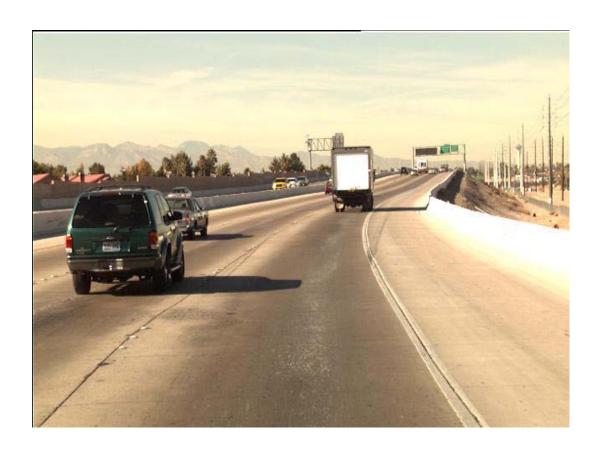
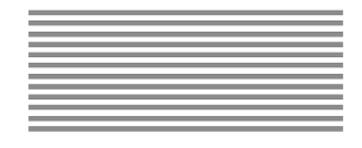
GEOTECHNICAL REPORT

I-515 NORTHBOUND SOUNDWALLS LAS VEGAS, NEVADA

JULY 2008







MATERIALS DIVISION

STATE OF NEVADA DEPARTMENT OF TRANSPORTATION MATERIALS DIVISION GEOTECHNICAL SECTION

GEOTECHNICAL REPORT I-515 NORTHBOUND SOUNDWALLS BETWEEN NELLIS BOULEVARD AND MOUNTAIN VISTA STREET, LAS VEGAS

July 2008

CLARK COUNTY, NEVADA

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Background, Purpose, and Scope

The purpose of this project is to relieve noise and enhance the quality of life for those who live along the I-515 corridor. Nevada Department of Transportation (NDOT) has determined that sound barriers are needed along the east edge of I-515's northbound lanes from south of Nellis Boulevard to north of Mountain Vista Street in Clark County. The project limits extend approximately 1.0 miles between station "L" 629+67.42 P.O.T. and station "L" 678+67.42 P.O.T. where existing soundwall begins. Two twelve foot tall combination traffic barrier/soundwall will be structurally attached to the existing Duck Creek structure (B-1455) and Mountain Vista Street structure (H-1454) bridge rails. Four post and panel style soundwalls are proposed along the rest of the project length. These soundwalls will stand 12 feet above existing concrete pavement and extend into the roadway embankment fill slopes. The centerline of the post and panel soundwalls will be located 3 feet, measured horizontally, away from the outside edge of the existing Type A Concrete Barrier Rails.

The purpose of this report is to present the results of our investigation and to provide recommendations and other pertinent information regarding the design and construction of the proposed foundations to support the post and panel soundwalls. The scope of our work included surface reconnaissance, subsurface exploration, soil sampling, field/laboratory testing, and engineering analyses.

Exploration & Testing Program

Between March 3rd and March 7th, 2008 the Geotechnical Engineering Section drilled ten borings below the existing concrete pavement, to depths ranging from 19 feet to 26.5 feet, to investigate the subsurface ground conditions at the project site. Borings SWA 1 through SWA 10 were drilled along the post and panel soundwall alignments. Drilling was accomplished using a Diedrich D-120 drill rig equipped with 6-inch hollow stem auger flights. Representative soil samples were obtained using SPT (Standard Penetration Testing) and CMS (California Modified Sampler) equipment and procedures. All soil samples were classified using the Unified Soil Classification System (USCS) in accordance with ASTM D 2487. Boring locations and ground surface elevations were determined using original roadway elevation and alignment information contained in construction plan sheets from NDOT Contract 2154 and converted to current project stationing using a station equation provided by NDOT's Southern Nevada Engineering Services Division. Copies of the boring logs, a boring location map, and a key to boring symbols are provided in Appendix A.

Soil samples were transported to NDOT's Materials Division in Carson City for laboratory testing. Atterberg Limits tests, natural moisture content measurements, particle size analyses, unit weight measurements, and direct shear tests were completed to assist in sample identification, classification, and evaluation. Final results from all these tests are presented in the Summary of Results, Particle Size Distribution Reports and Direct Shear Test Report sheets included in Appendix B.

Site Conditions

The first section of post and panel wall will be constructed along the east edge of northbound I-515 between station "L" 629+67.42 and the Nellis Boulevard structure, approximately station "L" 633+50. One boring (SWA 1) was completed along the alignment of the first section. 11.5 inches of P.C.C.P. were encountered. The roadway embankment fill height varies between 2 feet and 16 feet along this section. The majority of the embankment fill is classified as dry medium dense to dense clayey sand with gravel. Beneath the embankment fill the native soil is classified as very hard to hard, dry to moist sandy clay.

The second section of post and panel wall will be constructed along the east edge of northbound I-515 between Nellis Boulevard structure and Duck Creek structure from about station "L" 635+00 to "L" 644+10. Two borings (SWA 2, SWA 3) were drilled along the alignment of the second section. The borings encountered 12 inches and 11 inches of P.C.C.P. respectively. The roadway embankment fill height varies between 0 and 14 feet along this section. The embankment fill varies from loose to dense clay, silt, sand and gravel mixtures. The native soil beneath the fill is classified as medium stiff, dry to moist, lean clay with sand and very dense clayey sand with gravel.

The third section of post and panel wall will be constructed along the east edge of northbound I-515 between Duck Creek Structure and Mountain Vista Street structure from approximately station "L" 646+60 to "L" 664+50. Four borings (SWA 4 through SWA 7) were drilled along the alignment of the third section. All borings encountered 11.5 inches to 14 inches of P.C.C.P. The roadway embankment fill height varies between 15 feet and 30 feet along this section. The majority of the embankment fill is classified as dense to very dense, dry, silty and/or clayey sand with gravel. The native soil beneath the embankment fill is logged in borings SWA 4 and SWA 5 and is classified as stiff to very stiff, dry to moist, sandy lean clay.

The fourth section of post and panel wall will be constructed along the east edge of northbound I-515 between Mountain Vista Street structure and existing soundwall from about station "L" 667+00 to "L" 678+67+50. Three borings (SWA 8, SWA 9, SWA 10) were drilled along the alignment of the fourth section. All borings encountered 11.5 inches to 12 inches of P.C.C.P. The roadway embankment fill height varies between 12 feet and 25 feet along this section. The embankment fill is generally classified as dense, dry, clayey and/or silty sand with gravel. The native soil beneath the embankment fill was logged in Boring SWA 10. The native soil is classified as very stiff, moist to wet, sandy lean clay. Free water was encountered at a depth of 25 feet.

Discussion and Recommendations

We recommend using 3-foot diameter drilled shafts with a length of 14 feet for the foundation of each post. Drilled shaft lengths were determined using the design loading conditions and analytical methods recommended by AASHTO (American Association of State Highway and Transportation Officials, References No. 1). Design loading conditions were provided to us by NDOT's Southern Nevada Engineering Services Division and are as follows. The design dead load on each drilled shaft is 27 kip. The design wind force generates a moment of 51.2 kip-ft and a shear force of 6.4 kip at the top of each drilled shaft. Design seismic forces cause a moment of 24.2 kip-ft and a shear force of 3.0 kip at the top of each drilled shaft. Drilled shaft length was determined using the design moment and shear force created by the wind, as this is the greatest load combination. The lateral deflection of the drilled shafts was analyzed using computer software LPILE. Estimated deflections are within the allowable limits.

Parts of I-515's roadway surface drainage system (ditched inlets, reinforced concrete pipes, manholes, etc.) are in close proximity to the proposed soundwall alignments. We recommend that drilled shafts be located at least 3 feet away (measured edge to edge) from these items to avoid construction conflicts.

Drilled shafts will need to be strategically located in order to avoid conflicting with two water lines of unknown depths. One 96-inch diameter waterline crosses the soundwall alignment between stations "L" 640+00 and "L" 642+00. The other waterline, with unknown diameter, crosses the soundwall alignment between stations "L" 669+00 and "L" 671+00. These are not the only utilities that exist within the project limits. The plans should be reviewed for additional details of utility locations.

Construction Considerations

The contractor should be aware that NDOT specifications allow rocks up to 3 feet in diameter to be placed within embankment fills. Therefore, the contractor should consider the effect that these cobbles and boulders might have on drilling operations. Such effects may include, but not be limited to, longer drilling times, more difficult drilling conditions, oversized shaft diameters, increase in concrete quantities, etc. Specialized equipment or drilling techniques may also be required.

The groundwater table was measured at an approximate elevation of 1783 feet in boring SWA 10, drilled at approximately station "L" 677+57. Drilled shaft bottom elevations from station "L" 675+57 to the end of the project at station "L" 678+67.42 are estimated to be less than 10 feet above the recorded groundwater table elevation. If the groundwater table is higher during construction, shafts drilled in this area may require wet construction methods in accordance with Section 509 of NDOT's "Standard Specifications for Road and Bridge Construction 2001".

REFERENCES

- 1) AASHTO, "Standard Specifications for Structural Supports for Highway Signs, Luminaires and Traffic Signals," 4th Edition, 2001. Washington D.C., 2001.
- 2) AASHTO, "Guide Specifications for Structural Design of Sound Barriers, 1989" Washington D.C., 1989. Includes Interims published in 1992, 2002.
- 3) AASHTO, "Standard Specifications for Highway Bridges," 17th Edition, 2002. Washington D.C., 1996. Includes Interims published in 1997 through 2002.

APPENDIX A

Key to Boring Logs Boring Logs Boring Location Map

KEY TO BORING LOGS

CLAY	SILT		SAND		GR	AVEL	COBBLES	BOULDERS
		FINE	MEDIUM	COARSE	FINE	COARSE		

USCS GROUP	TYPICAL SOIL DESCRIPTION
GW	Well graded gravels, gravel-sand mixtures, little or no fines
GP	Poorly graded gravels, gravel-sand mixtures, little or no fines
GC	Clayey gravels, poorly graded gravel-sand-clay mixtures
sw	Well graded sands, gravelly sands, little or no fines
SP	Poorly graded sands, gravelly sands, little or no fines
SM	Silty sands, poorly graded sand-silt mixtures
SC	Clayey sands, poorly graded sand-clay mixtures
ML	Inorganic silts and very fine sands, rock flour, silty or clayey fine sands with slight plasticity
CL	Inorganic clays of low to medium plasticity, gravelly clays, sandy clays, silty clays, lean clays
OL	Organic silts and organic silt-clays of low plasticity
MH	Inorganic silts, micaceous or diatomaceous fine sandy or silty soils, elastic silts
CH	Inorganic clays of high plasticity, fat clays
OH	Organic clays of medium to high plasticity
CS	Claystone/Siltstone
PT	Peat and other highly organic soils

MOISTURE CON	DITION CRITERIA	SOIL CEMENTATION CRITERIA					
Description	Criteria	Description	Criteria				
Dry	Absence of moisture, dusty, dry to touch.	Weak	Crumbles or breaks with handling or little finger pressure.				
Moist	Damp, no visible free water.	Moderate	Crumbles or breaks with considerable				
Wet	Visible free water, usually below		finger pressure.				
	groundwater table.	Strong	Won't break or crumble w/finger pressure				
∇	Groundwater Elevation Symbols						

	GRANULAR SOIL	C	CLAYEY SOIL				
BLOWS/FT	DENSITY	BLOWS/FT	CONSISTENCY				
0 - 4	VERY LOOSE	0 - 1	VERY SOFT				
5 - 10	LOOSE	2 - 4	SOFT				
11 - 30	MEDIUM DENSE	5 - 8	MEDIUM STIFF				
31 - 50	DENSE	9 - 15	STIFF				
OVER 50	VERY DENSE	16 - 30	VERY STIFF				
	tration Test (N) 140 lb hammer l on 2 inch O.D. x 1.4 inch I.D. sampler.	31 - 60 OVER 60	HARD VERY HARD				

Blow counts on Calif. Modified Sampler (Ncms) can be converted to Nspt by:

(Ncms)(0.62) = Nspr Blow counts from Automatic or Safety Hammer can be converted to Standard SPT N60 by:

(Nautomatic)(1.25) =N60 (Nsafety)(1.17) =N60

TE	ST ABBREVIATIONS		SAMPLER NOTATION
CD CH CM CU D DS E G H HC	CONSOLIDATED DRAINED CHEMICAL (CORROSIVENESS) COMPACTION CONSOLIDATED UNDRAINED DISPERSIVE SOILS DIRECT SHEAR EXPANSIVE SOIL SPECIFIC GRAVITY HYDROMETER HYDRO-COLLAPSE PERMEABILITY	O ORGANIC CONTENT OC CONSOLIDATION PI PLASTICITY INDEX RQD ROCK QUALITY DESIGNATION RV R-VALUE S SIEVE ANALYSIS SL SHRINKAGE LIMIT U UNCONFINED COMPRESSION UU UNCONSOLIDATED UNDRAINED UW UNIT WEIGHT W MOISTURE CONTENT	CMS CALIF. MODIFIED SAMPLER CPT CONE PENETRATION TEST CS CONTINUOUS SAMPLER CSS CALIFORNIA SPLIT SPOON P PUSHED (NOT DRIVEN) PB PITCHER BARREL RC ROCK CORE SH SHELBY TUBE SPT STANDARD PENETRATION TEST TP TEST PIT
soi	L COLOR DESIGNATIONS ARE FRO EXAMPLE: (7.5 YR 5/3) BROV	OM THE MUNSELL SOIL COLOR CHART.	①- I.D.= 2.421 inch ②- I.D.=3.228 inch with tube; 3.50 inch w/o tube ③- NXB I.D.= 1.875 inch ④- I.D.= 2.875 inch

LAST MODIFIED: October 11, 2004

NEVADA	START DATE	3/3/08	EXPL	ORATIO	N LOG			SI
DEPARTMENT OF	END DATE	3/3/08				STATION	632+1	7
TRANSPORTATION	I JOB DESCRIPTIO	OFFSET	52' RT					
	LOCATION Nellis Blvd to Mountain Vista St ENGIN							ıni
	BORING	SWA-01				EQUIPMENT	Diedric	h D-120
	E.A. #		GROU	JNDWATER	LEVEL	OPERATOR	White	
	GROUND ELEV.	(ft)	DATE	DEPTH ft	ELEV. ft	DRILLING METHOD	6" H.S.	.A.
GEOTECHNICAL ENGINEERING	HAMMER DROP	SYSTEM Automatic				BACKFILLED	Yes	_ DATE _

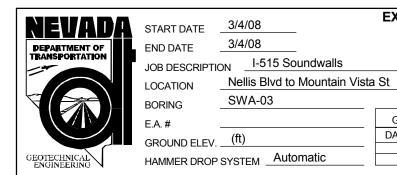
SHEET 1 OF 1

ENGIN	INICAL EERING			MMER DR				[BACKFILLED Yes DATE 3/3/08
ELEV. (ft)	DEPTH (ft)		MPLE TYPE	BLOW Co 6 inch Increments	OUNT Last 1 foot	Percent Recovid	LAB TESTS	USCS Group	MATERIAL DESCRIPTION REMARKS
()	-				7.1001				P.C.C.P. @ 11 1.00 Clayey Sand with Gravel dry, dense to medium dense. P.C.C.P. @ 11 Thick
	3.00	Α	SPT	10 19 12	31	80	PI, S, W	sc	Bulk Sample 1, 3'-5'.
-	5.00	В	SPT	7 7 11	18	95	PI, S, W		
	7.50	С	CMS	5 10	26	100	PI, S, W	GC	7.00 Clayey Gravel with Sand medium dense. 8.20 Silty Sand with Gravel dense.
	9.00	D	SPT	16 10 35	35	80	PI, S, W	CL	9.00 Lean Clay with Sand hard. 10.00
-	11.40	E	SPT	41 43 40/.4'	40/.4'	100	PI, S, W		Clayey Sand very dense.
	12.50	F	SPT	25 50/.4'	50/.4'	100	PI, S, W	sc	
-	15 								
	17.50			36					Sandy Lean Clay very hard to hard, dry to moist.
	19.00	G	SPT	50 50	100	115	PI, S, W	-	
-	20 _							CL	
	22.50	Н	SPT	10 21 24	45	100	PI, S, W		24.00
-	25								B.O.H.

NEVADA	START DATE	3/4/08	EXPL	ORATIO	N LOG			SH
DEPARTMENT OF	END DATE	3/4/08				STATION	637+21	
TRANSPORTATION	JOB DESCRIPTION	OFFSET	52' RT					
	LOCATION Nellis Blvd to Mountain Vista St ENGINE							ni
	BORING	SWA-02				EQUIPMENT	Diedrich	า D-120
	E.A. #		GROU	JNDWATER	LEVEL	OPERATOR	White	
	GROUND ELEV.	(ft)	DATE	DEPTH ft	ELEV. ft	DRILLING METHOD	6" H.S.A	۹.
GEOTECHNICAL ENGINEERING	HAMMER DROP	SYSTEM Automatic				BACKFILLED	Yes	DATE _

SHEET 1 OF 1

ELEV.	DEPTH		MPLE TYPE	BLOW Co 6 inch Increments	OUNT Last	Percent Recov'd	LAB TESTS	USCS Group	MATERIA	L DESCRIPTION	REMARK
(ft)	(ft)	NO.	TYPE	Increments	1 foot	Recov'd		Group	P.C.C.P.		P.C.C.P. @
									1.00		Thick
									Sandy Lean Cla	ay with Gravel loose, light brown.	
	-										
	3.00										
			ODT	4		7.	DI 0 144	CL			Bulk Sample 3'-5'.
	4.50	Α	SPT	3 5	8	75	PI, S, W				0-0.
	5.00										
		В	SPT	2 4	21	95	PI, S, W		6.00		
	6.50	Ь	0 1	17	21	33	1 1, 3, W	GC	Clavey Gravel	with Sand medium dense, white to	_
	7.00							 	7.00 pale brown.		-
		С	CMS	3 8	42	100	PI, S, W,	CL	1.10	Clay medium dense.	_
	8.50		5.410	34		100	UW	GC	8.50	with Sand dense.	
		D	SPT	25	54	120	PI, S, W			ith Gravel very dense.	
	9.50			54				1			
•	10.00 10.50	Е	SPT	50/.46'	50/.46'	100	PI, S, W]			
	_										
	-							SC			
	14.00										
		-		6			D. C. V.				
	15 15.50	F	SPT	20 32	52	100	PI, S, W				
	10.00										
									47.00		
	_								17.00 Lean Clay with	Sand hard, dry to moist.	-
	_									<u> </u>	
	19.00										
	10.00			9				1			
	-20 20.50	G	SPT	14 17	31	130	PI, S, W				
	20.50			17				1			
								CL			
	-										
	0.4.05										
	24.00			9				-			
	25	Н	SPT	13	40	115	PI, S, W				
	25.50			27					25.50 B.O.H .		-
	-								В.∪.П.		



	EXPL	ORATIO	N LOG		011557 4 05 4
					SHEET 1 OF 1
				STATION	642+26
				OFFSET	52' RT
'ista	St			ENGINEER	Ablahani
				EQUIPMENT	Diedrich D-120
_	GROL	JNDWATER	LEVEL	OPERATOR	White
_	DATE	DEPTH ft	ELEV. ft	DRILLING METHOD	6" H.S.A.
_				IVILITIOD	

BACKFILLED

DATE 3/4/08

Yes

ELEV. (ft)	DEPTH (ft)		MPLE TYPE	BLOW Co 6 inch Increments	Last	Percent Recov'd	LAB TESTS	USCS Group	MATERIAL DESCRIPTION	REMARKS
									0.90 P.C.C.P.	P.C.C.P. @ 11" Thick
	2.50								Silty Sand with Gravel dense, dry to moist, light brown.	
	4.00	Α	SPT	7 13 18	31	95	PI, S, W	SM	450	Bulk Sample 1, 3'-5'.
-	5 5.00	В	SPT	6 9	20	105	PI, S, W	SC SM	4.50 Silty Clayey Sand medium dense, dry to moist, light brown. 6.20	
	6.50			11				CL	Sandy Lean Clay very stiff.	
	9.50	С	CMS	5 8 11	19	100	PI, S, W, UW	OL.	9.50	
-	-10 11.00	D	SPT	3 6 12	18	95	PI, S, W	SC SM	Clayey Sand medium dense.	
	12.50			5			PI, S, W,	 	12.00 Sandy Lean Clay medium stiff, tan.	_
	14.00		CMS	4 3 2 2	10	100	PI, S, W			
-	15 15.50	-	JOF 1	8	10	100	F1, 3, W	CL		
	17.50		0.10	14				_		
	19.00 20.00		CMS SPT	18 18 38 43	36 43	110	PI, S, W	-	18.70 Silty, Clayey Sand with Gravel dense.	_
-	-20							SC SM		
	-									
-	25.00 25			4	_			CL		
	26.50	ı	SPT	7 13	20	120	PI, S, W		26.50 B.O.H.	_
- -	_									

NE	VAI	10	STAR ¹	T DATE	3/	5/08			EXPL	ORATION	l LOG	
DEPA	RTMENT OF PORTATION		END D	DATE DESCRIP		5/08 I-515	 Soundwalls					STATION OFFSET
		И	LOCA	TION	Ne	ellis Blvd	l to Mountair	n Vista	St			ENGINEER
\forall		\perp	BORIN	NG	SV	VA-04						EQUIPMENT
		/	E.A. #						GROL	JNDWATER	LEVEL	OPERATOR
			GROL	JND ELE	V. 17	83.55 (1	t)		DATE	DEPTH ft	ELEV. ft	DRILLING METHOD
GEOTEC ENGIN	CHNICAL VEERING		HAMM	MER DRO	OP SYS	TEM A	utomatic	t				BACKFILLED
ELEV. (ft)	DEPTH (ft)		TVDE 6	BLOW CO 5 inch rements	UNT Last 1 foot	Percent Recov'd	LAB TESTS	USCS Group		MATE	RIAL DE	SCRIPTION

2.50

4.00

7.50

9.00

10.00

11.50 12.00

13.50

17.00

18.50

22.00

23.50

-25

D SPT

Ε

F SPT

1773.6

1768.6 + 15

1763.6 + 20

I-515 SOUNDWALLS.GPJ NV_DOT.GDT 8/26/08

NV_DOT

1758.6

1778.6 +5_5.50

17

24

28

13

23

30

17

22

32

17

27

30

13

25

23

6

6

8

5

6

7

SPT

100

100

100

105

105

95

95

B.O.H.

52

53

54

57

48

14

13

SPT

SPT

SPT С

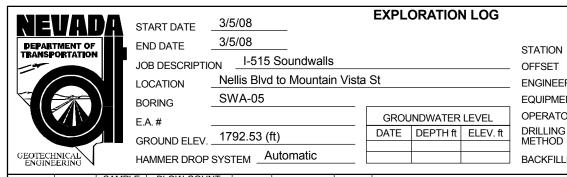
SPT

В

to Mountain	1 VISTA	ડા			ENGINEER	Abiana		100
					EQUIPMENT	Diedric	วท D-1	120
		GROU	NDWATER	LEVEL	OPERATOR	White		
t)		DATE	DEPTH ft	ELEV. ft	DRILLING METHOD	6" H.S	.A.	
utomatic					BACKFILLED _	Yes	_ DAT	E 3/5/08
LAB TESTS	USCS		MATE	ם ואום:	SCRIPTION			REMARKS
LAB ILOTO	USCS Group			INIAL DI	ESCRIPTION			
			P.C.C.P.					P.C.C.P. @ 14" Thick
		1.20	Silty, Clay	ev Sand w	vith Gravel very of	dense dr	,	
			<u> </u>	<u>.,</u>	<u></u> .e., .	,		
	-							
PI, S, W							1	Bulk Sample 1,
							;	3'-5'.
PI, S, W								
1 1, 0, 11	SC							
	SM							
PI, S, W								
		9.50						
		_ 9.50_	Clayey Sa	nd with G	ravel very dense	 to dense,		
			dry.					
PI, S, W								
	sc							
PI, S, W								
		15.00						
			Sandy Lea	an Clay Sur	f, dry to moist.			
PI, S, W								
. 1, O, VV								
	CL							
								(G) Foreign
PI, S, W							5	substance,
		23.50					1	oossibly organic.

