# GEOTECHNICAL INVESTIGATION SUMMERLIN HOV FLYOVER BRIDGE U.S. 95 WIDENING PROJECT

LAS VEGAS, NEVADA

**MAY 2008** 





PBS&J

Black Eagle Consulting, Inc. - Geotechnical & Construction Services



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Mr. Thomas Cotton PBS&J 2270 Corporate Circle, Suite 100 Henderson, NV 89074 May 30, 2008 Project No.: 0324-01-7

#### RE: Geotechnical Investigation Summerlin HOV Flyover Bridge U.S. 95 Widening Project Las Vegas, Nevada

Dear Mr. Cotton:

Attached please find five copies of the geotechnical report and an electronic copy on CD-ROM for the referenced project. Three of the copies are for PBS&J. Parviz Noori of the Nevada Department of Transportation (NDOT) Materials Division requests that one copy be forwarded to the project manager, John Terry, and one to the Assistant District 2 Engineer, Mohamed Rouas. We are forwarding 6 copies directly to NDOT Materials Division in Carson City, with distribution as listed below.

We appreciate being of service to you on the project. If you have any questions, or require any additional information, please contact us.

Sincerely,

Black Eagle Consulting, Inc.

Larry J.

Senior Consultant

JWP:LJJ:lmk

Copies to:



Addressee (5 copies, 1 CD-ROM) Parviz Noori, NDOT Materials Division (6 copies) (NDOT Materials Division, 2 copies plus CD-ROM) (Nancy Kennedy, NDOT Bridge Division, 1 copy) (Sharon Foershler, NDOT Construction Division, 1 copy) (Natalie Caffaratti, NDOT Roadway Design, 1 copy) (Terry Philbin FHWA, 1 copy)

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## **GEOTECHNICAL INVESTIGATION**

## SUMMERLIN HOV FLYOVER BRIDGE U.S. 95 WIDENING PROJECT

### LAS VEGAS, NEVADA

## **1.0 INTRODUCTION**

Presented herein is the Black Eagle Consulting, Inc. (BEC) geotechnical investigation for the proposed Summerlin Parkway High-Occupancy-Vehicle (HOV) Flyover bridge (Structure I-2744) that is proposed as part of the U.S. 95 / Rainbow Road / Summerlin Parkway interchange in Las Vegas, Nevada. The investigation was performed for the Post, Buckley, Schuh, and Jurnigan (PBS&J) design team, which is preparing the project design for the Nevada Department of Transportation (NDOT). The objectives of this study were to:

- 1. Determine general soil conditions pertaining to the 100 percent design and construction of the proposed bridge and associated retaining walls.
- 2. Provide recommendations for design and construction of the proposed bridge and approaches, as related to these geotechnical conditions.

Our investigation included field exploration, laboratory testing, and engineering analysis to determine the physical and mechanical properties of the various on-site materials. Results of our field exploration and testing programs are included in this report and form the basis for all conclusions and recommendations. This report includes pertinent borings from previous BEC investigations on this site (BEC, 2002a, 2002b, 2002c, 2003). The services described herein were conducted in accordance with the Subcontract Addendum to PBS&J Sub-consultant Contract for *PBS&J Project 511300.01 with Black Eagle Consulting, Inc., with the short title, "U.S. 95 HOV Flyover."* 

## 2.0 PROJECT DESCRIPTION

### **2.1 Site Conditions**

The Summerlin HOV Flyover bridge will be located within the U.S. 95/Summerlin Parkway/Rainbow Road interchange in the City of Las Vegas. The interchange is contained in Sections 26 and 27, Township 20 South, Range 60 East, Mount Diablo Meridian.

The east half of the HOV Flyover bridge will be located in the future centerline of U.S. 95 freeway, which is being reconstructed for widening. The U.S. 95 freeway in the Summerlin/Rainbow Road interchange is generally depressed as much as 2 to 3 m below original natural grade. The footprint of the south half of the bridge at the time of our exploration was approximately level at the adjacent freeway grade with gravel surfacing and no improvements. Existing traffic on U.S. 95 was moved from the northeast side of the proposed bridge to the southwest side of the proposed bridge shortly before our exploration.

