011	Encountered groundwater at 1.52 M. 1 83 Becoming more sandy, some fine gravel.	6% fines
412.3	2 1.98						SM SP	RED BROWN SLIGHTLY SILTY SAND wet, medium dense, line to medium grained sand.	
411.3	_ 3 3.05	С	мс	7	4	W,UW,S,PI	SC	2 90 LIGHT_BROWN_VERY_CLAYEY_SAND wet, stiff, fine grained sand, moderate plasticity fines.	Pl = 16 47% fines
410.3	3,51							3.51 Gray.	47 % tines
	-					i			
409.3	5								
408.3	_ 6								
407.3	7								
406.3	_ 8					÷			
*06.3				1	1				
05.3	- ⁹ -								

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PROJECT NO. 30-1348-15.002

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LOG OF BORING CARSON FREEWAY

CARSON CITY, NEVADA

PLATE

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TART	DATE:	5/15/97

END DATE:

5/15/97

JOB DESCRIPTION CARSON FREEWAY LOMPA REALIGNMENT AT U.S. 50

LOCATION BR-22 **BORING**

30-1348-15.002 E.A. #

DATE DEPTH ELEV. GROUND ELEV 1412.75 m 5-15-97 1.2 1411.5 HAMMER DROP SYSTEM AUTOMATIC

EXPLORATION LOG

GROUNDWATER LEVEL

SHEET 1 OF 1 STA 1+00 STATION

0 OFFSET

ENGINEER _J. FORGA EQUIPMENT _CME 55

OPERATOR SPECTRUM

HOLLOW STEM AUGER

BACKFILLED YES DATE 5/15/97

		,			YSTEM		BACKFILLED YES D	715 0710757
ELEV. (m)	DEPTH (m)	NO.	TYPE	BLOWS/	Recovery LAB TESTS	USCS	MATERIAL DESCRIPTION	REMARKS
	0.30				<u> </u>	SM	LIGHT BROWN SILTY SAND moist, medium	79.00
		Α	GRAB		S,PI,RV		dense, fine to coarse grained sand, non-plastic.	25% fines
	0.91						3	
1411.7								
							Frequentered convention at 1 22 M	
	1.52	B	МС	36	w.uw.s		Encountered groundwater at 1,22 M, Wet, dense, fine to medium grained sand,	30% fines
1410.7	2 1.98							
					į.		ā.	
						SC	2 44	
	[]				i		YELLOW BROWN CLAYEY SAND wet, medium dense, fine to coarse grained sand, low to	
1409.7	3 3.05	С	MC	14	W,UW,S		moderate plasticity fines.	13% fines
	3.51						351	13.6 111163
	3.31			4			No free water encountered.	
1408.7	L4				i i		003540	
	-	ĺ				- 1	***	
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407.7	_ 5							
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403.7	_9			-				
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PROJECT NO. 30-1348-15.002

KLEINFELDER

4875 LONGLEY LANE, SUITE 100 RENO, NEVADA 89502 Tel. (702) 689-7800

LOG OF BORING CARSON FREEWAY

CARSON CITY, NEVADA

PLATE

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START DATE: __5/15/97 END DATE:

5/15/97

JOB DESCRIPTION CARSON FREEWAY

LOCATION

U.S. 50 EAST RAMP

BORING

BR-23

E.A. #

30-1348-15.002

GROUND ELEV_1412.75 m

CALLED BOOK OVOTER ALITOMATIC

EXPLORATION LOG

GROUNDWATER LEVEL

DATE DEPTH ELEV.

5-15-97 2.1 1410.6

SHEET 1 OF 1

STATION

STA 2+00 0

ENGINEER

J. FORGA

EQUIPMENT _CME 55

OPERATOR _ SPECTRUM

DRILLING METHOD

HOLLOW STEM AUGER

i						UTOMATIC		BACKFILLED YES DA	TE 5/15/97
ELEV.	DEPTH	NO.	TYPE	AMPLE BLOWS/	Recovery	LAB TESTS	USCS	MATERIAL DESCRIPTION	REMARKS
							SC	O.15 Approx. 5 cm of AC, 15 cm of Aggregate Base. LIGHT BROWN CLAYEY SAND moist, loose, fine to medium grained sand, moderate plasticity fines.	
1411.7	1 	8	MC	9		W,UW,S,PI		Very line grained sand, Wet	PI= 15
1410.7	2 1.98							Encountered groundwater at 2,13 M.	19% fines
1409.7	_ 3 3.05 _ 3.51	С	MC	31		W,UW,S,PI		Dense, fine to medium grained sand, low plasticity fines.	PI≃5 33% fines
1408.7	_ 4		;					Sec	
1407.7	_ 5						;		
1406.7	- 6 - 6						}		
1405.7	7				1				
1404.7	. 8								
1403.7	. 9								
							-		

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PROJECT NO. 30-1348-15.002

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CARSON FREEWAY

LOG OF BORING

CARSON CITY, NEVADA

PLATE

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4675 1 250	51185
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START DATE:

5/15/97

5/15/97 END DATE:

JOB DESCRIPTION CARSON FREEWAY

LOCATION BORING

U.S. 50 EAST RAMP

BR-24

30-1348-15.002 E.A. # GROUND ELEV_ 1413.66 m

HAMMER DROP SYSTEM AUTOMATIC

EXPLORATION LOG

GROUNDWATER LEVEL

DATE | DEPTH | ELEV.

1412.1

SHEET 1 OF 1

STATION OFFSET

STA 6+00 0 J. FORGA

ENGINEER EQUIPMENT

CME 55 SPECTRUM

OPERATOR

HOLLOW STEM AUGER

BACKELLED YES DATE 5/15/07

ATE <u>5/15/97</u>	BACKFILLED YES DA		UTOMATIC						ì
REMARKS	ATERIAL DESCRIPTION	ISCS Iroup	LAB TESTS	Recovery (%)	AMPLE BLOWS/ 300mm	S. TYPE	NO.	DEPTH (m)	ELEV. (m)
21% fines	BROWN SILTY SAND slightly moist, a dense, fine to coarse grained sand.	SM	S,RV			GRAB	A	0.30	
	BOWN CLAYEY SAND moist, medium	SC 1.0						0.91	1412.7
PI= 13 41% fines	small cemented pieces, fine grained w plasticity fines.		W.UW,S,PI		16	мс	В	1,52 - - - 2 1.98	1411,7
	ROWN SLIGHTLY SILTY SAND wet, ine to medium grained sand, slightly	SM SP	į					_ 3_3.05	1410,7
9% fines		3.5	W,UW,S		30	MC	С	3,51	
	V/s							4	1409.7
								5	408.7
								_ 6	407.7
			:					7	406.7
		;						- 8 	405.7
				, , , , , , , , , , , , , , , , , , ,				9	404.7
									-

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LOG OF BORING CARSON FREEWAY