SHEET 1 OF 1

647+30 52' RT Ablahani



			SHEET 1 O	F 1
		STATION	652+35	
		OFFSET	52' RT	
St		ENGINEER	Ablahani	
		EQUIPMENT	Diedrich D-120	
GROUNDWATER	I EVEI	OPERATOR	White	
DATE DEPTH ft	ELEV. ft	DRILLING METHOD	6" H.S.A.	
		BACKFILLED	Yes DATE 3/5/08	

ELEV. (ft)	DEPTH (ft)		MPLE TYPE	BLOW C 6 inch Increments	Last 1 foot	Percent Recov'd	LAB TESTS	USCS Group	MATERIAL DESCRIPTION	REMARKS
									P.C.C.P. 1.00	P.C.C.P. @ 12 Thick
	-								Silty Sand with Gravel very dense to dense.	
	2.50									Bulk Sample 1 2'-5'.
	_	Α	SPT	18 42	50/.46'	100	PI, S, W			2-3.
	4.00			50/.46'				SM		
1787.5 -	5 5.00			11						
	_	В	SPT	16	34	100	PI, S, W			
	6.50			18						
	7.50			4					Sandy Lean Clay stiff to very stiff.	
	9.00	С	SPT	7 7	14	85	PI, S, W			
				'						
1782.5 -	10 ^{10.00}			5						
	11.50	D	SPT	6 10	16	100	PI, S, W			
	12.50							CL		
	-	E	SPT	6 12	22	6E	DI C W			
	14.00		371	21	33	65	PI, S, W			
1777.5 -	15									
1777.5	13								16.00	
	-								Sandy Silt loose to medium dense.	
	17.50									
	-	F	SPT	4 5	13	100	PI, S, W	ML		
	19.00			8						
1772.5 -	_20									
	-								21.00 Lean Clay with Sand very stiff.	.
	 - <u>-</u>								Lean Gray with Sand very Sun.	
	22.50			3				CL		
	24.00	G	SPT	6 10	16	105	PI, S, W		24.00	
									В.О.Н.	
1767.5 -	⊢25									1



PLORATION LOG

SHEET 1 OF 1 657+39

I-515 Soundwalls JOB DESCRIPTION

52' RT

LOCATION **BORING**

Nellis Blvd to Mountain Vista St SWA-06

ENGINEER EQUIPMENT

STATION

OFFSET

Ablahani Diedrich D-120

E.A.#

1806.95 (ft)

OPERATOR GROUNDWATER LEVEL DRILLING METHOD DATE | DEPTH ft | ELEV. ft

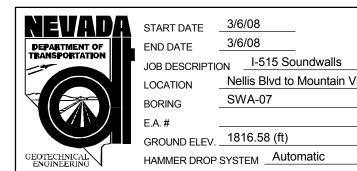
6" H.S.A.

HAMMER DROP SYSTEM Automatic

DATE 3/5/08 Yes BACKFILLED

White

ELEV. (ft)	DEPTH (ft)		IPLE TYPE	BLOW Co 6 inch Increments	Last 1 foot	Percent Recov'd	LAB TESTS	USCS Group	MATERIAL DESCRIPTION	REMARKS
(IL)	(11)			Increments	1 foot	Recova		эхэг	P.C.C.P. 1.00	P.C.C.P. @ 12' Thick
									Silty, Clayey Sand with Gravel dense to very dense, dry.	
	2.50 2.90		ODT	0.47.41	04/4	400	DI 0 14/		20.025, 2.7.	
	-		SPT	24/.4'	24/.4'	100	PI, S, W			(A) 10 blows w no progress. Bulk Sample 1, 2.5'-5'.
1802.0 -	5 5.00			14						
	6.50	В	SPT	24 36	60	95	PI, S, W			
	7.50			40						
	9.00	С	SPT	18 22 27	49	105	PI, S, W			
1797.0 -	10.00									
	11.50	D	SPT	16 17 25	42	100	S, W			
	12.50							SC SM		
	14.00	Е	SPT	13 21 27	48	75	PI, S, W			(E) Pulverized Rock in sample
1792.0 -	—15									
	- - 17.50									
	-	_	ODT	12		0.5	DI 0 144			
	19.00	F	SPT	30 30	60	95	PI, S, W			
4707.0	00									
1787.0 -	20									
	-									300 psi down
	-									21'-21.7'. Hard Drilling
	-							<u> </u>	23.00 Clayey Sand with Gravel dense, brown.	21'-23'.
	-									
1782.0 -	25.00 25							sc		
		G	SPT	9 16	38	85	PI, S, W			
	26.50			22					26.50 B.O.H.	\dashv
	-									



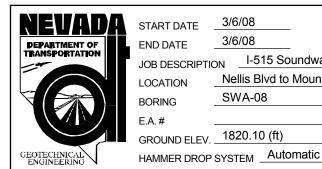
	EXPL	ORATIO	N LOG			OUEET 4 OF 4
						SHEET 1 OF 1
				STATION	662+44	
				OFFSET	52' RT	
/ista	St			ENGINEER	Ablahani	
				EQUIPMENT	Diedrich D-12	20
	GROL	INDWATER	LEVEL	OPERATOR	White	
_	DATE	DEPTH ft	ELEV. ft	DRILLING METHOD	6" H.S.A.	
_				WILTHOU		

BACKFILLED

Yes

DATE __3/6/08

ŀ	ELEV.	DEPTH	SAI	MPLE	BLOW C	TNUC			LISCS		
	(ft)	(ft)	NO.	TYPE	6 inch Increments	Last 1 foot	Percent Recov'd	LAB TESTS	USCS Group	MATERIAL DESCRIPTION	REMARKS
										P.C.C.P.	P.C.C.P. @11.5" Thick
										1.00	THICK
										<u>Clayey Sand with Gravel</u> dense to very dense, dry.	
										ury.	
		2.50									Bulk Sample 1, 2'-5'.
					13						(A) No progress.
		3.70	Α	SPT	41	30/.2	90	PI, S, W			
		3.70			30/.2						
	1811.6 -	5.00									
	1011.0				20						(B) Pulverized rock in sample.
			В	SPT	25	42	95	PI, S, W			Tock in sample.
		6.50			17						
		7.50									
					12						
			С	SPT	17	41	100	PI, S, W			
		9.00			24						
	1806.6 -	10.00							sc		
	1000.0 -	10			8				36		
			D	SPT	9	20	85	PI, S, W			
		11.50			11						
]		
		12.50									
					17						
			Ε	SPT	22	53	100	PI, S, W			
		14.00			31						
8	1801.6 -	15.00									
8/26/08	1001.0				8						
ቪ		<u> </u>	F	SPT	15	35	95	PI, S, W			
9		16.50			20						
≥											
2											
LS.G											
MAL											
										19.00	Hard dilling
5 SC										B.O.H.	began at 18.5'. No progress,
NV_DOT I-515 SOUNDWALLS.GPJ NV_DOT.GDT	1796.6 -	-20									terminate drilling
[BO	1130.0 -	-20									at 19'.
≥											



		EXPLORATION LOG
RT DATE	3/6/08	

I-515 Soundwalls

Nellis Blvd to Mountain Vista St

SWA-08

1820.10 (ft)

GROUNDWATER LEVEL DATE | DEPTH ft | ELEV. ft

667+48 STATION

ENGINEER

EQUIPMENT

OPERATOR

52' RT OFFSET

> Ablahani Diedrich D-120

SHEET 1 OF 1

White

DRILLING METHOD 6" H.S.A. DATE 3/6/08 Yes BACKFILLED

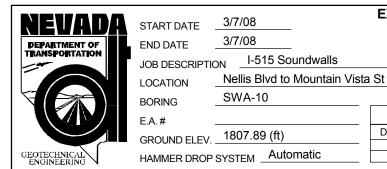
	DEDTIL	SAI	MPLE	BLOW C	OUNT					
ELEV. (ft)	DEPTH (ft)		TYPE		Last 1 foot	Percent Recov'd	LAB TESTS	USCS Group	MATERIAL DESCRIPTION	REMARKS
	, ,								P.C.C.P.	P.C.C.P. @ 12"
									1.00	Thick
									Silty Sand with Gravel dense, dry.	
										Bulk Sample 1,
	2.50			11						2'-5'.
	-	Α	SPT	15	35	80	PI, S, W			(A) Rock in
	4.00		0	20			1 1, 0, 11			sampler shoe.
								-		
1815.1 -	5.00									
1015.1	,			16				SM		
	_	В	SPT	23	43	95	PI, S, W			
	6.50			20						
	7.50			8						
	-	С	SPT	14	43	80	PI, S, W			
	9.00			29			, -,			
									9.50	
1810.1 -	10.00								Clayey Sand with Gravel dense, dry.	
1010.1				10						
	_	D	SPT	16	32	95	PI, S, W	SC		
	11.50			16				-		
	12.50								2.00 Silty Sand with Gravel dense to very dense, dry.	
	12.50			10				_	across to very defice, ary.	(E) Pulverized
		Е	SPT	11	48	100	PI, S, W			rock in sample.
	14.00			37						
1805.1 -	15.00									
		_	ODT	11	0.4	0.5	DI 0 14/			
		F	SPT	14 17	31	95	PI, S, W			
	16.50			17				SM		
	-									Hard Drilling
										17'-17.5'.
	_									
1800.1 -	20.00			4.5						
		G	SPT	16	61	100	DI C W			
	21 50		321	28 33	01	100	PI, S, W		24.50	
	21.50								21.50 B.O.H.	-
	[
			1	I			l		<u> </u>	l .

NV_DOT I-515 SOUNDWALLS.GPJ NV_DOT.GDT 8/26/08

NEVADA	START DATE	EXPLORATION LOG		SHEET 1
DEPARTMENT OF	END DATE <u>3/6/08</u>		STATION	672+53
TRANSPORTATION	JOB DESCRIPTION I-515 Soundwalls		OFFSET	52' RT
	LOCATION Nellis Blvd to Mountain Vista	a St	ENGINEER	Ablahani
	BORING SWA-09		EQUIPMENT	Diedrich D-120
	E.A.#	GROUNDWATER LEVEL	OPERATOR	White
	GROUND ELEV. 1817.15 (ft)	DATE DEPTH ft ELEV. ft	DRILLING METHOD	6" H.S.A.
GEOTECHNICAL ENGINEERING	HAMMER DROP SYSTEM Automatic		BACKFILLED	Yes DATE 3/6/08
CAMI	DIF DIOM COUNT			

SHEET 1 OF 1

ELEV. (ft)	DEPTH (ft)		IPLE TYPE	BLOW CO 6 inch Increments	Last	Percent Recov'd	LAB TESTS	USCS Group	MATERIAL DESCRIPTION	REMARKS
(IL)	(it)			increments	1 1001	Recova			P.C.C.P. 1.00	P.C.C.P. @ 12 Thick
	<u> </u>								Clayey, Silty Sand with Gravel dense, dry.	
	2.50							SC		Dulle Campala 4
	-	А	SPT	12 20	41	75	PI, S, W	SC SM		Bulk Sample 1 2.5'-5'.
	4.00			21				_	4.50	
1812.2 -	5 5.00			9					Clayey, Silty Sand dense, dry.	
	- 6.50	В	SPT	15 25	40	100	PI, S, W			
	6.50			23				-		
1007.0	10.00									
1807.2 -	10	С	SPT	18 14	42	100	PI, S, W			
	11.50		01 1	28		100	11, 0, 11	SC SM		
	-									
	-									
	_									
1802.2 -	15.00			14				_		
		D	SPT	14	26	95	PI, S, W			
	16.50			12				_		
									Clayey Sand with Gravel medium dense, light tan.	
	20.00							sc		
1797.2 -	20.00	E	SPT	5 6	18	80	PI, S, W			
	21.50		OI" I	12	10	00	1 1, 0, 11		21.50	
	-								В.О.Н.	
	-									



3/7/09	EXPLORATION LO	•

I-515 Soundwalls

1807.89 (ft)

