GEOTECHNICAL INVESTIGATION

I-80 EASTBOUND WIDENING FROM MCCARRAN BOULEVARD TO KEYSTONE AVENUE, PHASE 2B, RENO, NEVADA

EA 74191 OCTOBER 2023





| **NEVADA DEPARTMENT OF TRANSPORTATION** | MATERIALS DIVISION | | GEOTECHNICAL SECTION | 1263 STEWART ST, CARSON CITY, NEVADA 89712 |

STATE OF NEVADA DEPARTMENT OF TRANSPORTATION MATERIALS DIVISION GEOTECHNICAL SECTION

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1. Introduction

The Nevada Department of Transportation (NDOT) plans to widen eastbound US Interstate Highway 80 (I-80) in Reno, Nevada between McCarran Boulevard and Keystone Avenue to accommodate the addition of an auxiliary lane under project 74191 Phase 2B. The project scope includes widening Bridge Structure H-1162E on I-80 over Stoker Avenue, the construction of retaining structures separating the eastbound and westbound I-80, and soundwalls.

This report presents the findings and recommendations developed from our geotechnical engineering investigation for the proposed improvements. The investigation was conducted in accordance with American Association of State Highway and Traffic Administration (AASHTO) and Federal Highway Administration (FHWA) guidelines, 2020 (AASHTO 2020).

1.1 Project Description

The project consists of adding an auxiliary lane by widening the highway from two lanes to three lanes to improve traffic movement in the eastbound direction. The scope includes widening to the inside (north) of the existing Bridge H-1162E that spans over Stoker Avenue. The existing piers and abutments on both structures are supported on spread footings. To support the widening, two columns will be added on both the west and east sides, Piers 1 and 2 respectively. The piers will be supported on spread footings bearing on native soils. The existing spread footing abutments will be extended, bearing on new embankment fill, and new wingwalls will be constructed.

To facilitate the widening, a median retaining wall is proposed between eastbound and westbound I-80. The median retaining wall is a cast-in-place (CIP) cantilever retaining wall with an integral continuous slope barrier rail. Soundwalls supported on spread footings are proposed at the top of the embankment on both sides of the highway within the project limits.

The project Vicinity Map and Exploration Map are shown in Appendix A on Figures A-1 and A-2.

1.2 Purpose and Scope of Work

The purpose of this investigation was to evaluate the suitability of the project site from a geotechnical perspective. The main objectives of the investigation were to characterize the subsurface materials, assess the appropriate engineering characteristics of the subsurface soils, perform engineering analyses, develop geotechnical recommendations for design and construction, and document our findings and recommendations in this report.

The scope of our geotechnical investigation includes the following:

- A review of published geologic and geotechnical information pertaining to the site vicinity;
- A field exploration consisting of drilling one boring to a maximum depth of 60 feet below ground surface (bgs) to obtain information to evaluate the subsurface conditions;
- Perform geotechnical laboratory testing on select soil samples collected from the boring;
- Perform engineering analyses to develop geotechnical design criteria and recommendations for the proposed project; and

• Preparation of this report.

1.3 Other Reports and Investigations

Additional explorations and laboratory testing have been completed by NDOT and by others as part of previous design phases for the I-80 Widening Project. Two borings were completed by NDOT during the initial design phase and three additional borings were completed during the I-80 westbound bridge widening phase. This report incorporates the applicable information collected from the previous studies of the H-1162 Eastbound and Westbound Bridge projects. These reports are listed in the References section of this report. Boring logs from previous explorations are presented in Appendix C.

2. Field Exploration and Laboratory Testing

2.1 Field Exploration

The exploration performed by NDOT included one boring drilled on May 11th and 12th, 2021 at the approximate location shown on Figure A-2. The boring was advanced using a 6-inch hollow stem auger to a depth of approximately 60 feet below ground surface utilizing a truck-mounted Diedrich D-120 (NDOT 1627) drill rig. Samples were collected using Standard Penetration Test samplers driven by an automatic hammer with a weight of 140 pounds and a drop of 30 inches.

The number of blows required to drive the sampler 6-inches were recorded for the 18-inch drive and are presented in the boring logs. The blow counts presented in the logs are uncorrected and are shown as they were recorded in the field. Normalizing the blow counts for use in analysis was performed utilizing corrections for sampler type, rod length, auger diameter, hammer efficiency, and overburden stress. Both the samples and drill cuttings were visually classified in the field based on the Unified Soil Classification System (USCS) in general accordance with ASTM D2488.

Logs of the boring were prepared based on the field logging and the results of laboratory testing in general accordance with ASTM D2487. The stratification lines shown on the exploration logs represent the approximate boundary between soil types even though the actual transition may be more gradual. Care should be taken in interpolating between and beyond exploration points due to the heterogeneity of natural soil deposits. The boring logs and key are presented in Appendix B.

2.2 Geotechnical Laboratory Testing

Laboratory testing was conducted on select soil samples recovered during the field exploration. Geotechnical laboratory test results are summarized and presented in Appendix D. Tests conducted include the following:

- Method of Test Sieve Analysis of Coarse and Fine Aggregate (Nev. T206);
- Standard Method of Test for Laboratory Determination of Moisture Content of Soils (AASHTO T265);
- Method of Test for Determining the Liquid Limit, Plastic Limit, and Plasticity Index of Soil (Nev. T210, T211, and T212).

3. Site and Subsurface Conditions

3.1 Site Conditions

The site is located along the I-80 corridor between McCarran Boulevard and Keystone Avenue in Reno, Nevada. The existing Stoker Avenue underpass consists of two structures, H-1162E and H-1162W. The edge-to-edge distance between the eastbound bridge and westbound bridge is about 25 feet. Stoker Avenue runs in a generally north/south direction and slopes to the south at approximately 3 percent within the project area. Average clearance between the bottom of bridge structures and the Stoker grade is about 14 feet. From the bridge abutments, the surface slopes down to Stoker Avenue at an approximate 2H:1V (Horizontal:Vertical) grade. Concrete slope paving covers the surface from the abutments to the sidewalks adjacent to Stoker Avenue. Adjacent the slope paving, exposed embankment fills consist primarily of granular soils. Surface runoff and drainage generally flow south towards the Truckee River.

3.2 Subsurface Conditions

3.2.1 General Geology and Faulting

The project site is located in the western portion of the Basin and Range geomorphic province. The project site is mapped as primarily Quaternary alluvium (Qa). The alluvium includes beach and sand dune deposits and sandy gravel deposits.

There are no active faults mapped crossing the project site. The nearest active faults (activity within the last 15,000 years) are the Mount Rose Fault, approximately 1.6 miles to the southeast, and the Peavine Peak Fault Zone, approximately 5.7 miles to the northwest. Slip rates of the two faults are on the order of 1 to 5 mm/year (Sawyer, 2003). Although there is no record of significant recent (within the last 150 years) earthquakes associated with these faults, the Mount Rose Fault Zone is predicted capable of generating a M7 earthquake (dePolo, 1997).

3.2.2 Subsurface Materials

The results of our field exploration and laboratory analyses indicate the soil profile generally consists of about 7 inches of asphalt concrete underlain by 5 inches of aggregate base underlain by very dense, poorly graded GRAVEL (GP-GC) with varying amounts of sand and clay extending to the maximum depth explored of 60.5 feet bgs.

Thin, intermittent, very hard clayey sand and cobble layers were encountered throughout the profile. Drilling was difficult in the granular soils from depths of about 25 to 45 feet.

Previous explorations in the area generally indicate near surface granular soils with varying amounts of non-plastic and plastic fines. Boulders were also identified in embankment soils.

3.2.3 Groundwater Conditions

Groundwater was not encountered in the boring for this project and we do not anticipate that it will affect the construction or performance of the new structure. According to nearby well log data from the Nevada Division of Water Resources, groundwater was encountered at approximately 65 feet bgs in a well located

about 400 feet south of the project. Groundwater depths and soil moisture may change over time due to seasonal fluctuations, regional pumping, and other contributing factors.

4. Recommendations

We understand that the proposed bridge piers, bridge abutments, soundwalls, and median retaining walls will be supported on shallow foundations. Based on the results of this exploration, the site is suitable for the proposed improvements. Provided herein are the recommendations for use in design and construction of shallow foundations.

4.1 Site Preparation and Earthwork

4.1.1 Site and Subgrade Preparation

Prior to construction, it is recommended that any unsuitable soils and vegetation be removed from below areas which will ultimately support structural loads. Unsuitable soils consist of topsoil, organic soils, undocumented fill, disturbed native soils, and any other deleterious material. General site preparation should follow procedures outlined in the 2014 Edition of the Nevada Department of Transportation Standard Specifications for Road and Bridge Construction (Silver Book), Section 201. The removal of any existing structures or obstructions should follow Silver Book Section 202. Any soft or loose areas at the base of excavations should be stabilized prior to the placement of structural fill. After excavations we recommend compacting the exposed subgrade to not less than 90% of the maximum density as determined by Test Method No. Nev. T108 in accordance with Silver Book Section 206.03.01. Upon completion of subgrade preparation, granular backfill should be placed as described below.

4.1.2 Backfill and Embankment

Embankment and backfill should be properly placed and compacted according to the Silver Book sections 203 and 207 respectively. Based on the materials encountered in this exploration, most native soils generated from the widening excavations are anticipated to be suitable for reuse as backfill or embankment. The excavated material encountered in the slope, sidewalk and roadbed sections such as concrete, asphaltic concrete or deleterious substances may not be used for embankment borrow and this material should be disposed of off-site.

Borrow and Selected Borrow shall meet the specifications presented in Silver Book Section 704.03.12 and 704.03.13 respectively.

4.1.3 Excavations

Excavations should be possible using conventional equipment in the dense to very dense granular soils. Occasional cobble layers were encountered and may require ripping or other means to aid in excavation. Borings TH-50 and TH-51 performed for Contract 1230 indicate the presence of boulders within the fill. It is anticipated that oversized material, such as boulders, will be encountered in the excavations associated with the project improvements and may require the use of rock hammer attachments or other non-conventional excavation techniques.

Temporary excavations and shoring should conform to OSHA standards. Based on the subsurface materials encountered in our exploration, the on-site clayey, sandy gravels can be classified as Type C (OSHA 1926). Vertical excavations should not exceed 4 vertical feet. Excavations greater than 4 vertical feet should be sloped in accordance with OSHA 1926 or shored. Temporary slope surfaces should be

moistened to minimize sloughing and raveling. Protection of workers and adjacent structures, shoring design, and the stability of all temporary slopes are the sole responsibility of the contractor.

4.1.4 Cut and Fill Slopes

Permanent fill slopes should have a maximum slope of 2H:1V and should be overbuilt and trimmed to limits on the staking. Slopes should be constructed in accordance to Silver Book 203.03.06. All slopes should be stabilized from wind and rain erosion in accordance with Silver Book Section 211.

4.2 Bridge Widening

Table 1 describes the bridge demands and effective shallow foundation dimensions for Bridge H-1162E for which NDOT has provided geotechnical design recommendations. Widened foundations will bear at the same elevation as the existing foundations. The depth of bearing is on the order of 5 feet for both abutments and 8 to 9 feet for both piers.

Table 1: Bridge Demands and Effective Shallow Foundation Dimensions

Shallow Foundation Support Location	Factored Service Limit State Bearing Demand, ksf	Factored Strength Limit State Bearing Demand, ksf	Factored Extreme Limit State Bearing Demand, ksf	
Abutments 1 and 2	4.8 (3 feet by 22 feet)	7.14 (3 feet by 22 feet)	5.44 (3 feet by 22 feet)	
Piers 1 and 2	5.7 (8 feet by 18 feet)	8.2 (8 feet by 17 feet)	14.2 (6.8 feet by 8 feet)	

Note: Effective dimensions for each limit state are provided following the associate demands in parentheses.

4.2.1 Shallow Foundations

Shallow footings founded on the native soils or engineered fill are planned to support both the new columns and the abutments. The results of this exploration indicate that the soils are capable of supporting the anticipated loads provided the recommendations are followed.

4.2.1.1 Shallow Foundation Bearing Resistance

Each new column and abutment of the widened Bridge H-1162E will be supported on shallow footings bearing on the native granular soils. Table 2 summarizes the calculated shallow foundation resistances for the effective foundation sizes.

The shear resistance between the foundation and the supporting soil is taken as the friction coefficient multiplied by the total load at the interface. A nominal sliding resistance of 0.67V is recommended for the soils described above, where V is the total vertical force. The shear resistance should be factored by 0.8 for the Strength Limit State.

Support Location	Factored Service Limit State Bearing Resistance (ksf)	Strength Limit State Factored Bearing Resistance (ksf) (φ _b =0.45)	Extreme Limit State Factored Bearing Resistance (ksf) (φ _b =1)	
Abutments	4.8	7.2	16	
Piers	5.7	13.5	30	

4.2.1.2 Shallow Foundation Settlement

Based on the dense granular soils encountered in the exploration, it is anticipated that the total settlement will be less than 0.5-inch and the differential settlement will be on the order of half of the total settlement. These settlement calculations were based on the anticipated loading conditions and utilizing the elastic methods. The maximum total settlement given is based on immediate settlement calculations.

4.2.1.3 Global Stability

Global stability analyses of the bridge abutments were conducted using SLIDE2 v.9.019, a two-dimensional limit equilibrium analysis program by Rocscience. Design parameters were estimated via the Mohr-Coulomb failure criterion. A seismic coefficient of 0.25g was used in the pseudo static analysis, based on one half of the PGA, as recommended for design. The results of the global stability analysis indicate that the embankments meet the required factors of safety as designed.

4.2.2 Earth Pressure

The lateral pressures imposed on a retaining structure depend on its ability to resist rotation (or movement) and its rigidity. Retaining walls that are free to slightly rotate develop an active lateral soil pressure, and walls that are not permitted to rotate laterally develop an at-rest lateral earth pressure. Resistance to lateral loads on foundations may be achieved by frictional resistance between the foundations and underlying soils and by passive earth pressures of backfills placed against the sides of foundations. Walls may be designed using the total lateral force as the given equivalent fluid pressures multiplied by the height of the walls. The total force is applied at one-third the wall height.

The table below provides recommended static and seismic design parameters for level backfill for NDOT granular backfill. The lateral earth pressure coefficients were calculated using Rankine theory. The soil parameters used were for the granular backfill material and included a friction angle of 34 degrees, a cohesion of 0 psf, and a moist unit weight of 125 pcf. The lateral earth pressure coefficients were calculated using the Mononobe-Okabe method with the horizontal seismic acceleration coefficient of 0.25g, or one-half of the peak ground acceleration (PGA) for the site of 0.5g.

Table 3: Static and Seismic Lateral Earth Coefficients and Equivalent Fluid Pressures for Stoker Bridge Widening

Lateral Earth Pressure Coefficients for Level Backfill	Lateral Earth Pressure Coefficient	Equivalent Fluid Pressure (pcf)	
Active Condition Ka	0.28	35	
At-Rest Condition Ko	0.44	55	
Passive Condition Kp	3.54	443	
Seismic Active Coefficient, Kae	0.45	56	

4.3 Median Retaining Wall and Soundwalls

Cast-in-place (CIP) cantilever retaining walls with an integral continuous slope barrier rail is proposed at the median, retaining the westbound I-80 embankment. Soundwalls are proposed along both the north and south limits of the westbound and eastbound limits of I-80 respectively. The soundwalls are proposed to bear on spread footings in embankment fill.

Presented below are the parameters for use in design. The material properties were conservatively assigned based on review of previous explorations; observations of the exposed embankment soil; and information presented on the NDOT Standard Plans for Road and Bridge Construction, 2020.