The HOV Flyover bridge will cross over the Rainbow Boulevard bridge and Ramp R7 bridge. North of Rainbow Boulevard, the bridge will cross above U.S. 95 southbound, Ramp R6, and various landscaped areas within the interchange. Landscaping is minimal, consisting of scattered trees or shrubs and gravel hardscape. The west end of the bridge approaches the west end of the Ramp 11 overpass, which connects from U.S. 95 northbound to Summerlin Parkway westbound. The west abutment and Piers 9 through 11 will be built over an existing 2H:1V (Horizontal:Vertical) fill slope for Ramp 11 which is approximately 11 m high.

Various utilities including traffic control loops, overhead lighting wiring, water and storm drain are present in the area. A 1,060-mm-diameter water supply pipeline and a storm drain box culvert cross beneath the flyover bridge footprint south of Rainbow Road.

#### 2.2 Structure/Development Information

The HOV flyover bridge will provide a route for HOV traffic from the median of U.S. 95 northbound to the median of Summerlin Parkway westbound, and from the median of Summerlin Parkway eastbound to the median of U.S. 95 southbound. The bridge will carry one travel lane in each direction, with wide shoulders in each direction and a central barrier rail. The bridge will be 18.3 m wide and 740 m long, with 12 spans between 55 and 69 m in length. Abutment 1 will be located in a Mechanically Stabilized Earth (MSE) fill approximately 5 m high in the median of U.S. 95. Mechanically Stabilized Earth walls H1 and H2 will be as high as 4 and 5 m high

extending 71 to 111 m to the southeast, on the northeast and southwest sides of the HOV lanes, respectively. Abutment 2 will be located on an existing 2H:1V fill for the adjacent Ramp 11 overpass. Mechanically Stabilized Earth walls H3 and H4 will be as high as 4 and 11 m high, extending 33 and 75 m to the west, on the north and south sides of the HOV lanes, respectively.

Two concrete cantilever walls, H-5 and H-6, are planned to support the Ramp 11 embankment adjacent to new bridge piers. Wall H-5, which will be up to 3.9 m high and 17 m long, will support the edge of the Ramp 11 adjacent to Pier 11. Wall H-5 will be founded on a 4H:1V slope immediately uphill from Pier 11. Wall H-6, which will be up to 2.65 m high and 4.5 m long, replaces the south end of an existing tieback wall which must be excavated to construct Pier 10. Wall H-6 has level ground at its base and supports a 2H:1V slope below an existing bridge abutment.

The bridge will consist of steel plate girders with a concrete deck supported on cast-in-place concrete piers. Piers will have a single rectangular column, typically 1.8 by 5 m in cross section, and columns will vary in height between 2.5 and 14.1 m above final grade. Abutment 1 will be a closed abutment. Abutment 1 and Piers 1 to 8 will be supported on spread footings that will be least 2 m below the ground surface. The footings for Piers 9 through 11 are proposed to be 1 to 4 m above adjacent interchange grade within existing 2H:1V embankment slopes. The footing for Abutment 2 is proposed to be between 1 to 8 m above adjacent interchange grade. If built at lower elevation, the Abutment 2 footing would be entirely in existing embankment fill. If Abutment 2 is built at the highest elevation, the existing embankment would be widened by the addition of new borrow fill. The north side of the abutment footing would be on existing embankment fill overlying existing embankment fill.

The bridges will be designed using Load and Resistance Factor Design (AASHTO, 2007). Vertical service loads (consisting of dead plus live loads, load factor of 1.0) were 27,000 kN for typical pier foundations. Abutments will be subject to soil and bridge deck longitudinal loading.

## **3.0 GEOLOGIC CONDITIONS AND SEISMICITY**

#### **3.1 Regional Geologic Conditions**

The project lies within the Las Vegas Valley. The Las Vegas Valley occupies a topographic and structural basin transitional between the "younger" Basin and Range topography of the Great Basin of Nevada and Utah, and the "older" Basin and Range topography characteristic of the

Mojave and Gila Deserts of Arizona and California. Extensional or normal faulting started in mid- to late-Tertiary time resulting in the basin-and-range topography currently observed in the Las Vegas area. Following the peak of tectonic activity in Miocene time, and continuing through Quaternary time, a thick, semi-continuous sequence of terrestrial sediments accumulated in deep structural basins, including the structural basin that forms the Las Vegas Vegas Valley.