Automatic

OG

GROUNDWATER LEVEL

DATE | DEPTH ft | ELEV. ft

677+57

STATION OFFSET

52' RT Ablahani SHEET 1 OF 1

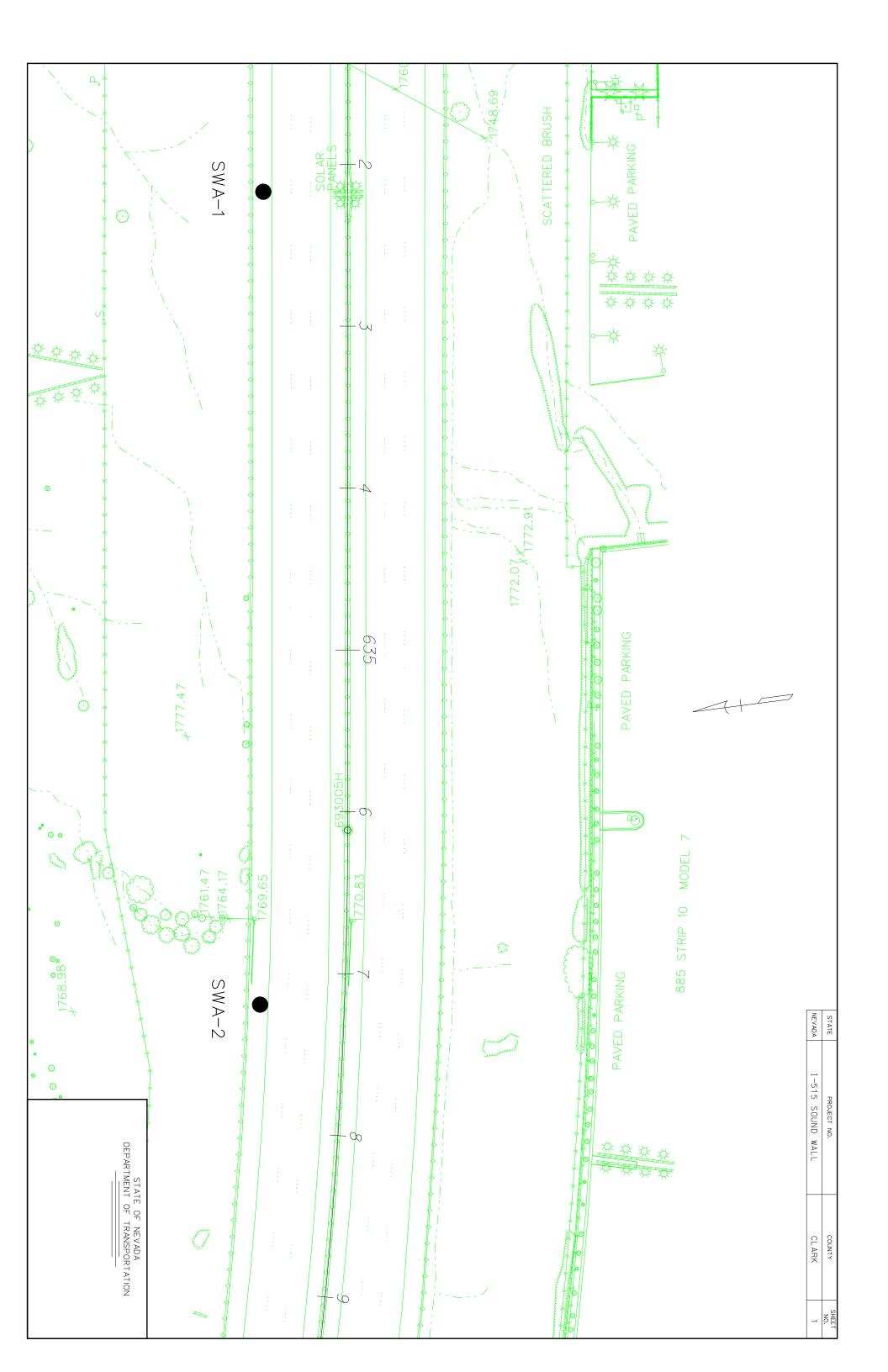
ENGINEER EQUIPMENT

Diedrich D-120 White

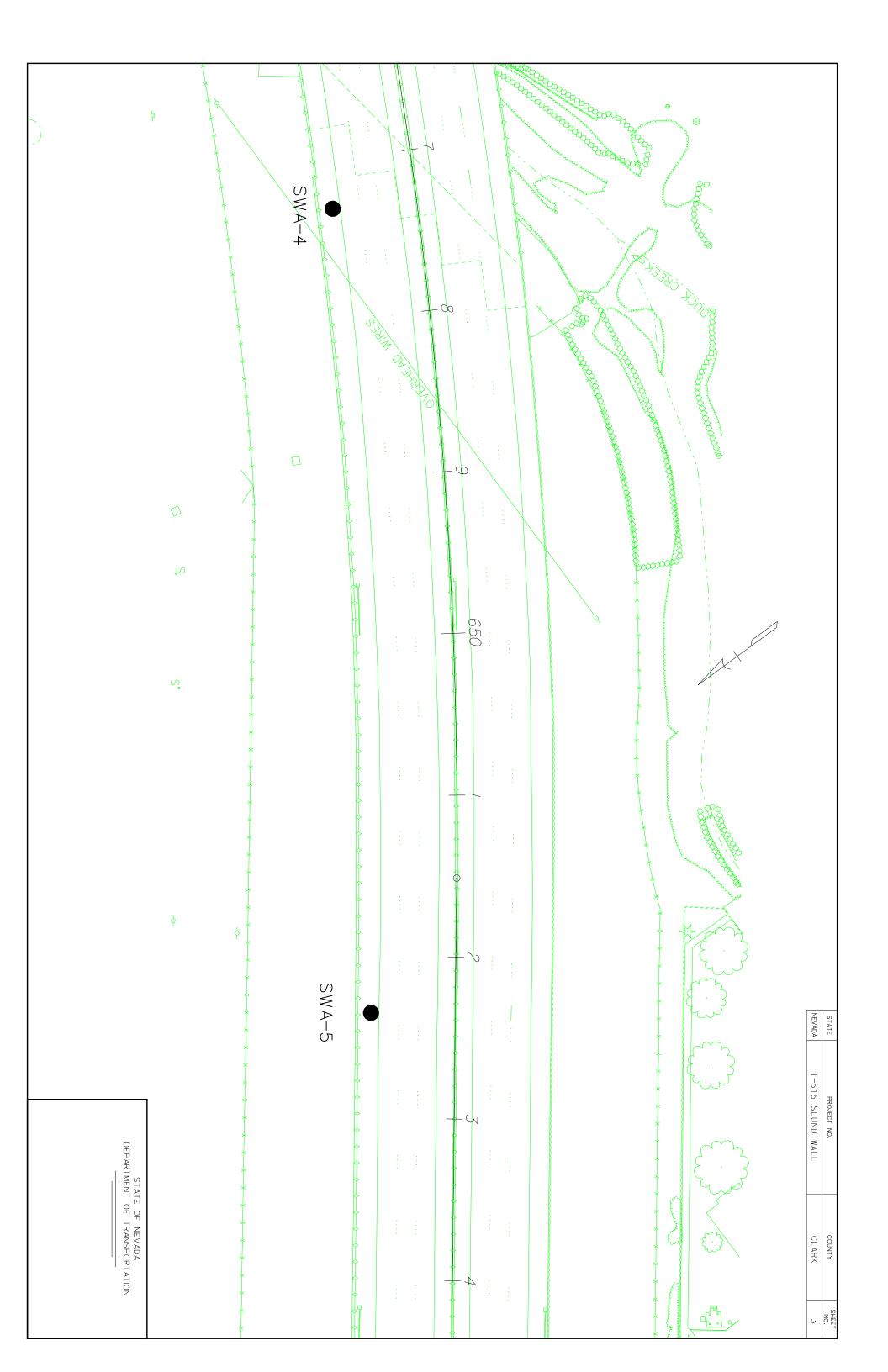
OPERATOR DRILLING METHOD

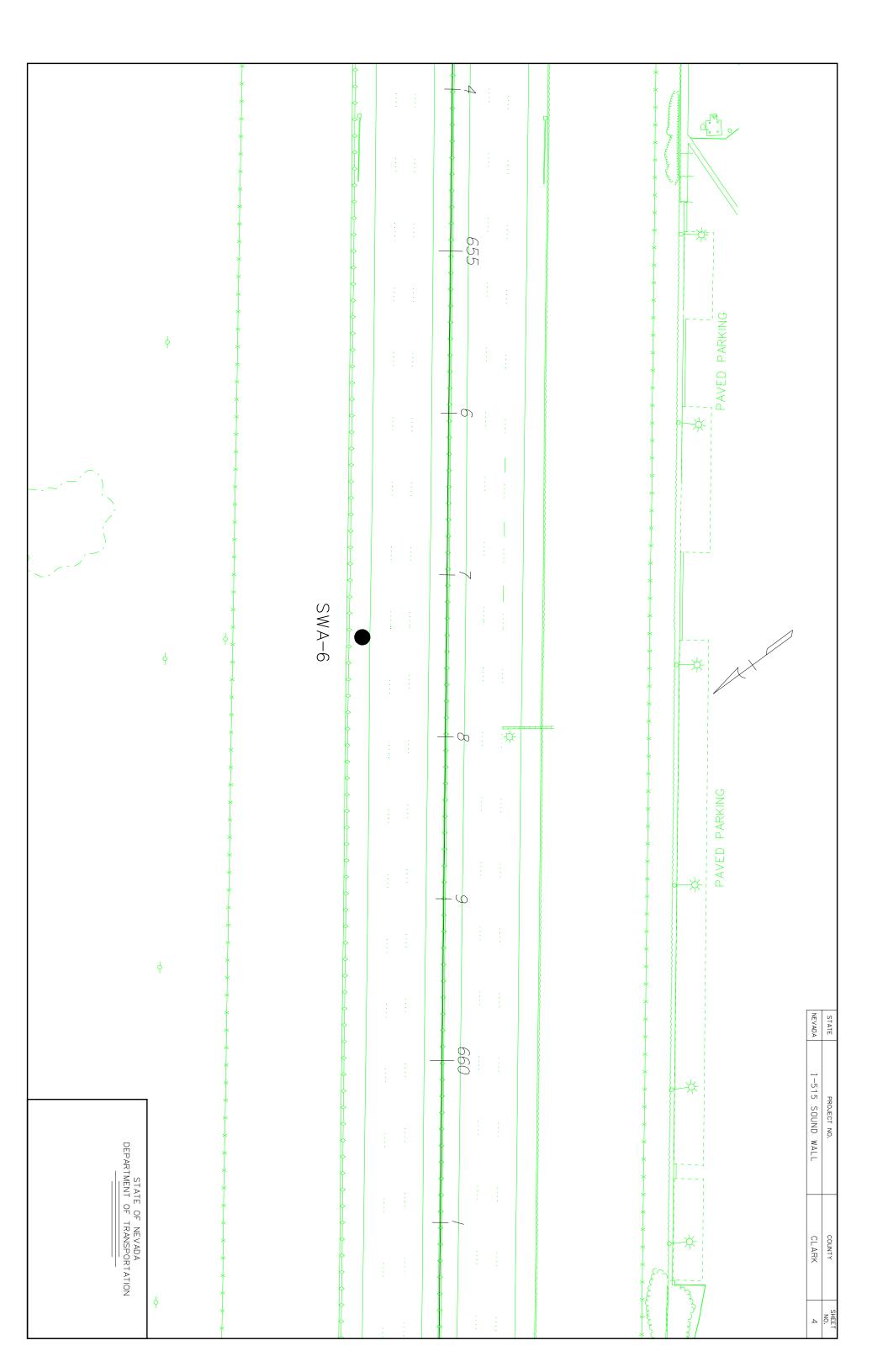
6" H.S.A. DATE 3/7/08 BACKFILLED Yes

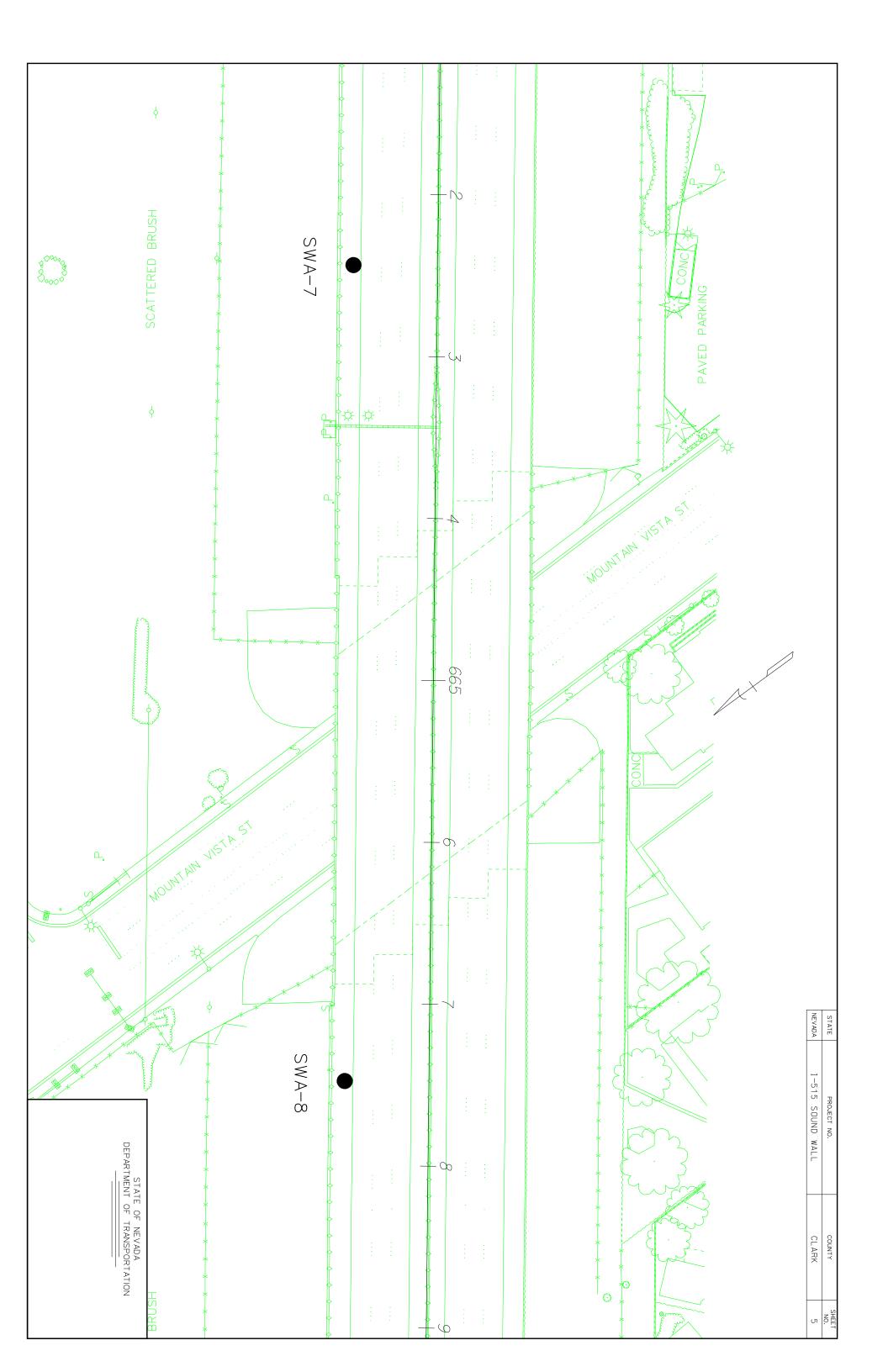
(ft) (1	2.50		SPT	morements		0	LAB TESTS	SC SC	MATERIAL DESCRIPTION P.C.C.P. 1.00 Clayey Sand with Gravel very dense, dry. 3.50 Silty Sand with Gravel dense, dry.	P.C.C.P. @ 11. Thick Bulk Sample 1, 2'-5'. (A) 10 blows wino progress. No
1802.9 -5	5.00 6.50 7.50	В		12 16				sc	1.00 Clayey Sand with Gravel very dense, dry.	Thick Bulk Sample 1, 2'-5'. (A) 10 blows wino progress. No
1802.9 -5	5.00 6.50 7.50	В		12 16				sc	Clayey Sand with Gravel very dense, dry.	Thick Bulk Sample 1, 2'-5'. (A) 10 blows wino progress. No
-	6.50 7.50		SPT	16	38	05			3.50	(A) 10 blows w
-	6.50 7.50		SPT	16	38	05			Sitty Sand with Graver dense, dry.	Recovery.
-	7.50		SPT		38	0-		SM		Very hard drilli 2.5'-3.5'.
		С				95	PI, S, W	_	7.00	Probable bould
	9.00	С							Sandy Lean Clay medium stiff.	
1797.9			SPT	3 2 5	7	115	PI, S, W	CL		
1797.9 10	00.00								10.00	
<u> </u>	11.50	D	CMS	7 8 15	23	100	PI, S, W, UW		Clayey Sand with Gravel medium dense to dense.	
1	13.00	Е	SPT	12 12 36	48	80	PI, S, W			(E) Pulverized rock in sample
-								sc		
1792.9 15	<u>15.00</u>							1		(E) E
	16.50	F	SPT	2 6 12	18	35	PI, S, W	_		(F) Rock in sampler shoe.
									18.00 Sandy Lean Clay very stiff, moist to wet.	_
1787.9 - 20	20.00									
	21.50	G	SPT	1 5 14	19	100	PI, S, W	-		
-								CL		
- 1782.9 - 2 5	25.00									
	26.50	Н	SPT	4 9 12	21	105	PI, S, W		26.50	(H) Free water
-									В.О.Н.	
-										

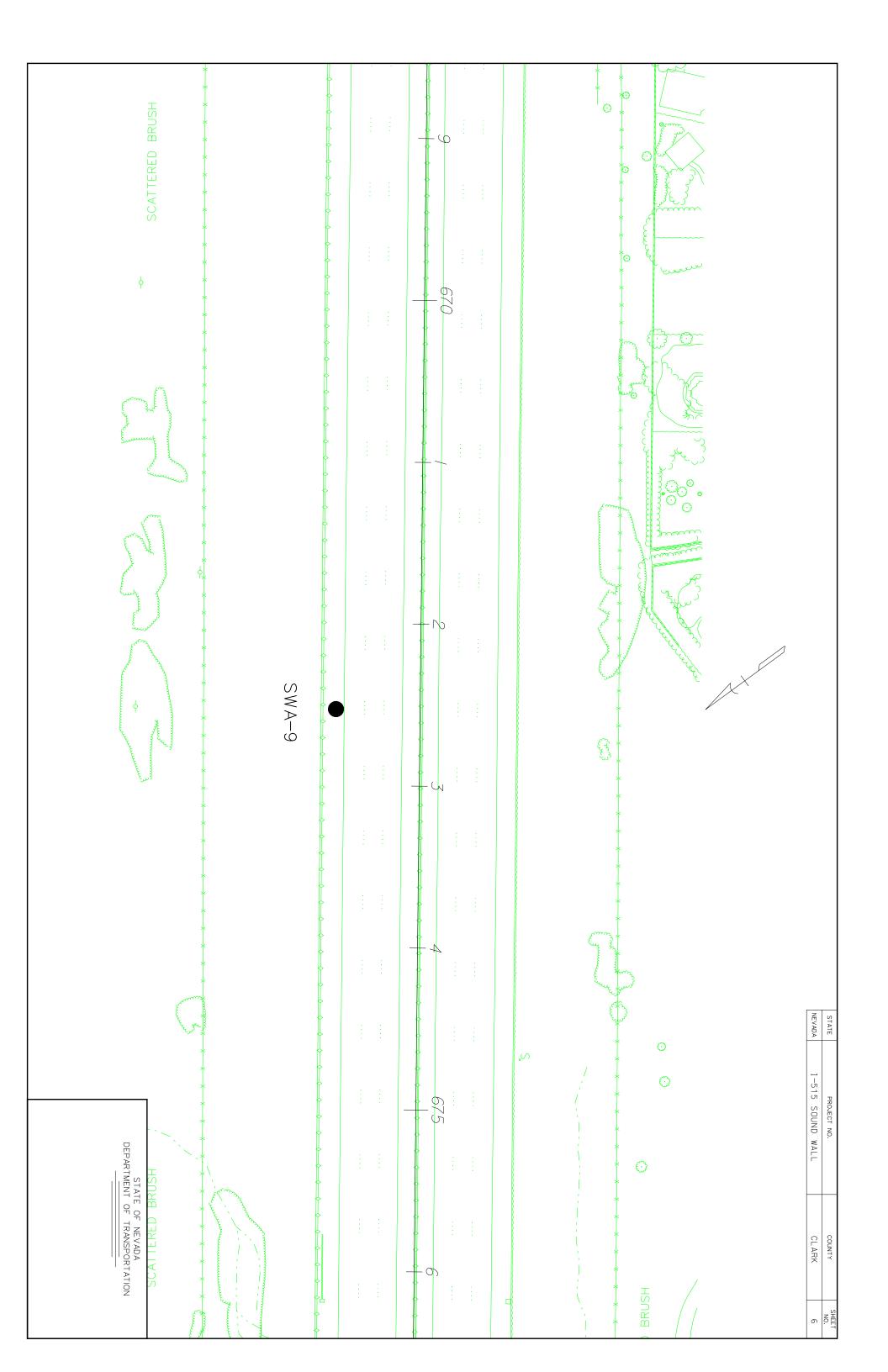


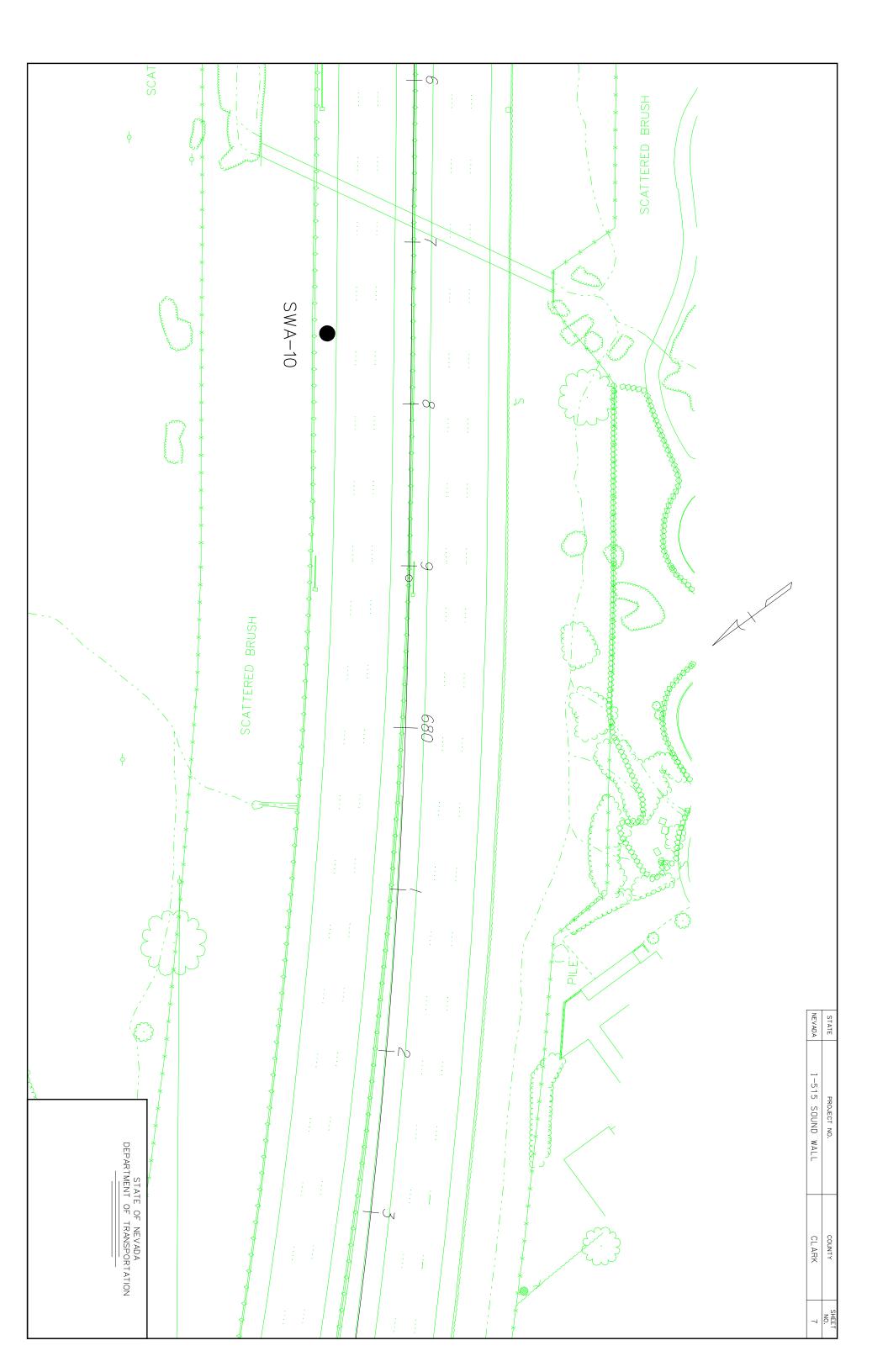












APPENDIX B

Summary of Test Results Tables Particle Size Distribution Reports Direct Shear Test Reports

EA/Cont #

Job Description I - 515 Soundwalls, Nellis to Mtn. Vista

Boring No. **SWA - 1** Elevation (ft) 03/11/2008 Station Date

	SAMPLE	SAMP-	N			DRY	%					STR	ENGTH T	EST		
SAMPLE NO.	DEPTH (ft)	LER TYPE	BLOWS per ft.	SOIL GROUP	W%	UW pcf	PASS #200	LL %	PL %	PI %	TEST TYPE	φ deg.	C psi	φ deg.	C psi	COMMENTS
												Pe	eak	Res	idual	
А	3.0 - 4.5	SPT	31	SC	10.8		25.2	33	23	10						
В	5.0 - 6.5	SPT	18	SC	15.4		39.6	38	19	19						
C1	7.7 - 8.2	CMS	26	GC	16.3		38.3	43	17	26						
C2	8.2 - 8.7	CMS		SM	17.5		49.1	44	17	27						
D	9.0 - 10.0	SPT	35	CL	11.7		76.8	35	16	19						
Е	10.0 - 11.4	SPT	R	SC	14.3		38.2	49	17	32						
F	12.5 - 13.4	SPT	R	SC	12.6		29.0	33	18	15						
G	17.5 - 19.0	SPT	100	CL	16.7		55.5	36	19	17						
Н	22.5 - 24.0	SPT	45	CL	21.9		52.6	33	19	14						
BULK 1	3.0 - 5.0			SC			22.7	37	24	13						RV = 72

CMS = California Modified Sampler 2.40" ID

SPT = Standard Penetration 1.38" ID

CS = Continuous Sample 3.23" ID

RC = Rock Core

PB = Pitcher Barrel

CSS = Calif. Split Spoon 2.42" ID

CPT = Cone Penetration Test

TP = Test Pit

P = Pushed, not driven

R = Refusal

Sh = Shelby Tube 2.87" ID

U = Unconfined Compressive

UU = Unconsolidated Undrained

CD = Consolidated Drained

CU = Consolidated Undrained

DS = Direct Shear

φ = Friction

C = Cohesion

N = No. of blows per ft., sampler

N = Field SPT $N = (N_{css})(0.62)$ H = Hydrometer

S = Sieve

G = Specific Gravity

PI = Plasticity Index

LL = Liquid Limit

PL = Plastic Limit

NP = Non-Plastic

OC = Consolidation

Ch = Chemical

RV = R - Value MD = Moisture Density CM = Compaction

E = Swell/Pressure on Expansive Soils

SL = Shrinkage Limit

UW= Unit Weight

W = Moisture Content K = Permeability

O = Organic Content

D = Dispersive

RQD = Rock Quality Designation

X = X-Ray Defraction

^{* =} Average of subsamples

EA/Cont #

Job Description I - 515 Soundwalls, Nellis to Mtn. Vista

Boring No. SWA - 2 Elevation (ft) 03/11/2008 Station Date

	SAMPLE	SAMP-	N			DRY	%					STR	ENGTH T	EST		
SAMPLE NO.	DEPTH	LER TYPE	BLOWS per ft.	SOIL GROUP	W%	UW pcf	PASS #200	LL %	PL %	PI %	TEST TYPE	φ deg.	C psi	φ deg.	C psi	COMMENTS
NO.	(ft)	IIFL	per it.	GROOF		рсі	#200	/0	/0	/0	IIFL		eak		dual	
А	3.0 - 4.5	SPT	8	CL	19.4		53.7	44	16	28						
B1	5.0 - 6.0	SPT	21	CL	22.5		53.6	48	17	31						
B2	6.0 - 6.5	SPT			8.4		17.8									
C1	7.2 - 7.7	CMS	42	CL	23.2	79.0	69.7	46	18	28						
C2	7.7 - 8.2	CMS		GC	12.2	101.3	28.6	48	19	29						
D	8.5 - 9.5	SPT	54	SC	14.2		38.6	40	18	22						
Е	10.0 - 11.5	SPT	R	SC	10.0		32.4	35	17	18						
F	14.0 - 15.5	SPT	52	SC	18.6		48.0	37	19	18						
G	19.0 - 20.5	SPT	31	CL	24.8		84.7	39	19	20						
Н	24.0 - 25.5	SPT	40	CL	24.3		66.9	31	21	10						
BULK 1	3.0 - 5.0			SC			40.8	37	16	21						RV = 20

CMS = California Modified Sampler 2.40" ID

SPT = Standard Penetration 1.38" ID

CS = Continuous Sample 3.23" ID

RC = Rock Core

PB = Pitcher Barrel

CSS = Calif. Split Spoon 2.42" ID

CPT = Cone Penetration Test

TP = Test Pit

P = Pushed, not driven

R = Refusal

Sh = Shelby Tube 2.87" ID

U = Unconfined Compressive

UU = Unconsolidated Undrained

CD = Consolidated Drained

CU = Consolidated Undrained

DS = Direct Shear

φ = Friction

C = Cohesion

N = No. of blows per ft., sampler

N = Field SPT $N = (N_{css})(0.62)$ H = Hydrometer

S = Sieve

G = Specific Gravity

PI = Plasticity Index

LL = Liquid Limit

PL = Plastic Limit

NP = Non-Plastic

OC = Consolidation

Ch = Chemical RV = R - Value

MD = Moisture Density

CM = Compaction

E = Swell/Pressure on Expansive Soils

SL = Shrinkage Limit

UW= Unit Weight

W = Moisture Content

K = Permeability O = Organic Content

D = Dispersive

RQD = Rock Quality Designation

X = X-Ray Defraction

^{* =} Average of subsamples

EA/Cont #

Job Description I - 515 Soundwalls, Nellis to Mtn. Vista

Boring No. **SWA - 3** Elevation (ft) 03/11/2008 Station Date

	SAMPLE	SAMP-	N			DRY	%					STR	ENGTH T	EST		
SAMPLE NO.	DEPTH (ft)	LER TYPE	BLOWS per ft.	SOIL GROUP	W%	UW pcf	PASS #200	LL %	PL %	PI %	TEST TYPE	φ deg.	C psi	φ deg.	C psi	COMMENTS
												Pe	ak	Res	idual	
Α	2.5 - 4.0	SPT	31	SM	9.5		21.3	27	NP	NP						
B1	5.0 - 6.2	SPT	20	SC-SM	13.5		44.4	26	21	5						
B2	6.2 - 6.5	SPT			17.9			43	17	26						
C1	8.2 - 8.7	CMS	19	CL	20.5	100.0	58.7	46	18	28						
C2	8.7 - 9.2	CMS		CL	22.3	97.5	58.1	42	15	27						
D	9.5 - 11.0	SPT	18	SC-SM	13.0		43.2	36	22	14						
E1	12.7 - 13.2	CMS	7	CL-ML	13.1	86.3	55.6	26	22	4						
E2	13.2 - 13.7	CMS		CL	17.6	80.0	75.0	29	18	11	DS	29	0.07	29	0	
F	14.0 - 15.5	SPT	10	CL	13.4		50.2	30	20	10						
G1	17.7 - 18.2	CMS	36	CL	20.6	98.0	72.4	39	17	22						
G2	18.2 - 18.7	CMS		CL	24.5	94.6	54.8	46	18	28						
Н	19.0 - 20.0	SPT	43	SC-SM	11.1		36.9	25	18	7						

G = Specific Gravity

CMS = California Modified Sampler 2.40" ID

SPT = Standard Penetration 1.38" ID

CS = Continuous Sample 3.23" ID

RC = Rock Core

PB = Pitcher Barrel

CSS = Calif. Split Spoon 2.42" ID

CPT = Cone Penetration Test

TP = Test Pit

P = Pushed, not driven

R = Refusal

Sh = Shelby Tube 2.87" ID

U = Unconfined Compressive

UU = Unconsolidated Undrained

CD = Consolidated Drained

CU = Consolidated Undrained

DS = Direct Shear

φ = Friction

C = Cohesion

N = No. of blows per ft., sampler

N = Field SPT $N = (N_{css})(0.62)$ H = Hydrometer CM = Compaction

S = Sieve E = Swell/Pressure on Expansive Soils

SL = Shrinkage Limit

PI = Plasticity Index UW= Unit Weight

W = Moisture Content LL = Liquid Limit

PL = Plastic Limit K = Permeability

NP = Non-Plastic O = Organic Content

OC = Consolidation D = Dispersive

Ch = Chemical RQD = Rock Quality Designation

RV = R - Value X = X-Ray Defraction

MD = Moisture Density HCpot = Hydro-Collapse Potential

^{* =} Average of subsamples

EA/Cont #

Job Description I - 515 Soundwalls, Nellis to Mtn. Vista

Boring No. **SWA - 3** Elevation (ft) 03/11/2008 Station Date

	SAMPLE	SAMP-	N			DRY	%					STR	ENGTH T	EST		
SAMPLE	DEPTH	LER	BLOWS	SOIL	W%	UW	PASS	LL	PL	PI	TEST	φ	С	φ	С	COMMENTS
NO.	(ft)	TYPE	per ft.	GROUP		pcf	#200	%	%	%	TYPE	deg.	psi	deg.	psi	
												Pe	ak	Res	idual	
1	25.0 - 26.5	SPT	20	CL	31.9		95.0	49	19	30						
BULK 1	3.0 - 5.0			SC			20.4	32	23	9						RV = 64
										_						

CMS = California Modified Sampler 2.40" ID

SPT = Standard Penetration 1.38" ID

CS = Continuous Sample 3.23" ID

RC = Rock Core

PB = Pitcher Barrel

CSS = Calif. Split Spoon 2.42" ID

CPT = Cone Penetration Test

TP = Test Pit

P = Pushed, not driven

R = Refusal

Sh = Shelby Tube 2.87" ID

U = Unconfined Compressive

UU = Unconsolidated Undrained

CD = Consolidated Drained

CU = Consolidated Undrained

DS = Direct Shear

φ = Friction

C = Cohesion

N = No. of blows per ft., sampler

N = Field SPT $N = (N_{css})(0.62)$ H = Hydrometer

S = Sieve

G = Specific Gravity

PI = Plasticity Index

LL = Liquid Limit

PL = Plastic Limit NP = Non-Plastic

OC = Consolidation

Ch = Chemical

RV = R - Value MD = Moisture Density CM = Compaction

E = Swell/Pressure on Expansive Soils

SL = Shrinkage Limit

UW= Unit Weight

W = Moisture Content

K = Permeability O = Organic Content D = Dispersive

RQD = Rock Quality Designation

X = X-Ray Defraction

^{* =} Average of subsamples

EA/Cont #

Job Description I - 515 Soundwalls, Nellis to Mtn. Vista

Boring No. SWA - 4 Elevation (ft) 03/11/2008 Station Date

	SAMPLE	SAMP-	N			DRY	%					STR	ENGTH T	EST		
SAMPLE NO.	DEPTH (ft)	LER TYPE	BLOWS per ft.	SOIL GROUP	W%	UW pcf	PASS #200	LL %	PL %	PI %	TEST TYPE	φ deg.	C psi	φ deg.	C psi	COMMENTS
												Pe	eak	Res	idual	
Α	2.5 - 4.0	SPT	52	SC	8.3		18.6	32	23	9						
В	4.0 - 5.5	SPT	53	SM	6.6		16.2	34	25	9						
С	7.5 - 9.0	SPT	54	SC-SM	7.4		16.5	29	22	7						
D	10.0 - 11.5	SPT	57	SC	8.7		19.9	30	22	8						
E	12.0 - 13.5	SPT	48	SC	10.1		33.2	32	21	11						
F	17.0 - 18.5	SPT	14	CL	16.5		59.0	29	18	11						
G	22.0 - 23.5	SPT	13	CL	20.3		66.8	30	20	10						
BULK 1	3.0 - 5.0			SC			18.5	29	21	8						RV = 75