PLATE

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PROJECT NO. 30-1348-15.002

KI	EINFELD
ELEV.	DEPTH (m)
	0.30
1413.9	0.91
1412.9	2 1.98
1411.9	3 3.05
1410.9	3.51 - 4
	_

			START	DATE: _	5/15/97			EXPLORATION LOG			SHEET 1 OF 1
_			END DA		5/15/97					CT4 40 - 0	
				· -		SON FREEWA	v		31711014	STA 10+0	0
						AST RAMP			011361 _		
			LOCATI			AST FAIVE				J. FORGA	
K	LEINFELDI	ER	BORING	BORING BR-25					EQUIPMENT _		
			E.A. #	-	30-1348-1			GROUNDWATER LEVEL		SPECTRU	M
			GROUN	D ELEV_	1414.88 m	1		DATE DEPTH ELEV. 5-15-97 0.9 1414.0	DRILLING METHOD _	HOLLOW	STEM AUGER
			HAMME	R DROP	SYSTEM	AUTOMATIC		3-13-37 0.3 1414.0	BACKFILLED	YES (DATE _5/15/97
		1									
ELEV.	DEPTH (m)	NO.	TYPE	BLOWS	Recovery	LAB TESTS	USCS	MATERIAL DE	SCRIPTION		REMARKS
\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\	1 1117		1	SUCHEM		i	SC				
	0.30		GRAB	[S.RV		LIGHT BROWN VERY C moist, loose, fine to me	LAYEY SAND sli	ightly	
	[A	Grab			15,50		plasticity fines.	aioin grameu sa	ino, iow	42% fines
	[1							
1413.9	0.91							Encountered groundwa	ter at 0.91 M.		
İ	-				ĺ	1		1 22			
	1.52		1	1			SP	YELLOW BROWN CLEA	N SAND wet, fin	e to	
	-	В	MC	12		W,UW,S		medium grained sand.			3% fines
1412.9	2_1.98			ĺ		i				1	
1412.9	2-1-30										
	<u> </u>			 							
								2.59			
							SC	LIGHT BROWN CLAYE	/ SAND wet lose	a fina to	
1411.9	3 3.05							medium grained sand, I			
	-	C	MC	10		W,UW,S		fines			41% fines
1	3.51	i						3.51			
	-					li l			-		·
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1410.9	-4	ĺ				7					
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		- 1				8	- 1				
1409.9	_ 5						- 1				
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1408.9	_ 6					i				Í	1
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1407.9	7										
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1405.9



PROJECT NO. 30-1348-15.002

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CARSON FREEWAY

LOG OF BORING

CARSON CITY, NEVADA

PLATE

F-26

START DATE: 5/15/97

END DATE: 5/15/97

JOB DESCRIPTION __CARSON FREEWAY

LOCATION U.S. 50 WEST RAMP

BORING _

BR-26

E.A. # 30-1348-15.002 GROUND ELEV_1414.27 m

HAMMER DROP SYSTEM AUTOMATIC

EXPLORATION LOG

GROUNDWATER LEVEL

DATE | DEPTH | ELEV.

5-15-97 1.7 1412.6

SHEET 1 OF 1
FATION STA 10+00

STATION STATION OFFSET 0

ENGINEER __J. FORGA

EQUIPMENT CME 55
OPERATOR SPECTRUM

DRILLING

HOLLOW STEM AUGER

BACKFILLED YES DATE 5/15/97

			HAMME	R DROP S	SYSTEML	AUTOMATIC		BACKFILLED YES D	ATE <u>5/15/97</u>
ELEV.	DEPTH (m)	NO.	S. TYPE	AMPLE BLOWS/	Recovery	LAB TESTS		MATERIAL DESCRIPTION	REMARKS
	0.30	A	GRAB			S,RV	SC	LIGHT BROWN CLAYEY SAND slightly moist, medium dense, fine to medium grained sand, low plasticity fines.	29% fines
413.3	0.91								
	1.52	В	MC	15		w.uw.s	SM SP	GPAY SLIGHTLY SILTY SAND wet, medium dense, fine to medium grained sand.	9% fines
1412.3	<u>21.98</u>							Encountered groundwater at 1,68 M.	
411.3	_ 3 _{3.05}	С	MC	14		w.uw.s	SC	3.05 LIGHT_BROWN_CLAYEY_SAND wet, medium 3.51 dense, fine to medium grained sand, low to	26% fines
410.3	4							moderate plasticity fines	
	-								
409.3	5								
408.3	6						}		
407.3	- 7								
!	-								
406.3	- 8								
405.3	9					ĺ			

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PROJECT NO. 30-1348-15.002

KLEINFELDER

4875 LONGLEY LANE, SUITE 100 RENO, NEVADA 89502 Tel. (702) 689-7800 CARSON FREEWAY

LOG OF BORING

CARSON CITY, NEVADA

PLATE

F-27

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EXPLORATION LOG START DATE: __5/15/97 SHEET 1 OF 1 5/15/97 END DATE: STA 5+00 STATION JOB DESCRIPTION CARSON FREEWAY 0 U.S. 50 WEST RAMP **ENGINEER** _J. FORGA LOCATION CME 55 **BR-27** EQUIPMENT **BORING** KLEINFELDER SPECTRUM 30-1348-15.002 GROUNDWATER LEVEL **OPERATOR** E.A. # DRILLING METHOD DATE | DEPTH | ELEV. GROUND ELEV 1413.05 m HOLLOW STEM AUGER 5-15-97 1411.5 HAMMER DROP SYSTEM AUTOMATIC BACKFILLED YES DATE 5/15/97 SAMPLE | BLOWS/ Recovery | 300mm (%) USCS ELEV. DEPTH LAB TESTS MATERIAL DESCRIPTION REMARKS NO. TYPE (m) (m) SC LIGHT BROWN CLAYEY SAND moist, loose to 0.30 GRAB S,RV medium dense, fine to moderate grained sand, 26% fines low to moderate plasticity, estimated 20 to 30% fines. 0,91 1412.1 SM SP Encountered groundwater at 1.5 M. MC 12 W,UW,S В 19% fines GRAY_SLIGHTLY_SILTY_SAND wet, medium dense, fine to medium grained sand, estimated 5 2_1.98 1411.1 to 12% fines. 1410.1 3 3.05 MC 14 1409.1 1408.1 L 5 1407.1 6 1406.1 1405.1 _ 8 1404.1 _ 9

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LOG OF BORING CARSON FREEWAY