Table 4: Design Parameters for I-80 Media	an Retaining Walls and Soundwalls
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Parameter	Value
Internal Angle of Friction, Phi (degrees)	32
Unit Weight (pcf)	120
Cohesion (psf)	0
Equivalent Active Fluid Pressure (pcf)	36
Equivalent Passive Fluid Pressure (pcf)	390
Seismic Active Coefficient, Kae	0.48
Seismic Active Force, Pae (pcf)	58
Coefficient of Friction Between CIP Concrete and Soil	0.625
Coefficient of Friction Between Precast Concrete and Soil	0.5
Service Limit Bearing Resistance (psf)	4,000

Note: The passive and shear resistances should be factored in accordance with AASHTO 2020 Article 10.6.3.4-1

4.4 Seismic Design

4.4.1 Seismic Design Criteria

The seismic design criteria for the bridge site (39.5268°N, 119.8379°W) were developed utilizing the USGS seismic hazards tool in accordance with AASHTO 2020, considering the site location, and the subsurface information obtained from our geotechnical investigation. In addition, minimum seismic criteria for use in

design are listed by county in the NDOT Structures Manual. Peak ground acceleration (PGA) and both the short period (S_s) and long period (S_1) spectral acceleration coefficients were provided in the manual, but the remaining site factors were computed using the tables and equations found in AASHTO 2020, Articles 3.10.3.2 and 3.10.4. Both the USGS Mapped values and the NDOT structures manual values are presented below. The greater value should supersede.

Table 5: Seismic Design Parameters

Parameter	USGS Mapped Value	NDOT Structures Manual Values
Site Class	С	С
Peak ground acceleration (PGA)	0.481 g	0.5 g
Short Period Spectral Acceleration Coefficient (Ss)	1.155 g	1.25 g
Long Period Spectral Acceleration Coefficient (S1)	0.42 g	0.5 g
Peak ground acceleration coefficient (FPGA)	1.0	1.0
Site coefficient (Fa)	1.0	1.0
Site coefficient (F _v)	1.38	1.30
Mapped MCE peak ground acceleration (As)	0.481 g	0.5 g
Design Spectral Acceleration for short period (S _{DS})	1.155 g	1.25 g
Design Spectral Acceleration for 1 sec period (S _{D1})	0.58 g	0.65 g

4.4.2 Liquefaction

Liquefaction potential was assessed as part of this investigation. Liquefaction is the loss of stability and intergranular strength in water-saturated soil due to the increase of pore pressures during a dynamic event such as an earthquake. The potential for liquefaction is based upon several factors, such as the grain size distribution, relative density, overburden pressures of the soil, and the magnitude and duration of the seismic event. The site is categorized as a Seismic Zone 4 because the on-site acceleration coefficient S_{D1} is 0.65 (AASHTO 2020 Article 3.10.6). However, based on the density of the soil and the absence of groundwater within the depths explored, liquefaction potential is considered negligible.

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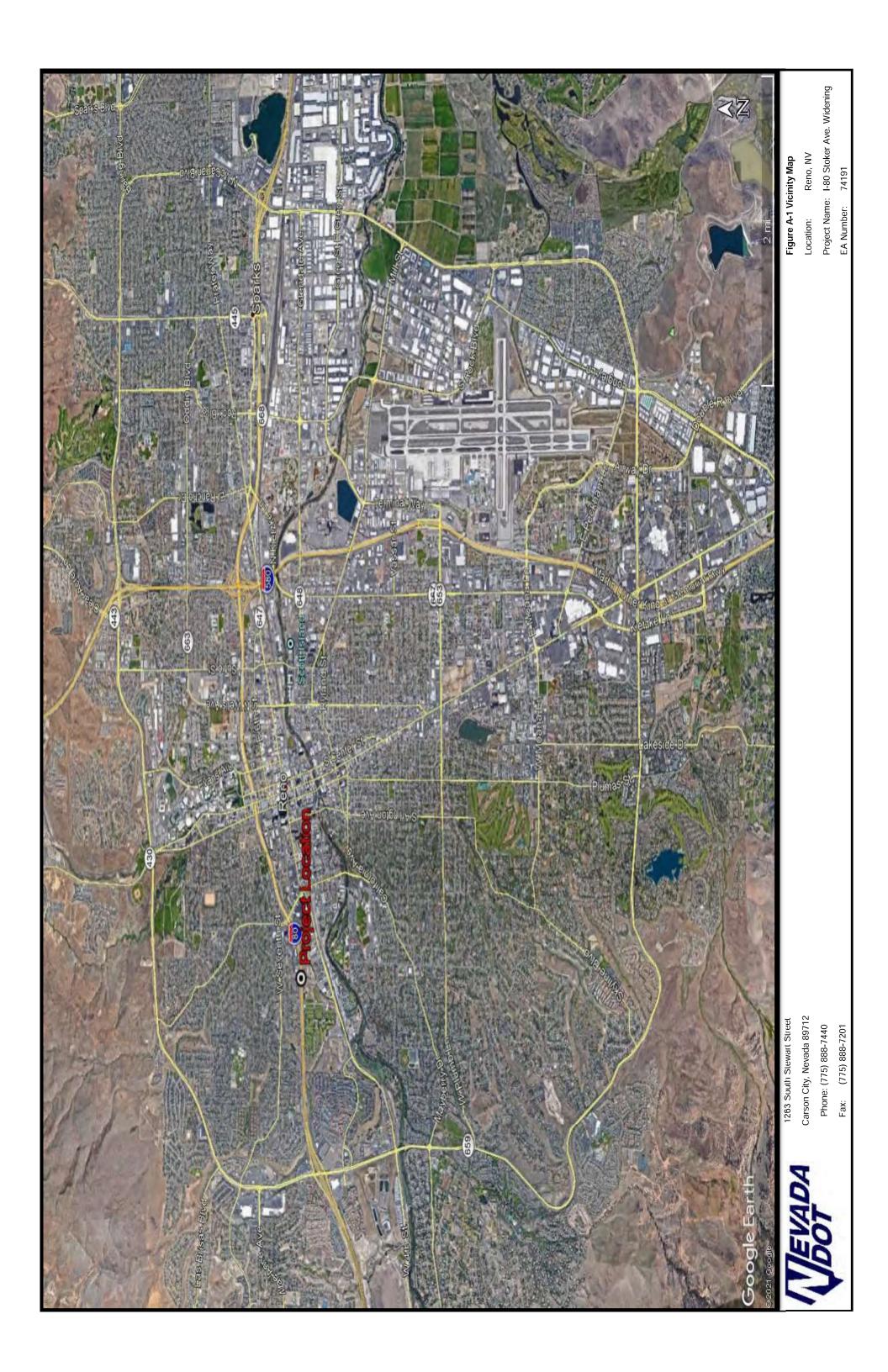
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6. Limitations

This report has been prepared by Nevada Department of Transportation (NDOT) Geotechnical Section under the supervision of those whose signatures appear herein. The interpretation of data, findings, and recommendations presented in this report were developed from our geotechnical investigation.

Variations from the conditions portrayed in the explorations often occur which are sometimes sufficient to require modifications in the design. The recommendations provided in this report apply only to the proposed improvements described herein. If the proposed project is modified or relocated, or if the subsurface conditions found during construction differ from those described in this report, NDOT Geotechnical Section should be contacted immediately to assess the new information or changed conditions and determine if additional recommendations are required.

Appendix A Figures





Appendix B Logs of Borings

KEY TO BORING LOGS

PARTICLE SIZE LIMITS								
CLAY	SILT		SAND		GR	AVEL	COBBLES	BOULDERS
		FINE	MEDIUM	COARSE	FINE	COARSE		
.00	2 mm #	200 #	#40 #	10 #	4 ¾ ii	nch 3	inch 12	inch

USCS GROUP	TYPICAL SOIL DESCRIPTION
GW	Well graded gravels, gravel-sand mixtures, little or no fines
GP	Poorly graded gravels, gravel-sand mixtures, little or no fines
GC	Clayey gravels, poorly graded gravel-sand-clay mixtures
SW	Well graded sands, gravelly sands, little or no fines
SP	Poorly graded sands, gravelly sands, little or no fines
SM	Silty sands, poorly graded sand-silt mixtures
SC	Clayey sands, poorly graded sand-clay mixtures
ML	Inorganic silts and very fine sands, rock flour, silty or clayey fine sands with slight plasticity
CL	Inorganic clays of low to medium plasticity, gravelly clays, sandy clays, silty clays, lean clays
OL	Organic silts and organic silt-clays of low plasticity
MH	Inorganic silts, micaceous or diatomaceous fine sandy or silty soils, elastic silts
СН	Inorganic clays of high plasticity, fat clays
ОН	Organic clays of medium to high plasticity
CS	Claystone/Siltstone
PT	Peat and other highly organic soils

MOISTURE CONDITION CRITERIA

MOISTURE CONDIT	ION CRITERIA	SOIL CEMENTATION CRITERIA		
<u>Description</u> Dry	<u>Criteria</u> Absence of moisture, dusty, dry to touch.	<u>Description</u> Weak	<u>Criteria</u> Crumbles or breaks with handling or little finger pressure.	
Moist	Damp, no visible free water.	Moderate	Crumbles or breaks with considerable finger pressure	
Wet	Visible free water, usually below groundwater table.	Strong	· Won't break or crumble w/finger pressure	

 ∇

Groundwater Elevation Symbols

	STANDARD PENETRATION	CLASSIFIC	
	GRANULAR SOIL		CLAYEY SOIL
BLOWS/FT	DENSITY	BLOWS/FT	CONSISTENCY
0 - 4	VERY LOOSE	0 - 1	VERY SOFT
5 – 10	LOOSE	2 - 4	SOFT
11 - 30	MEDIUM DENSE	5 - 8	MEDIUM STIFF
31 - 50	DENSE	9 - 15	STIFF
OVER 50	VERY DENSE	16 - 30	VERY STIFF
	ration Test (N) 140 lb hammer on 2-inch O.D. x 1.4 inch I.D. sampler.	31 - 60 OVER 60	HARD VERY HARD

Blow counts on Calif. Modified Sampler (NCMS) can be converted to NSPT by:

(NCMS)(0.62) = NSPT

Automatic Hammer Engergy: Rig # 1627: 82.5% Rig # 1082: 84%

SAMPLER NOTATION

TEST ABBREVIATIONS

CD CONSOLIDATED DRAINED CH CHEMICAL (CORROSIVENESS) CM COMPACTION CU CONSOLIDATED UNDRAINED D DISPERSIVE SOILS DS DIRECT SHEAR E EXPANSIVE SOIL G SPECIFIC GRAVITY H HYDROMETER HC HYDRO-COLLAPSE K PERMEABILITY	O ORGANIC CONTENT OC CONSOLIDATION PI PLASTICITY INDEX RQD ROCK QUALITY DESIGNATION RV R-VALUE S SIEVE ANALYSIS SL SHRINKAGE LIMIT U UNCONFINED COMPRESSION UU UNCONSOLIDATED UNDRAINED UW UNIT WEIGHT W MOISTURE CONTENT	CMS CALIF. MODIFIED SAMPLER ¹ CPT CONE PENETRATION TEST CS CONTINUOUS SAMPLER ² CSS CALIFORNIA SPLIT SPOON P PUSHED (NOT DRIVEN) PB PITCHER BARREL RC ROCK CORE ³ SH SHELBY TUBE ⁴ SPT STANDARD PENETRATION TEST TP TEST PIT
SOIL COLOR DESIGNATIONS ARE FROM EXAMPLE: <u>(7.5 YR 5/3) BROWN</u>	THE MUNSELL SOIL COLOR CHART.	1- I.D.= 2.421 inch 2- I.D.=3.228 inch with tube; 3.50 inch w/o tube 3- NXB I.D.= 1.875 inch 4- I.D.= 2.875 inch

Revised June 2018

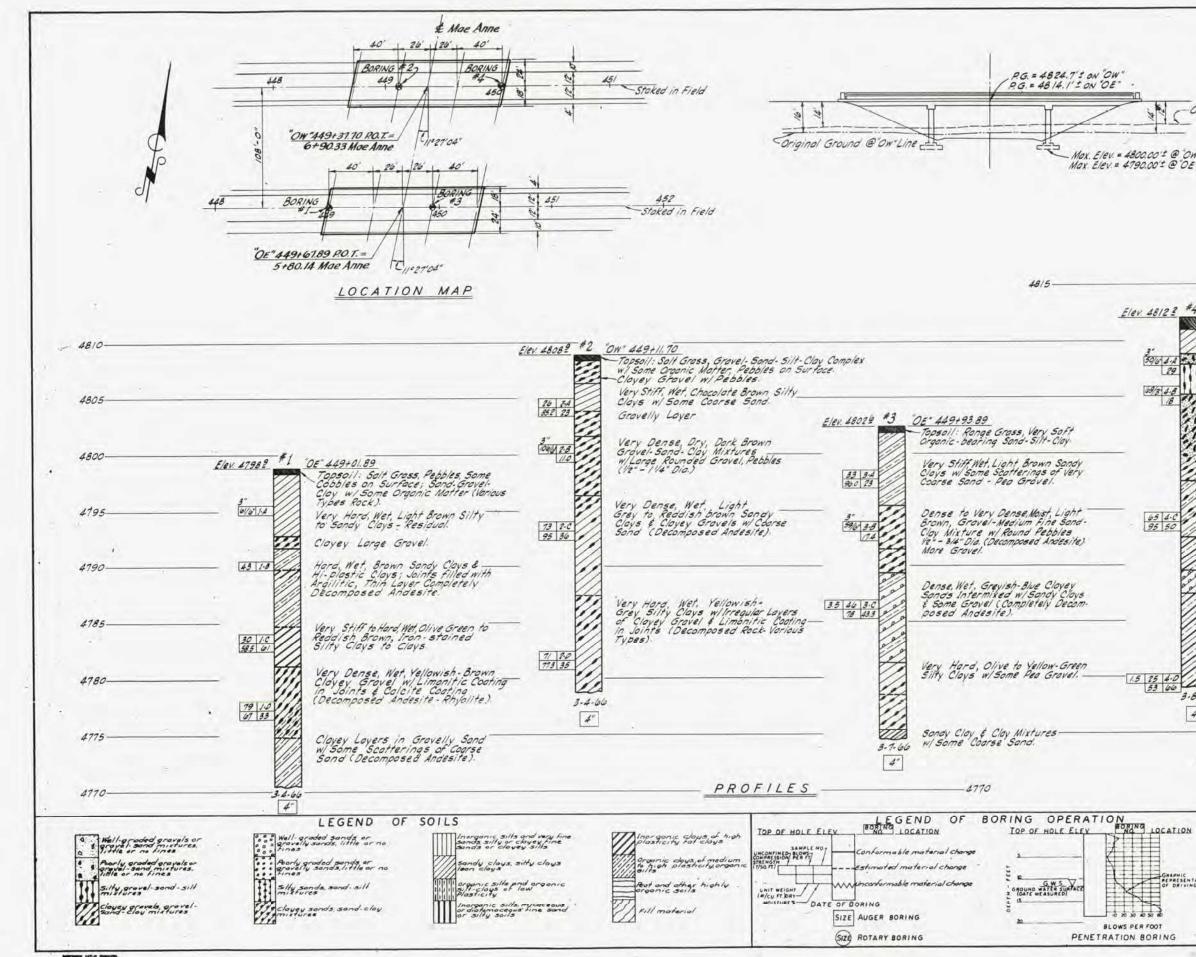
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	Carson	City, NV 0	9/12		TOTAL D			61.5										KFILLED	, _,	Yes	DA	TE 5/12/2	21
┝								(9															
	(ft) (ft)	DEPTH (ff)	SAMPLE NO.	ТҮРЕ	BLOWS / 6"	Uncorrected N Value	Recovery (%)	MOISTURE CONTENT (%)	% PASSING NO.200	LIQUID LIMIT	PLASTICITY INDEX				DE	MATE ESCR	IPTIO					REMAR	KS
												FILL	FILL: A	sphalti	c Concre	ete 7",	Aggreg	jate base	e co	urse 5	".		
	4601.0 -	-1										CL/	<u> </u>					/, stiff, m					
	4600.0 -	_	1		17 26 31	57	53		13				subang	ular gra	GRAVE avel, low k brown	plastic	clay an ity clay	d sand (v, very d	GP- lens	GC), e, mois	st,		
	4599.0 - 4598.0 -		2		26 33 44	77	93	6	11	23	4												
	4597.0 -	5																					
	4596.0 -	E	3		23 40 50/6"		93		12	23	4												
	4595.0 -	-7																					
	4594.0 -		4		41 60/6"	0	93		7	23	4												
	4593.0 -	-9			50/3"																		
	4592.0 -	- 10																					
	4591.0 -		5		50 50/3.5"		53		14			• GP • GC											
	4590.0 -	12																					
_	4589.0 -	13																					
8/3/21	4588.0 -	14																					
GDT	4587.0 -	15																					
18.10.10		16	6		41 50/4.5"		53		11														
0G 20	4585.0 -	17																					
MART L	4584.0 -	18																					
VDOT S	4583.0 -	19																				18.5-19.0 H drilling-grin	
A L A	4582.0 -	20																				on rock.	
NING.0	4581.0 -	21	7	N	14 42 50/4.5"		93	12	20	27	6							C-GM), S e, moist					
WIDE	4580.0 -	- 22			50/4.5							GC											
KER AVE		23										GM											
0 STOK	4578.0 -	24										• GP	Poorly (GRAVF	 Lwith s	ilty cla	y and sa	 nd			23.5-25 Grinding or	ו ו
<u>و</u>		_										•GC/	(GP-ĞC	Č), Sub	angular			obble la		low		rock, cobbl	
SMART SOIL LOG 180 STOKER AVE. WIDENING GPJ NDOT SMART LOG 2018 10 10 GDT 8/3	Stand Penet Test								FILL			USC Plas Clay		US Po Gr Cla	avel with	ded	USCS Claye Grave	S Silty y I		USCS Sand	5 Silty	USC Grav	S Silty el