CMS = California Modified Sampler 2.40" ID

SPT = Standard Penetration 1.38" ID

CS = Continuous Sample 3.23" ID

RC = Rock Core

PB = Pitcher Barrel

CSS = Calif. Split Spoon 2.42" ID

CPT = Cone Penetration Test

TP = Test Pit

P = Pushed, not driven

R = Refusal

Sh = Shelby Tube 2.87" ID

U = Unconfined Compressive

UU = Unconsolidated Undrained

CD = Consolidated Drained

CU = Consolidated Undrained

DS = Direct Shear

φ = Friction

C = Cohesion

N = No. of blows per ft., sampler

N = Field SPT $N = (N_{css})(0.62)$ H = Hydrometer

S = Sieve

G = Specific Gravity

PI = Plasticity Index

LL = Liquid Limit

PL = Plastic Limit NP = Non-Plastic

OC = Consolidation

Ch = Chemical

RV = R - Value MD = Moisture Density CM = Compaction

E = Swell/Pressure on Expansive Soils

SL = Shrinkage Limit

UW= Unit Weight

W = Moisture Content

K = Permeability O = Organic Content

D = Dispersive

RQD = Rock Quality Designation

X = X-Ray Defraction

^{* =} Average of subsamples

EA/Cont #

Job Description I - 515 Soundwalls, Nellis to Mtn. Vista

Boring No. **SWA - 5** Elevation (ft) 03/11/2008 Station Date

	SAMPLE	SAMP-	N			DRY	%					STR	ENGTH T	EST		
SAMPLE NO.	DEPTH (ft)	LER TYPE	BLOWS per ft.	SOIL GROUP	W%	UW pcf	PASS #200	LL %	PL %	PI %	TEST TYPE	φ deg.	C psi	φ deg.	C psi	COMMENTS
										_		PE	ak	Res	idual	
Α	2.5 - 4.0	SPT	R	SM	6.9		20.9	26	23	3						
В	5.0 - 6.5	SPT	34	SM	6.7		15.7	28	25	3						
С	7.5 - 9.0	SPT	14	CL	17.1		53.8	29	21	8						
D	10.0 - 11.5	SPT	16	CL	16.0		54.2	38	17	21						
Е	12.5 - 14.0	SPT	33	CL	16.9		61.4	38	15	23						
F	17.5 - 19.0	SPT	13	ML	13.8		55.0	24	21	3						
G	22.5 - 24.0	SPT	16	CL	20.1		73.6	25	17	8						
BULK 1	20 5.0			SC			18.1	28	20	8						RV = 56

CMS = California Modified Sampler 2.40" ID

SPT = Standard Penetration 1.38" ID

CS = Continuous Sample 3.23" ID

RC = Rock Core

PB = Pitcher Barrel

CSS = Calif. Split Spoon 2.42" ID

CPT = Cone Penetration Test

TP = Test Pit

P = Pushed, not driven

R = Refusal

Sh = Shelby Tube 2.87" ID

U = Unconfined Compressive

UU = Unconsolidated Undrained

CD = Consolidated Drained

CU = Consolidated Undrained

DS = Direct Shear

φ = Friction

C = Cohesion

N = No. of blows per ft., sampler

N = Field SPT $N = (N_{css})(0.62)$ H = Hydrometer

S = Sieve

G = Specific Gravity

PI = Plasticity Index

LL = Liquid Limit

PL = Plastic Limit

NP = Non-Plastic

OC = Consolidation

Ch = Chemical

RV = R - Value MD = Moisture Density CM = Compaction

E = Swell/Pressure on Expansive Soils

SL = Shrinkage Limit

UW= Unit Weight

W = Moisture Content K = Permeability

O = Organic Content

D = Dispersive

RQD = Rock Quality Designation

X = X-Ray Defraction

^{* =} Average of subsamples

EA/Cont #

Job Description I - 515 Soundwalls, Nellis to Mtn. Vista

 Boring No.
 SWA - 6
 Elevation (ft)
 Station
 Date
 03/11/2008

	SAMPLE	SAMP-	N			DRY	%					STR	ENGTH T	EST		
SAMPLE NO.	DEPTH (ft)	LER TYPE	BLOWS per ft.	SOIL GROUP	W%	UW pcf	PASS #200	LL %	PL %	PI %	TEST TYPE	φ deg.	C psi eak	φ deg.	C psi idual	COMMENTS
												PE	ак	Res	duai	
Α	2.5 - 2.9	SPT	R	SM	3.2		18.8	30	26	4						
В	5.0 - 6.5	SPT	60	SC-SM	6.2		12.4	24	19	5						
С	7.5 - 9.0	SPT	49	SC-SM	8.0		17.9	30	21	9						
D	10.0 - 11.5	SPT	42		10.8		27.0									
Е	12.5 - 14.0	SPT	48	SM	9.6		28.2	40	27	13						
F	17.5 - 19.0	SPT	60	SC	9.7		19.5	41	25	16						
G	25.0 - 26.5	SPT	38	SC	13.0		49.1	36	15	21						
BULK 1	2.5 - 5.0			SC-SM			17.0	27	20	7						RV = 77

CMS = California Modified Sampler 2.40" ID

SPT = Standard Penetration 1.38" ID

CS = Continuous Sample 3.23" ID

RC = Rock Core

PB = Pitcher Barrel

CSS = Calif. Split Spoon 2.42" ID

CPT = Cone Penetration Test

TP = Test Pit

P = Pushed, not driven

R = Refusal

Sh = Shelby Tube 2.87" ID

U = Unconfined Compressive

UU = Unconsolidated Undrained

CD = Consolidated Drained

CU = Consolidated Undrained

DS = Direct Shear

φ = Friction

C = Cohesion

N = Field SPT

N = No. of blows per ft., sampler

H = Hydrometer

S = Sieve

G = Specific Gravity

DI Disatisita Index

PI = Plasticity Index

I = Flasticity fluex

LL = Liquid Limit

PL = Plastic Limit NP = Non-Plastic

OC = Consolidation

Ch = Chemical

RV = R - Value MD = Moisture Density CM = Compaction

E = Swell/Pressure on Expansive Soils

SL = Shrinkage Limit

UW= Unit Weight

W = Moisture Content

K = PermeabilityO = Organic Content

D = Dispersive

RQD = Rock Quality Designation

X = X-Ray Defraction

HCpot = Hydro-Collapse Potential

 $N = (N_{css})(0.62)$

^{* =} Average of subsamples

EA/Cont #

Job Description I - 515 Soundwalls, Nellis to Mtn. Vista

Boring No. SWA - 7 Elevation (ft) 03/11/2008 Station Date

	SAMPLE	SAMP-	N			DRY	%					STR	ENGTH T	EST		
SAMPLE NO.	DEPTH (ft)	LER TYPE	BLOWS per ft.	SOIL GROUP	W%	UW pcf	PASS #200	LL %	PL %	PI %	TEST TYPE	φ deg.	C psi	φ deg.	C psi	COMMENTS
												Pe	ak	Res	idual	
Α	2.5 - 3.7	SPT	R	SC	6.8		22.2	28	19	9						
В	5.0 - 6.5	SPT	42	SC	8.5		22.4	35	23	12						
С	7.5 - 9.0	SPT	41	SC	8.3		21.6	32	22	10						
D	10.0 - 11.5	SPT	20	SC	11.5		27.7	37	23	14						
Е	12.5 - 14.0	SPT	53	SC	9.0		21.4	32	21	11						
F	15.0 - 16.5	SPT	35	SC	10.7		23.0	39	23	16						
BULK 1	2.0 - 5.0			SC-SM			16.0	24	18	6						RV = 68

CMS = California Modified Sampler 2.40" ID

SPT = Standard Penetration 1.38" ID

CS = Continuous Sample 3.23" ID

RC = Rock Core

PB = Pitcher Barrel

CSS = Calif. Split Spoon 2.42" ID

CPT = Cone Penetration Test

TP = Test Pit

P = Pushed, not driven

R = Refusal

Sh = Shelby Tube 2.87" ID

U = Unconfined Compressive

UU = Unconsolidated Undrained

CD = Consolidated Drained

CU = Consolidated Undrained

DS = Direct Shear

φ = Friction

C = Cohesion

N = No. of blows per ft., sampler

N = Field SPT $N = (N_{css})(0.62)$ H = Hydrometer

S = Sieve

G = Specific Gravity

PI = Plasticity Index

LL = Liquid Limit

PL = Plastic Limit NP = Non-Plastic

OC = Consolidation

Ch = Chemical

RV = R - Value MD = Moisture Density CM = Compaction

E = Swell/Pressure on Expansive Soils

SL = Shrinkage Limit

UW= Unit Weight

W = Moisture Content K = Permeability

O = Organic Content

D = Dispersive

RQD = Rock Quality Designation

X = X-Ray Defraction

^{* =} Average of subsamples

EA/Cont #

Job Description I - 515 Soundwalls, Nellis to Mtn. Vista

Boring No. **SWA - 8** Elevation (ft) 03/11/2008 Station Date

	SAMPLE	SAMP-	N			DRY	%					STR	ENGTH T	EST		
SAMPLE NO.	DEPTH (ft)	LER TYPE	BLOWS per ft.	SOIL GROUP	W%	UW pcf	PASS #200	LL %	PL %	PI %	TEST TYPE	φ deg.	C psi	φ deg.	C psi	COMMENTS
												Pe	eak	Res	idual	
А	2.5 - 4.0	SPT	35	SM	10.1		26.2	37	27	10						
В	5.0 - 6.5	SPT	43	SM	8.6		21.6	35	27	8						
С	7.5 - 9.0	SPT	43	SM	9.3		22.9	38	28	10						
D	10.0 - 11.5	SPT	32	SC	10.8		24.3	39	24	15						
Е	12.5 - 14.0	SPT	48		8.2		19.8									
F	15.0 - 16.5	SPT	31	SM	10.8		28.6	40	31	9						
G	20.0 - 21.5	SPT	61	SM	8.4		15.3	29	23	6						
BULK 1	2.0 - 5.0			SC			24.8	40	24	16						RV = 26

CMS = California Modified Sampler 2.40" ID

SPT = Standard Penetration 1.38" ID

CS = Continuous Sample 3.23" ID

RC = Rock Core

PB = Pitcher Barrel

CSS = Calif. Split Spoon 2.42" ID

CPT = Cone Penetration Test

TP = Test Pit

P = Pushed, not driven

R = Refusal

Sh = Shelby Tube 2.87" ID

U = Unconfined Compressive

UU = Unconsolidated Undrained

CD = Consolidated Drained

CU = Consolidated Undrained

DS = Direct Shear

φ = Friction

C = Cohesion

N = No. of blows per ft., sampler

N = Field SPT $N = (N_{css})(0.62)$ H = Hydrometer

S = Sieve

G = Specific Gravity

PI = Plasticity Index

LL = Liquid Limit

PL = Plastic Limit NP = Non-Plastic

OC = Consolidation

Ch = Chemical

RV = R - Value MD = Moisture Density CM = Compaction

E = Swell/Pressure on Expansive Soils

SL = Shrinkage Limit

UW= Unit Weight

W = Moisture Content

K = Permeability

O = Organic Content

D = Dispersive

RQD = Rock Quality Designation

X = X-Ray Defraction

^{* =} Average of subsamples

EA/Cont #

Job Description I - 515 Soundwalls, Nellis to Mtn. Vista

Boring No. SWA - 9 Elevation (ft) 03/11/2008 Station Date

	SAMPLE	SAMP-	N			DRY	%					STR	ENGTH T	EST		
SAMPLE NO.	DEPTH (ft)	LER TYPE	BLOWS per ft.	SOIL GROUP	W%	UW pcf	PASS #200	LL %	PL %	PI %	TEST TYPE	φ deg.	C psi	φ deg.	C psi	COMMENTS
												Pe	eak	Res	dual	
Α	2.5 - 4.0	SPT	41	SM	8.6		23.2	39	29	10						
В	5.0 - 6.5	SPT	40	SM	12.9		30.0	47	32	15						
С	10.0 - 11.5	SPT	42	SC	11.4		29.0	30	22	8						
D	15.0 - 16.5	SPT	26	SM	8.4		16.9	37	29	8						
E	20.0 - 21.5	SPT	18	SC	13.8		44.9	32	15	17						
BULK 1	2.5 - 5.0			SC			27.7	40	23	17						RV = 56

CMS = California Modified Sampler 2.40" ID

SPT = Standard Penetration 1.38" ID

CS = Continuous Sample 3.23" ID

RC = Rock Core

PB = Pitcher Barrel

CSS = Calif. Split Spoon 2.42" ID

CPT = Cone Penetration Test

TP = Test Pit

P = Pushed, not driven

R = Refusal

Sh = Shelby Tube 2.87" ID

U = Unconfined Compressive

UU = Unconsolidated Undrained

CD = Consolidated Drained

CU = Consolidated Undrained

DS = Direct Shear

φ = Friction

C = Cohesion

N = No. of blows per ft., sampler

N = Field SPT $N = (N_{css})(0.62)$ H = Hydrometer

S = Sieve

G = Specific Gravity

PI = Plasticity Index

LL = Liquid Limit

PL = Plastic Limit NP = Non-Plastic

OC = Consolidation

Ch = Chemical

RV = R - Value MD = Moisture Density CM = Compaction

E = Swell/Pressure on Expansive Soils

SL = Shrinkage Limit

UW= Unit Weight

W = Moisture Content

K = Permeability O = Organic Content

D = Dispersive

RQD = Rock Quality Designation

X = X-Ray Defraction

^{* =} Average of subsamples

EA/Cont #

Job Description I - 515 Soundwalls, Nellis to Mtn. Vista

Boring No. SWA - 10 Elevation (ft) 03/11/2008 Station Date

	SAMPLE	SAMP-	N			DRY	%					STR	ENGTH T	EST		
SAMPLE NO.	DEPTH (ft)	LER TYPE	BLOWS per ft.	SOIL GROUP	W%	UW pcf	PASS #200	LL %	PL %	PI %	TEST TYPE	φ deg.	C psi	φ deg.	C psi	COMMENTS
												Pe	eak	Res	idual	
В	5.0 - 6.5	SPT	38	SM	9.8		23.2	35	28	7						
С	7.5 - 9.0	SPT	7	CL	18.3		55.1	40	15	25						
D1	10.2 - 10.7	CMS	23	SC	15.5		45.1	36	14	22						
D2	10.7 - 11.2	CMS			18.5	107.0	61.6									
Е	11.5 - 13.0	SPT	48	SC	10.6		40.5	31	16	15						
F	15.0 - 16.5	SPT	18	SC	14.3		45.7	38	19	19						
G	20.0 - 21.5	SPT	19	CL	24.8		59.0	30	18	12						
Н	25.0 - 26.5	SPT	21	CL	23.4		53.6	28	17	11						
BULK 1	2.0 - 5.0			SC			26.2	26	18	8						RV = 72

CMS = California Modified Sampler 2.40" ID

SPT = Standard Penetration 1.38" ID

CS = Continuous Sample 3.23" ID

RC = Rock Core

PB = Pitcher Barrel

CSS = Calif. Split Spoon 2.42" ID

CPT = Cone Penetration Test

TP = Test Pit

P = Pushed, not driven

R = Refusal

Sh = Shelby Tube 2.87" ID

U = Unconfined Compressive

UU = Unconsolidated Undrained

CD = Consolidated Drained

CU = Consolidated Undrained

DS = Direct Shear

φ = Friction

C = Cohesion

N = No. of blows per ft., sampler

N = Field SPT $N = (N_{css})(0.62)$ H = Hydrometer

S = Sieve

G = Specific Gravity

PI = Plasticity Index

LL = Liquid Limit

PL = Plastic Limit NP = Non-Plastic

OC = Consolidation

Ch = Chemical

RV = R - Value MD = Moisture Density CM = Compaction

E = Swell/Pressure on Expansive Soils

SL = Shrinkage Limit

UW= Unit Weight

W = Moisture Content

K = Permeability O = Organic Content

D = Dispersive

RQD = Rock Quality Designation

X = X-Ray Defraction

^{* =} Average of subsamples

EA/Cont #

Job Description I - 515 Soundwalls, Nellis to Mtn. Vista

Boring No. **SWB - 1** 03/11/2008 Station Date Elevation (ft)

	SAMPLE	SAMP-	N			DRY	%				STRENGTH TEST					
SAMPLE	DEPTH	LER	BLOWS	SOIL	W%	UW	PASS	LL	PL	PI	TEST	φ	С	φ	С	COMMENTS
NO.	(ft)	TYPE	per ft.	GROUP		pcf	#200	%	%	%	TYPE	deg.	psi	deg.	psi	
												Pe	ak	Res	idual	
Α	2.5 - 4.0	SPT		SM	8.9		23.2	31	27	4						
C1	7.5 - 8.5	SPT		SM	10.1		21.6	36	29	7						
C2	8.5 - 9.0	SPT		CL	21.7		64.0	40	20	20						
D1	10.2 - 10.7	CMS		SM	9.6	101.6	38.4	33	25	8						
D2	10.7 - 11.2	CMS		СН	19.7	86.3	65.9	58	26	32	DS	32	1.39	32	1.39	
Е	11.5 - 13.0	SPT		SC-SM	8.3		36.6	29	22	7						
F1	15.2 - 15.7	CMS		sc	9.3	95.8	20.5	37	22	15						
F2	15.7 - 16.2	CMS		SC	24.6	84.6	48.5	51	24	27						
G	20.0 - 21.5	SPT		СН	28.7		86.3	52	20	32						
Н	25.0 - 26.5	SPT		CL	31.3		96.3	47	23	24						
BULK 1	2.0 - 5.0			sc			24.0	34	23	11						RV = 71

CMS = California Modified Sampler 2.40" ID

SPT = Standard Penetration 1.38" ID

CS = Continuous Sample 3.23" ID

RC = Rock Core

PB = Pitcher Barrel

CSS = Calif. Split Spoon 2.42" ID

CPT = Cone Penetration Test

TP = Test Pit

P = Pushed, not driven

R = Refusal

Sh = Shelby Tube 2.87" ID

U = Unconfined Compressive

UU = Unconsolidated Undrained

CD = Consolidated Drained

CU = Consolidated Undrained

DS = Direct Shear

φ = Friction

C = Cohesion

N = No. of blows per ft., sampler

N = Field SPT $N = (N_{css})(0.62)$ H = Hydrometer

S = Sieve

G = Specific Gravity

PI = Plasticity Index

LL = Liquid Limit

PL = Plastic Limit NP = Non-Plastic

OC = Consolidation

Ch = Chemical

RV = R - Value

MD = Moisture Density

CM = Compaction

E = Swell/Pressure on Expansive Soils

SL = Shrinkage Limit

UW= Unit Weight

W = Moisture Content K = Permeability

O = Organic Content

D = Dispersive

RQD = Rock Quality Designation

X = X-Ray Defraction

^{* =} Average of subsamples

EA/Cont #

Job Description I - 515 Soundwalls, Nellis to Mtn. Vista

Boring No. SWB - 2 Elevation (ft) 03/11/2008 Station Date

	SAMPLE	SAMP-	N			DRY	%					STR	ENGTH T	EST		
SAMPLE NO.	DEPTH (ft)	LER TYPE	BLOWS per ft.	SOIL GROUP	W%	UW pcf	PASS #200	LL %	PL %	PI %	TEST TYPE	φ deg.	C psi	φ deg.	C psi	COMMENTS
												Pe	eak	Res	idual	
А	2.5 - 4.0	SPT		SM	10.0		24.7	33	27	6						
B1	5.0 - 6.0	SPT		SC	16.1		44.4	41	19	22						
B2	6.0 - 6.5	SPT		SC	10.3		45.0	33	24	9						
C1	7.7 - 8.2	CMS		SC-SM	11.1	104.2	45.1	28	21	7						
C2	8.2 - 8.7	CMS			11.2	103.1	44.4									
D	9.0 - 10.5	SPT		SC-SM	9.1		41.6	27	21	6						
Е	12.5 - 14.0	SPT		SC	8.6		38.1	30	21	9						
F	17.5 - 19.0	SPT		SC	20.9		45.4	42	19	23						
G	25.0 - 26.5	SPT		СН	49.6		89.0	66	23	43						
BULK 1	2.0 - 5.0			SC			26.8	35	24	11						RV = 48