PLATE

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PROJECT NO. 30-1348-15.002

MOISTURE CONTENT AND UNIT WEIGHT RAMP AND REALIGNMENT BORINGS

BORING (meters) (%) (kN/cu m) BR-1 1.83 7.8 17.43 BR-2 1.83 4.8 16.33 BR-3 1.83 7.3 16.96 BR-4 1.83 5.6 16.18 BR-4 3.35 11.3 17.28	
BR-2 1.83 4.8 16.33 BR-3 1.83 7.3 16.96 BR-4 1.83 5.6 16.18	
BR-3 1.83 7.3 16.96 BR-4 1.83 5.6 16.18	
BR-4 1.83 5.6 16.18	
BR-4 3.35 11.3 17.28	
BR-4 4.88 9.0 15.71	
BR-4 6.40 10.9 16.18	
BR-4 7.92 9.2 17.75	
BR-5 3.35 9.2 18.93	
BR-6 1.83 6.0 16.65	
BR-7 1.83 7.7 18.06	
BR-7 3.35 7.2 16.65	
BR-8 1.83 6.7 16.81	
BR-8 3.35 9.4 15.71	
BR-9 1.83 9.0 16.81	
BR-9 3.35 6.4 16.02	
BR-10 1.83 6.8 16.49	
BR-10 3.35 10.8 17.43	
BR-11 1.83 18.1 16.33	
BR-11 3.35 5.6 17.43	
BR-12 1.83 12.8 17.43	
BR-12 4.88 14.5 17.28	
BR-12 6.40 12.0 18.33	
BR-13 1.83 12.2 17.28	
BR-14 1.83 12.7 17.75	
BR-14 3.35 16.5 17.12	
BR-15 1.52 0.6	
BR-15 4.57 0.5	
BR-16 1.83 14.0 17.28	
BR-16 3.35 12.8 16.65	
BR-17 1.83 10.9 17.12	
BR-17 3.35 21.8 15.85	
BR-18 1.83 15.9 16.96	
BR-18 3.35 14.4 17.28	
BR-19 1.83 13.6 18.06	
BR-19 3.35 12.9 16.81	
BR-20 1.83 24.0 14.61	
BR-20 3.35 23.3 15.55	
BR-21 1.83 5.1 16.81	
BR-21 3.35 34.7 13.04	
BR-22 1.83 21.4 16.02	
BR-22 3.35 15.6 17.28	
BR-23 1.83 24.7 14.92	

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MOISTURE / DENSITY TABLE

CARSON FREEWAY

F-25

PLATE

PROJECT NO. 30-1348-15.002

CARSON CITY, NEVADA

30 M

MOISTURE CONTENT AND UNIT WEIGHT RAMP AND REALIGNMENT BORINGS

	DEPTH	MOISTURE CONTENT	DRY UNIT WEIGHT
BORING	(meters)	(%)	(kN/cu m)
BR-23	3.35	24.4	14.45
BR-24	1.83	21.7	16.02
BR-24	3.35	18.4	16.49
BR-25	1.83	6.2	16.33
BR-25	3.35	23.3	15.23
BR-26	1.83	18.7	16.02
BR-26	3.35	16.5	16.96
BR-27	1.83	19.1	16.33

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CARSON FREEWAY

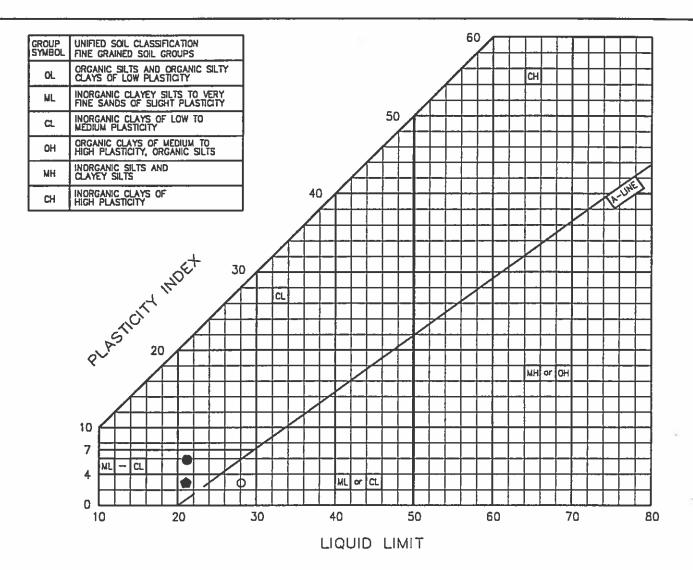
MOISTURE / DENSITY TABLE

CARSON CITY, NEVADA

PLATE

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TEST SYMBOL	SAMPLE NO.	SAMPLE (DEPTH)	LIQUID LIMIT	PLASTICITY INDEX	CLASSIFICATION
*	BR-4	3.81		NP	Brown Slightly Silty Sand (SM) -200=10%
•	BR-5	3.35	21	3	Brown Clayey Sand (SC) -200=22%
•	BR-10	0.30	21	6	Brown Clayey Sand (SC) -200=31%
A	BR-10	3.35		NP	Yellow Brown Slightly Silty Sand (SM) -200=8%
	BR-12	1.83		NP	Light Brown Clayey Sand (SC) -200=14%
M	BR-13	1.83		NP	Light Brown Clayey Sand (SC) -200=20%
0	BR-14	3.35	28	3	Light Brown Clayey Sand (SC) -200=14%

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PROJECT NO. 30-1348-15,002

KEY SOIL CLASSIFICATION AND TERMS

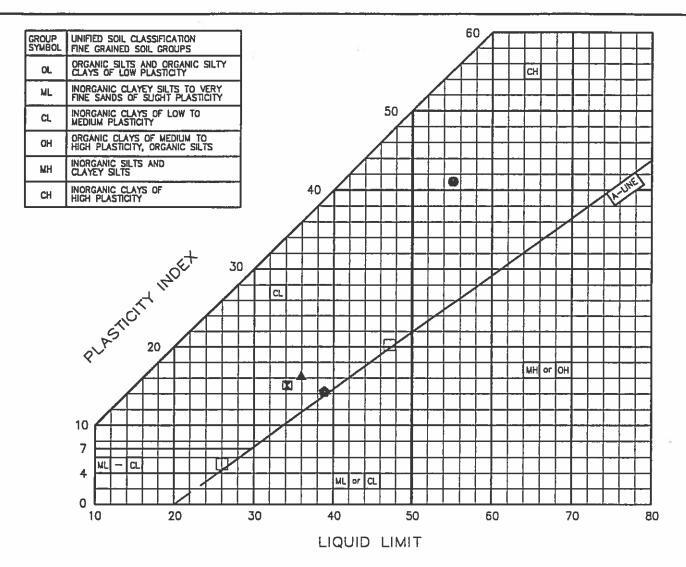
CARSON FREEWAY
CARSON CITY, NEVADA

PLATE

F-30

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3



TEST SYMBOL	SAMPLE NO.	SAMPLE (DEPTH)	LIQUID LIMIT	PLASTICITY INDEX	CLASSIFICATION
*	BR-18	1.83		NP	Brown Clayey Sand (SC) -200 =11%
•	BR-20	1.83	39	14	Light Brown Clayey Sand (SC) -200=29%
•	BR-20	3.35	55	41	Light Bown Clayey Sand (SC) -200=28%
A	BR21	3.35	36	16	Light Brown Very Clayey Sand (SC) -200=47%
	BR-22	0.30		NP	Light Brown Silty Sand (SC) —200= 25%
×	BR-23	1.83	34	15	Light Brown Clayey Sand (SC) -200=19%
	BR-23	3.35	26	5	Light Brown Clayey Sond (SC) -200=33%

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KEY SOIL CLASSIFICATION AND TERMS

CARSON FREEWAY
CARSON CITY, NEVADA

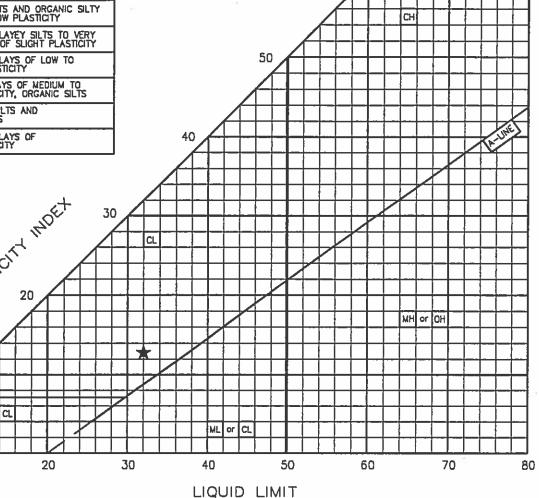
PLATE

F-31

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TEST SYMBOL	SAMPLE NO.	SAMPLE (DEPTH)	LÌQUID LIMIT	PLASTICITY INDEX	CLASSIFICATION
*	BR-24	1.83	32	13	Light Brown Clayey Sand (SC) -200=41%