		-				5/11	/21			BOR	ING LO	CG				39.5268	SHEET 2 OF 3
IN EVAL	DA		TART D		-	5/12		_							LATITUDE LONGITUDE	-119.8379)
SAFE AND CONNEG	TIO		ND DAT ROJEC [.]		-			– er Av	/e. W	/ideniı	ng					J. Crosby	
					-		o, N\				0				OPERATOR	O. Altimira	ano
Materia l s Divisi			.A. #	21 N	_	7419					-						D-120 (1627)
Geotechnical Sec			ORING		-	B - 1							ATER LE		METHOD	6" HSA	
1263 S. Stewart Carson City, NV 8			ROUNE			4602	2.0				DATE	TIME	ft	ft	HAMMER	Automatic	;
	5112		OTAL D			61.5									BACKFILLED	Yes	ATE 5/12/21
	Ö		-	7			0	F	~								
ELEV. (ff) (ff) (ff)	SAMPLE NO.	TYPE	BLOWS / 6"	Uncorrected N Value	Recovery (%)	MOISTURE CONTENT (%)	% PASSING NO.200	LIQUID LIMIT	PLASTICITY INDEX	GRAPHIC LOG			N	IATEF	RIAL		REMARKS
DE EL	AMPI	∣≻∣	ROW	Ncor N <	Rec (%	NUTE	NO.	GUIE	LASL	GRA					PTION		
	Ś					20	8	5	₽.		nlaatio	itu diffic	ult to dril		donao maiat d	lark brown /	boulder layer.
4576.0 -26	8	N	49 49		100		15	28	20						dense, moist, d d (GC-GM), Si		boulder layer.
4370.0 20			50/4"								low pla	sticity,	very dens	se, mois	st, dark brown.		26.5-26.9 Rig
4575.0 - 27																	Chatter, rough
4574.0 - 28										GC							granular material.
4573.0 - 29										GM4							
4572.0 - 30			46														
4571.0 -31	9		50/4"		53		15			• GP	Poorly	araded	GRAVE	with c	ay and sand (G		
4570.0 - 32										GC/	subang	gular gra	avel, low moist, d	plastic	ity, very hard co	bble layer	
4569.0 - 33											Silty cl	ayey GF	RAVELW	ith sand	d (GC-GM), Su difficult drilling	bangular,	
4568.0 - 34										P	36') m	oist, dar	k brown.	e (very		1011 35 10	
4567.0 - 35																	
4566.0 - 36	10		50/5"		47		16										Rock bit
4565.0 - 37										P							grinding on cobble
4564.0 + 38																	37.5-40 Very Hard Drilling
4563.0 + 39																	riara Drining.
4562.0 + 40																	
4561.0 + 41	11		57 50/4.5"		87		17	26	6	GC-							
4560.0 -42										GM/							
4559.0 + 43																	
4558.0 + 44																	
4557.0 + 45																	
4556.0 + 46	12		50/3.5"		47		20										
4555.0 - 47																	
4554.0 + 48																	
4553.0 + 49																	
-																	
Standard Penetration Test						ا ا	FILL			USC Plas Clay	S Low ticity	Po Gr	SCS oorly-grad avel with ay	led	USCS Silty Clayey Gravel	USCS Silty Sand	USCS Silty Gravel

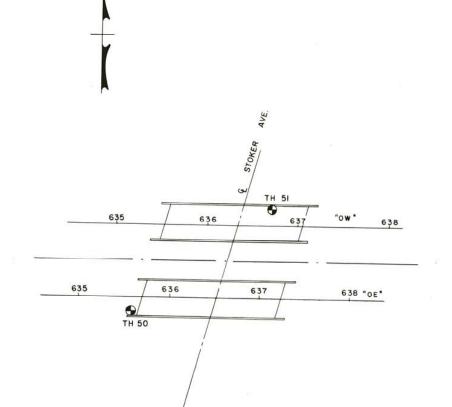
SMART SOIL LOG 1-80 STOKER AVE. WIDENING GPJ NDOT SMART LOG 2018 10.10.GDT 8/3/21

ſ			2					5/11	/21			BOR	ING L	OG				39.52	268	SHEET 3 OF 3
		EVAL	DA	-	START D		-	5/12		_							LATITUDE	110		
	V	DOT	-		ND DAT		-			- or Av	ر م	/ideniı	ha				LONGITUD			
	V SAR	E AND CONNEC			ROJEC		-		0, NV		/C. V	nuerm	iy				ENGINEER		timirai	
	Mater	ia l s Divisi	on		OCATIC	DN	-	7419									_ OPERATOR	·		-120 (1627)
	Geotecl	nnical Sec	ction		E.A. #		-	B-1	21				GF	OUNDW	ATER LI			6" HS		120 (1027)
	1263 \$	S. Stewart	St		BORING		-	4602	2 0				DATE	TIME	DEPTH	ELEV.		Auto		
	Carson (City, NV 8	9712		GROUNE		v. n	61.5									HAMMER	Vaa		5/12/21
				Т	OTAL D)EPTH	lft_										BACKFILLE	D	DA	TE 5/12/21
	ELEV. (ff)	DEPTH (ft)	SAMPLE NO.	түре	BLOWS / 6"	Uncorrected N Value	Recovery (%)	MOISTURE CONTENT (%)	% PASSING NO.200	LIQUID LIMIT	PLASTICITY INDEX	GRAPHIC LOG				MATEI ESCRI	RIAL PTION			REMARKS
	4551.0 -	51	13		19 41 50/3 <u>.</u> 5"		87		30	28	4	SM	Silty S moist,	AND (Sl reddish	M), few s to dark	subangı brown.	ular gravel, ve	ery dense,		
	4550.0 -	- 52										• • •	 Silty G	RAVEL	with sar	 nd (GM)	Subangular	r gravel ve	rv –	
	4549.0 -	53										GМ	dense	, moist,	dark bro	wn.	. essengele	g ,	.,	
		54 55										SM	dense	, moist,	reddish	o dark l			very	
		56	14	N	57 50/3"		53		13			k	Silty G alterna	RAVEL	with sar nd layers	nd (GM) , very d	: Subangula ense, moist,	r with dark browr	ı.	
	4545.0 -	57																		
	4544.0 -	- 58										GM								
	4543.0 -	- 59										T								
	4542.0 -	-60	15		80/5"		53	8	14											
	4541.0 - 4540.0 -	61 62													ated at 6	1.5 feet.	Groundwate	er was not		
	4539.0 -	63											encou	ntered						
F 8/3/21	4538.0 -	-64																		
0.10.GD	4537.0 -	65																		
G 2018.1	4536.0	- 66 - 67																		
IART LO	4535.0 - 4534.0 -	68																		
DOT SM	4533.0 -	- 69																		
GPJ N	4532.0 -	-70																		
DENING	4531.0 -																			
AVE. WI	4530.0 -	-72																		
)KER /	4529.0 -	-73																		
I-80 STC	4528.0 -	-74																		
SMART SOIL LOG 1-80 STOKER AVE. WIDENING GPJ NDOT SMART LOG 2018.10.10.GDT 8/3/21	Standa Penetr Test								FILL			USC Plast Clay	S Low icity	Po Gr	SCS oorly-gra avel with ay	ded	USCS Silty Clayey Gravel	USC Sand	S Silty	USCS Silty Gravel

Appendix C Previous Explorations



PED ROAD BTATE CONTROL STATE SHEET TOTAL PROJECT COUNTY 63 NEVADA I-080-1(19)7 HASHOS 31-082 5-9A -Original Ground @ "OE" Line Mox. Elev. = 4800.00' @ "OW" LINE Max Elev = 4790.00' COE" LINE 4815 Elev. 48123 #4 "ON" 450+03.70 SIMI ? Topsoil: Range Gross, Very Loose Sand-Silt-Clay Complex, Some Organic Matter 4810 Very Hard, Wet, Grey-Brown Sondy Clays. 39/6 d.A . Gravel- Sand Mixture w/Pebbles. 29 Very Dense, Moist, Buff, Lorge Gravel-Sond-Silt Mixtures W/Layer Pebbles. 49/3 4-8 4805 Very Dense Gravel-Sand Wilrregular Layers of Clayey Gravel (Decomposed Andesite). 4800 Dense, Wet, Yellow-Brown to Buff Cloyey Gravels Alternating wisondy Clays, Limonitic Cooting along Joints 4795 (Decomposed Andesite). 65 4.C 95 50 Silty Clays w/ Irregular Loyers of Clay. 4790 Loyer of Sond-Cloy Mixture. 4785 Very Stiff Wet, Olive Green Clays WI Trregular Layers of Silty Clays. 4780 1.5 25 4.0 11 3-8-66 4" 4775 Note: No Ground Water Encountered. (tester R.E. No. 1643 APPROVED: ____ STATE OF NEVADA DEPARTMENT OF HIGHWAYS MAE ANNE GRADE SEPARATION GRADHIC AEPACSENTATION OF DRIVING H-767E & H-767W LOG OF TEST BORINGS APRIL 11. 1966 SPROUT ENGINEERS, INC. 950 INDUSTRIAL WAY W-1088-F JWH/LH SPARKS. NEVADA

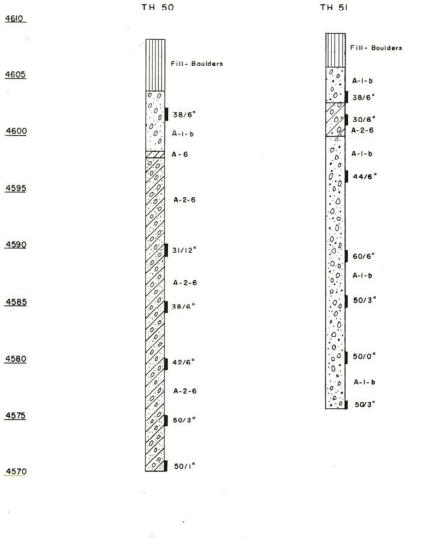


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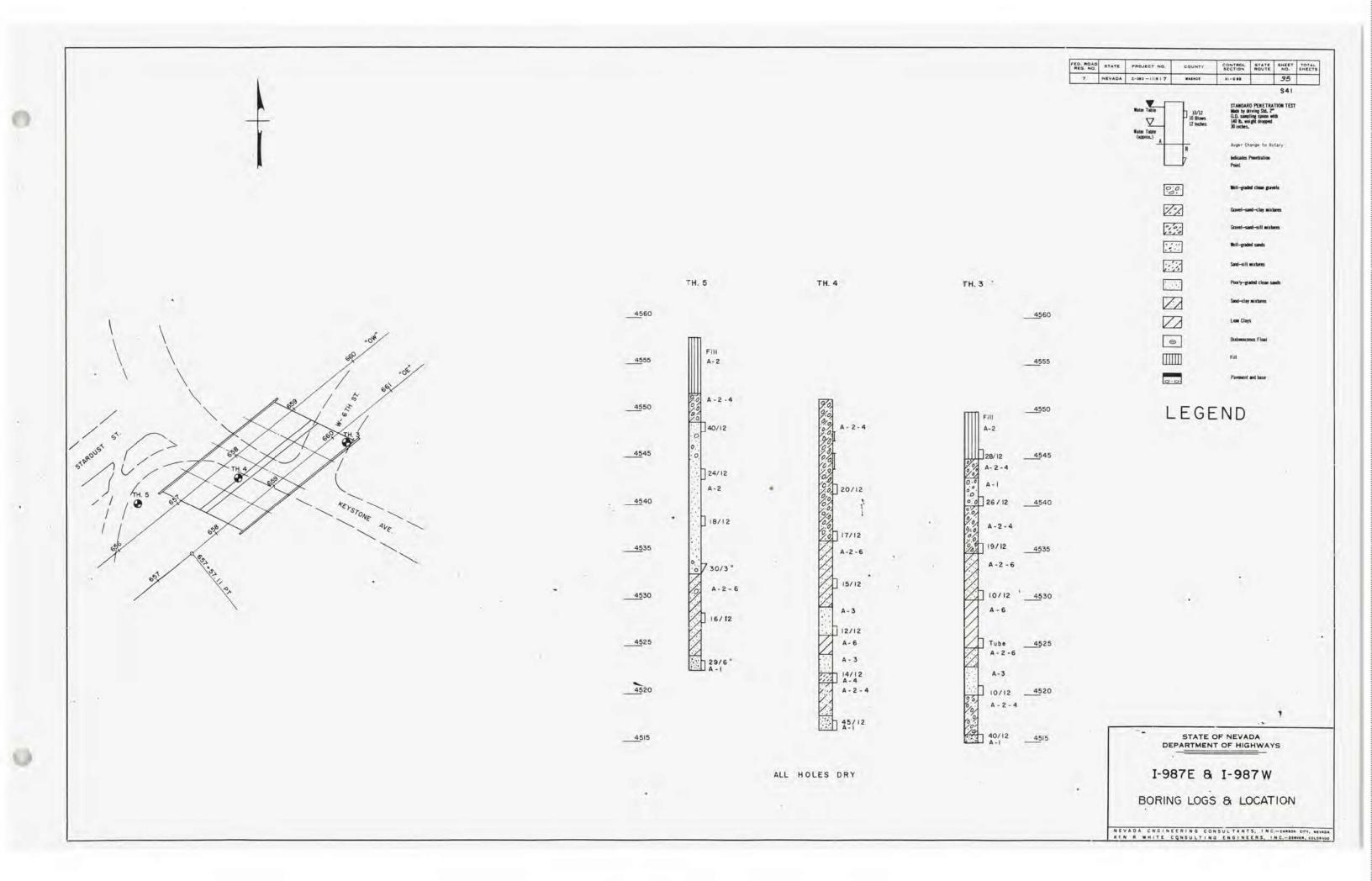
NEVA	D A	ENGIN	EERIN	G	CONSULTANTS, INC CARSON CITY, NET	VADA
KEN	R.	WHITE	CONSU	LT	ING ENGINEERS, INC DENVER, COLOR	RADO