The center of the Las Vegas structural basin is characterized by a gradual alluvial plain sloping from the west and northwest to the east and southeast, that is crossed by a series of generally north-south-trending normal faults that extend discontinuously across the center of the valley. The majority of the faults show movement down to the east and extend to depths that approach 150 m.

## **3.2 Surficial Geology**

The HOV flyover bridge alignment is on soils that have been mapped by the Nevada Bureau of Mines and Geology (NBMG) (Matti, et al., 1987) as intermittently active alluvium (Holocene) overlying Older Alluvium of the Red Rock Fan (Pleistocene). According to the NBMG, the Holocene alluvium consists of slightly to moderately consolidated sand and pebble to cobble gravel. The Older alluvium of the Red Rock Fan ("older fan deposits") consist of mostly moderately well-consolidated and cemented, pebble to small cobble gravel with pebble-bearing sand.

## 3.3 Seismicity

The Las Vegas area is relatively quiet seismically compared to the northern portions of the state of Nevada. The mountain ranges and deep alluvial basin in the Las Vegas area were formed primarily by Tertiary tectonic movements, i.e., activity greater than 1.6 million years before present. The strongest historic seismic activity in the project vicinity has been related seismic response to filling of Lake Mead (M 5.0; several earthquakes between 1938 and 1952) or energy released from underground atomic testing north of Las Vegas in Yucca Flats (maximum magnitude M 5.8).

American Association of State Highway Transportation Officials (2007) shows horizontal rock acceleration potential to be 0.10g for a 10 percent probability of exceedance within 50 years in this area.

### **3.4 Faults**

Area Quaternary faults have been mapped by the NBMG and are presented in the *Map of Faults and Earth Fissures in the Las Vegas Area* (dePolo and Bell, 2000). This map identifies traces of potential Quaternary age tectonic faults approximately four kilometers (km) west and two km east of the site, but there is no evidence of faulting in the site vicinity.

## **3.5 Ground Subsidence**

Regional land subsidence in the Las Vegas Valley related to ground water withdrawal has been monitored since 1935 (Bell and Price, 1993; Bell et al., 2001, 2002). A map included in the 1993 open-file report, titled *Subsidence in Las Vegas Valley 1963 Through 1986/87*, identifies three major Las Vegas subsidence centers located about seven km north, seven km east, and eight km southeast of the site. The map also shows that subsidence of as much as 150 mm may have occurred in the site vicinity between the years 1963 and 1987. The more recent work (Bell, et al., 2001) indicates that subsidence within the project area has been arrested since 1987 and no additional settlement has occurred over the last two decades.

#### **3.6 Ground Fissures**

The nearest areas of ground fissures have been mapped approximately three to four km east of the site. No fissures have been mapped by others and no evidence of fissuring was observed in explorations at the project site.

#### **3.7 Liquefaction Potential**

There is no potential for liquefaction at the site because design earthquake motions and magnitude are low and the ground water is located at a depth below 30 meters.

#### **4.0 EXPLORATION**

#### 4.1 Drilling

Portions of the bridge alignment were explored in 2001 to 2002 by drilling hollow-stem auger borings to a maximum depth of 30.5 meters. The borings were drilled with 152-millimeter (mm), outside-diameter (O.D.), 83-mm-inside-diameter (I.D.) augers using a truck-mounted Foremost

B90 and a track-mounted Diedrich D-50 Turbo drill rig. The locations of closest previous test borings with sufficient depth (B-01, B-02, B-04, B-07 through B-11, B-13, B-18 and B-19) are shown on Plate 1 - Plot Plan. Numerous other borings were performed at greater distances from the HOV bridge alignment, including borings by others, which are shown in BEC (2002b).