CMS = California Modified Sampler 2.40" ID

SPT = Standard Penetration 1.38" ID

CS = Continuous Sample 3.23" ID

RC = Rock Core

PB = Pitcher Barrel

CSS = Calif. Split Spoon 2.42" ID

CPT = Cone Penetration Test

TP = Test Pit

P = Pushed, not driven

R = Refusal

Sh = Shelby Tube 2.87" ID

U = Unconfined Compressive

UU = Unconsolidated Undrained

CD = Consolidated Drained

CU = Consolidated Undrained

DS = Direct Shear

φ = Friction

C = Cohesion

N = No. of blows per ft., sampler

N = Field SPT $N = (N_{css})(0.62)$ H = Hydrometer

S = Sieve

G = Specific Gravity

PI = Plasticity Index

LL = Liquid Limit

PL = Plastic Limit

NP = Non-Plastic

OC = Consolidation

Ch = Chemical

RV = R - Value MD = Moisture Density CM = Compaction

E = Swell/Pressure on Expansive Soils

SL = Shrinkage Limit

UW= Unit Weight

W = Moisture Content K = Permeability

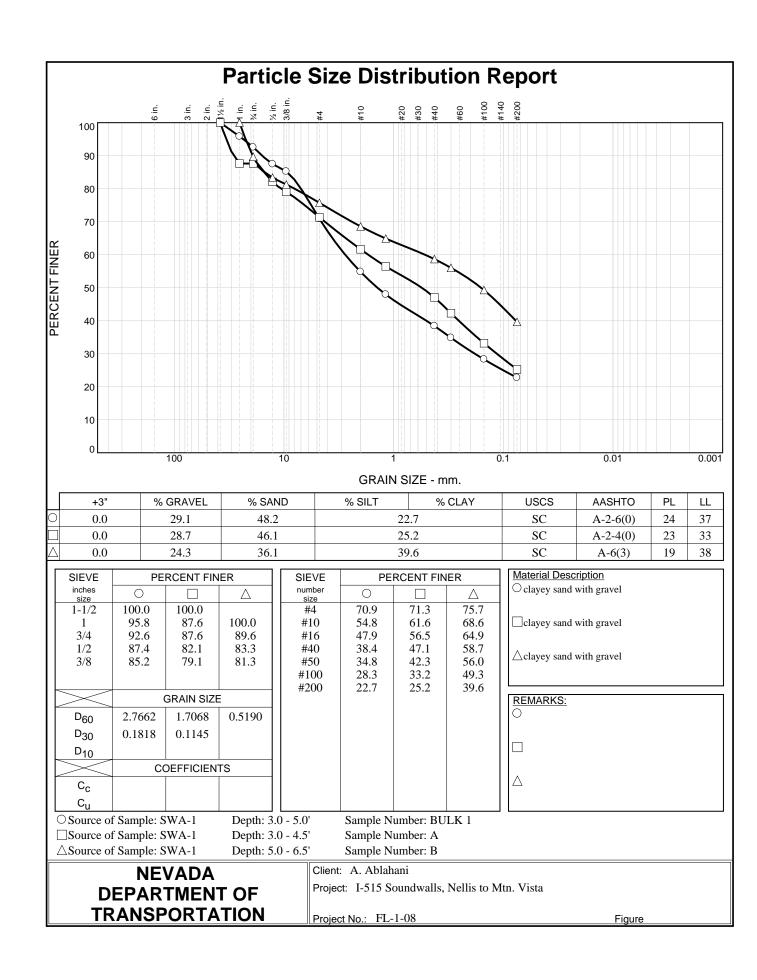
O = Organic Content

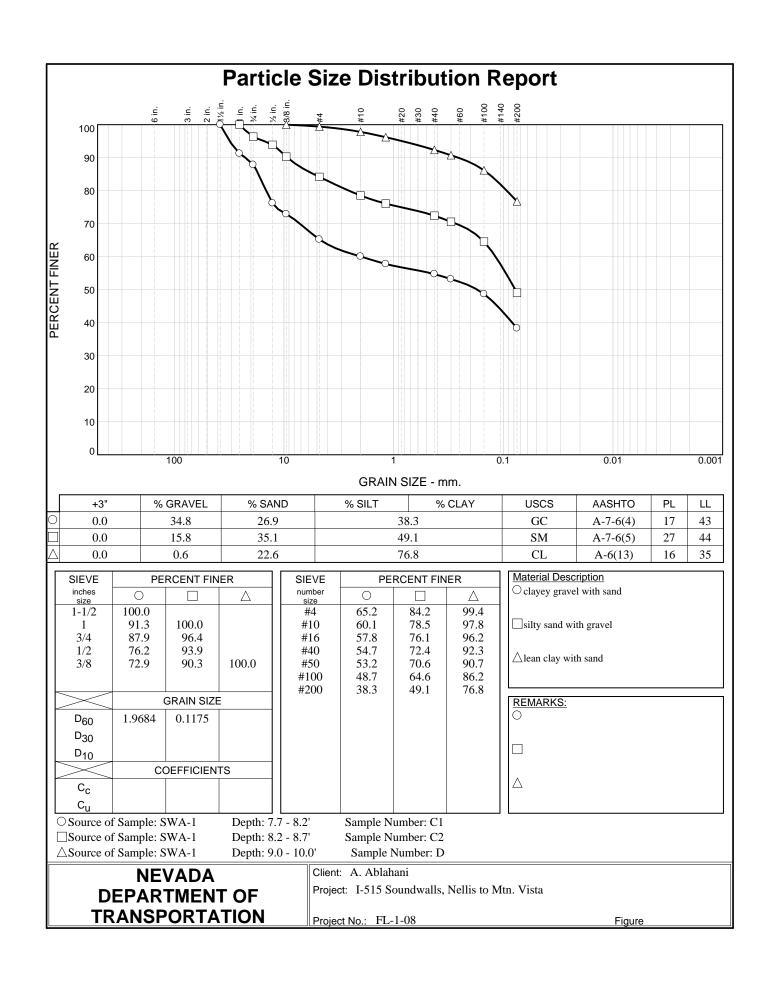
D = Dispersive

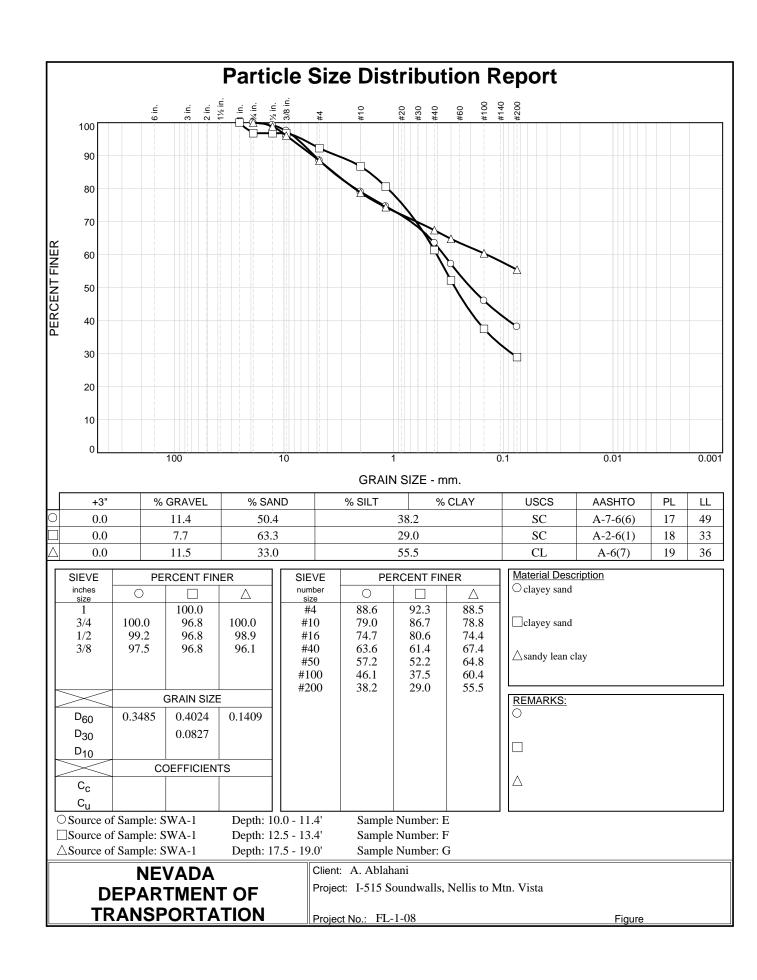
RQD = Rock Quality Designation

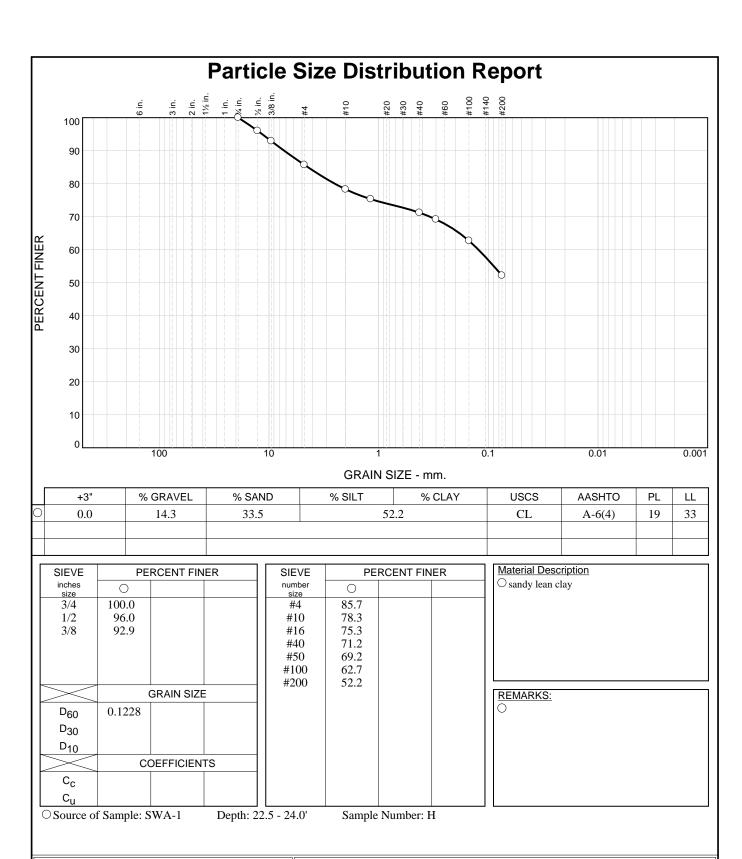
X = X-Ray Defraction

^{* =} Average of subsamples









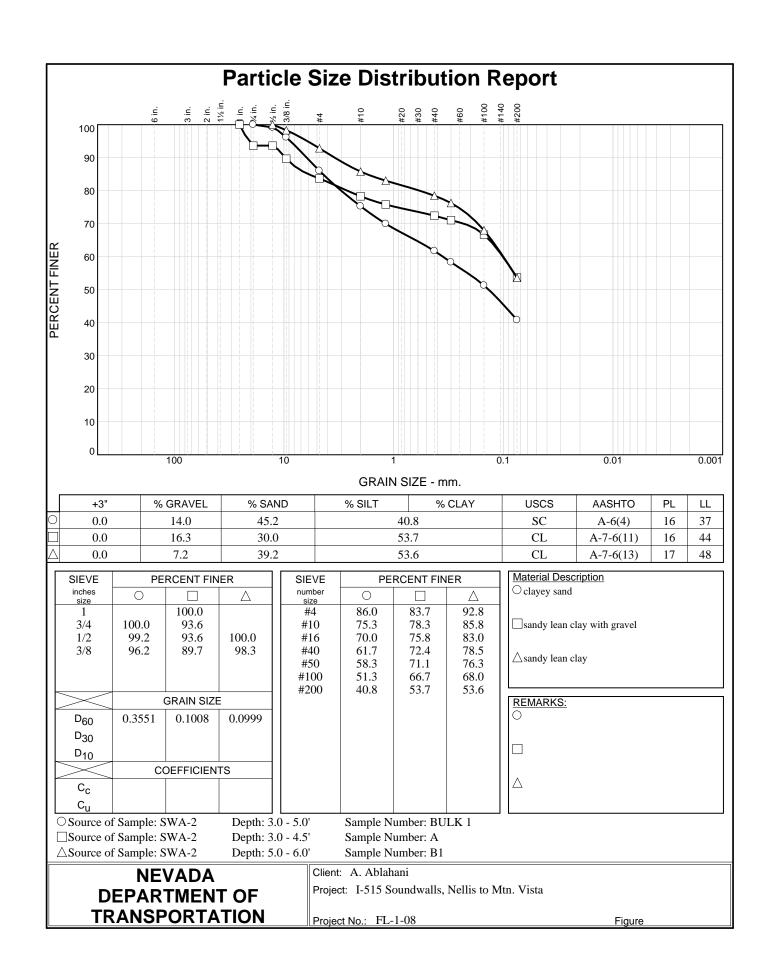
NEVADA
DEPARTMENT OF
TRANSPORTATION

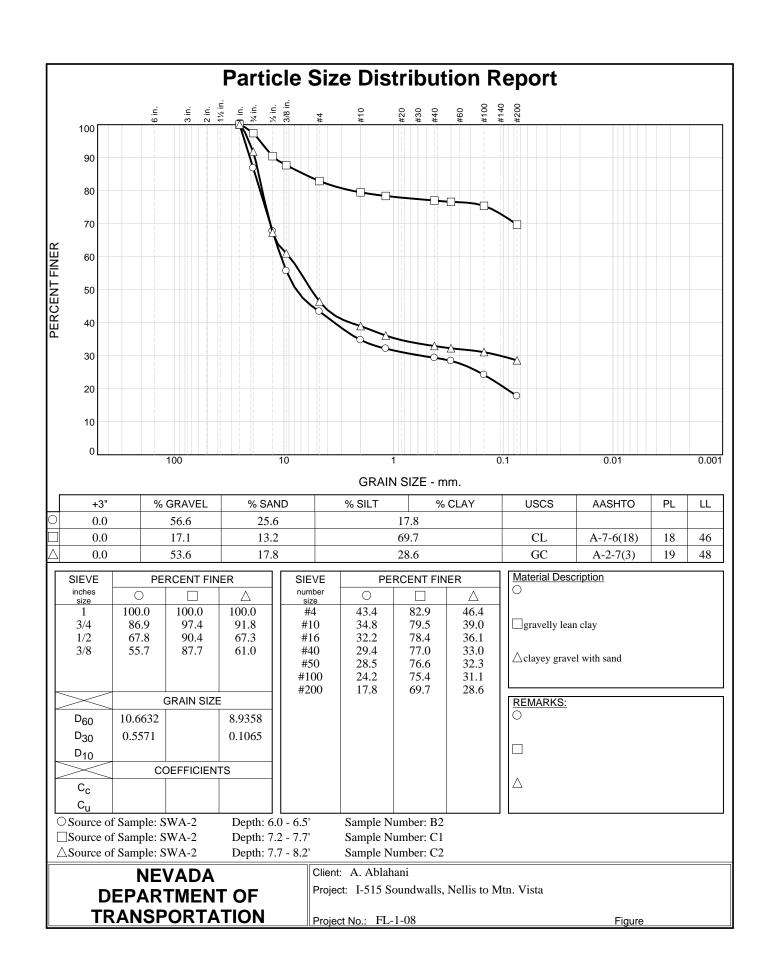
Client: A. Ablahani

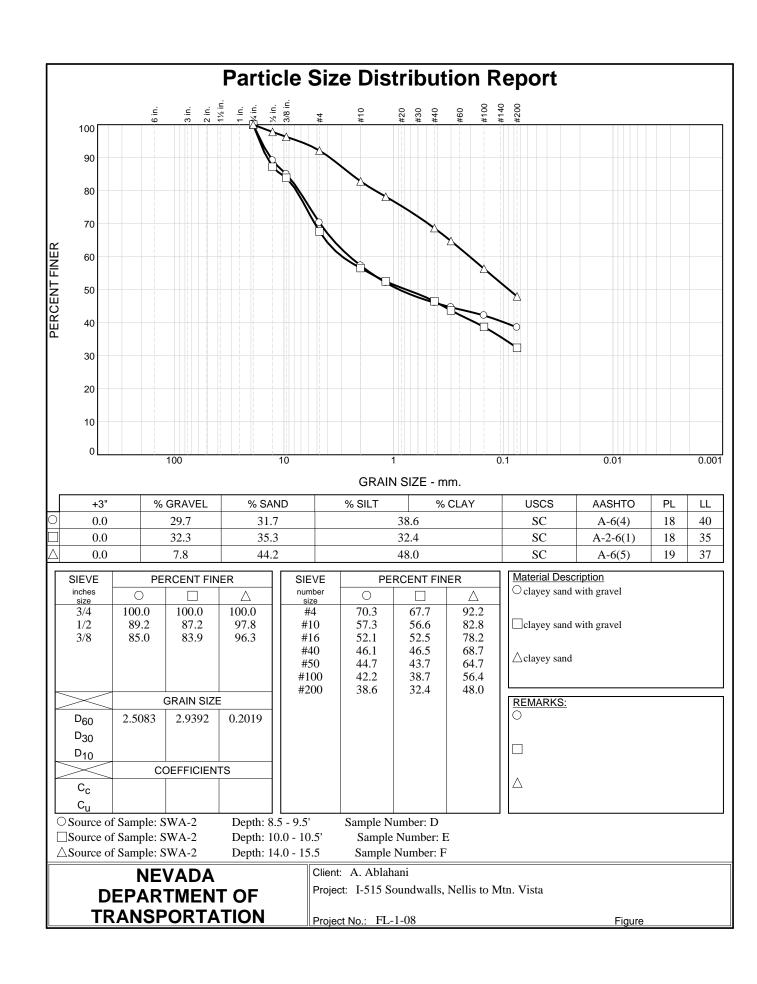
Project: I-515 Soundwalls, Nellis to Mtn. Vista

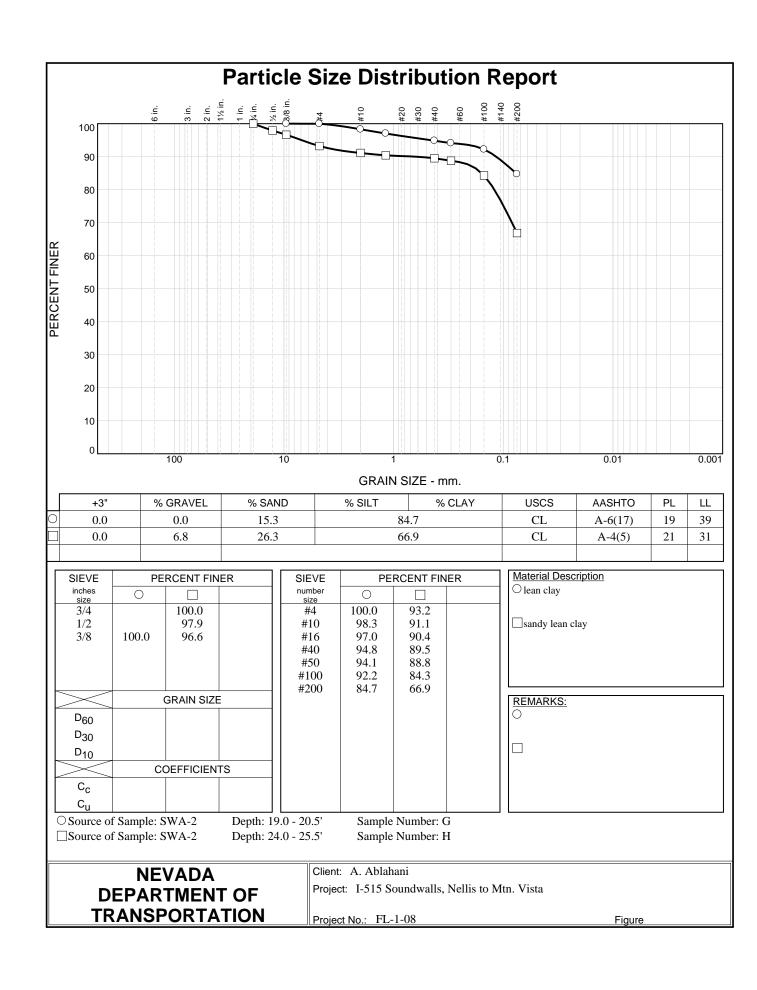
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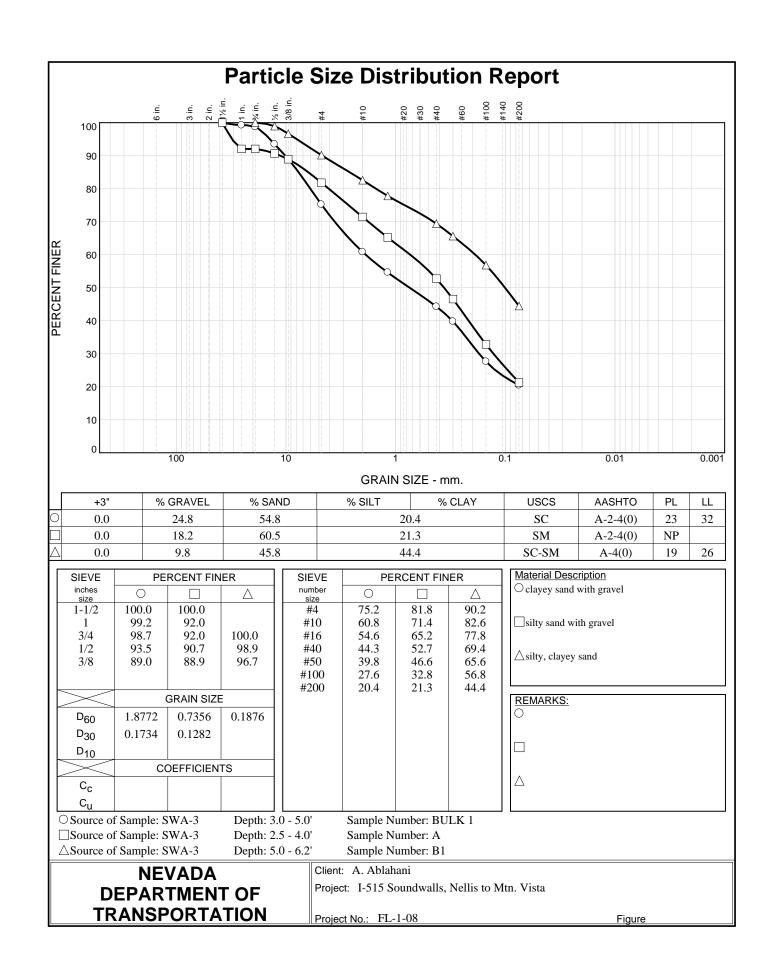
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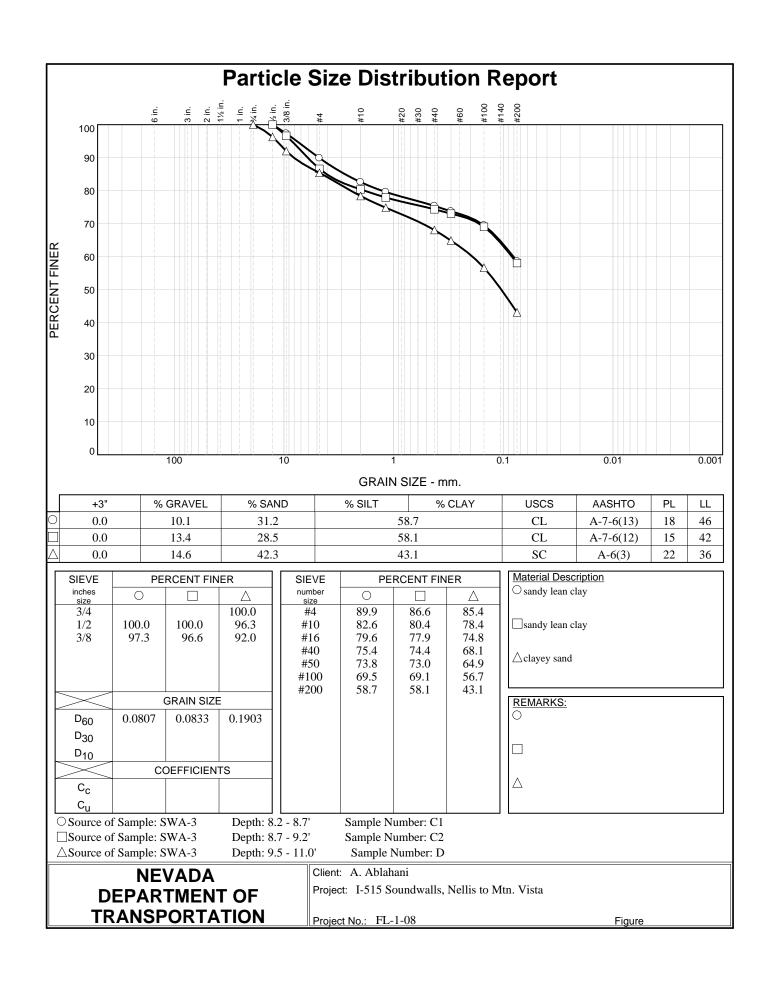