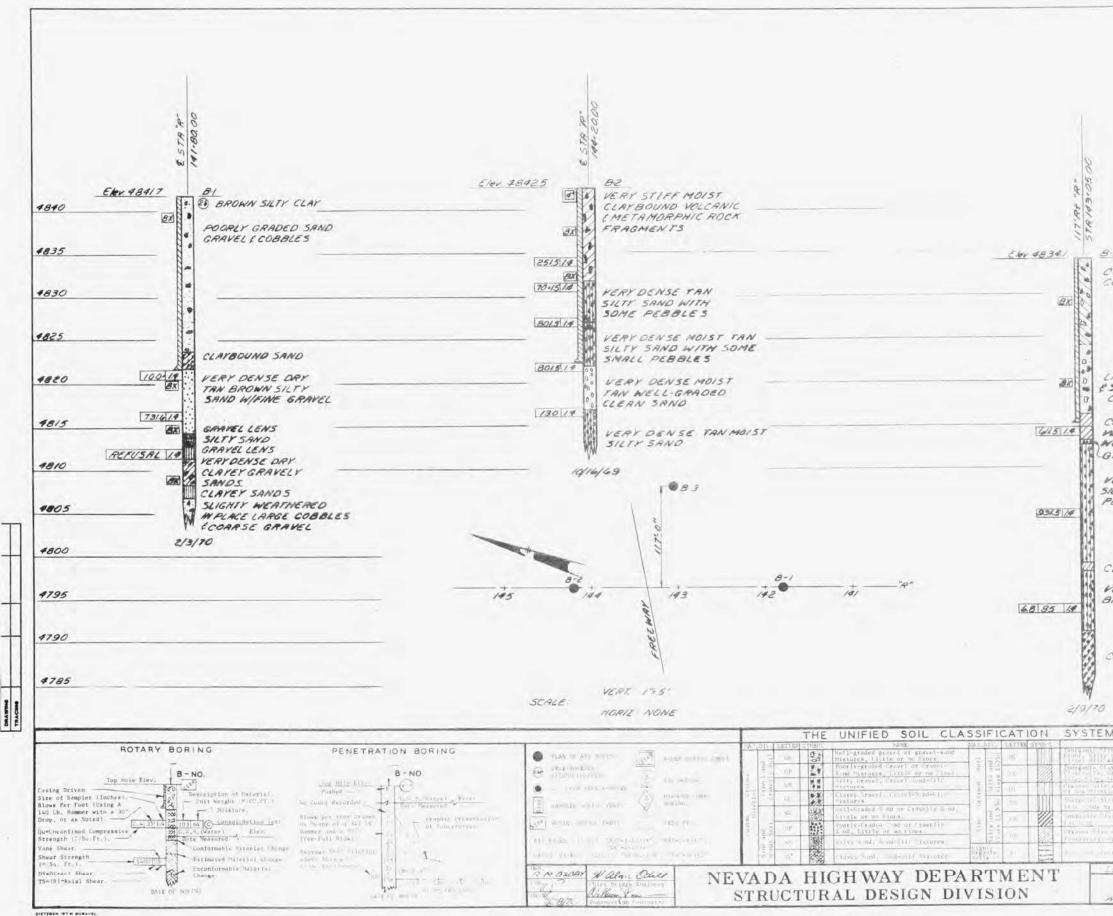
BORING LOGS & LOCATION

H-1162E & H-1162W

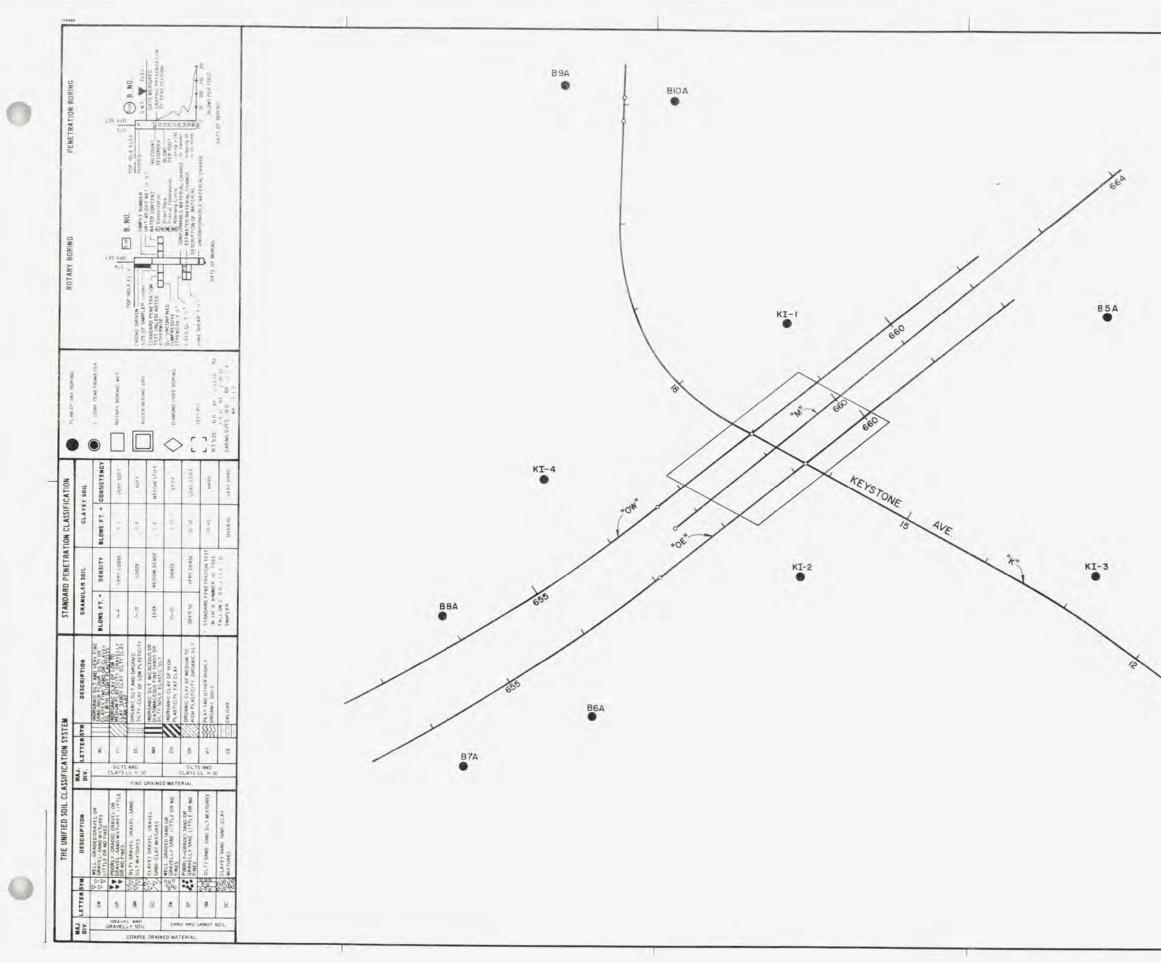
STATE OF NEVADA

FED. ROAD STATE PROJECT NO. CONTROL STATE SHEET TOTAL SECTION ROUTE NO. CHEETS COUNTY 7 NEVADA I-080-1(19)7 WASHOE 31-082 82 S28 Water Table Water Table (approx.) STANDARD PENETRATION TEST Made by driving Std. 2" O.D. sampling spoon with 140 lb. weight dropped 30 inches. 10/12 10 Blows 12 Inches Indicates Penetration Point 0.0. Well-graded clean gravels 0% Gravel-sand-clay mixture 0,00 Gravel-sand-silt mixtures Well-graded sands 4610 1.1 Sand-silt mixtures Poorly-graded clean sand 4605 \mathbb{Z} Sand-clay mixtures \mathbb{Z} Lean Clays Θ Diatomaceous Float 4600 TTTT Fill 0.0 Pavement and base 4595 4590 4585 4580 4575 4570



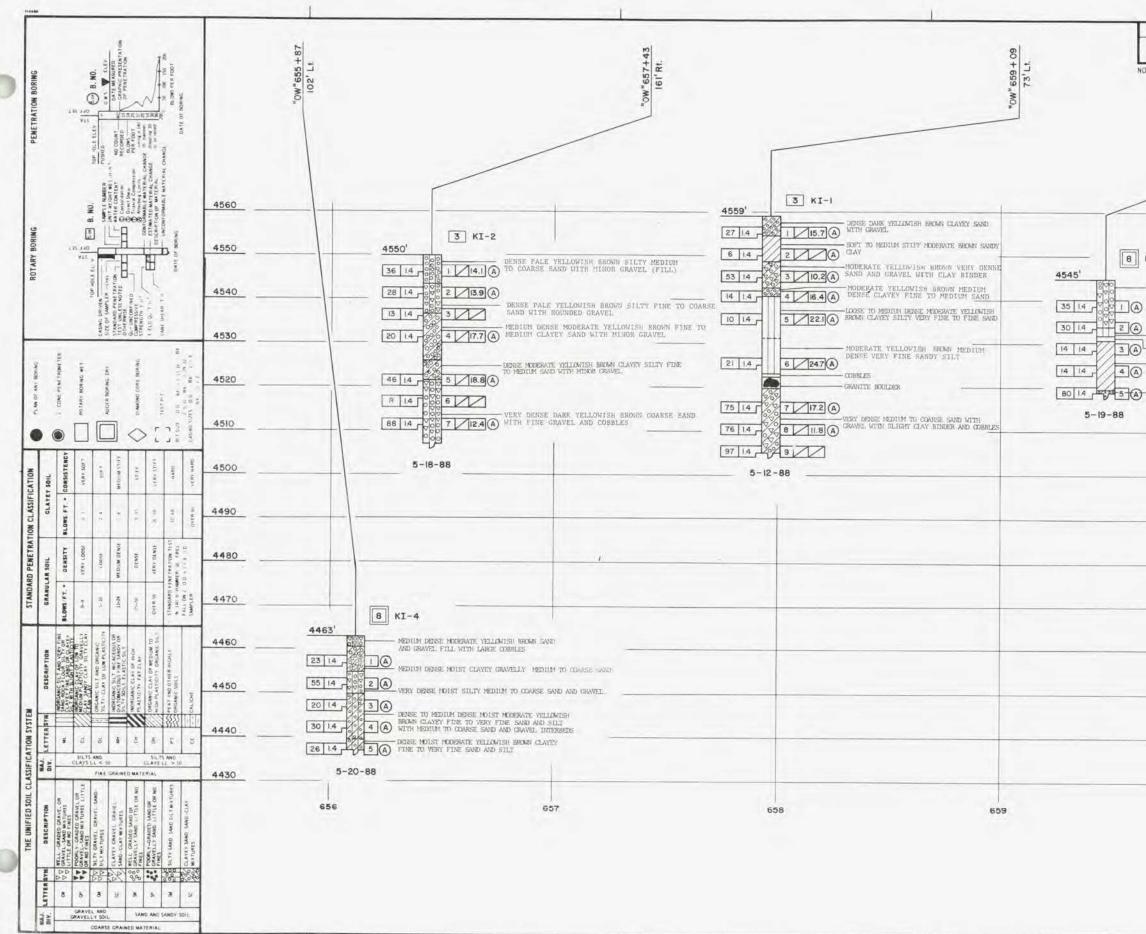


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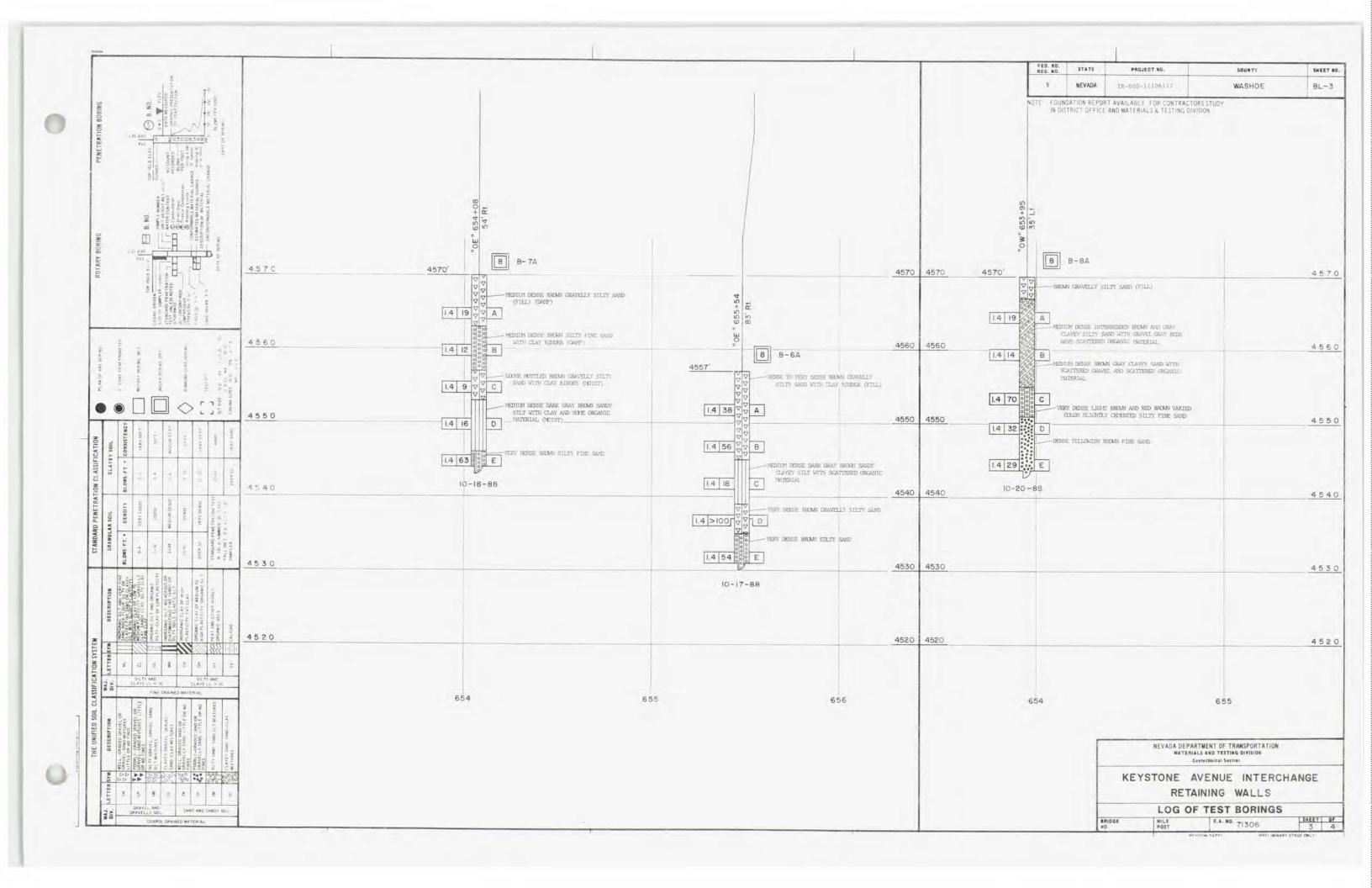


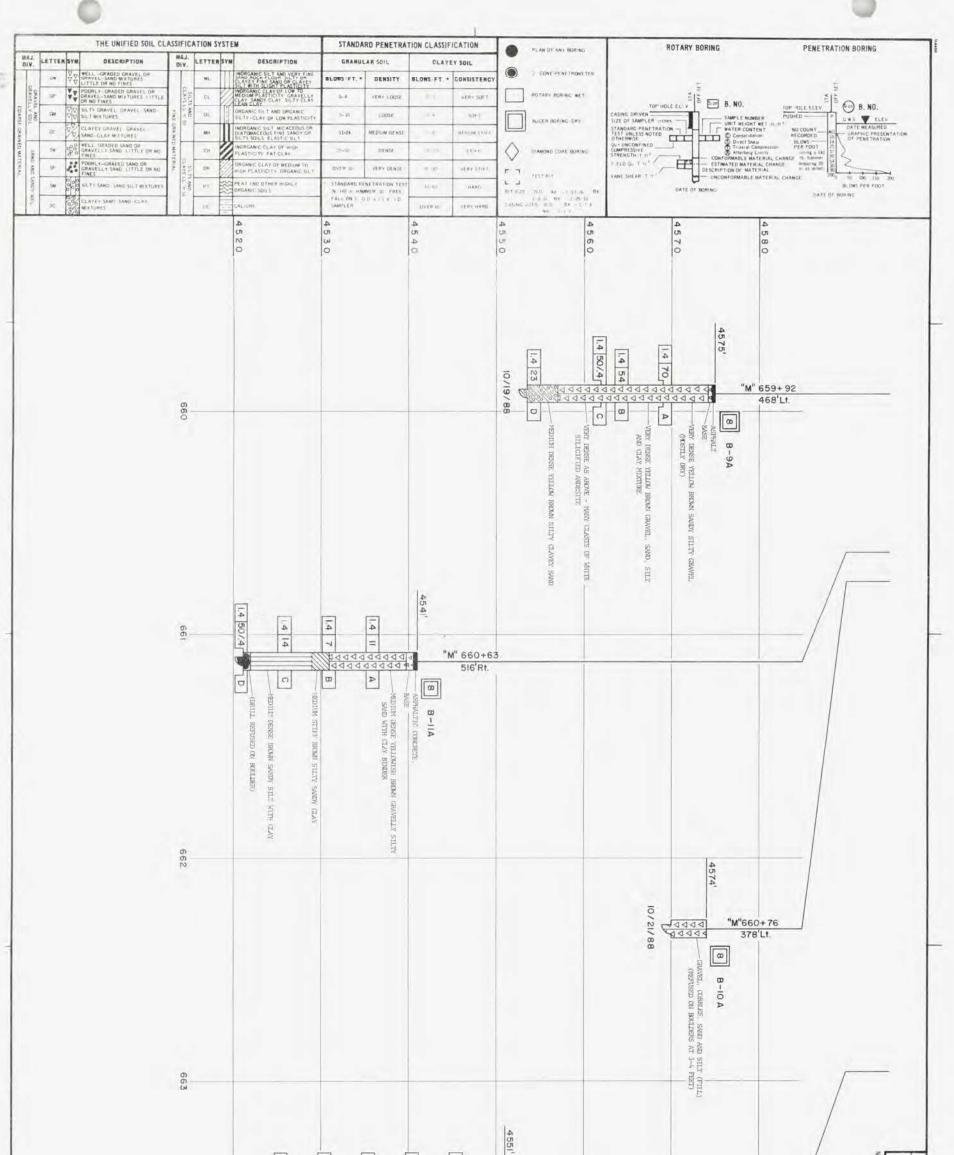
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KEY TO BORING LOGS