Seven borings (FB-01 through FB-07) were drilled in April 2007. Three borings (FB-08 through FB-10) were drilled in October 2007 on the Ramp 11 embankment between Pier 9 and Abutment 2. The borings were drilled with 152-mm O.D., 83-mm I.D. augers to a maximum depth of 24.4 m using a track-mounted Diedrich D-50 Turbo drill rig.

Native soils were sampled in place every 600 to 1,500 mm by use of a standard, 51-mm, O.D., Split-Spoon Sampler or an 89-mm O.D. Split-Spoon Sampler (ASTM D 3550), both driven with a standard 63.6-kilogram (kg) drive hammer and a 760-mm stroke (Standard Penetration Test, AASHTO T 206). The number of blows in a Standard Penetration Test (SPT) is an indication of the density and consistency of the material. Also, at various locations, where the split spoon samplers were not able to retrieve an adequate sample in the cemented, coarse, granular soils, grab samples were acquired from the auger spoils to obtain a sample of sufficient size for determining the approximate maximum particle size and particle gradation.

Coring was attempted in two borings (FB-04 and FB-05) from 1.5 to 4.5 m depth using rock coring equipment. Recovery of intact core was poor, indicating that dense gravel soils are not highly cemented.

The horizontal and vertical locations of each boring were resurveyed by PBS&J after drilling. Logs of borings are included in Appendix B (Subsurface Exploration Data).

## **4.2 Material Classification**

A geological engineer examined and identified all soils in the field in accordance with ASTM D 2488. Additional soil classification was subsequently performed on soil samples in accordance with ASTM 2487 (Unified Soil Classification System [USCS]) upon completion of laboratory testing. Where soil tests are not listed in the appropriate column of the boring log, or soil gradation in the material description column is listed as "estimated," the USCS symbols and terminology are based on manual identification (ASTM D 2488) rather than laboratory classification. A classification and symbol key is provided as Plate B-2 in Appendix B.

Some unavoidable bias in the grain size distributions is present in the soil identification and classification due to drilling and sampling methods. A majority of samples were collected from driven samples which met refusal (greater than 50 blows per 15 cm driven interval). Sampler refusal frequently results in crushing of material at the sampler tip and a greater fraction of smaller particles and some non-plastic fines (rock flour). Samples were also obtained from auger cuttings, where the auger lifting operation also tends to fracture and degrade larger particles into smaller particles. One sample (FB-04 at 1.8 m) was collected by rock coring techniques. Coring can reduce crushing or breaking of larger-sized particles (up to the diameter of the core barrel) but can result instead in washing out of fine particles. The cored sample had only 2 percent fines, 7 percent sand, and 91 percent gravel-sized particles. For comparison, SPT and auger cutting samples at adjacent depths consistently had 5 to 20 percent fines, 30 to 70 percent sand, and 10 to 65 percent gravel.

### 4.3 Drive Hammer Calibration

Borings in the 2007 investigation (FB-01 through FB-10) were sampled with a calibrated automatic hammer with an efficiency of 69 percent (Foundation Tech, LLC, 2007). The field SPT values should be multiplied by 1.15 to obtain a standard 60 percent efficiency. Where borings were performed with a calibrated automatic hammer, the hammer efficiency is also listed in the "remarks" column on each boring log.

Previous explorations were performed with a down-hole hammer operated with a wire cable winch system. The wire cable is raised and lowered by a hydraulic winch. Hammer efficiency for this system is unknown but is generally low.

## 4.4 Shear-Wave Velocity Survey

Redpath Geophysics conducted a shear-wave velocity survey during July 2001 in Boring B-01. The velocity study used conventional down-hole survey methods, where travel times of the shearand compression-wave arrivals are measured progressively from the surface to the bottom of the borehole. The velocities are shown on Table 1 – Shear-Wave Velocities in Boring B-1.

TABLE 1 – SHEAR-WAVE VELOCITIES IN BORING B-01										
Depth Range in m         Shear-Wave Velocity, m/s         Comment										
0 – 1.5	380	Probably compacted fill								
1.5 – 6	710	Probably compacted fill								
6 – 29	1,300	Native								

## **5.0 LABORATORY TESTING**

All soils testing performed in the BEC soils laboratory were conducted in accordance with the standards and methods described Nevada Department of Transportation (NDOT, 2001), American Association of State Highway Transportation Officials (AASHTO, 2004) and ASTM (2005).