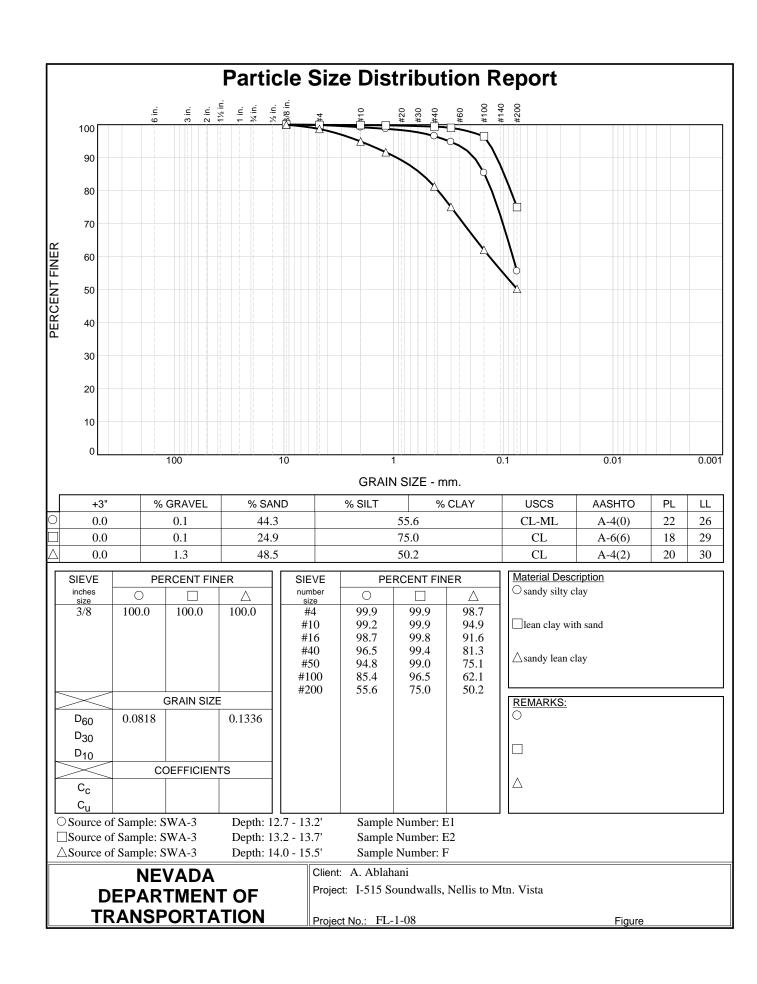


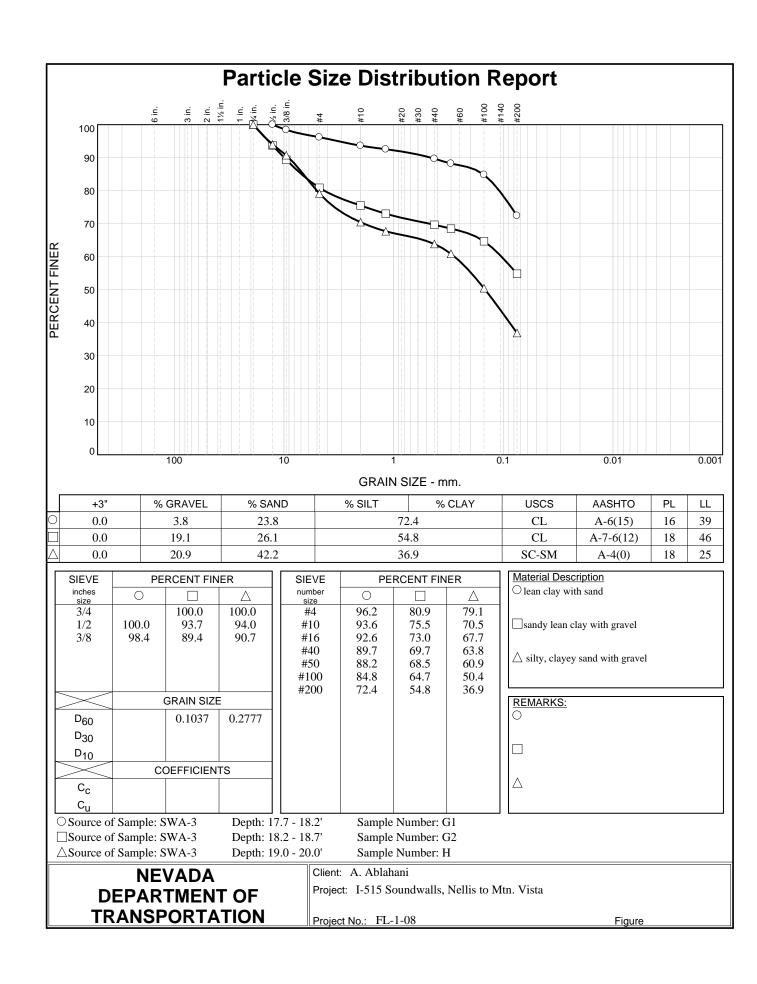


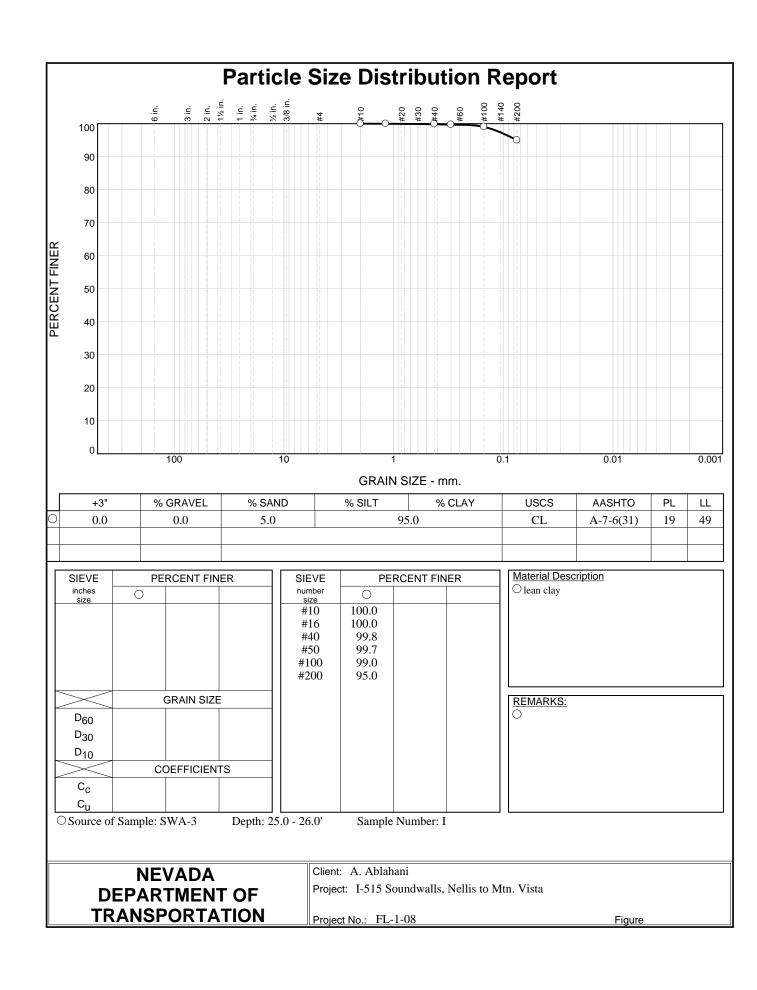


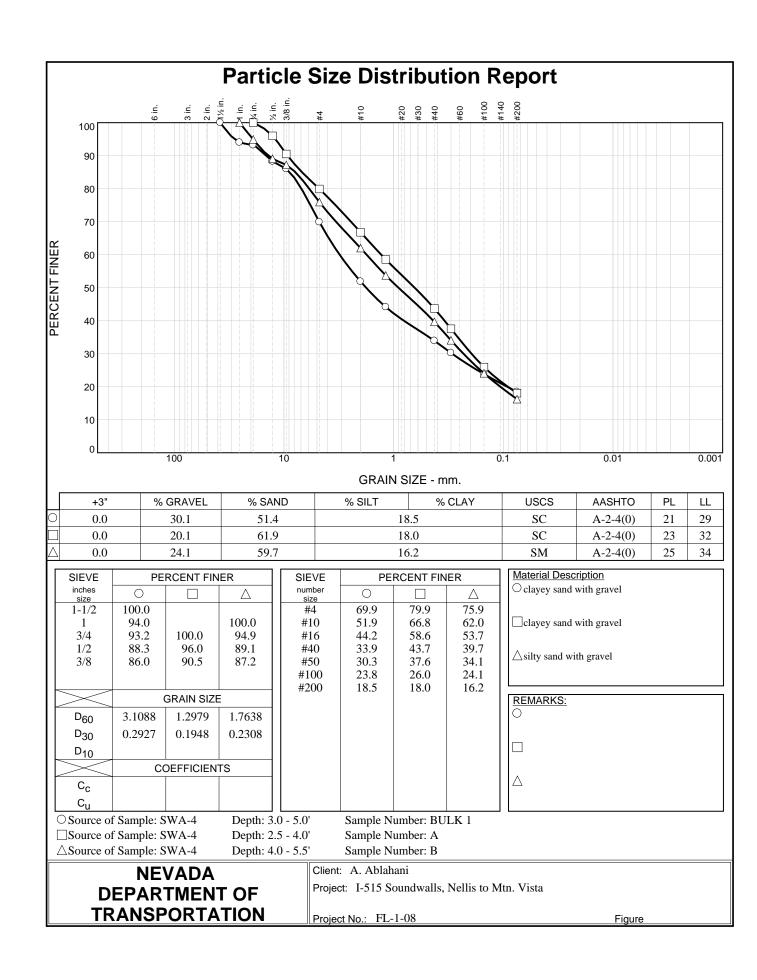


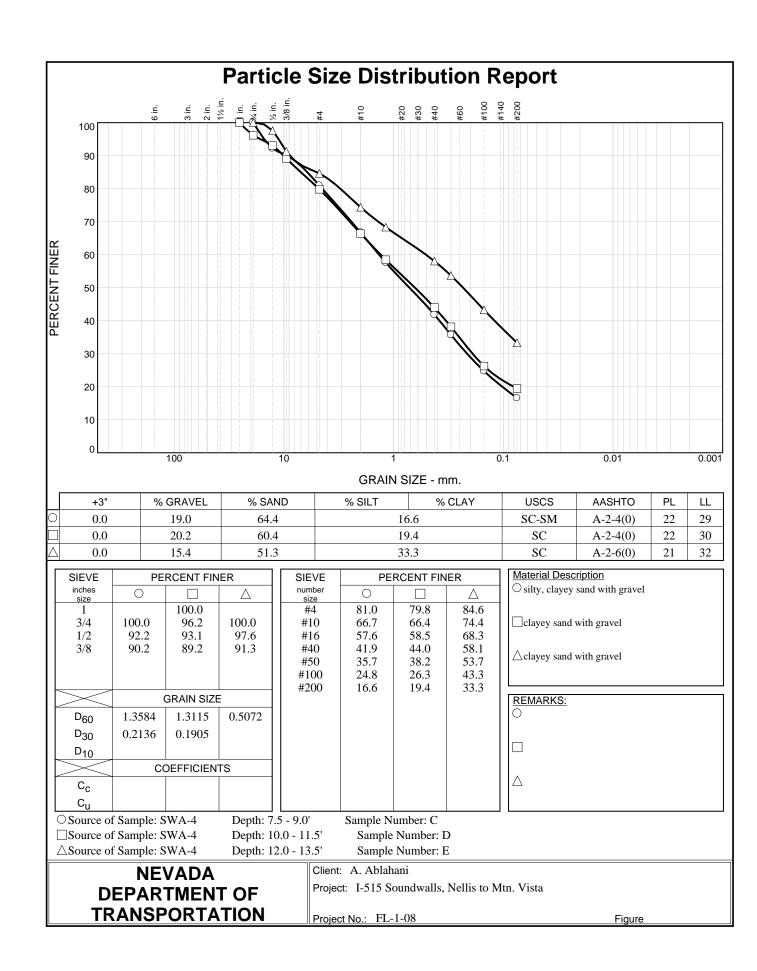


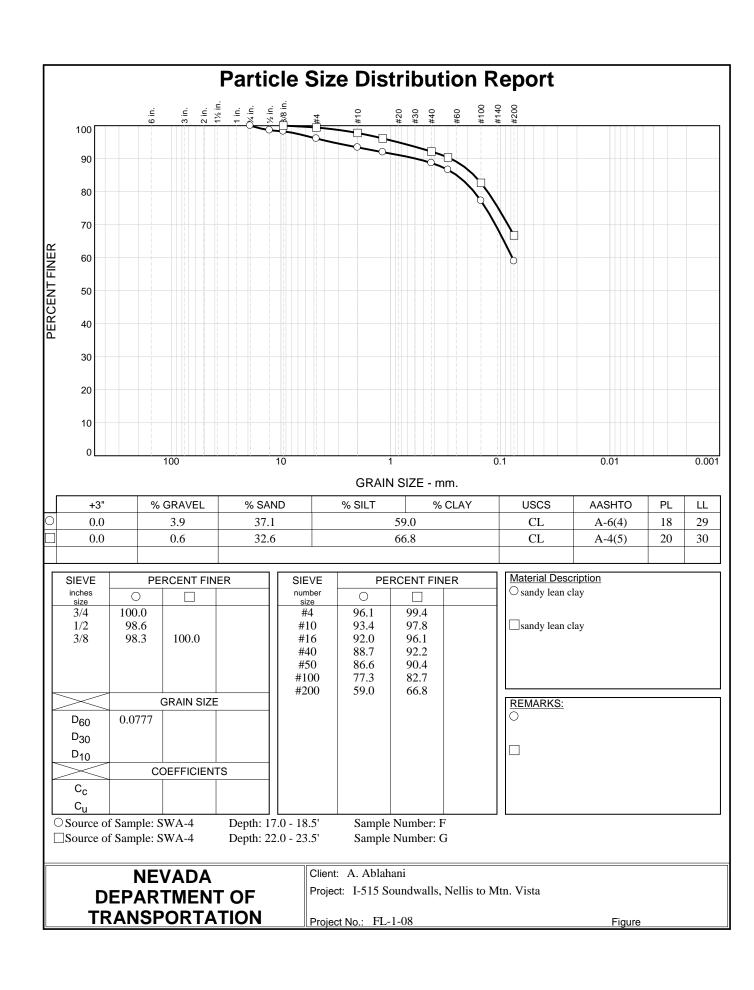


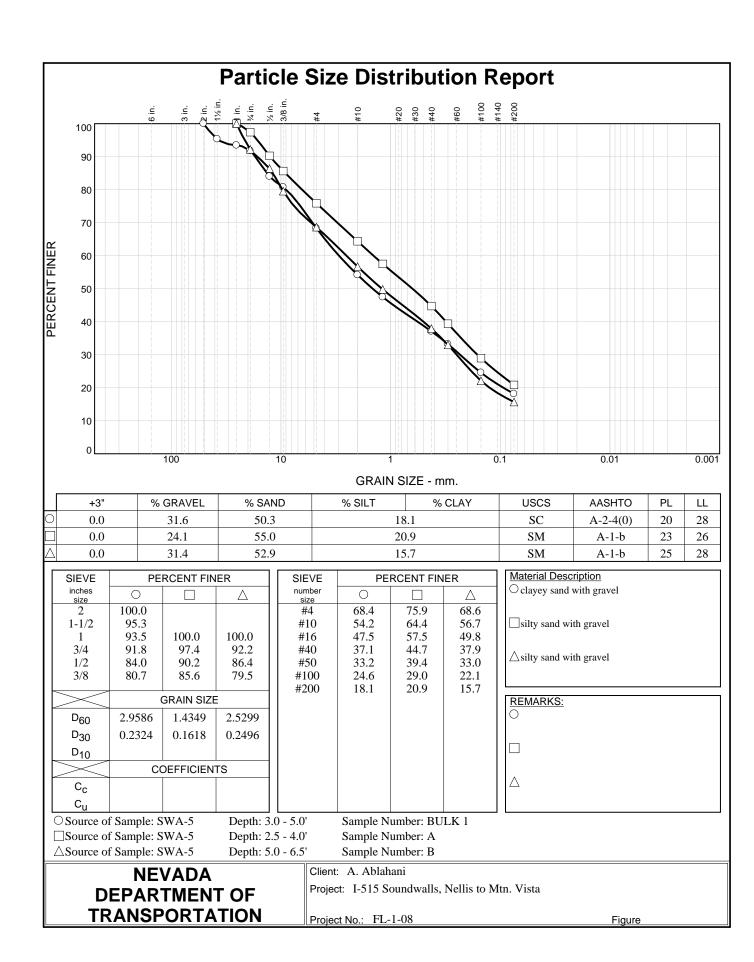


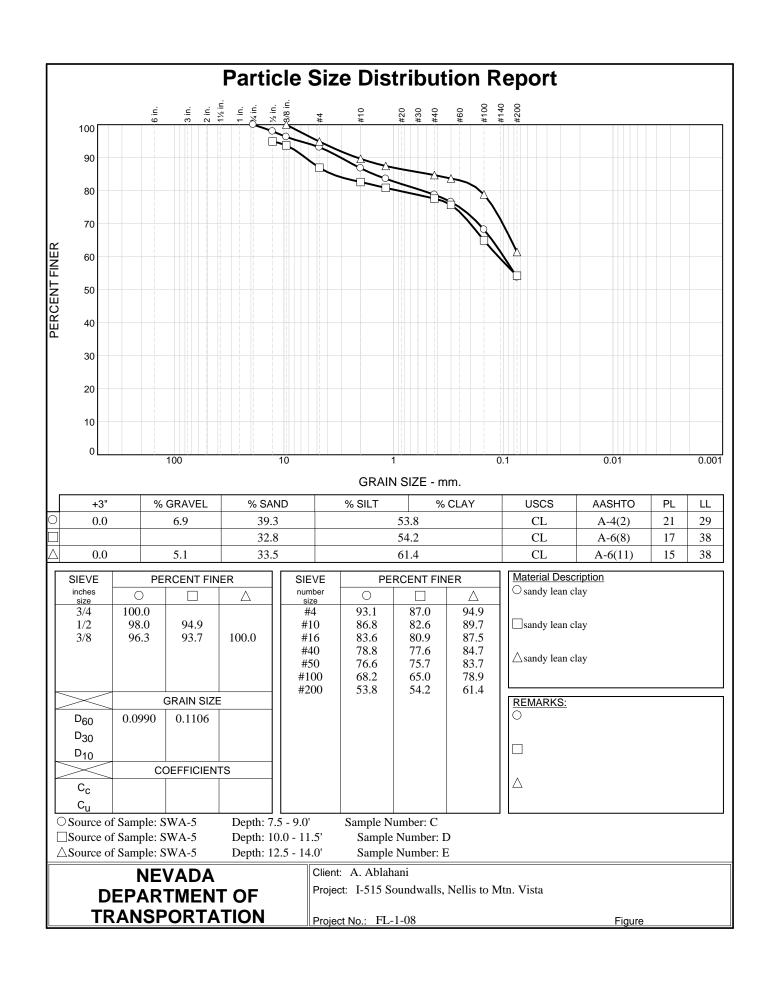


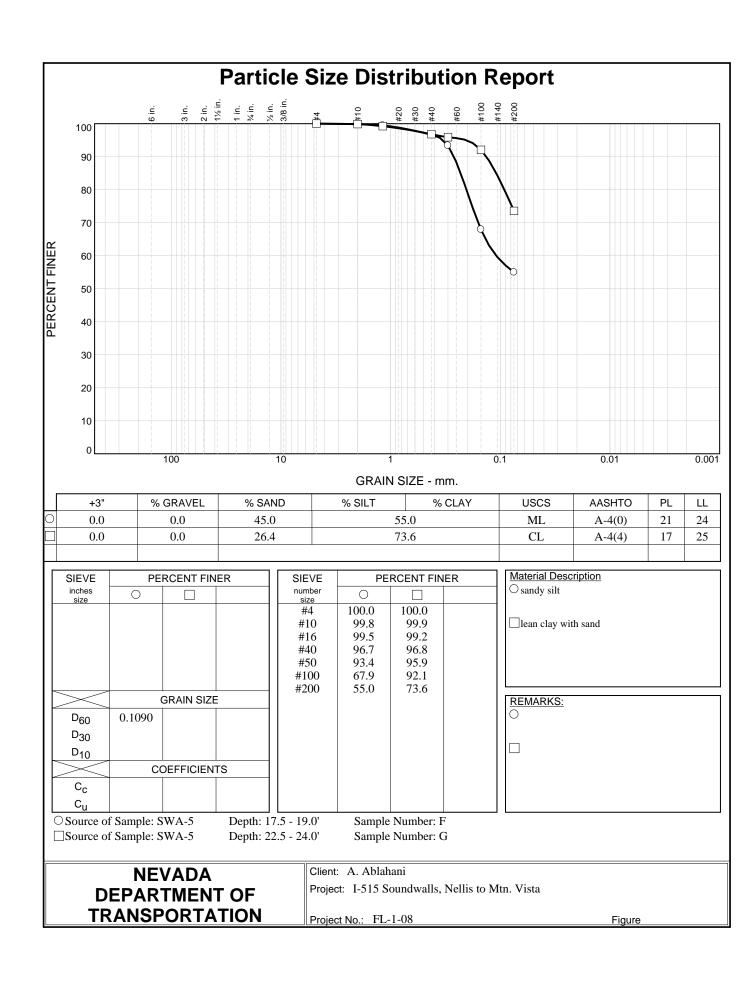


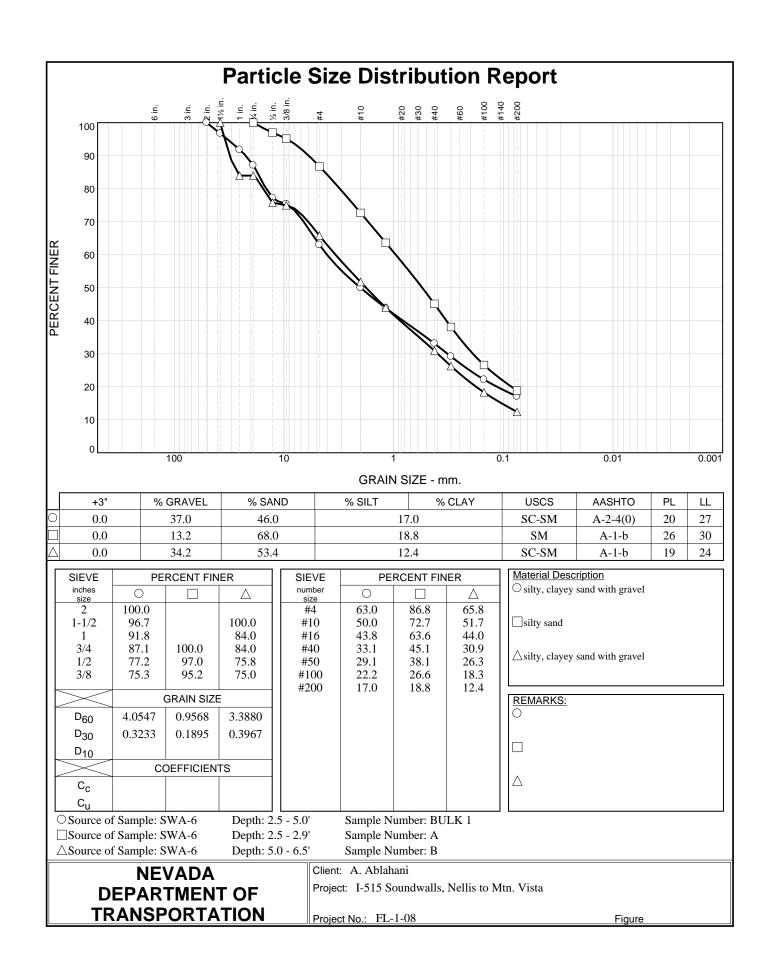


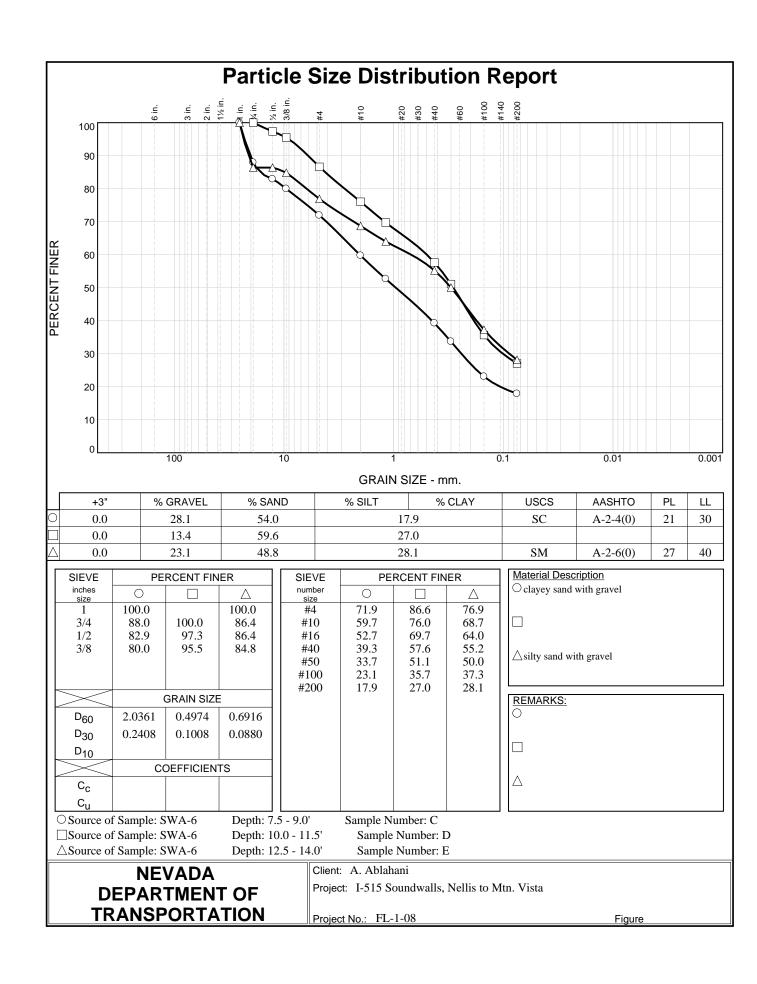