00 mm	0.002 mm •200 •40 •10 •4 19 mm 75 mm 300 mm	mm 7	4 19	ö •	40	•200 •	m	0.002
		COARSE	FINE	COARSE	MEDIUM COARSE FINE COARSE	FINE		
COBBLES BOULDERS	COBBLES	GRAVEL	ଦୁ		SAND		SILT	CLAY SILT
	N N	PARTICLE SIZE LIMITS	SIZE	ICLE	PART			

Wet	Moist	Dry	Description	NOIS	⋻⋓ҕ⋶⋹∊∊ゃゃゃゃゃゃゃゃ	UCSC (
				TURE		GROUP
Visible free woter, usually below groundwater table	Damp, no visible free water	Absence of moisture, dusty, dry to touch	Criterio	MOISTURE CONDITION CRITERIA	Wellgraded gravels, gravel-sand mixtures, little or no fines Poorly graded gravels, porty graded gravel-sand-silt mixtures Sity gravels, poorly graded gravel-sand-silt mixtures Clayey gravels, poorly graded gravel-sand-clay mixtures Poorly graded sands, gravelly sands, little or no fines Poorly graded sands, gravelly sands, little or no fines Sity sands, poorly graded sand-clay mixtures Clayely sands, poorly graded sand-clay mixtures Clayely sonds, poorly graded sand-clay mixtures Clayels and organic silt-clays of low plosticity, gravely clays, sandy clays, sity cl Gradnic silts and arganic silt-clays of low plosticity Inorganic clays of high plosticity, fot clays Organic clays of medium to high ploticity Coliche Peot and ather highly arganic solls	TYPICAL SOIL
Strong	Moderote	Weak	Description	SOIL C	ixtures, little or no mixtures, little or el-sand-clay mixtures tel-sand-clay mixtures sl, little or no fine t mixtures clay mixtures clay mixtures clay mixtures clay structures clay structures clay structures clay structures clay structures clay structures clay structures clay structures clay structures clay structures structures platicity	
Won't break or crumble with finger pressure	Crumbles or breaks with considerable finger pressure	Crumbles or breoks with hondling or little finger pressure	Criterio	CEMENTATION CRITERIA	Wellgraded gravels, gravel-sand mixtures, litte or no fines Poorly graded gravels, gravel-sand mixtures, litte or no fines Sity gravels, poorly graded gravel-sand-clay mixtures Clayey gravels, poorly graded gravel-sand-clay mixtures Poorly graded sands, gravely sands, litte or no fines Sity sands, poorly graded sand-clay mixtures Clayely sonds, poorly graded sand-clays in plasticity, gravelly clays, sondy clays, sity clays, lean clays Organic silts and arganic silt-clays of law plasticity, gravelly clays, sondy clays, elastic silts Inorganic silts and arganic silt-clays of law plasticity Organic clays of high plasticity, fot clays Organic clays of medium to high platicity Coliche Peot and ather highty arganic soils	DESCRIPTION

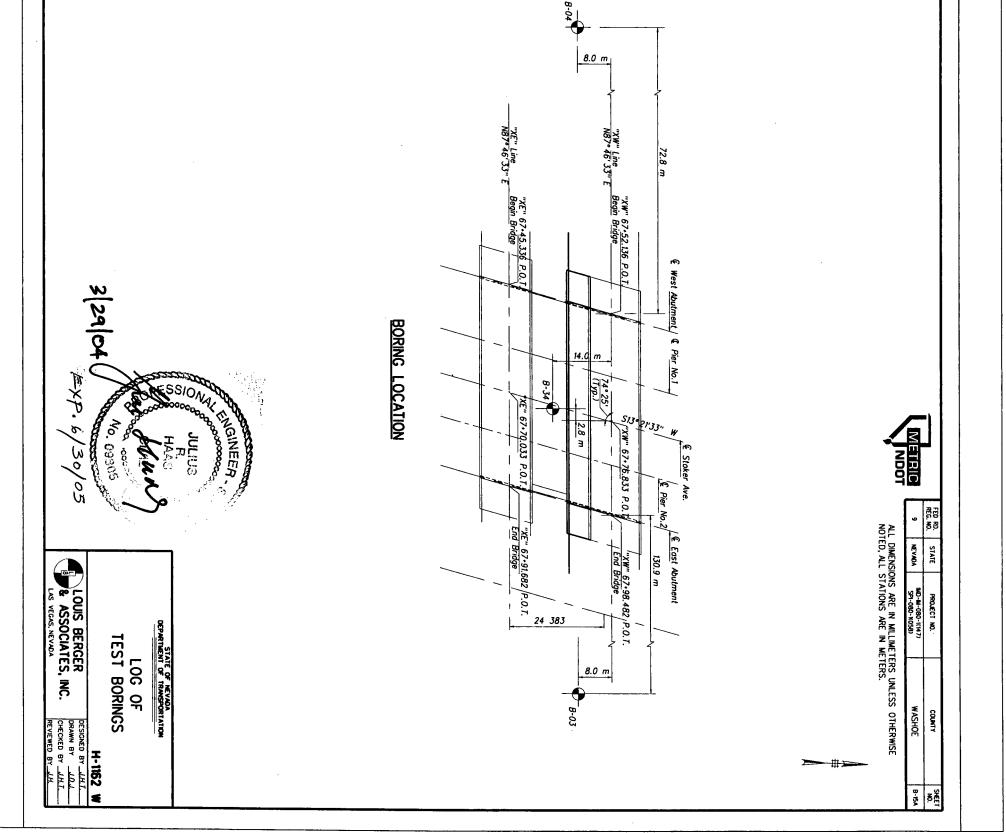
STANDARD PENETRATION CLASSIFICATION.

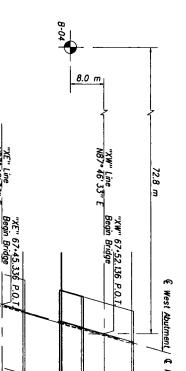
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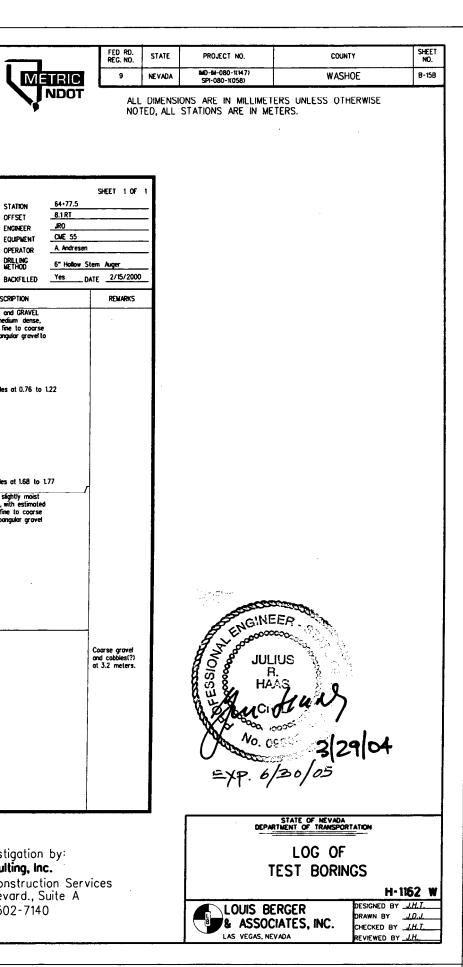
Groundwater Elevation Symbols

SOIL COLOR DESIGNATIONS ARE FROM THE MUNSELL SOIL COLOR CHART. EXAMPLE: (7.5 YR 5/3) BROWN	CD CONSOLIDATED DRAINED 0 ORCANIC CONTENT CH CHEMICAL (CORROSIVENESS) PI PLASTICITY INDEX CU CONSOLIDATED UNDRAINED ROD ROCK QUALITY DESIGNATION D DISPERSIVE SOIL E EXPANSIVE SOIL G SPECIFIC GRAVITY USES SV SIEVE AWALYSIS E EXPANSIVE SOIL H HYDROMETER H CONFOLLAPSE UN UNCONFINED COMPRESSION H HYDROMETER W MOISTURE CONTENT	TEST ABBRE VIATIONS	
UD. • 88.9 mm w/o tube NXW 1.D. • 73 mm	CMS CALF. MODIFIED SAMPLER CPT CONE PENETRATION CS CONTINUOUS SAMPLER CSS CALFORNIA SPLIT SPOON P PILSHED (NOT DRIVEN) PB PITCHER BARRREL RC ROCK CORE@ SPT STANDARD PENETRATION TEST TP TEST PIT D I.D. 61.5 mm (2) I.D. 61.5 mm	SAMPLER NOTATION	





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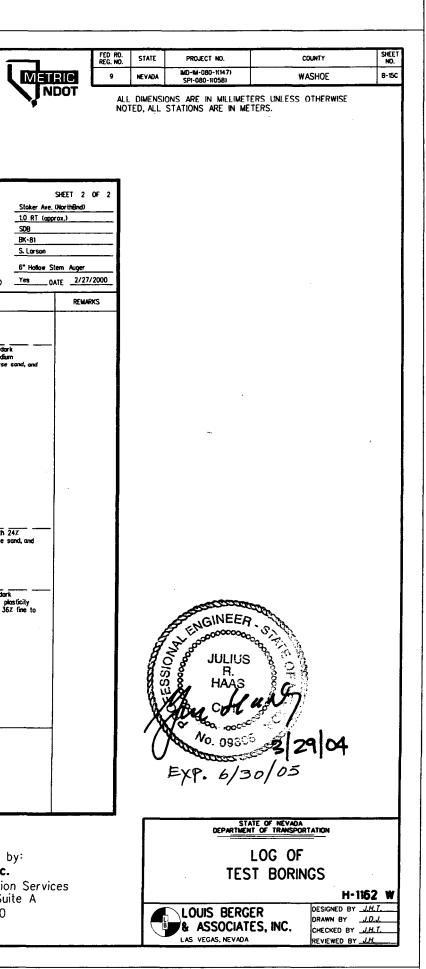


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			E.	A.•	02	12-01-1			GROUNDW	_		OPERATOR	A. Andresen	
- S.			ci ci	ROUND ELEN	, 140	0.56 (m)		OATE DEP	THm	ELEV. m	DRILLING	6" Hollow S	item Auger
	- 7 7	ł		MMER DROP	P SYSTE		atheod					BACKFILLED	Yes 0	ATE 2/15/2000
ELÉV. (m)	DEPTH (m)	NO.	TYPE	BLOW C 150 mm Increments	CUNT Lost 300_mo	Percent Recov'd	LAB TESTS	USCS Group	00001 X 0		MATERIAL D	ESCRIPTION GRAVEL (Fill)		REMARKS
	0.00			4					dark brown	, very	moist to n	loist, very dense,		
	0.38	3A	SPT	9 50/75	50/75	80			fine to coo	dark brown, very moist to moist, very dense, with estimated 5x non-plastic lines, 55-65% fine to coarse sand, 30-40% fine to coarse, subangular gravel to +25mm in diameter. Coarse graveland cobbles at 0.3 to 0.76 meters (base of fill).				
			-					SP	Coarse gra					
	0.76	Butk	AC						0.76					
		1	[[•]		[CLAYEY SA	ND br	own, moist, low okreti	dense, with fines 60-657 fin	- •	1
1399.56 -	-1	38	SPT	9 21 26	47	83		SC	to coorse s	estimated 25-302 low plastic fines,60-65% fine to coarse sand, 10% fine to coarse, angular gravelto +19mm in diameter.				
	1.22													
	1.37								1.37					
	-			t					SILTY, CLAY	EY S	AND brown,	moist, medium		
	1.52								65-70% fine	estime to c	oted 15-202 coarse sand	low plastic fines 15% fine to coar	, se.	
	[зc	SPT	5	18	100		SC SM	angular grad grades to f	rei to	•19mm in c	formeter. Unit		
	1.98			9				-						
1398.56 -	-2 1.30													
									2.13	10 6-		The TRA		
	2.29					•						dense, with 35% to coarse sand,		
									8% fine to	coarse	e, angular to	subangular grave	4	
	[i	30	SPT	7	43	100	MC, SV, PI		to +9.5mm	n 000	meter.]
	-	30	351	32	73	~~~	mu, 3v, ri							
	2.74													
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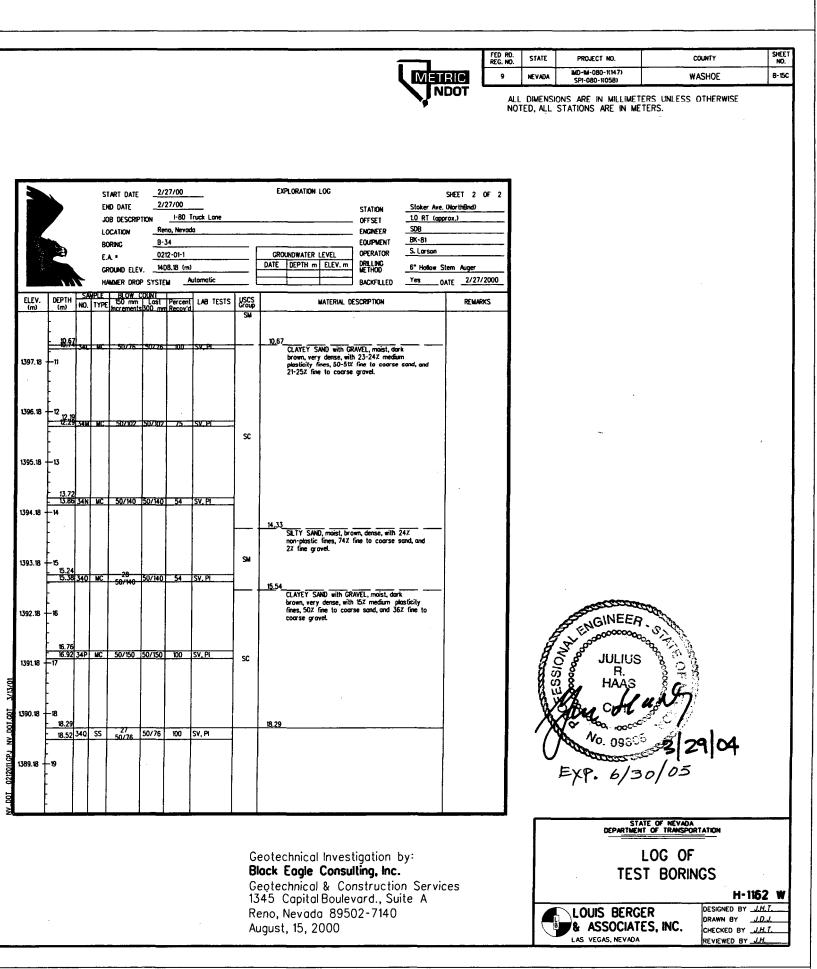
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24	a			RING	_	12-01-1		1	GROUNDWATER LEVEL	OPERATOR	A. Andresen
			-	A. •)8.18 (m)			DATE DEPTH m ELEV.m	DRILLING	6" Hollow S
1. A.				OUND ELEV	·		othead			4	
				MMER DROP					I	J BACKFILLED	<u></u> [
ELEV. (m)	DEPTH (m)	NO.	TYPE	BLOW C 150 mm Increments		Percent	LAB TESTS	USCS	MATERIAL (ESCRIPTION	
	0.00	4.	SPT	7 10 16	26	100	MC, SV, PI		WELL GRADED SAND with SU (Fill) dark brown, very moist, with 87 non-plastic fines, 59 fines, 347 fine to coarse, su +25mm in diameter.	medium dense, 7 fine to coorse	
1407.18 -	- 0.76 - 0.81	4 H	SPL	50/50	50750	- 10		SW SM	Coarse graveland col meters.	obles at 0.76 to 1	.22
	<u>1.52</u>			20							
	1.75	4C	SPT	50/75	50/75	100			1,77 Coarse graveland cat	obles at 1.68 to 1.	77
	-								Vineters. POORLY GRADED SAND brow	n cliahtlu moiet	
1406.18 -	-2 2.29								to moist, dense to very den (5% non-plastic fines, 85-95) sand, 0-10% fine, angular to s to +9.5mm in diameter.	se, with estimated (fine to coarse	
	- 2.74	4D	SPT	13 18 30	48	100		59			
	-										
1405.18 -	-3 3.05										
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Geotechnical Investigation by: Black Eagle Consulting, Inc. Geotechnical & Construction Services 1345 Capital Boulevard., Suite A Reno, Nevada 89502-7140 August, 15, 2000