## **5.1 Index Testing**

Samples of each significant soil type were analyzed to determine their in-situ moisture content (NDOT T 206F), grain size distribution, and plasticity index (NDOT 210E, 211E, and 212E). The results of these tests are in Appendix C.1 - Laboratory Test Results.

#### **5.2 Strength Tests**

Direct Shear Tests (AASHTO T236-92) were performed on representative samples of soil from the project alignment. Since even ring samples were disturbed and all had particles greater than the 2 mm (No. 10) sieve, all direct shear test samples were prepared by removing particles greater retained on the No. 10 sieve and recompacting near the optimum water content. Direct shear test samples in the present investigation were also split on the No. 200 sieve, and then recombined to provide the same fines content (percentage of material passing the No. 200 sieve) as was present in the original bulk soil sample. Results of these tests are shown in Appendix C-2 – Strength Test Results.

A Harvard Miniature compaction test (NDOT T101E) was performed on one of the screened samples for direct shear testing (FB-10 at 9.1 - 10.7 m) for comparison of tested and compacted densities. The test results showed the maximum dry density of 19.8 kN/m<sup>3</sup> (126 pcf) at an optimum moisture content of 9 percent by dry sample weight. The tested sample at that location had an average density equal to 97 percent of the maximum dry density by this test method.

### **5.3 Corrosion Potential Tests**

Chemical testing was performed on representative samples of site foundation soils to provide data for corrosion potential evaluation. Chemical testing was subcontracted to Western Environmental Testing Laboratory of Sparks, Nevada. Testing for pH was performed in accordance with Environmental Projection Agency (EPA) Method 9045B (AASHTO T289-91). Testing for soil resistivity was performed in accordance with EPA Method 2510B (AASHTO T288-91). Testing for soluble chloride and soluble sulfate was performed in accordance with EPA 600/4-79-020-300.0 (AASHTO T290-94 and T90-95). These test results are shown in Appendix C-3 – Chemical Test Results.

## 6.0 DISCUSSION OF INVESTIGATION RESULTS

### 6.1 Geologic and Geotechnical Conditions

The proposed HOV flyover alignment is mapped by the Nevada Bureau of Mines and Geology [NBMG] (Matti et al., 1987) as Pleistocene Older Alluvium of the Red Rock Fan, which is described as *mostly pebble to small cobble gravel, with pebble-bearing sand that is moderately well consolidation and cemented*. The materials encountered in our borings were consistent with the description of the Older Alluvium by the NBMG. Because the interchange is cut 2 to 3 m below original grade, recent (Holocene) deposits are not present at foundation levels. Soils were uniform in consistency across the site and at all depths, and consisted of clayey and silty sands with gravel, clayey and silty gravel, poorly-graded and well-graded sands or gravels with silt or clay, and rarely poorly graded sands or gravels. Samples ranged in proportion from 5 to 25 percent low- to medium-plasticity fines, and 20 to 65 percent gravel content. Some cobbles are likely present but were not extensively observed due to the relatively-small-diameter sampling methods.

The soils are very dense or slightly cemented, indicated by high penetration resistance and shearwave velocities in excess of 600 meters per second. Soils were not sufficiently cemented to allow successful intact core recovery. No hard caliche layers were encountered that resulted in refusal of the hollow-stem-auger drilling equipment.

Fills for the existing embankment of Summerlin Parkway westbound (Ramp 11) were found to be well-compacted, dense, granular soils similar in grain size characteristics (coarse silty, sandy gravel with occasional cobbles) and density to the underlying native soils.

Ground water was not encountered during the explorations to depths in excess of 30.5 m, and is at a depth that would not affect construction.

#### 6.2 Geologic Hazards

A moderate potential for dust generation is present if grading is performed in dry weather. Regional subsidence has occurred at this site but is not occurring presently, due to more-controlled ground water withdrawal from the aquifers under the site. Regional ground subsidence, if it recurs, is sufficiently gradual in magnitude and widespread in horizontal extent that it should not impact structural performance. No other geologic hazards were identified.