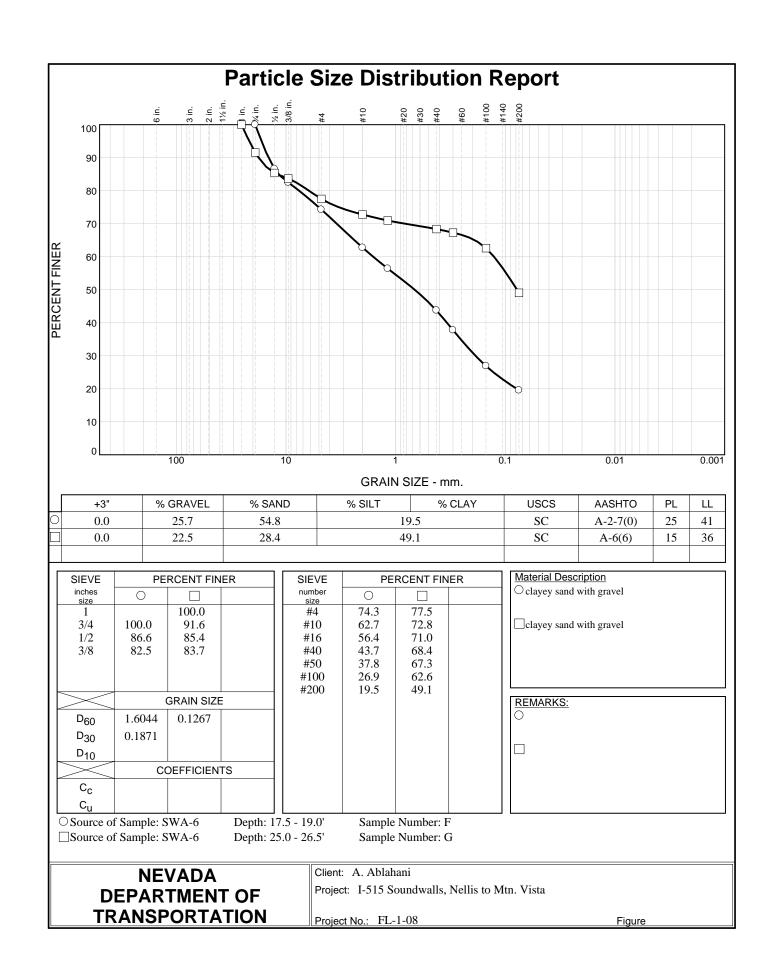


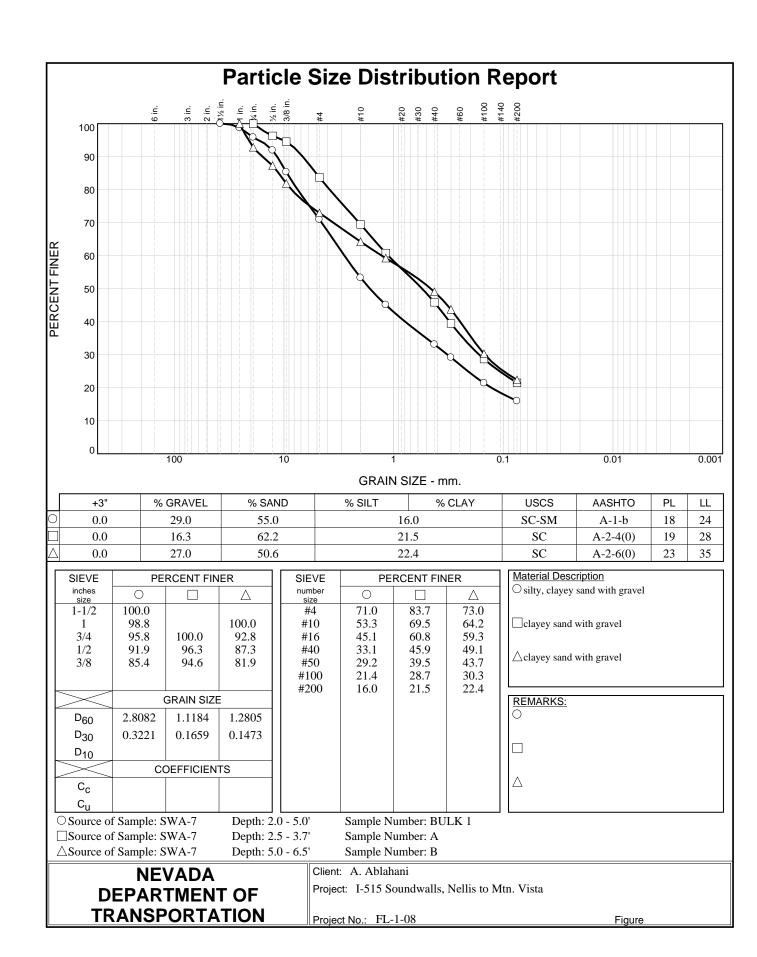


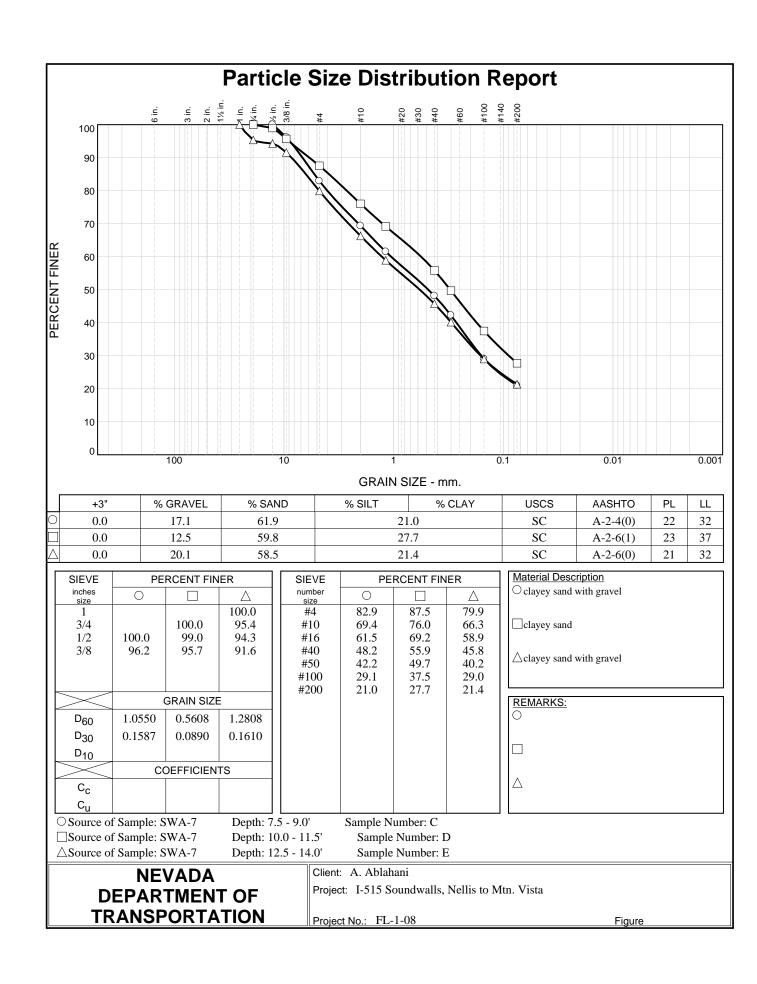


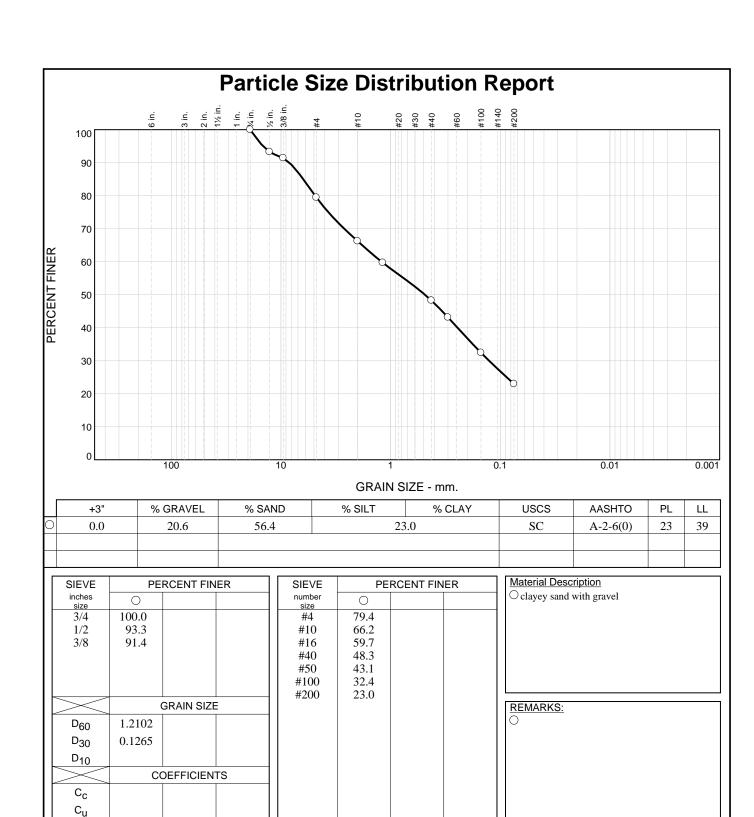












O Source of Sample: SWA-7

Depth: 15.0 - 16.5'

Sample Number: F

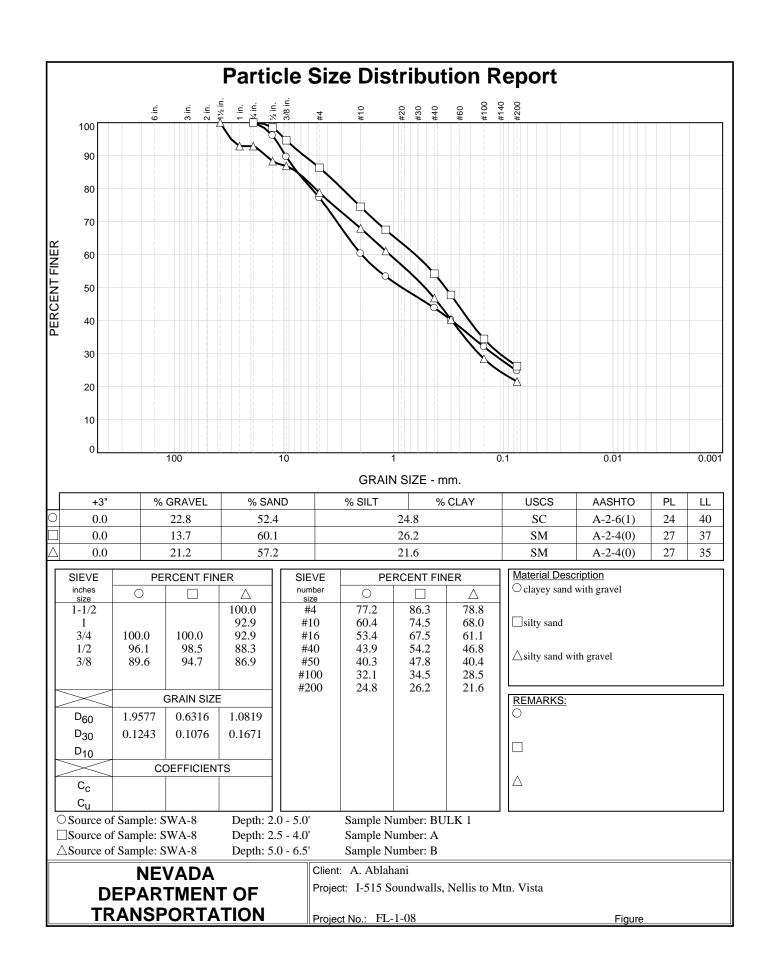
NEVADA DEPARTMENT OF TRANSPORTATION

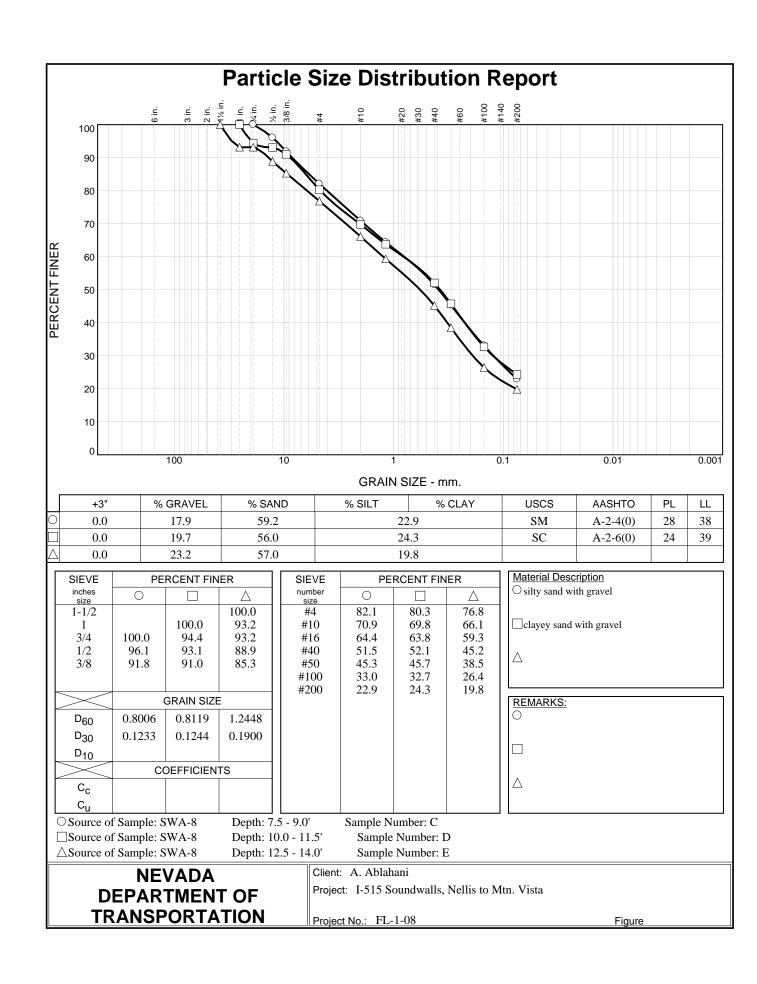
Client: A. Ablahani

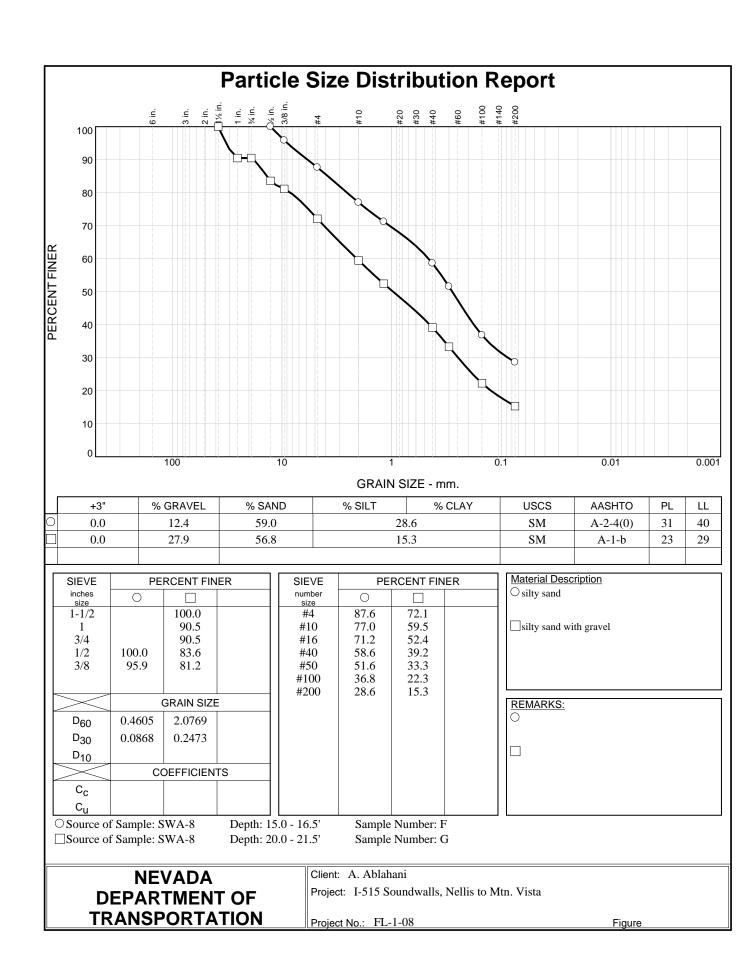
Project: I-515 Soundwalls, Nellis to Mtn. Vista