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KEY SOIL CLASSIFICATION AND TERMS

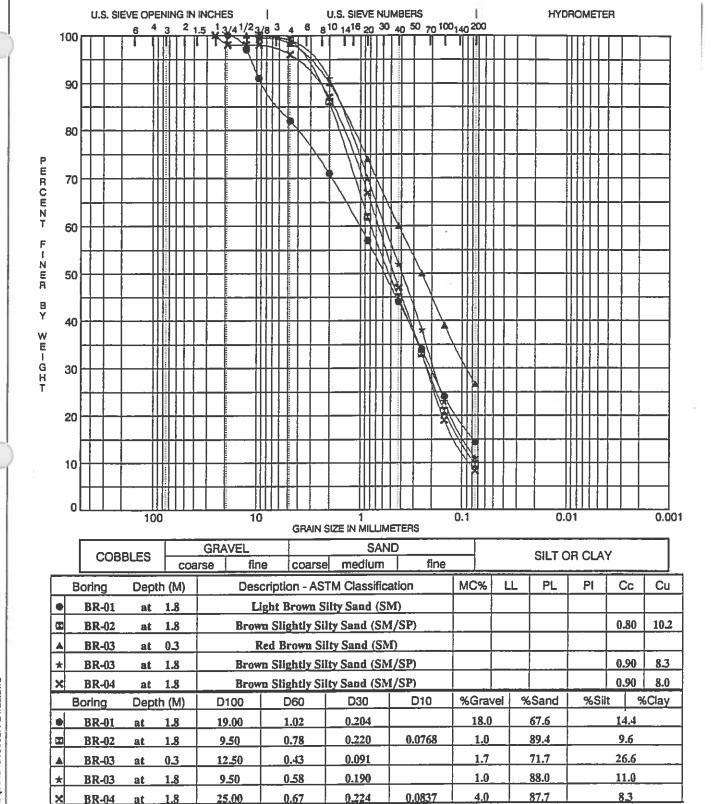
CARSON FREEWAY
CARSON CITY, NEVADA

PLATE

F-32

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4875 LONGLEY LANE, SUITE 100 RENO, NEVADA 89502 Tel. (702) 689-7800 GRAIN SIZE ANALYSES

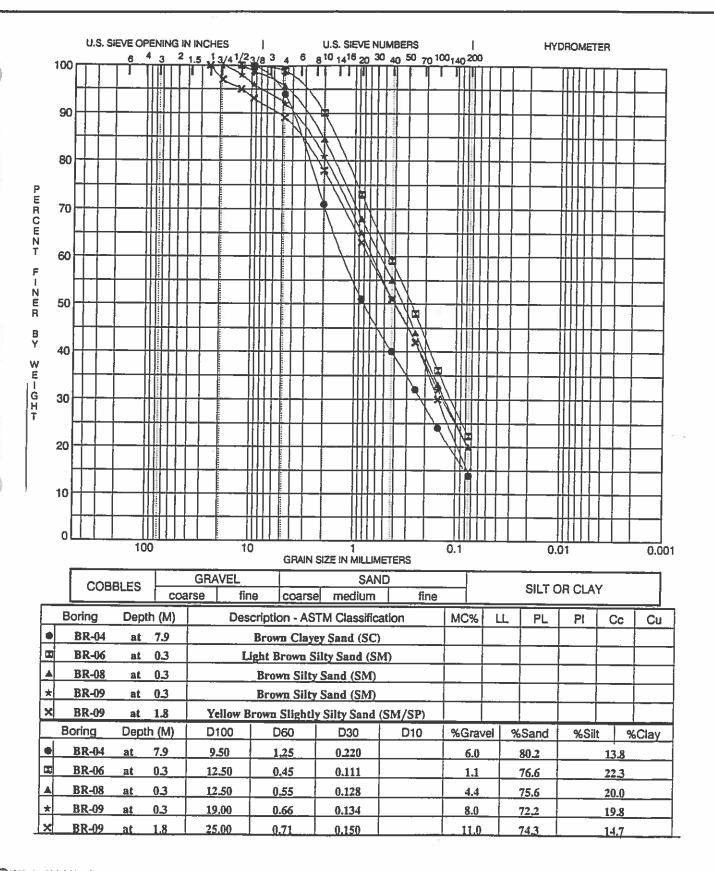
CARSON FREEWAY

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PLATE

PROJECT NO. 30-1348-15.002





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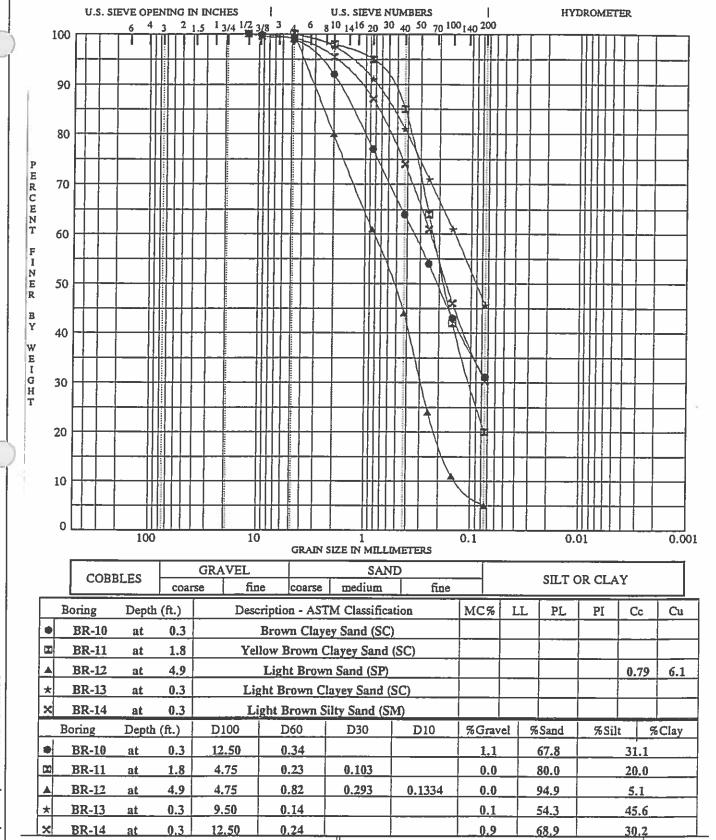
GRAIN SIZE ANALYSES