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19 19							12-01-1		_	GROUN	DWATER L	rvri	OPERATOR	S. Larson		
2. 2				-	A. •		08.18 (m)		-		DEPTH m		DRILLING	e" Hollow		A
					IOUND ELEV			utomotic					METHOD	6" Hollow		
		1	٢_		WWER DROP				I	l.			BACKFILLED	Yes	DATE	2/27/20
ELEV. (m)	DEPT (m)			IPLE TYPE	BLOW C 150 mm Increments	CUNT Lost 1300 mm	Percent Recov'd	LAB TESTS	USCS Group			ATERIAL DI	SCRIPTION		T	REMARKS
	<u> </u>	24					<u> </u>	L	GW	1 0.24	SPHALT C					
	<u> </u>	49	34A	MC	50/76	50/76	67	SV, PI	- cc		nd sawd i	Fill movist	with SILTY CL	v dense	h	
1	F o	.76						[1	1 1	ith 8% low	plasticity	fines, 30% fine	lo coorse	11	
	-				28	<u> </u>			1		and, and 6	2% fine to	fines, 30% fine (coarse grave). with CLAY and		1	
1407.18	<u>t'</u> 1	22	348	MC	45 47	92	100	SV, PI	1 ~	5	AND, moist	t, brown, ver	y dense, with 93	X low	1	
							1		GW	į pi	lasticity fi	nes, 38% fin	ie to coarse sa	nd, and		
ļ		52		_	26				-	j g	37. fine to rovel.	COUTHE au	brounded to sul	ongular		
	[<u>ı</u>	80	34C	MC	50/127	50/127	100	SV, PI		-						
1406.18 -	-2					Γ			1	2.13						
1100.	-	29			i					G			with SAND, moi		-	
ļ			34D	MC	13	50/150	100	SV, PI	a		ark brown,	, hard, with	estimated 507 i	medium		
1	<u> </u>	<u>59</u>	<u> </u>		50/150					2.59 pi	osticity to ^7 fine to	nes, 207 mm	e to medium s unded to subrou	and, and whield	h	
1	ŀ.	_	1		1					🦉	ovel.		with GRAVEL, mo	ANC.	11	
1405.18	<u>-3 3.</u>		- 40		42					S	LTY, CLAY	EY SAND	with GRAVEL, mo e, with 18-23% kc	ist,	1	
1	<u> </u>	25	34E	MC	50/51	50/51	100	SV, PI	1	p p	losticity fir	nes, 45-497	fine to coorse	e sand, and		
ļ	ŀ	ł					i 1		SC SM	3	2-33% fine	to coorse	subrounded gr	ovel		
1		.81	1				1		•	1	•					
1404.18			34F	MC	50/150	50/150	67	SV. PI								
1404.10	[*			Ē	Γ I	[[]	[4.27						
1	[]					í :				CI			AVEL, moist, dar		1	
	4	54	140		50/13	50/13		\$¥; P		į br	own, very	dense, with	32% low to me to coarse sor	edum		
ł	ł	1	Ì						SC			coorse gro		10, 0nu		
1403.18	-5					l j	l.					••••				
ł	- 5.		1	L						5.33						
F			34H	MC	47/150	47/150	100	SV, PI		SI	LTY, CLAY	EY GRAVEL	with SAND, moi	ist,	٦	
t	F	1			, j		1		CC CM	oc far	rk brown, v=s. 39% f	very dense ine to coor	e, with 13% low p se sand, and 48	Nosticity 17 fine to		
	[_				1 1			GM	l cc	xorse grov		36 alerey er			
1402.18	-6.6.	19	340	wet	40789	40789	-79	SV PI		6.10 CI	AYEY SA	in with GR	AVEL, moist, dor		4	
ſ	-	٦								br	own, very	dense, with	247 low plostic	city fines,		
ł	-				1		1			48	3% fine to xorse grov	course so	nd, and 28% fine	: to		
ŀ	-		1								(u 30 y	en.				
1401.18	-7		1		1					Í.						
ŀ	-									l						
ŀ		.,								l					1	
F		88	54.3	NC	- 35/64	35/64	╞╍╤╡	SV, PI	SC	1					ł	
<u>I</u>		ł		1	1										1	
1400.18	8						1	•		į					1	
ļ	-	1				i				I.						
ŀ	-				1					i i						
ł	-	1			J	1]	ŀ	ļ	I						
1399.18 🕇	-9 9.1	4								9,14					Ì	
Ē			4K	MC_	50/150	50/150	33 9	SV, PI		SL			th GRAVEL, mois		-	
ŀ		Τ								da	rk brown, i	very dense,	with 14% low pl	losticity		
ł						1	1			C0	es, 44% tø arse grave	ne lo cues el.	se sand, and 42	7. Tahe Lo		
		1	- I	1			1		SC			,			1	

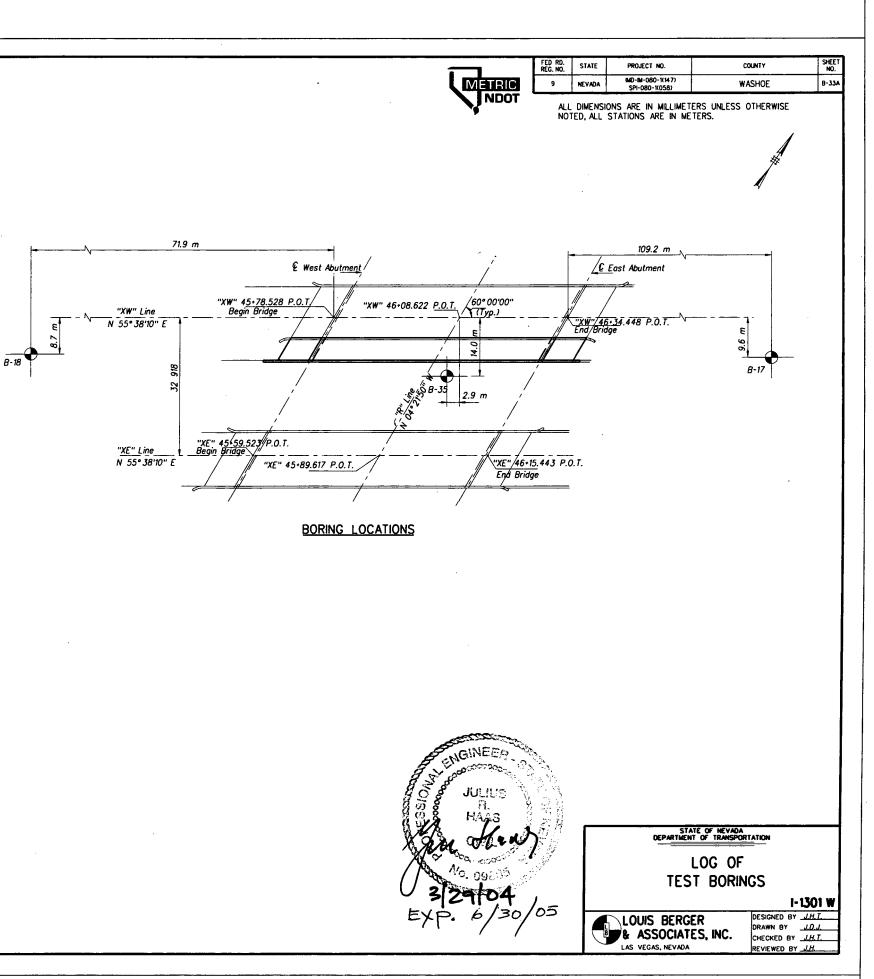


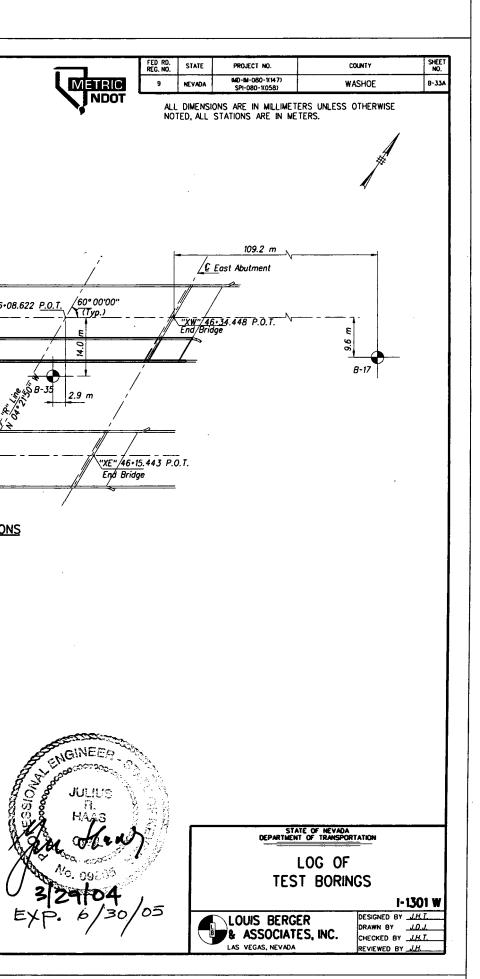
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KEY TO BORING LOGS

		DADT	CLE SIZE	LINALT	<u> </u>]			
CLAY	SILT	SAND		VEL	COBBLES	BOI	ILDERS			
CLAI	3121		COARSE FINE C			000				
0.002		•200 •40 •10			 75 mm 3	500 r				
0.002										
UCSC G		<u>.</u>			IL DESC					
GW GP		Well graded gravels Paarly graded gra								
GM GC		Silty gravels, paorl Clayey gravels, pao								
SW		Well groded sonds,				ui 65				
SP		Poorly groded san				S				
SM SC	1	Silty sands, poorly Clayely sands, poor								
ML		inorganic silts and				or clo	yey fine s	ands with s	light plasticity	
CL		Inorganic clays of				clay	s, sandy cl	oys, silty cl	ays, lean clays	
OL MH		Organic silts and a Inorganic silts, mice				v or	silty soils i	elastic silts		i i
СН		Inorganic clays of				,	5			
ОН	i	Organic cloys of m	nedium to high	platicit	y					
CE PT		Caliche Peot and other hig	hly organic sail	s						
MOIS	TURE		CRITERIA		SOIL C	EME	ΞΝΤΑΤΙΟ	N CRITE	ERIA]
Descript		Criteria		De	scription		iteria			
Dry		Absence of moistu dry to touch	re, dusty,	w	eak		umbles or t little finger	oreaks with	hondling	
Moist		Damp, no visible free water		Mo	oderate	Cri	-	oreaks with	considerable	
Wet		Visible free water, groundwater table	usually below	St	rong	Wo	-	or crumble v	with	
<u>v</u> <u></u>	<u>.</u>	Groundwater Eleva	tion Symbols							
	<u>ST</u>	NDARD PENE	TRATION (SIFICATIO	<u>)N</u> "				I
(GRANU	LAR SOIL	· · · · · · · · · · · · · · · · · · ·	CLAYEY	SOIL			nts on Calif	. Modified be converted	
BLOWS /	/ 0.3	m DENSITY	BLOWS / 0	.3 m	CONSISTE	NCY	to NSPT			
)-4 5-10	VERY LOOSE	0-1 2-4		VERY SO	FT	UTCHS/ C	.027 - NSP		
11-	-30	MEDIUM DENSE	5-8		SOFT MEDIUM S	TIFF			utomotic or	
OVER	-50 50	DENSE VERY DENSE	9-15 16-30		STIFF VERY ST	IFF		ammer con ard SPT Ne	be converted to by:	
		tration Test	31-60 OVER 60		HARD VERY HA			_{ic}) (1.25) -		
0.D. 63. 760 mm		follon 50.8 mm				Ň	(N Safety)) (1.17) - Ne	60	
		I.D. sampler								
<u>TEST</u>	ABE	BREVIATIONS						SAM	PLER NO1	ATION
CD		OLIDATED DRAINED	0	ORG	ANIC CONT	INT		CMS	CALIF. MODIFIE	D SAMPLER
CH CM		CAL (CORROSIVENE ACTION	SS) OC PI	CON	SOLIDATION			CPT CS	CONE PENETE CONTINUOUS	RATION
CU	CONS	OLIDATED UNDRAINE	.D RQC) ROC	K QUALITY	DESI	GNATION	CSS	CALIFORNIA S	PLIT SPOON
D DS	DIREC	RSIVE SOILS T SHEAR	RV SV	SIEV	ALUE 'E ANALYSIS	5		P PB	PUSHED (NOT PITCHER BAR	RRFI
E. G	EXPA	ISIVE SOIL FIC GRAVITY	SL U	Shr	INKAGE LIMI ONFINED CO	Т	SSION	RC SH	ROCK CORE	0
н	HYDR	DMETER	ŬU	UNC	ONSOLIDATE	D UN	DRAINED	SPT	STANUARU PE	NETRATION TES
HC K		D-COLLAPSÉ EABILTY	UW W		WEIGHT	ENT		TP ①	TEST PIT I.D. = 61.5 mm	n
				_				ě	I.D. • 82 mm	
		DESIGNATIONS ARE 7.5 YR 5/3) BROW		JNSELL	SOIL COLO	R CH	ART.		1.D 88.9 mr	m w∕o tube
								U U U	1.D. = 73 mm	
								L. Ŭ.		