## 7.0 DISCUSSION AND RECOMMENDATIONS

### 7.1 General Information

The proposed bridge alignment is on dense, slightly-cemented granular soils which provide good bearing conditions to support the proposed bridge on shallow foundations. Slight cementation may make excavation difficult with smaller equipment. Fills for the existing embankment of Summerlin Parkway westbound (Ramp 11) were found to be well-compacted, dense, granular soils similar to native materials. Gravel and likely cobbles are present that could make temporary excavation support with tiebacks or soil nails difficult, both due to difficult drilling and to possible caving or oversize excavation of drill holes.

#### 7.2 Seismic Design Requirements

For the purposes of this project, we recommend a minimum design acceleration value of 0.15g to be used in accordance with NDOT design policy. Soil Profile Type II is appropriate given the considerable depth of the Las Vegas soil basin under the site.

#### 7.3 Structure Foundation Recommendations

#### 7.3.1 Foundation Type Selection

Shallow foundations are appropriate for the Summerlin Parkway HOV flyover bridge, due to the strong, very dense, granular subgrade soils. If there are tight space constraints or high uplift

requirements, drilled shafts would be a suitable alternate foundation method. Driven piles would not be suitable due to very difficult conditions for pile driving.

For design, a friction angle of 37 degrees was used for native soils. The friction angle for native soils was considered to be at least 37 degrees based on high penetration resistance (e.g. FHWA 2007 Table 10.4.6.2.4-1). The selection of friction angle was further supported by the uniform, well-graded particle-size distribution, with sufficient gravel-size particles to provide interlocking of coarse-grained as well as sand-sized particles in the soil matrix. Geologic aging or other minor cementation also was considered in selecting the friction angle. Results of direct shear tests from the original investigation were reviewed. After removing particles larger than the No. 10 sieve, soils had a range of 25 to 48 percent fines content, but still had average friction angle of 37.5 degrees. Since the actual fines content of the original samples was only 10 to 17 percent, and coarse particles were present, lower strength values in the direct shear testing were discounted as not reflecting the actual material characteristics or strength.

A friction angle of 37 degrees was selected for existing embankment fill. Similar grain size distribution and similar to slightly higher penetration resistance was present as for the native soils; however, geologic aging and cementation would not be present. Direct shear testing of fill soils in this investigation used soils with the same fines content as the in situ material, with a resulting friction angle of 37 to 42 degrees; the lower bound was used for design.

#### 7.3.2 Shallow Footing Recommendations

#### Level Ground Footings (Abutment 1, Piers 1 through 8)

Shallow footings should be designed using the lesser of the factored bearing resistance determined from nominal bearing capacities or the bearing pressure determined from settlement limitations. Nominal bearing resistances for strength design and service bearing resistances providing less than 1 inch of settlement are presented versus footing width on Plate 2. Bearing resistance limits are for footings 2 m or greater depth below adjacent grade in dense native soils (Abutment 1 and Piers 1 through 8). Assumptions regarding footing length and depth of footing are stated on the Plate. If the final footing dimensions (including the effects of eccentric loading), depth, or horizontal locations vary substantially from these assumptions, we should provide additional analysis for the actual design geometry.

Major utilities, including a water main and a box culvert, cross under the bridge alignment south of Rainbow Boulevard. Culverts, utilities, or other infrastructure should lie above a 1½H:1V plane projected downward from the edge of the footings. This separation is recommended to

avoid undue stresses on culverts or pipe, to avoid damage to the bridge if failure of the utility occurs, and to allow for repair or replacement of the utility at a future time without requiring complete removal or extensive underpinning or shoring of the bridge abutment. For footings on native soils, abandoned utility trenches which extend under the footing zone of influence should be excavated and backfilled with slurry cement.

#### Footings in Sloped Embankment Fills (Piers 9 to 11, Abutment 2)

Nominal bearing resistances and service bearing resistances providing less than 1 inch of settlement are presented versus footing width on Plate 3 for footings founded in existing Ramp 11 embankment soils (Piers 9 through 11 and Abutment 2). These conditions are appropriate for where the footings will be supported in existing embankment fill or are founded above the lowest adjacent grade (above the toe of a slope). Design values on Plate 3 are based on a measured soil friction angle for the existing embankment fill of 37 degrees or greater.