CARSON FREEWAY

F-34

PLATE

CARSON CITY, NEVADA



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CARSON FREEWAY

F-35

PLATE

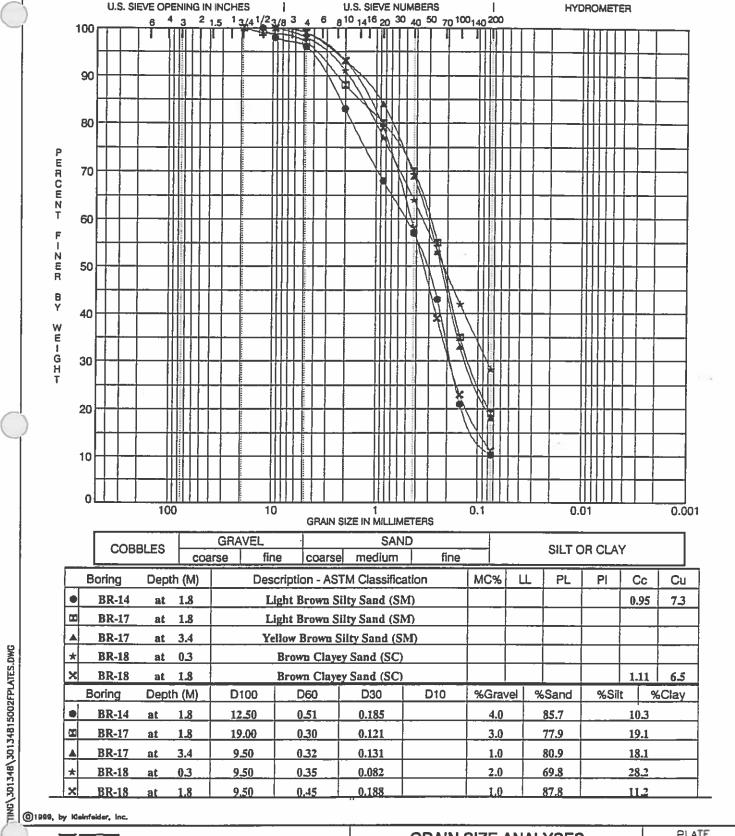
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CARSON CITY, NEVADA

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GRAIN SIZE ANALYSES

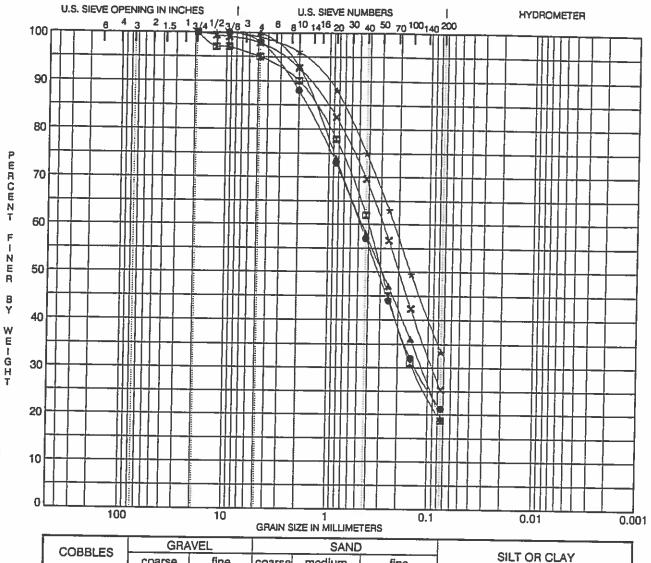
CARSON FREEWAY

PLATE

PROJECT NO. 30-1348-15.002

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3



		COB	COBBLES		GRAVEL		SAND				SULOPICIAN					
			CO			fine	coarse	medium	fine		SILT OR CLAY					
		Boring Depth (M)		Description - ASTM Classification			MC%	LL	PL	PI	Cc	1				
	•	BR-19	at	0.3		Brov	n Claye	Sand (SC)		<u> </u>					 	
	I	BR-19	at	1.8				Sand (SC)						†		
		BR-19	at	3.4	Brown Clayey Sand (SC)							1			\vdash	
	*	BR-21	at	0.3				yey Sand (S							\vdash	
	×	BR-22	at	0.3	Light Brown Silty Sand (SM)										\vdash	
		Boring	Dept	h (M)	D100		060	D30	D10	%Grav	/el	%Sand	%Silt	9	6Cla	
	•	BR-19	at	0.3	9.50	_ 0	.48	_ 0.132		2.0		76.8		21.2		
	M	BR-19	at	1.8	19.00	0	.40	0.142		5.0		76.3		18.7		
	A	BR-19	at_	3.4	4.75	0	.46	0.117		0.0	\neg	80.8		19.2		
	*	BR-21	at	0.3	19.00	0	22			1.0		65.7	33.3			
	x	BR-22	at	0.3	19.00	0	29	0.091		2.2		72.5		25.3		

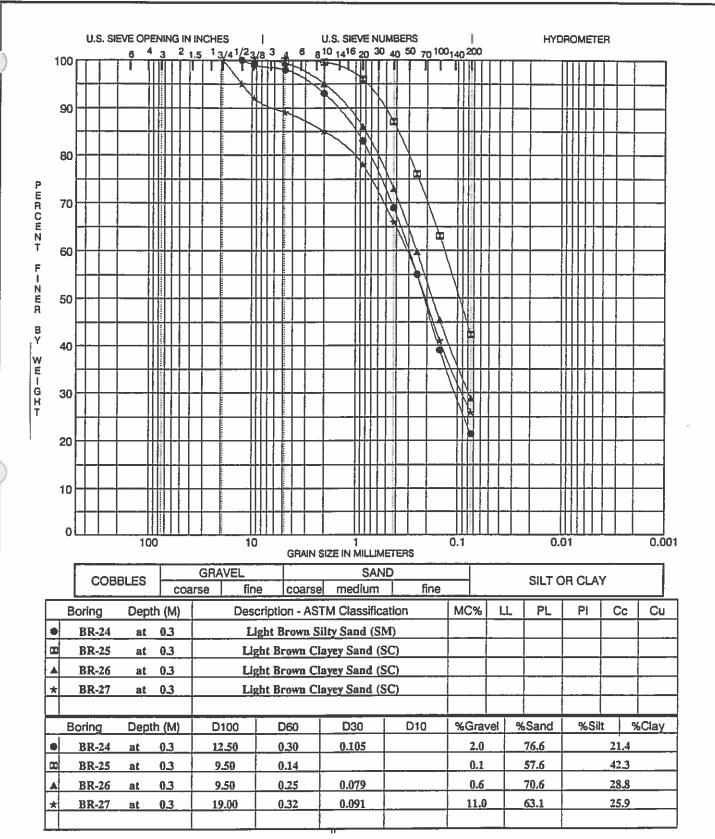


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GRAIN SIZE ANALYSES CARSON FREEWAY

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CARSON FREEWAY

F-38

PLATE

PROJECT NO. 30-1348-15.002

BORING NO.: BR-5 DEPTH:	1.83 m					
SAMPLE DESCRIPTION Light Brown Silty Sand						
OVERBURDEN PRESSURE, kPa	33					
PRECONSOLIDATION PRESSURE, KF	a 48					

	INITIAL	FINAL
DRY DENSITY - kN/cu m	16.81	17.67
WATER CONTENT - %	6.5	14.6
VOID RATIO	0.4314	0.3621
DEGREE OF SATURATION, %	37.00	99.00
SAMPLE HEIGHT - cm	2,5400	2.39