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			-	ART DATE		7/00 17/00			EXPLORATION LOG				SHEET 1 OF
				id date							STATION	45-13.5	· · · · · · · · · · · · · · · · · · ·
				6 Descrip			ruck Lone				OFFSET	<u>9.6 RT</u> JRO	
				CATION		no, Nevod	0				ENGINEER	CME 55	
÷.				ORING	<u>B-1</u>			i			EQUIPMENT	A. Andres	en
141			E./	A. •		2-01-1			GROUNDWATER I DATE DEPTH m		OPERATOR DRILLING		
2			GR	NOUND ELEV		i9.14 (m)			UALE DEFTH III	ELLV.III	METHOD		Stem Auger
		2	HA	MMER DROP	P SYSTE	<u> </u>	otheod]	BACKFILLED	Yes	DATE2/17/2000
ELEV.	DEPTH		PLE	BLOW C	OUNT	Percent	LAB TESTS	USCS		MATERIAL DE	SCRIPTION		REMARKS
(m)	(m)	NO.	TYPE	increments	300 mm	Recovid		Group					
	0.00	17A	SPT	7 14 20	34	100		SP	brown, mois non-plastic	it, dense, wil fines, 85%) dark brown to th estimated 5% fine to coarse s ular grovelto +3	sand, 10%	
	-								0.61 SILTY SAND	with GRAV	EL dark brown,	moist,	-
	<u>0.76</u> -	178	SPT	10	50/125	91	MC, SV, PI	SM	very dense fine to coo	, with 17% n rse sond, 15	on-plastic fines, 5% fine to coars 9mm in diamete	687 e,	
1468.14	-1 1.04	1/8	3*1	50/125	507 125		mu, 3V, F1		1.07	•	with SET and		_
	- 1.52							SP SW	GRAVEL da dense, with plastic fines fine to coo 1.46	rk brown to estimated 5 5,65-70% fi rse, angular	5-10% non-plastic 5-10% non-plastic ine to coarse so to subangular g	c to low and, 25%	
	-								CLAYEY SA	ND brown, a	moist, medium d	lense,	1
	-	17C	SPT	7 11 12	23	17		sc	50% fine to	coarse sa	r to medium pla nd, 10% fine, ang).5mm in diamet	utor to	
1467.14 -	-2-1.98								2.13				Auger Refusal
	-												ot 2.13 meters.
466.14	-3												
-													
465.14	-4												
ł													
-													
ł				ŀ									

				TART DATE	-	7/00			EXPLORATION LOG		
				d date		7/00				STATION	42+76.
			J	B Descrip			ruck Lane			- OFFSET	8.7 RT
			ιC	CATION		io, Nevod	0			ENGINEER	JRO
÷.		÷	BC	ORING	8-1	8				EQUIPMENT	CME 5
1.41	Cel .		E.	A٠	02	2-01-1			GROUNDWATER LEVEL	OPERATOR	A. ANU
20			G	ROUND ELEN	. 147	2.18 (m)			DATE DEPTH m ELEV. m	DRILLING	6" Hole
		1	H	wimer droi	P SYSTE	<u> </u>	otheod			BACKFILLED	Yes
ELEV. (m)	DEPTH (m)	SA NO.	TYPE	BLOW (150 mm Increments	DUNT Lost	Percent	LAB TESTS	USCS Group	MATERIAL D	ESCRIPTION	
MU	0.00			merements	<u>)</u>	Recov a		SP	POORLY GRADED SAN	D with SILT dorl	k
	L			4			ļ	SW	brown, very moist, me 0.24_estimated 5-10% non-	dium dense, with plactic to slightly) v olastic
		18A	SPT	8	19	83		<u> </u>	fines. 80-85% fine to	coarse sand, 102	X fine to ∶
	- 0.46								coarse, angular to sub	angular gravel to	•25mm
	0.10		<u> </u>					SC SM	n diameter. SANDSTONE BEDROCK		
	-							-	formation, brown, mais the soil index propertie	t, medium dense vsofo Sätv Clor	e, with new Sound
	0.76								<u>0.76</u> with Grovel with estimation	nted 20-25% kow	plastic
	1	188	SPT	11	50/62.5	82			fines, 50-55% fine to coarse, angular to sub		
1471.18 -	0.98	Ē	<u> </u>	50/62.5				4	in diameter. SANDSTONE BEDROCK		
1471.10	- 1							sc	SANDSTONE BEDROCK formation, brown, mais		
	Ļ							SM	soil index properties of	a Silty, Clayey	Sand with
				1				i i	Gravel with estimated : 40-45% fine to coors	20-257 low plas	itic fines,
	-								i angular to subangular	orovelto •25mm	r to cours 1 in
	1.52							 	1.52 diameter. SANDSTONE BEDROCK	-	
	\mathbf{F}			12					formation, brown, mois		
		18C	SPT	32	78	100			soil index estimated 52 plastic fines, 90% fine	non-plastic to	slightly
	Ī			46				SP	fine, angular to subang	ular graveito +9	0,5%.),5mm in
1470.18 -	1.98								diameter.		
	-								2,13		
	2.29								SANDSTONE BEDROCK formation, brown, mois		
i	2.23							1	index properties of a		
	-			8					estimated 15-25% low to medium sand.	plastic fines, 75	-85% fine
		180	SPT	15 29	44	100		SC SM	to metatan sana.		
	2.74			29				ľ			
	2.14							1			
							I		2.90 CLAYSTONE BEDROCK		
1469.18 -	-3 3.05								formation, brown, moist	, hard, with the	soil
				7				a	index properties of o l 742 medium plastic fi	nes, 26% fine to	međum
	-	18E	SPT	12 25	37	100	MC, SV, PI	1	sand. Very minor fine	gravel.	
ł				25							
ŀ	3.51								3.51		
f	•										
	.										
[
1468.18	-4		ł								
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L											
Γ											
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Geotechnical Investigation to Block Eagle Consulting, Inc. Geotechnical & Constructio 1345 Capital Boulevard., Su Reno, Nevada 89502-7140 August, 15, 2000

	550 RD				SHEET
IC	FED RD. REG. NO. 9	STATE	PROJECT ND.	WASHOE	NO. 8-338
OT	ALL	DIMENSIO		s unless otherwise	
	NU	ICU, ALL 3	TABUNS ALL IN METER		
Sheet	10F 1				
Auger					
	/2000				
REMA	RKS				
		ji. Ž			
				12	
			fue fla	129/04	
				3/29/04	
		E E	4P. 6/30	0/05	
				, 	
				OF NEVADA	
				dg of Borings	
	ices				-1301 W
Serv A			LOUIS BERGER	DESIGNED B	

	~		ST	TART DATE	_2/	26/00			EXPLORATION LOG SHEET 1.0
			EM	D DATE	2/	26/00			STATION W. McCarron (NorthBad)
			JC	DE DESCRIP	TION	1-80	ruck Lane		OFFSET 4.0 RT (approx)
			LC	CATION	Re	no, Nevac	lo 0l		ENCINEERJRO
			80	ORING	8-	35			EQUIPMENT BK-81
202			E.	٨.	02	12-01-1			GROUNDWATER LEVEL DPERATOR S. Lorson
			G	ROUND ELEY	v. <u>140</u>	56.09 (m)		DATE DEPTH m ELEV. m DRILLING 6" Hollow Stem Auger
		1	H/	wwer dro	P SYSTE	M _A	utomotic		BACKFELED Yes DATE 2/26/20
ELEV.	DEPTH	SA	MPLE	BLOW	COUNT			10000	
(m)	(m)	NO.	TYPE	BLOW (150 mm increments	Last sB00 mm	Percent Recovid	LAB TESTS	USCS	
	-					i i		<u> </u>	0.15 ASPHALT CONCRETE PAVEMENT
	-							<u> </u>	0.46
	0.76								SANDSTONE BEDROCK of the Hunter Creek formation, brown, maist, very dense, with the
1465.09 -	L, 10	35A	SPT	50/100	50/100	100	SV, PI	sc	soil index properties of a Clayey Sand with Gravel with 17% medium plastic fines, 44% fine
1403.09	-								to coarse sand, 39% fine to coarse, subangular
	1.52							<u> </u>	1.37 gravet to +25mm in diameter.
	-	358	SPT	12 28	50/125	100	SV. PI	sc	formation, brown, moist, very dense, with the
	1.96	338	351	50/125	30/ 23	100	34, 11	x	soil index properties of a Clayey Sand with 21% medium plastic fines, 78% fine to coarse sand,
1464.09 -	2.29		ļ					 	2,13 and trace fine gravel to <u>some</u> in diameter.
	-			14	1			1	formation, brown, very moist, very dense, with the soil index properties of a Sity Sand with
	2.74	35C	SPT	29 45	74	100	SV, PI	SM	the soil index properties of a Silty Sand with 28% slightly plastic fines, 72% fine to coarse
	_3 3.05			1				 	2.90 sand, and trace, fine gravel to +5mm in
1463.09 -		1		29			au a	1	SANDSTONE BEDROCK of the Hunter Creek
	. 3.45	35D	MC	49 50/100	50/100	100	SV, PI		formation, brown, maist to very maist, very dense, with the soilindex properties of a Silty
								SM	Sand with 18% non-plastic fines, 80% fine to
	3.81	⊢		- 11 -	÷				coarse sand, and trace fine gravel to +5mm in diameter.
1462.09 -	4.23	35E	SPT	42 50/113	50/113	100	SV, PI		
									4,42
	4.57			19				-	SANDSTONE BEDROCK of the Hunter Creek formation, brown, maist, very dense, with the
	5.02	35F	SPT	32 50/138	50/138	100	SV, PI	51	soilindex properties of a Poorly Graded Sand with Silt with 11% non-plastic fines, 84% fine to
1461.09 -	•			30/ 50					5.18 coarse sand, 5% fine, subangular grovel to
	5.33			15					SANDSTONE BEDROCK of the Hunter Creek
ł	5.79	35G	SPT	34 37	71	100	SV, PI	SM	formation, brown, very maist, very dense, with the soilindex properties of a Silly Sand with
ł				3/					5.94 38% non-plastic fines, 60% fine to coarse sand,
1460.09	-6 6.10			16					SANDSTONE BEDROCK of the Hunter Creek
[35H	SPT	28	62	100	SV, PI		formation, brown, very moist, very dense, with the solindex properties of a Saty Sand with
ŀ	6.55			34				SM	the soil index properties of a Silty Sand with 27% non-plastic fines, 73% fine to coarse sand.
ł	•								7.01
459.09	-7								SANDSTONE BEDROCK of the Hunter Creek
Ľ	:		ĺ				.		formation, brown, moist, very dense, with the sailindex properties of a Well Graded Sand
Ļ	7.62								with Silt with 10% non-plastic fines, 86% fine to
ŀ		351	SPT	17 25	63	100	SV, PI	SW	coarse sand, and trace fine gravel to +5mm in diameter.
458.09	-8 8.08			38				SM	
t									
L L									
-									8.84
457.09	-9 9.14							İ	SANDSTONE BEDROCK of the Hunter Creek formation, brown, maist, very dense, with the
F		35J	MC	29 52/100	52/100	80	SV, PI		solindex properties of a Poorly Graded Sand
t									with Silt with approximately 10% non-plastic fines, 90% fine to coarse sand, and trace fine
- r	1	1				1			gravelto +5mm in diameter.

	END JOB	ART DATE D DATE	2/	26/00			STATION	SH
								W. McCarron (N
	100	3 DESCRIPT	TION .	i-80 T	ruck Lane		0FFSET	4.0 RT (appro
		CATION	Ren	no, Nevad	0		ENGINEER	JRO
	805	RING	B	35			EQUIPMENT	BK-81
	E.A.		02	12-01-1			GROUNDWATER LEVEL OPERATOR	S. Lorson
		Dund elev	146	6.09 (m))		DATE DEPTH m ELEV. m DRILLING	6" Hollow Sten
		IMER DROP			utomotic	_	BACKFILLED	Yes DATE
							1	
	TYPE	BLOW C 150 mm Increments	Lost 300 mm	Percent Recovid	LAB TESTS	USCS	MATERIAL DESCRIPTION	
						SP SM		
- 10.67	 			L				1.
ист ор 1 35к	SPT	16 24	62	89	SV, PI			
455.09 -11 11.13		38		 				d
						1		
								1
							11,89	
1454.09 12 12.19							SILTSTONE BEDROCK of the Hunter Cree formation, dark brown, very moist, hard,	
	SPT	6	47	100	SV, PI	1	the solindex properties of a Sandy Lea	n Chory
- 12.65	581	19 28	•	100	58, 11	a	with 55% medium plastic fines, 45% fine medium sond.	10
F T								
453.09 + 13							13.11	
. † 1							SANDSTONE BEDROCK of the Hunter Cre formation, dark brown, moist to very ma	ek vist verv
							dense, with the solunder properties of a	Poorty
13.72	MC	33	51/113	100	SV. PI		graded Sand with estimated 5% non-plas fines, 85-90% fine to coarse sand, 5-10	stic 7. fine.
452.09 13.98 35		51/113	2010	- 60	58, 11	SP	subangular gravel to +12.5mm in diamete	я.
[]							14.78	
1451.09 - 15							SANDSTONE BEDROCK of the Hunter Cre formation, brown, very maist very dense	ek itte
15.24							formation, brown, very moist, very dense the soil index properties of a Well Graded with Silt with 10% non-plastic fines, 85%	Sana
35N	SPT	22 42	50/138	97	SV, PI		course sand, 5% fine, subangular gravel to	ine to
- 15.69		50/138				SW SM	•Smm in diameter. Unit contains thinly interbedded Silty Sand.	
450.09 - 16							interbedded Sitty Sand.	
							16.46	
16.76							SANDSTONE BEDROCK of the Hunter Cre formation, brown, slightly maist, very den	
449.09 -17 350	SPT	20 35	77	89	SV. PI		with the solindex properties of a Poorly	Graded
17.22	351	42		03	37, FI	SP	Sand with estimated (5% non-plastic fine 95-100% fine to medium sand. Unit contr	oins j
	T	T	T				thinly interbedded Silty Sand.	
							17.68	
							SANDSTONE BEDROCK of the Hunter Cree formation, light grey to white, moist, med	
148.09 + 18					[SP	dense to dense, with the soil index prope	rties of
18.29		14				-	a Poorly Graded Sand with estimated 101 18.44 medium to coarse sand	02
- 18.69 35P	SPT	45 50/100	50/100	88	SV, PI	SM	18.44 medium to coarse sand. SANDSTONE BEDROCK of the Hunter Creation of the Hunter Creatio	sk
F 1		, (A)					the soil index properties of a Sity Sand	with /j
147.09 - 19							25% non-plastic fines, 73% fine to coarse	esono, //
							and trace fine gravel to 5mm in diamet	<u>ei.</u>
								1
1 1								

Geotechnical Investigation by: Black Eagle Cansulting, Inc. Geotechnical & Construction Services 1345 Capital Boulevard., Suite A Reno, Nevada 89502-7140 August, 15, 2000

	FED RD. REG. NO.	STATE	PROJECT NO.	COUNTY	SHEE
RIC	REG. NO. 9	NEVADA	MD-M-080-1(147) SPI-080-1(058)	WASHOE	NO. 8-334
TOT `			NS ARE IN MILLIMETERS STATIONS ARE IN METER		
2 OF 2 #Brnd)					
uger 2/26/2000 REMARKS	-				
er added to rr to aid ng action.					
			GINEEA.	A.	
		ESSION.			
		CESSION.	ENGINEEA JULIUS B.	N	
		CESSION.	JULIUS HAAS	2/29/04	
		CESSION.	JULIUS HAAS No. 0360 EXP. 6/30	3/29/04 2/05	
		CESSION.	JULIUS HAAS No. 03800 YP. 6/30	SIZA 04 SIZA 04 OF NEVADA F TRANSPORTATION OG OF BORINGS	
		CESSION.	JULIUS HAAS No. 03800 YP. 6/30	S 29 04 S 20 04 S 2	I-1301 W

Appendix D Laboratory Test Results

		Date	
SUMMARY OF RESULTS N.D.O.T. GEOTECHNICAL SECTION	Job Description I-80 Over Stoker Ave.Eastbound Widening	Station	STRENGTH TEST
SUMMARY OF RESULTS D.T. GEOTECHNICAL SEC	I-80 Over Stoker /	4602	%
N.D.(cription	Elevation (ft) 4602	DRY
	Job Desi	Elevation	
			z
			SAMP-
	74191	В-1	VPLE

Boring No.