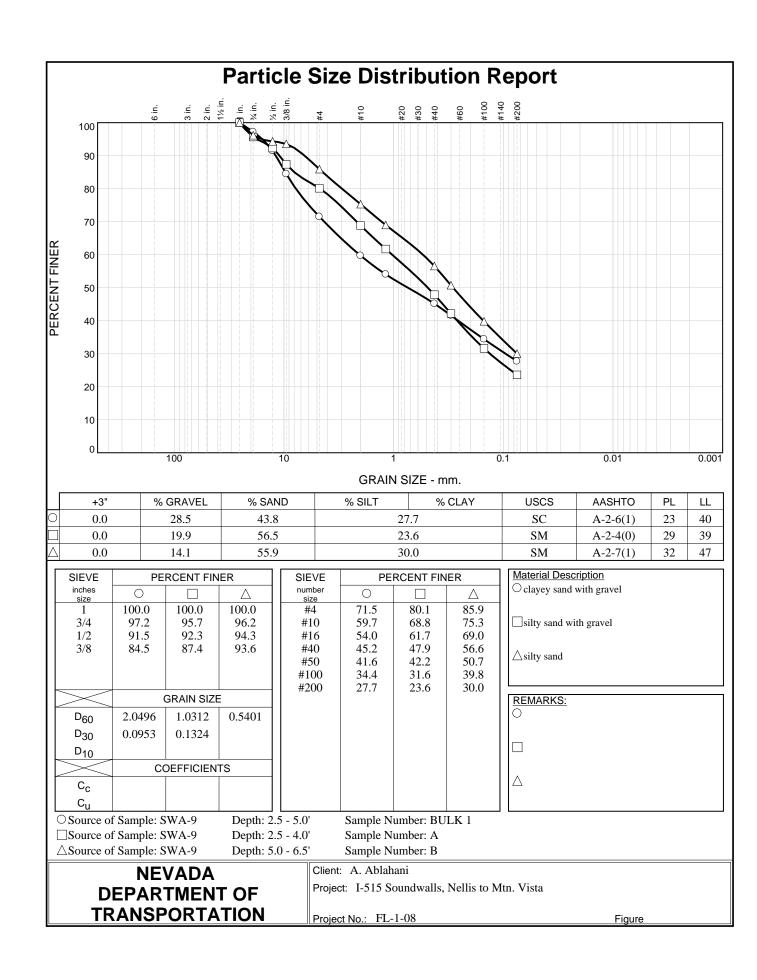
Project No.: FL-1-08

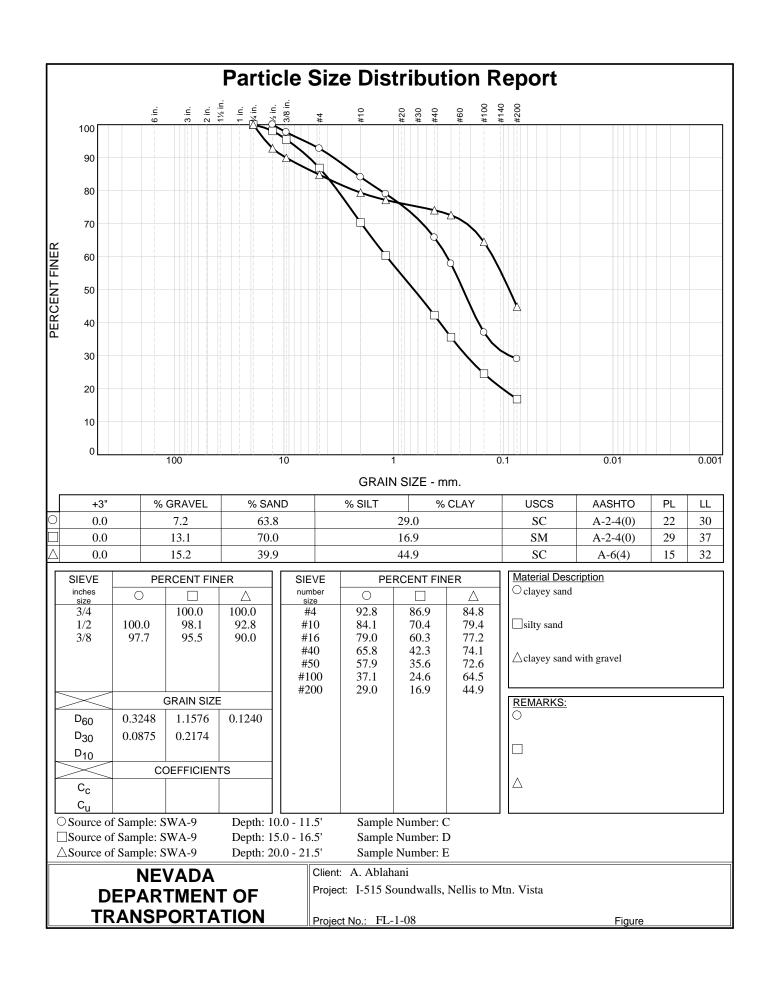
Figure

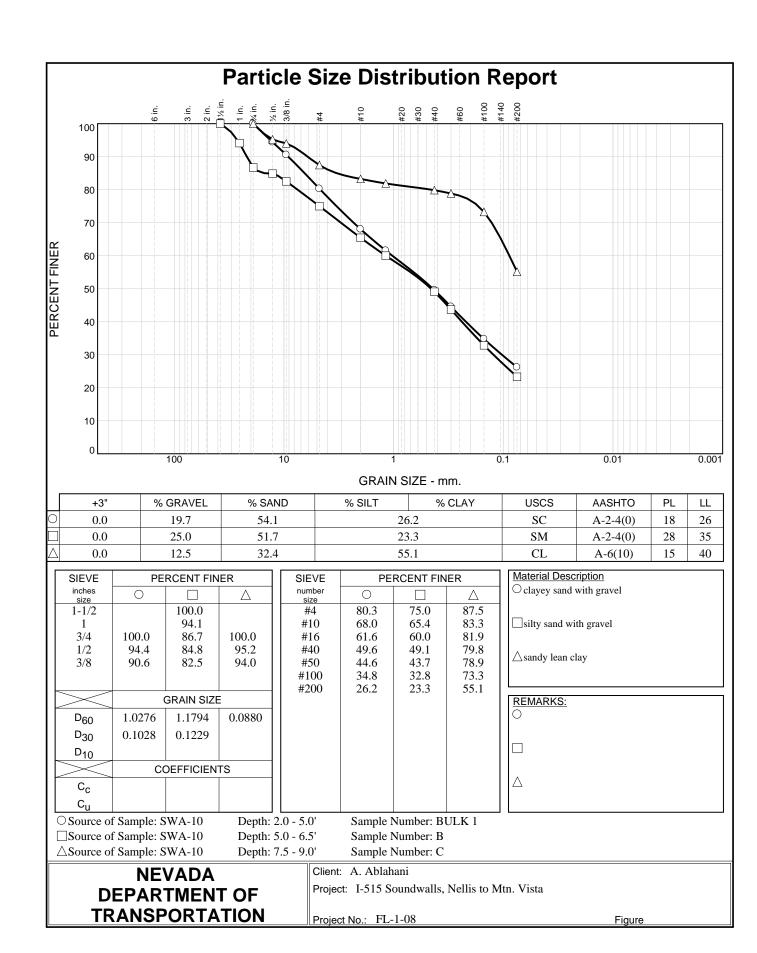


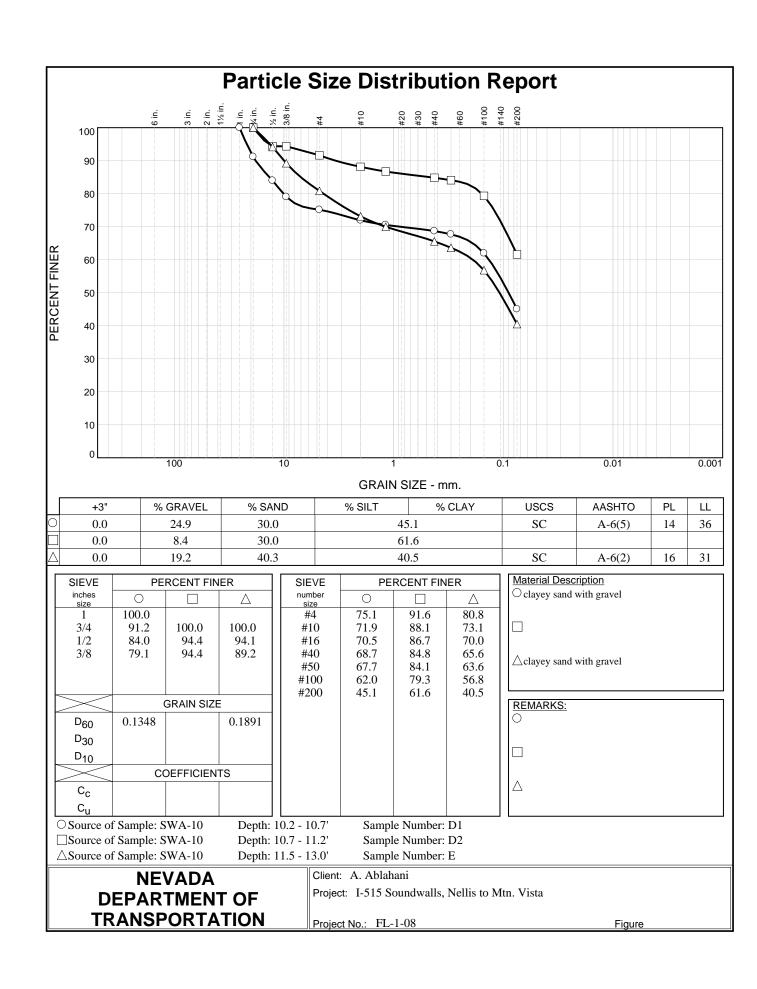


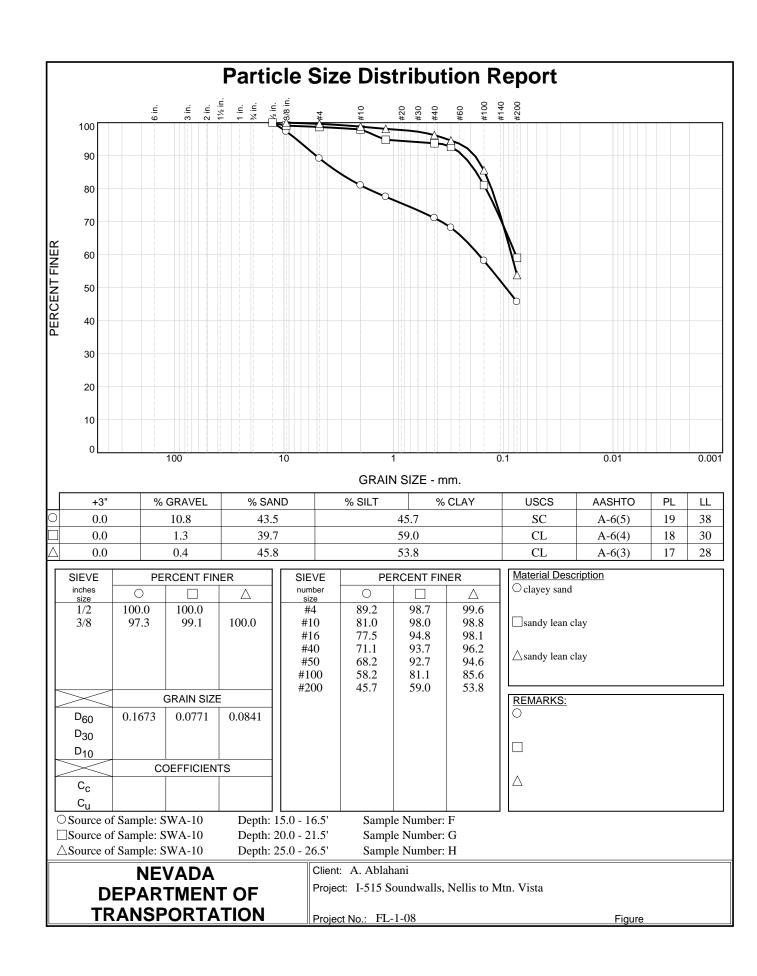


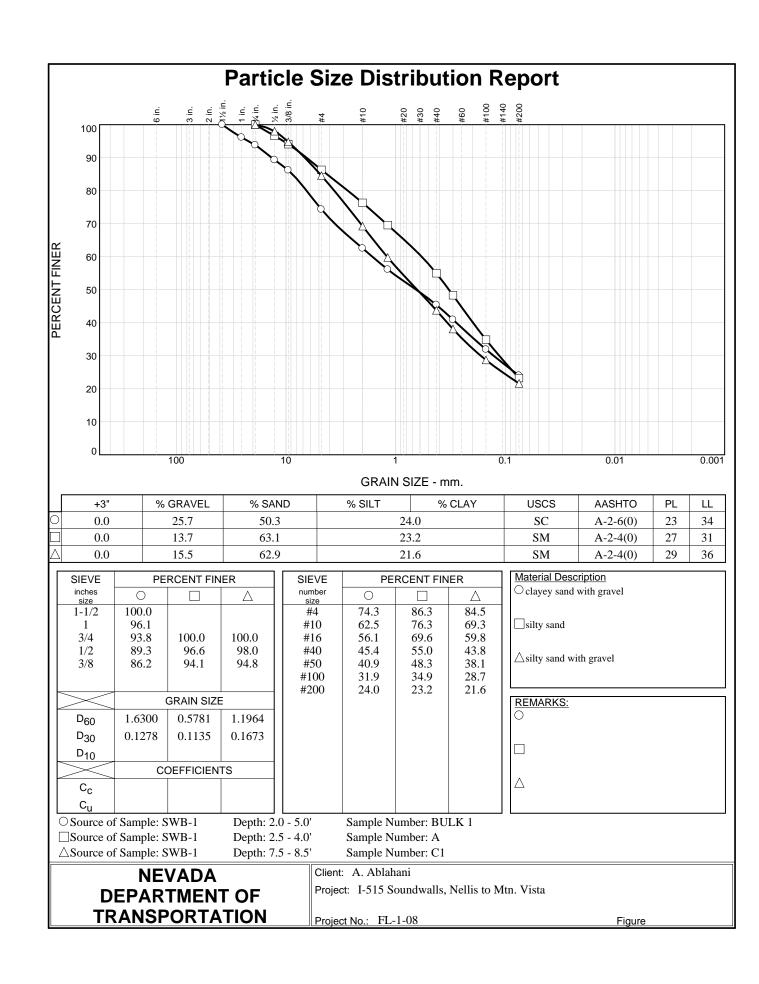


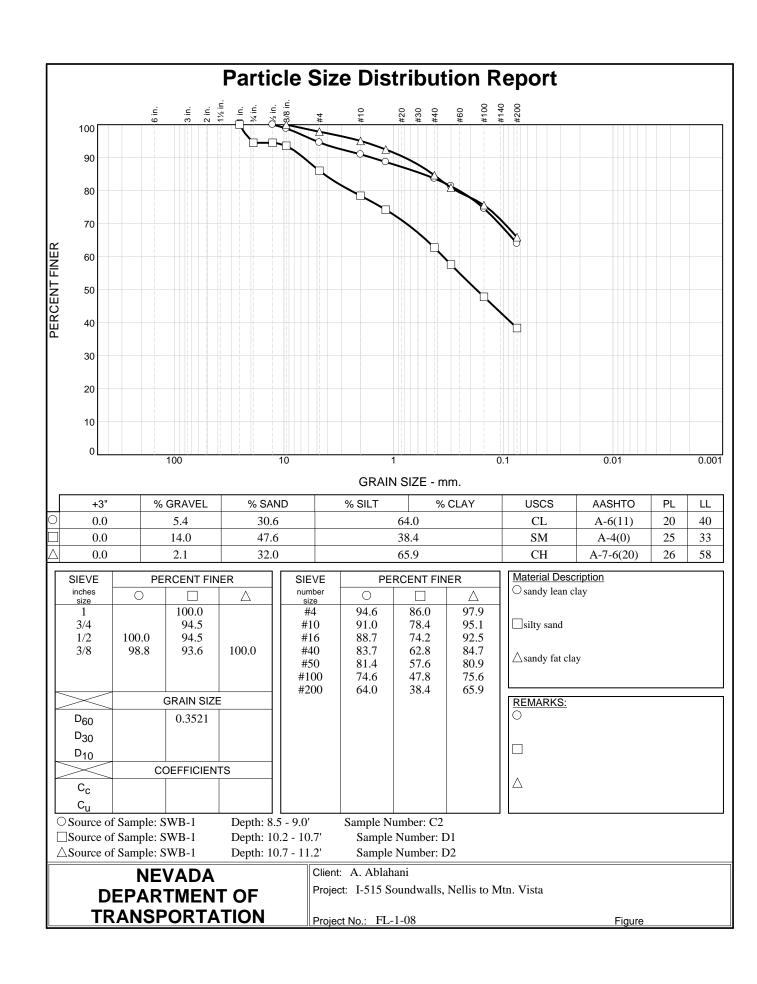


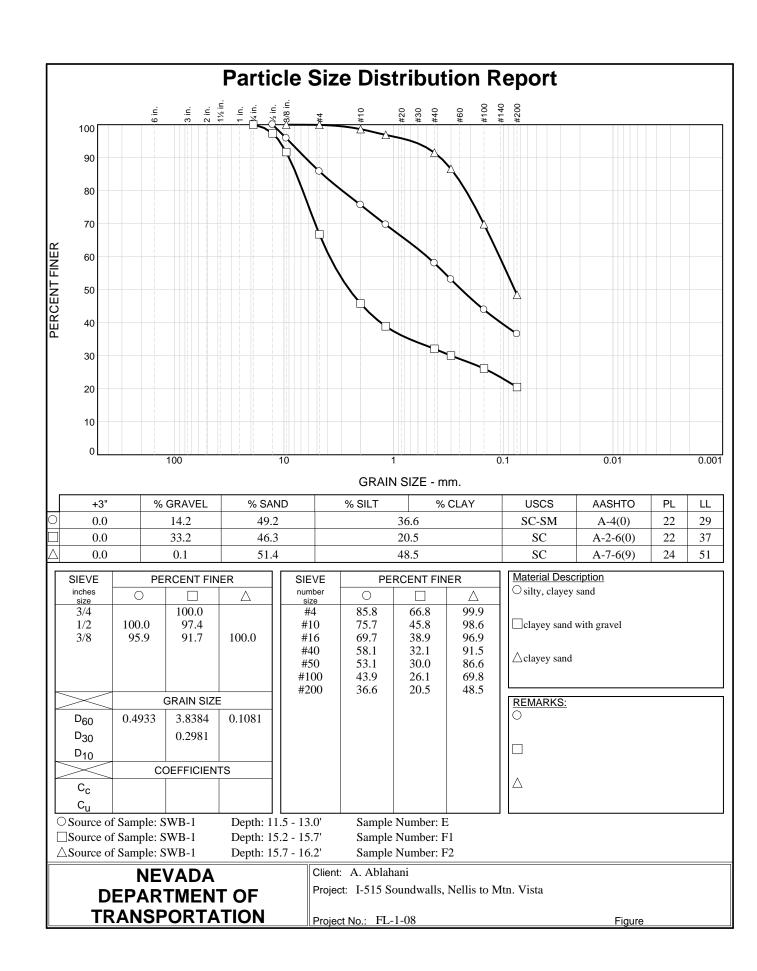


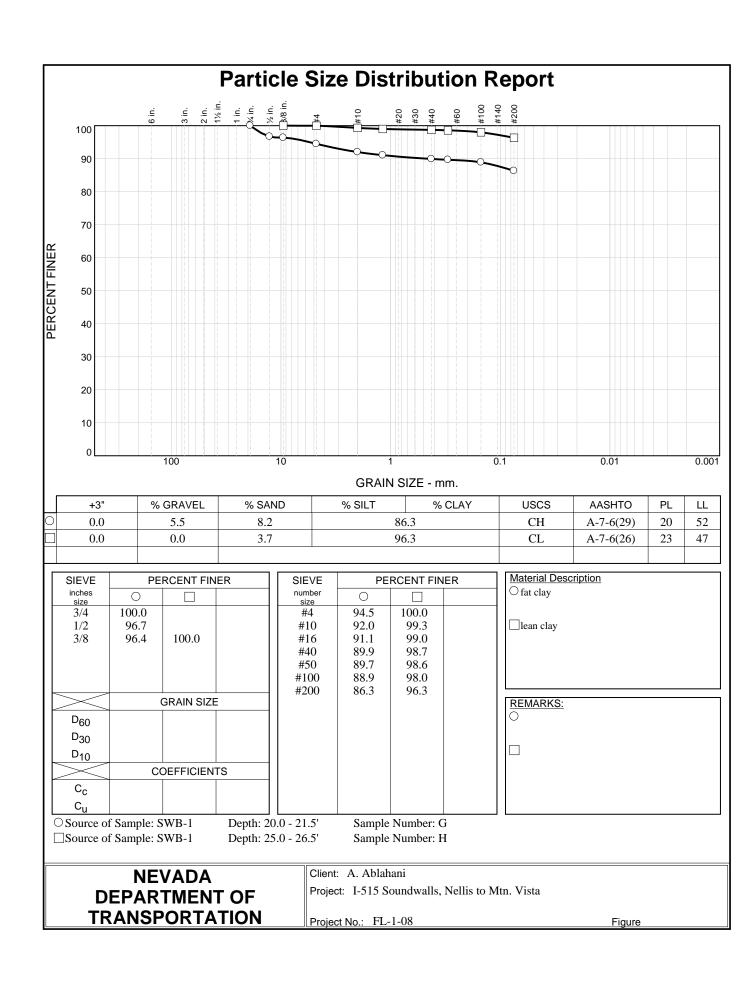


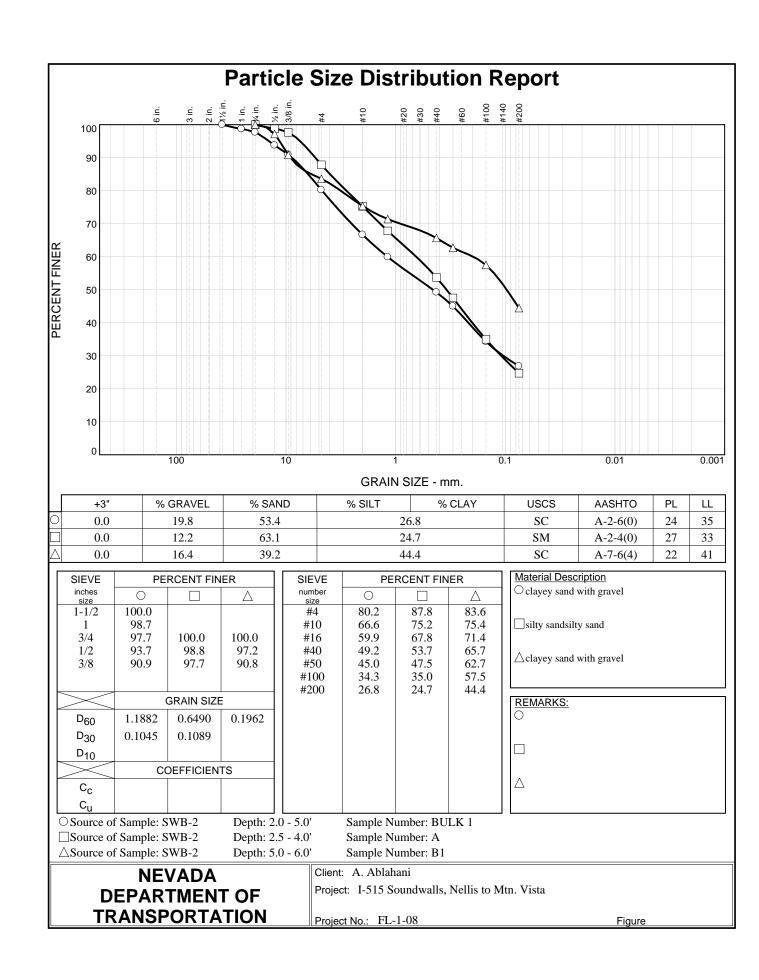


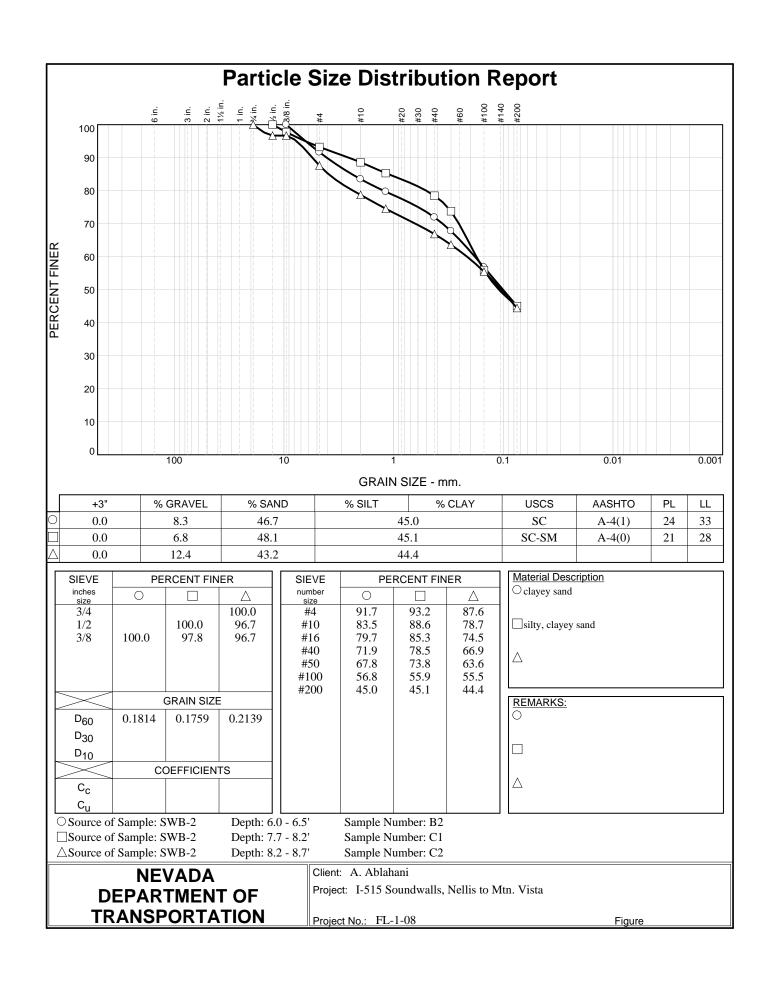


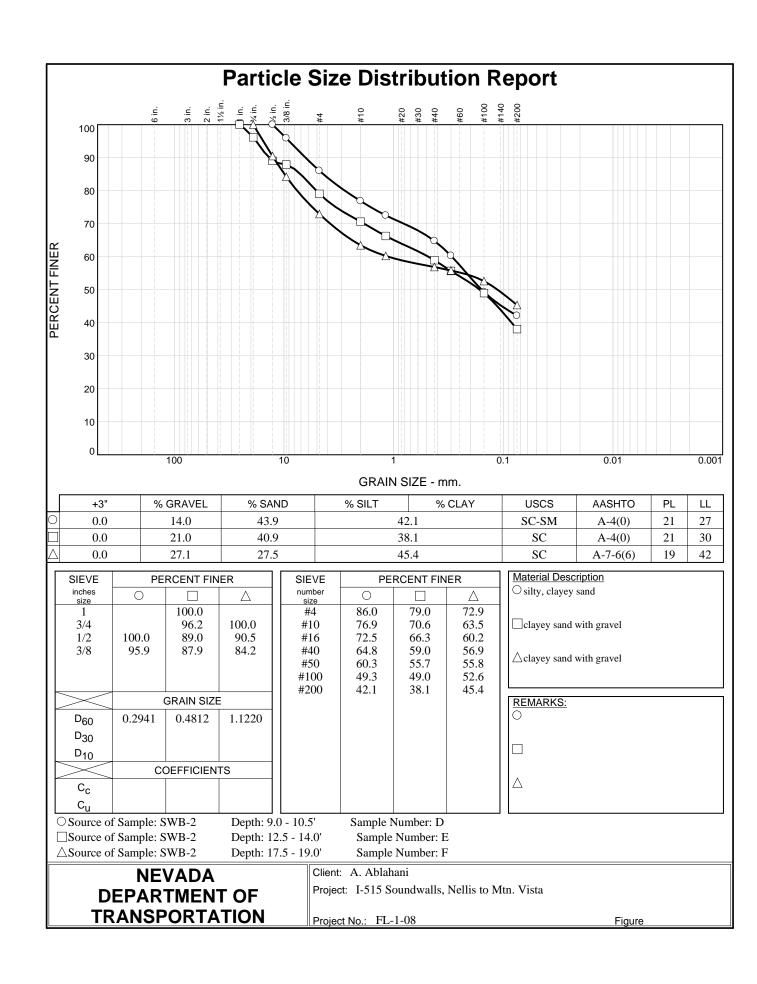


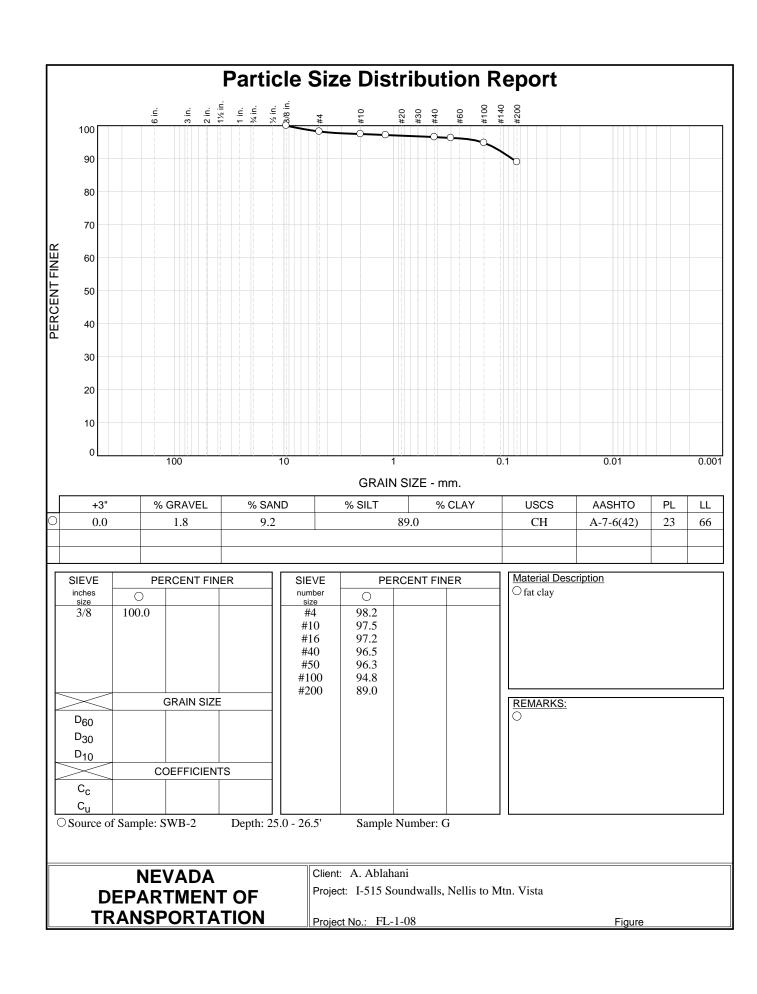




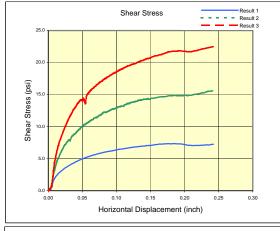


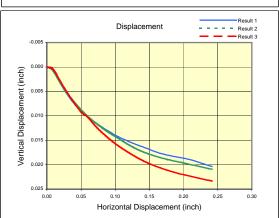


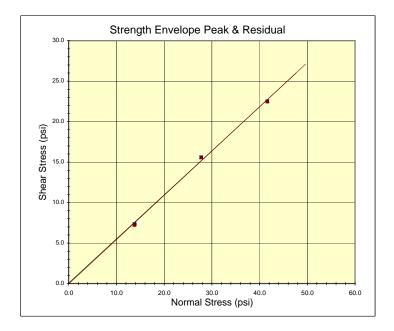




DIRECT SHEAR TEST REPORT







Strength Parameters										
Friction Angle =	Peak 29	degrees	Residual 29							
Cohesion =	0.07	psi	-0.06							

Project: FL-1-08 Boring: SWA-3 Sample: E2

	Result 1	Result 2	Result 3	
Specimen:	а	b	С	
Date Tested	03/13/2008	03/13/2008	03/13/2008	
Diameter (inch):	2.42	2.42	2.42	
Height (inch):	1.00	1.00	1.00	
Depth (ft):	10.00	10.00	10.00	
Moisture (%)	16.6	19.5	21.7	
Dry Unit Wt (pcf)	79.5	80.9	79.7	
SHEAR				
Displacement Rate(in/min)	0.0054	0.0054	0.0054	
Normal Stress (psi)	13.77	27.75	41.63	
Peak Shear Stress(psi)	7.34	15.60	22.50	
Residual Shear Stress(psi)	7.2	15.6	22.5	
Residual Point Picked @(in)	0.242	0.242	0.242	
Time @ Peak Failure (min)	33.4	44.0	44.0	

Specimen Comments

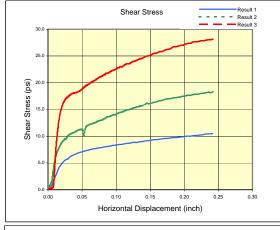
a Tan lean clay with sand. Shear at 2000 psf

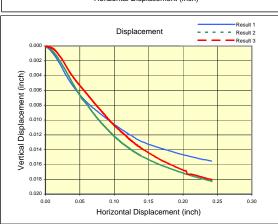
b Tan lean clay with sand. Shear at 4000 psf

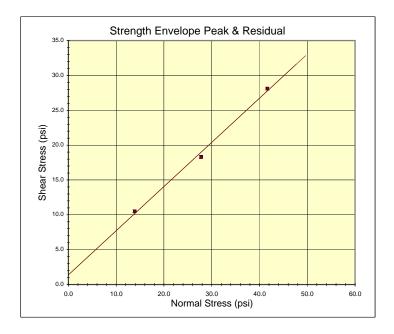
Tan lean clay with sand. Shear at 6000 psf



DIRECT SHEAR TEST REPORT







Strength Parameters										
Friction Angle =	Peak 32	degrees	Residual 32							
Cohesion =	1.39	psi	1.39							

Project: FL-1-08 Boring: SWB-1 Sample: D2

	Result 1	Result 2	Result 3	
Specimen:	а	b	С	
Date Tested	03/19/2008	03/19/2008	03/19/2008	
Diameter (inch):	2.42	2.42	2.42	
Height (inch):	1.00	1.00	1.00	
Depth (ft):	10.00	10.00	10.00	
Moisture (%)	25.7	23.8	24.5	
Dry Unit Wt (pcf)	84.1	85.1	84.9	
SHEAR				
Displacement Rate(in/min)	0.0039	0.0040	0.0040	
Normal Stress (psi)	13.87	27.76	41.66	
Peak Shear Stress(psi)	10.51	18.31	28.11	
Residual Shear Stress(psi)	10.5	18.3	28.1	
Residual Point Picked @(in)	0.241	0.242	0.242	
Time @ Peak Failure (min)	60.4	60.5	60.4	

Specimen Comments

- a Light brown sandy clay. Shear at 2000 psf. (Recompacted -No. 4 material)
- b Light brown sandy clay. Shear at 4000 psf. (Recompacted -No. 4 material)
- c Light brown sandy clay. Shear at 6000 psf. (Recompacted -No. 4 material)