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Tel. (702) 689-7800

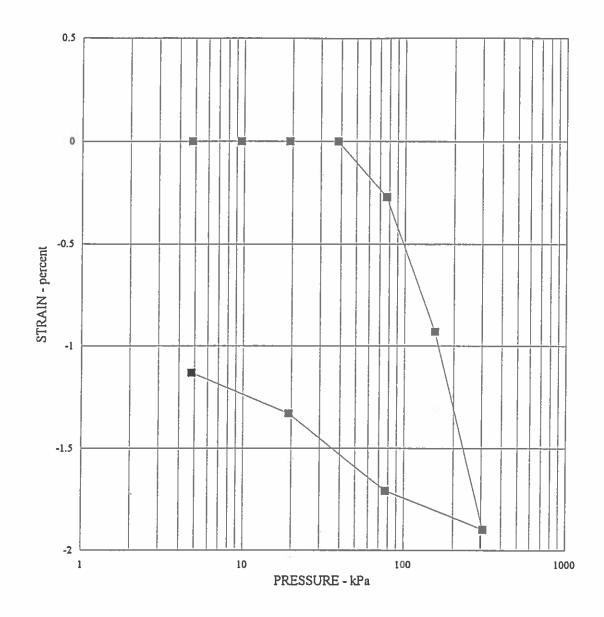
PROJECT NO. 30-1348-15.002

CONSOLIDATION TEST

CARSON FREEWAY

CARSON CITY, NEVADA

PLATE



BORING NO.: BR-12 DEPT	H: 3.96 m
SAMPLE DESCRIPTION Light E	Brown Clayey Sand
OVERBURDEN PRESSURE, kP	a 33
PRECONSOLIDATION PRESSU	RE, kPa 68

	INITIAL	FINAL
DRY DENSITY - kN/cu m	19.466	19.84
WATER CONTENT - %	7.8	8.5
VOID RATIO	0.2330	0.2103
DEGREE OF SATURATION, %	82.00	99.00
SAMPLE HEIGHT - cm	2.5400	2.39

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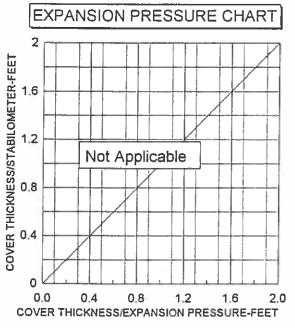
CONSOLIDATION TEST

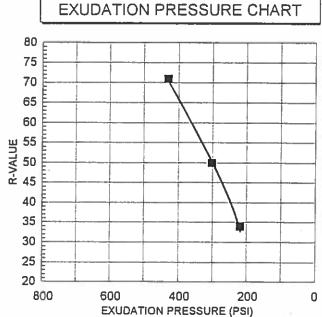
CARSON FREEWAY

PLATE

Sample Source: BR-3 @ .30-.91 meters

Sample Description: Red Brown Silty Sand





Specimen	A	В	С
Exudation Pressure, psi	431	303	220
Expansion Dial (.0001")	0.0000	0.0000	0.0000
Expansion Pressure, psf	0	0	0
Resistance Value, R	71	50	34
% Moisture at Test	8.9	9.5	10.0
Dry Density at Test, pcf	130.1	129.1	127.7
R Value by Expansion Pressure (TI=)		Not App	olicable
R Value at 300 psi Exudation Pressure		5	0

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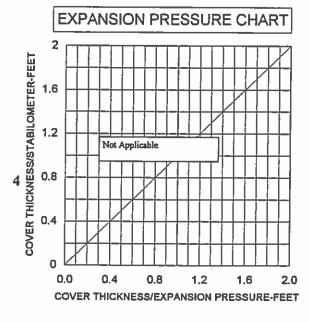
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CARSON FREEWAY

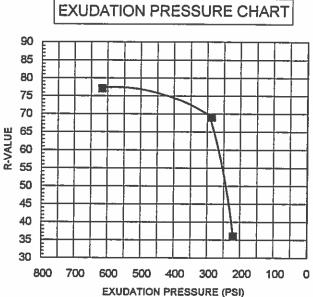
PLATE

F-41

PROJECT NO. 30-1348-15.002

Sample Source: BR-6 @ .30-.91 meters
Sample Description: Light Brown Silty Sand





Specimen	A	В	C
Exudation Pressure, psi	617	288	221
Expansion Dial (.0001")	0.0000	0.0000	0.0000
Expansion Pressure, psf	0	0	0
Resistance Value, R	77	69	36
% Moisture at Test	8.8	4.0	10.4
Dry Density at Test, pcf	129.6	129.0	126.7
R Value by Expansion Pressure (TI=)		Not Ap	plicable
R Value at 300 psi Exudation Pressure		7	0

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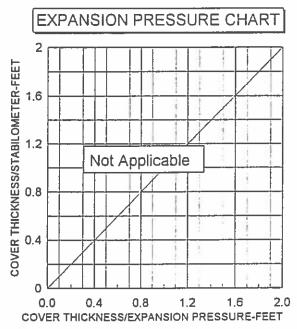
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CARSON FREEWAY

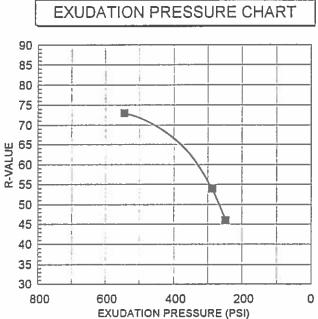
F-4

PLATE

Sample Source: BR-8 @ .03-91 meters

Sample Description: Brown Silty Sand w/Trace of Gravel





Specimen	A	В	С
Exudation Pressure, psi	545	288	249
Expansion Dial (.0001")	0.0000	0.0000	0.0000
Expansion Pressure, psf	0	0	0
Resistance Value, R	73	54	46
% Moisture at Test	9.5	10.6	11.1
Dry Density at Test, pcf	124.9	125.6	124.3
R Value by Expansion Pressure (TI=)		Not App	licable
R Value at 300 psi Exudation Pressure		56	5

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R-VALUE

F-43

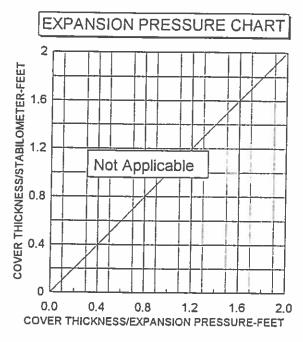
PLATE

PROJECT NO. 30-1348-15.002

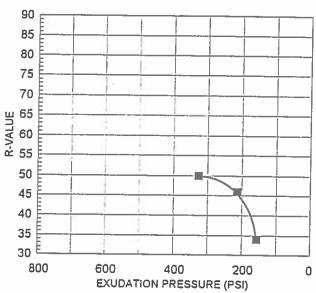
CARSON CITY, NEVADA

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Sample Source: BR-9 @ .30-.91 meters Sample Description: Brown Silty Sand



EXUDATION PRESSURE CHART



Specimen	A	В	С
Exudation Pressure, psi	328	215	158
Expansion Dial (.0001")	0.0000	0.0000	0.0000
Expansion Pressure, psf	0	0	0
Resistance Value, R	50	46	34
% Moisture at Test	9.9	10.4	10.9
Dry Density at Test, pcf	128.4	127.3	126.3
R Value by Expansion Pressure (TI=)		Not Ap	
R Value at 300 psi Exudation Pressure		4	