EA/Cont #

	SAMPLE	SAMP-				DRY	%		F			STRE	STRENGTH TEST	EST		
SAMPLE	DEPTH	LER	_	SOIL	%M	M	PASS	Ξ	Ч	L.,	TEST	Ð	c	Ð	ပ	COMMENTS
Ö N	(tt)	TYPE	per ft.	GROUP		pcf	#200	%	%	%	TYPE	deg.	psi	deg.	psi	
					_						1	Peak	ž	Res	Residual	
~	1.5	SPT					12.6									
7	3.0	SPT		SP-SC	6.0		11.3	53	19	4						
3	5.0	SPT		GP-GC			11.9	23	19	4						
4	7.5	SPT		GP-GC			7.2	23	19	4						
5	10.0	SPT					14.3									
9	15.0	SPT					11.1									
7	20.0	SPT		GC-GM	11.5		20.0	27	21	G						
8	25.0	SPT		ဒင			15.4	28	20	ω						
6	30.0	SPT					14.9									
10	35.0	SPT					16.1									
11	40.0	SPT		SC-SM			17.4	26	20	9						
12	45.0	SPT					20.4									
CMS = Califor	CMS = California Modified Sampler 2.42" ID	0	U = Unconfined Compressive	ad Compressi	ev.			H = Hydrometer	neter			CM = Compaction	ction			

CMS = California Modified Sampler 2.42" ID SPT = Standard Penetration 1.38" ID CS = Continuous Sample 3.23" ID RC = Rock Core PB = Pitcher Barrel CSS = Calif. Split Spoon 2.42" ID CPT = Cone Penetration Test TP = Test Pit
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* = Average of subsamples

 $N = (N_{css})(0.62)$

N = Field SPT

N = No. of blows per ft., sampler

C = Cohesion Φ = Friction

UU = Unconsolidated Undrained CD = Consolidated Drained

CU = Consolidated Undrained DS = Direct Shear

Ch = Chemicat RV = R - Vatue MD = Moisture Density H = Hydrometer S = Sieve G = Specific Gravity PI = Plasticity Index LL = Liquid Limit OC = Consolidation PL = Plastic Limit NP = Non-Plastic

E = Swell/Pressure on Expansive Soils HCpot = Hydro-Collapse Potential RQD = Rock Quality Designation W = Moisture Content K = Permeability O = Organic Content D = Dispersive X = X-Ray Defraction SL = Shrinkage Limit CM = Compaction UW= Unit Weight

6/1/2021

N.D.O.T. GEOTECHNICAL SECTION SUMMARY OF RESULTS

74191	
EA/Cont #	

Job Description I-80 Over Stoker Ave.Eastbound Widening

Elevation (ft)

	<u>.</u>	CM = Compaction E = Swell/Pressure on Expansive Soils	iction issure on E	CM = Compaction E = Swell/Pressure	υш		neter	H = Hydrometer S = Sieve	Υ		eq	Compressive dated Undrain	U = Unconfined Compressive UU = Unconsolidated Undrained		CMS = California Modified Sampler 2,42" ID SPT = Standard Penetration 1,38" ID	CMS = California Moo SPT = Standard Pene
				4.000000000000000000000000000000000000												
							-									
		1														
									13.5		8.3			SPT	60.0	15
									12.6					SPT	55.0	14
						4	24	28	30.4			SM		SPT	50.0	13
	idual	Residual	2	Peak	1		;	:		į						
COMMENTS	0	θ.	0	Ð.	TEST	E a	님	1:	PASS	MN 2	%M	SOIL	BLOWS	LER	DEPTH	SAMPLE
		EST	STRENGTH TEST	STRI					%	DRY				SAMP-	SAMPLE	<u> </u>

* = Average of subsamples

G = Specific Gravity Pt = Plasticity Index MD = Moisture Density OC = Consolidation PL = Plastic Limit NP = Non-Plastic LL = Liquid Limit

E = Swell/Pressure on Expansive Soils RQD = Rock Quality Designation W = Moisture Content X = X-Ray Defraction SL = Shrinkage Limit O = Organic Content UW= Unit Weight K = Permeability D = Dispersive

RV = R - Value Ch = Chemical

 $N = (N_{css})(0.62)$

HCpot = Hydro-Coliapse Potential

N = Field SPT C = Cohesion

N = No. of blows per ft., sampler UU = Unconsolidated Undrained CD = Consolidated Drained CU = Consolidated Undrained DS = Direct Shear

SPI = Standard Penetration 1.38" ID CS = Continuous Sample 3.23" ID CSS = Calif. Split Spoon 2.42" ID CPT = Cone Penetration Test PB = Pitcher Barrel RC = Rock Core

TP = Test Pit

P = Pushed, not driven

R = Refusal

Sh = Shelby Tube 2.87" ID

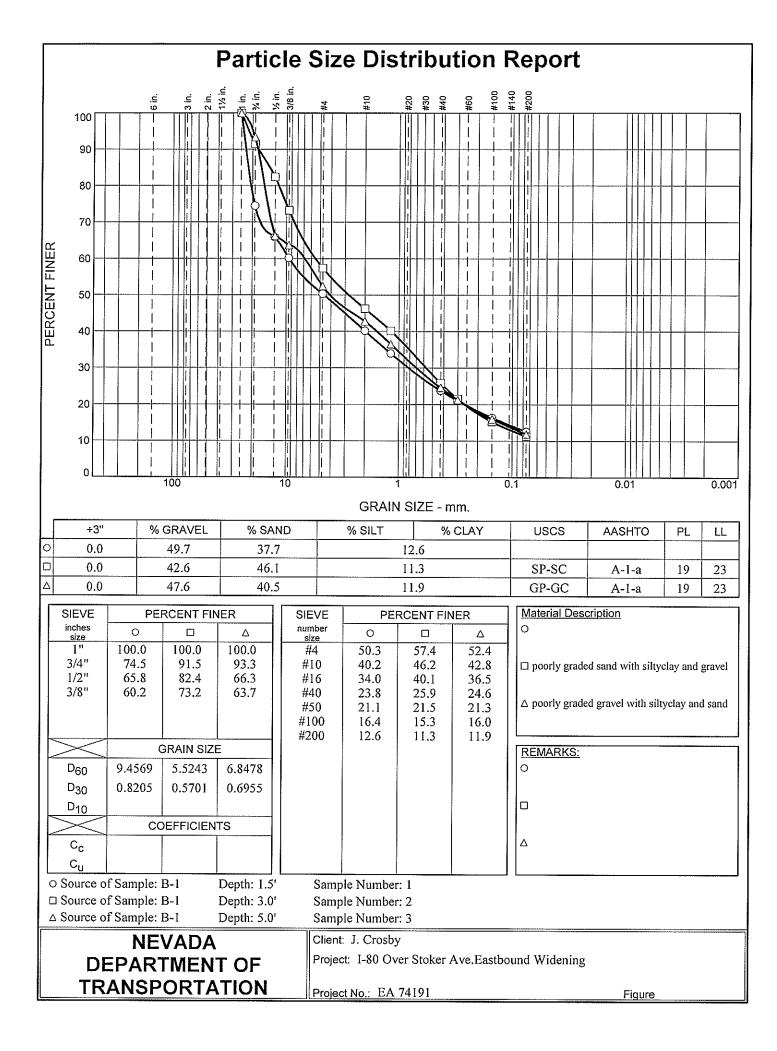
Boring No.

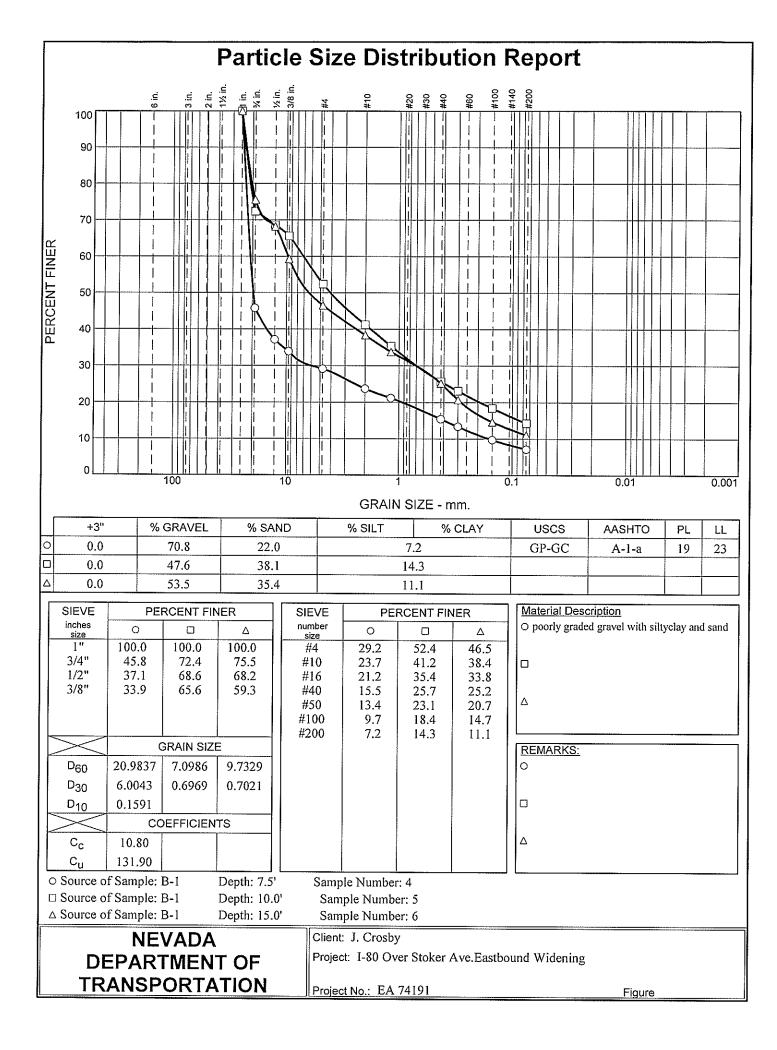
Ч.

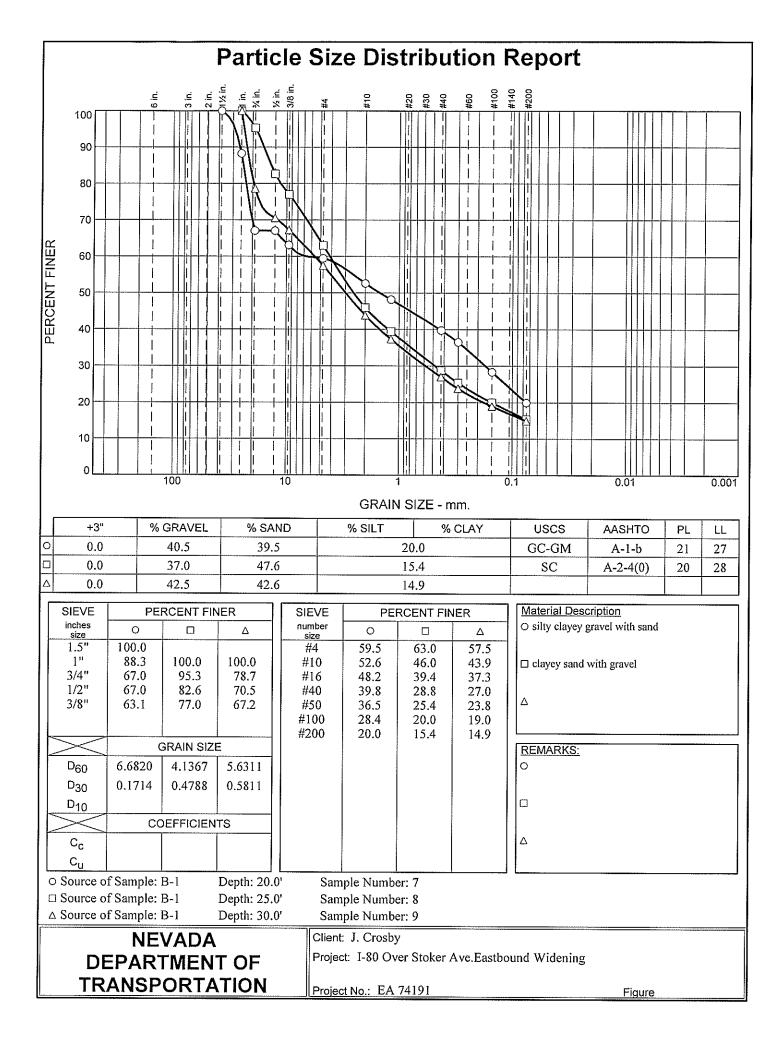
4602

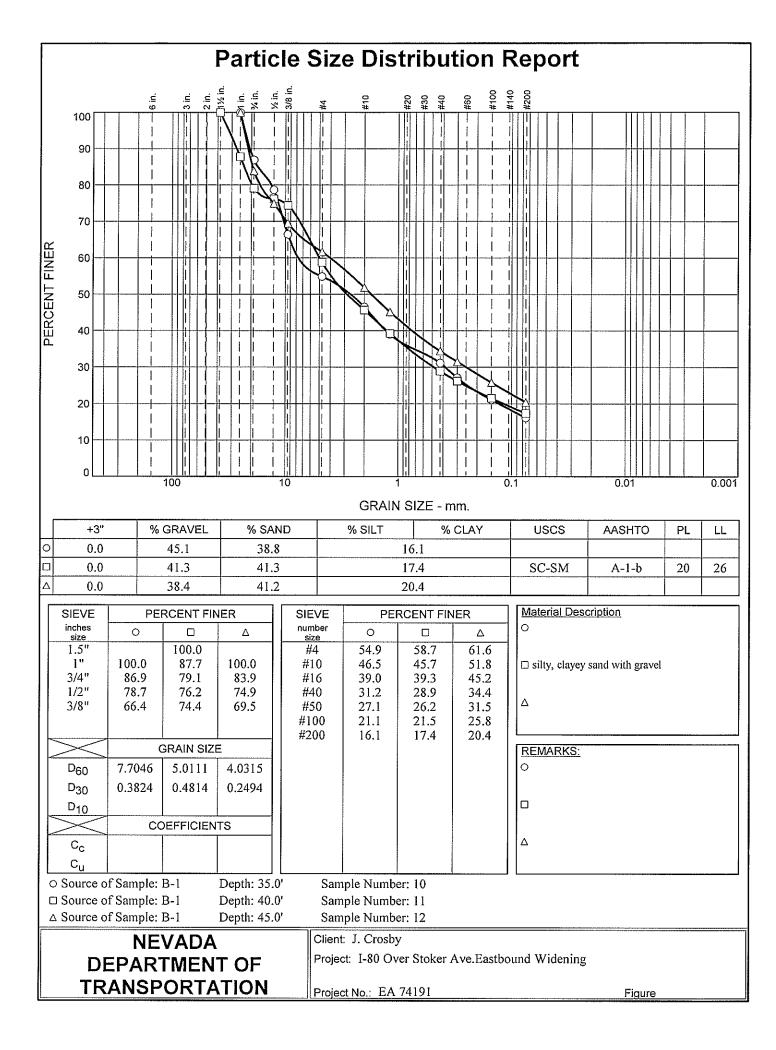
Station

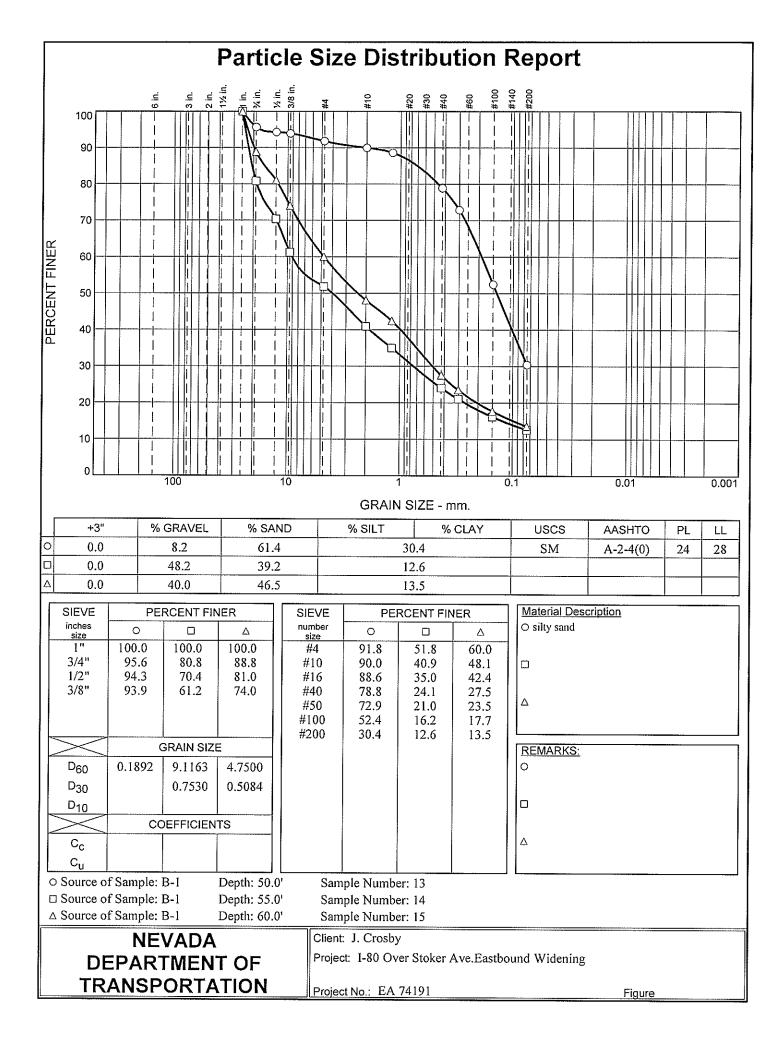
6/1/2021 Date











NEVADA DEPARTMENT OF TRANSPORTATION

Materials Division Geotechnical Section 1263 Stewart St, Carson City, NV 89712