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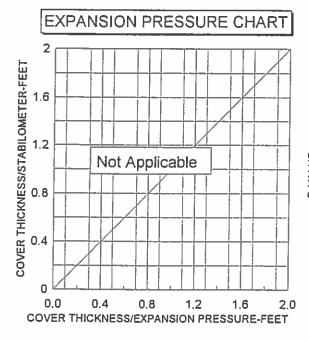
CARSON FREEWAY

PLATE

F-44

PROJECT NO. 30-1348-15.002

Sample Source: BR-10 @ .30-.91 meters Sample Description: Brown Clayey Sand



EXUDATION PRESSURE CHART 50 45 40 35 R-VALUE 20 30 30 20 15 10 5 0 800 600

400

EXUDATION PRESSURE (PSI)

200

0

Specimen	A	В	С
Exudation Pressure, psi	343	284	113
Expansion Dial (.0001")	0.0019	0.0020	0.0017
Expansion Pressure, psf	82	87	74
Resistance Value, R	35	15	7
% Moisture at Test	9.3	10.3	11,4
Dry Density at Test, pcf	124.3	126.5	124.9
R Value by Expansion Pressure (TI=)		Not App	olicable
R Value at 300 psi Exudation Pressure		1	7

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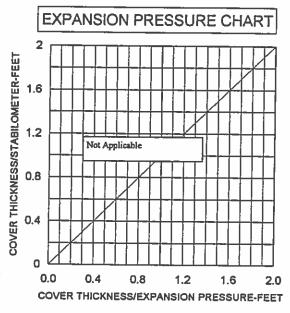
R-VALUE

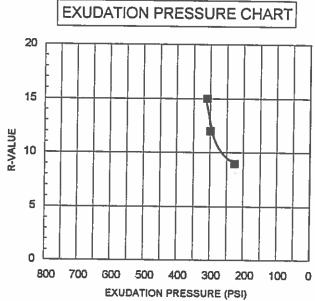
CARSON FREEWAY

PLATE

PROJECT NO. 30-1348-15.002

Sample Description: Light Brown Very Clayey Sand





Specimen	A	В	С
Exudation Pressure, psi	226	300	310
Expansion Dial (.0001")	0.0000	0.0000	0,0000
Expansion Pressure, psf	0	0	0
Resistance Value, R	9	12	15
% Moisture at Test	20.6	21.2	21.8
Dry Density at Test, pcf	104.7	106.3	105.1
R Value by Expansion Pressure (TI=)		Not Ap	
R Value at 300 psi Exudation Pressure		1	2

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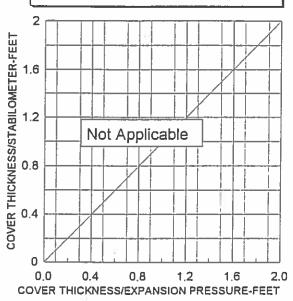
PLATE

F-46

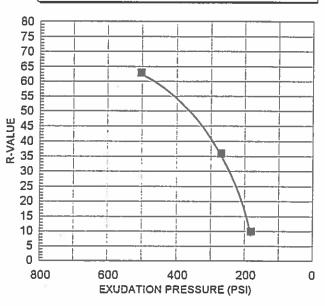
PROJECT NO. 30-1348-15.002

Sample Description: Light Brown Silty Sand

EXPANSION PRESSURE CHART



EXUDATION PRESSURE CHART



Specimen	A	В	С
Exudation Pressure, psi	503	270	182
Expansion Dial (.0001")	0.0000	0.0000	0.0000
Expansion Pressure, psf	0	0	0
Resistance Value, R	63	36	10
% Moisture at Test	10.4	11.5	12.5
Dry Density at Test, pcf	128.1	125.8	121.9
R Value by Expansion Pressure (TI=)		Not App	licable
R Value at 300 psi Exudation Pressure		40)

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CARSON FREEWAY

PLATE

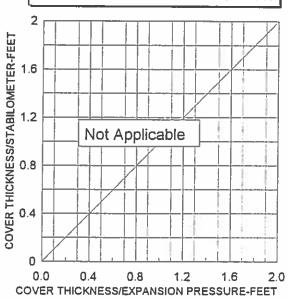
F-47

PROJECT NO. 30-1348-15.002

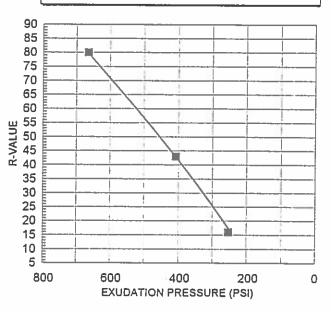
Sample Source: BR-18 @ .30-.91 meters

Sample Description: Brown Clayey Fine Sand

EXPANSION PRESSURE CHART



EXUDATION PRESSURE CHART



Specimen	A	В	С
Exudation Pressure, psi	666	408	254
Expansion Dial (.0001")	0.0000	0.0000	0.0000
Expansion Pressure, psf	0	0	0
Resistance Value, R	80	43	16
% Moisture at Test	8.3	9.4	10.4
Dry Density at Test, pcf	129.0	127.2	126.6
R Value by Expansion Pressure (TI=)		Not Ap	plicable
R Value at 300 psi Exudation Pressure		2	5

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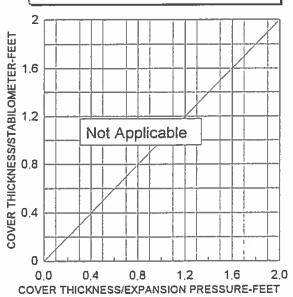
PLATE

F-48

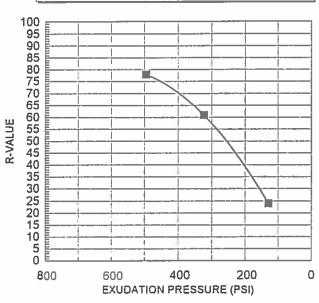
PROJECT NO. 30-1348-15.002

Sample Description: Brown Clayey Sand





EXUDATION PRESSURE CHART



Specimen	A	В	С
Exudation Pressure, psi	497	324	130
Expansion Dial (.0001")	0.0002	0.0000	0.0000
Expansion Pressure, psf	9	0	0
Resistance Value, R	78	61	24
% Moisture at Test	8.9	10.1	11.1
Dry Density at Test, pcf	129.7	128.1	126.2
R Value by Expansion Pressure (TI=)		Not App	licable
R Value at 300 psi Exudation Pressure		57	7

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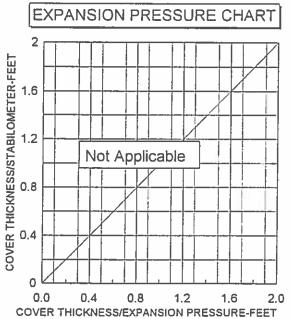
PLATE

F-49

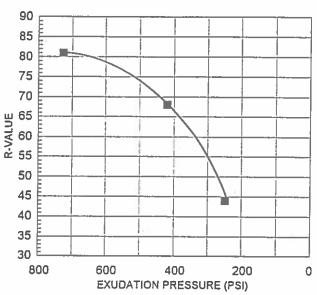
PROJECT NO. 30-1348-15.002

Sample Source: BR-21 @ .30-.91 meters

Sample Description: Light Brown Silty Sand



EXUDATION PRESSURE CHART



Specimen	A	В	С
Exudation Pressure, psi	728	420	249
Expansion Dial (.0001")	0.0002	0.0000	0.0000
Expansion Pressure, psf	9	0	0
Resistance Value, R	81	68	44
% Moisture at Test	10.5	10.9	11.5
Dry Density at Test, pcf	121.8	123.0	118.4
R Value by Expansion Pressure (TI=)		Not Ap	plicable
R Value at 300 psi Exudation Pressure		5	5

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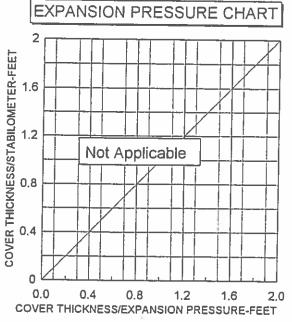
CARSON FREEWAY

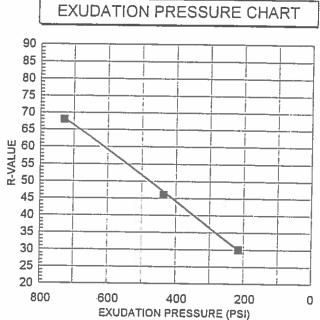
PLATE

PROJECT NO. 30-1348-15.002

Sample Source: BR-22 @ .30-.91 meters

Sample Description: Light Brown Silty Sand





Specimen	A	В	C
Exudation Pressure, psi	729	437	216
Expansion Dial (.0001")	0.0000	0.0000	0.0000
Expansion Pressure, psf	0	0	0
Resistance Value, R	68	46	30
% Moisture at Test	11.0	12.1	13.1
Dry Density at Test, pcf	121.1	119.8	117.2
R Value by Expansion Pressure (TI=)	<u> </u>	Not Applicable	
R Value at 300 psi Exudation Pressure	36		

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CARSON FREEWAY

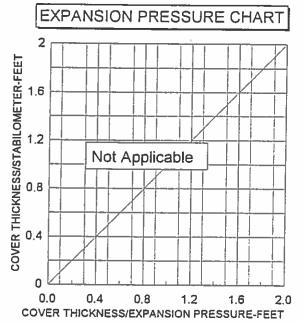
PLATE

F-51

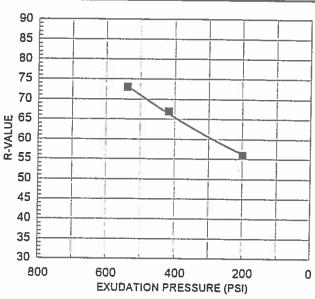
PROJECT NO. 30-1348-15.002

Sample Source: BR-24 @ .30-.91 meters

Sample Description: Light Brown Silty Sand



EXUDATION PRESSURE CHART



Specimen	A	В	С
Exudation Pressure, psi	539	418	200
Expansion Dial (.0001")	0.0000	0.0000	0.0000
Expansion Pressure, psf	0	0	0
Resistance Value, R	73	67	56
% Moisture at Test	10.7	11.2	11.8
Dry Density at Test, pcf	121.2	119.4	118.7
R Value by Expansion Pressure (TI=)	Not Applicab		plicable
Value at 300 psi Exudation Pressure		61	

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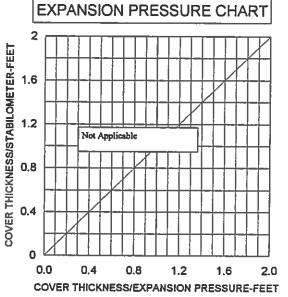
CARSON FREEWAY

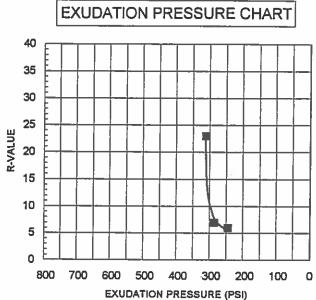
PLATE

F-52

PROJECT NO. 30-1348-15.002

Sample Description: Light Brown Very Clayey Sand





Specimen	A	В	С	
Exudation Pressure, psi	311	286	245	
Expansion Dial (.0001")	0.0005	0.0000	0.0000	
Expansion Pressure, psf	22	0	0	
Resistance Value, R	23	7	6	
% Moisture at Test	16.4	17.5	18.7	
Dry Density at Test, pcf	111.2	108.9	106.0	
R Value by Expansion Pressure (TI=)			Not Applicable	
R Value at 300 psi Exudation Pressure		9		

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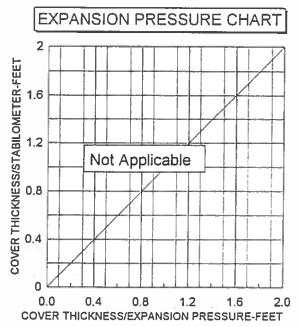
PLATE

F-53

PROJECT NO. 30-1348-15.002

Sample Source: BR-26 @ .30-.91 meters

Sample Description: Light Brown Clayey Sand



EXUDATION PRESSURE CHART 40 35 30 25 R-VALUE 20 15 10 5 0 800 600

400

EXUDATION PRESSURE (PSI)

200

0

Specimen	A	В	C
Exudation Pressure, psi	410	380	227
Expansion Dial (.0001")	0.0000	0.0000	0.0000
Expansion Pressure, psf	0	0	0
Resistance Value, R	7	3	1
% Moisture at Test	13.9	15.0	16.2
Dry Density at Test, pcf	118.5	115.7	112.4
R Value by Expansion Pressure (TI=)		Not Applicable	
R Value at 300 psi Exudation Pressure		2	

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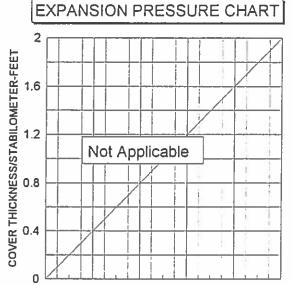
R-VALUE

CARSON FREEWAY

PLATE

PROJECT NO. 30-1348-15.002

Sample Description: Light Brown Clayey Sand



0.8

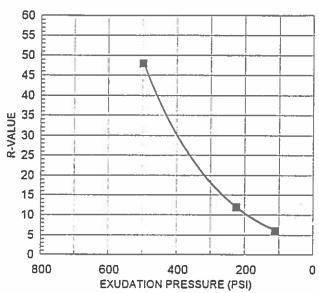
COVER THICKNESS/EXPANSION PRESSURE-FEET

1.2

1.6

2.0

EXUDATION PRESSURE CHART



Specimen	A	В	С	
Exudation Pressure, psi	500	225	112	
Expansion Dial (.0001")	0.0000	0.0000	0.0000	
Expansion Pressure, psf	0	0	0	
Resistance Value, R	48	12	6	
% Moisture at Test	17.5	16.3	15.1	
Dry Density at Test, pcf	117.1	116.2	114.7	
R Value by Expansion Pressure (TI=)	Not Applicable		plicable	
R Value at 300 psi Exudation Pressure	alue at 300 psi Exudation Pressure		18	

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R-VALUE CARSON FREEWAY

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PROJECT NO. 30-1348-15.002

0.0

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