For Abutment 2, design options include either a tall abutment founded near the base of the existing fill, or a shallow abutment founded partly above the existing embankment fill prism. In the latter case, the embankment would be widened to provide sufficient fill to support and embed the abutment, so that the outer edge of the abutment could potentially be supported by an additional 3 m thickness of new select borrow. Since the properties of the new select borrow are not determined, a friction angle of 34 degrees is appropriate for design (per NDOT policy). Per NDOT policy, bearing resistances on new embankment fill are typically limited to prescribed values; however, given the size of the proposed footing (9 by 18 m in the present design iteration) we recommend values higher than the prescriptive limit for footings wider than 6 m. Nominal bearing resistances and service bearing resistances providing less than 1 inch of settlement are presented versus footing width on Plate 4 for the Abutment 2 footing founded partly on new embankment fill and partly on existing Ramp 11 embankment fill.

## 7.4 Lateral Earth Pressures

Earth-pressure coefficients are provided below for lateral load design of retaining walls, abutments, or structures with imbalanced earth loads. Fill slopes and retaining walls will generate active or at-rest pressures that will impose loads on structures. For shallow foundations, wind, seismic, or earth pressure loads may be resisted by passive soil pressure and friction on the bottom of the footings.

Table 2 shows the recommended static and dynamic soil pressure coefficients. The values are based on Mohr-Coulomb analyses, Mononobe-Okabe analyses, and log-spiral passive charts by Caquot and Kerisel as referenced in AASHTO (2007). Table 2 provides recommendations for approximately level ground at the top and bottom of the wall, and different values will be needed if sloped conditions are present. We recommend neglecting the passive pressure where the base of Abutment 2 is adjacent to a 2H:1V slope. Retaining walls can use bearing resistance recommendations provided in the previous section.

### 7.5 MSE Wall Recommendations

Mechanically Stabilized Earth walls should be designed based on the parameters on Table 3. Retaining walls can use bearing resistance recommendations provided in the previous section. For seismic design, the horizontal seismic coefficient should be one-half the design peak ground acceleration, or 0.075g. Mechanically Stabilized Earth wall external stability design (except for global stability) was performed by PBS&J and is not included in this report.

TABLE 2 - LRFD LATERAL EARTH PRESSURE COEFFICIENTS FOR RETAINING STRUCTURES											
Embankment Fill and Granular Backfill (Abutments 1 and 2)											
Unit Weight $20 \ kN/m^3$											
Sliding Coefficient <sup>(1)</sup> , µ:		0.54(soil on cast-in-place [CIP] concrete) <sup>(1)</sup>									
Bearing Resistance	(Abutment 1 if	built on existing embankment soils, us	se Plate 3, if partly on new emban	kment fill, use Plate 4)							
Slope Condition (and assumed friction angle)	Active Coefficient, K <sub>a</sub>	Active Forthqueke Coefficient									
Level Ground ( <b>¢=34°</b> )(3)	0.26	0.30	0.44	3.9							
2H:1V (φ=34°)(3) (Wall H-6)	0.40	0.54	0.44	NA							
	Dense	Native Gravel to Clayey Sand v	with Gravel								
Unit Weight		20	$kN/m^3$								
Sliding Coefficient <sup>(1)</sup> , µ:		0.54(soil on	CIP concrete) <sup>(1)</sup>								
Bearing Resistance		(Abutment 1 on native soil 2m belo	ow lowest adjacent grade, use Plat	e 2)							
Passive Pressure Coefficient $\mathbf{K}_{\mathbf{p}}^{(1)}(\phi=37^{\circ})$											
Notes: (1) Values of passive pressure and sliding coefficients include resistance factors of 0.50 and 0.80, respectively for LRFD design (AASHTO, 2007 Table 10.5.5.2.2-1). (2) For earthquake design of a structure under static at-rest ( $K_o$ ) conditions, use the greater of the at-rest or active earthquake pressure ( $K_{ae}$ ) unless walls are rigidly held in place by											

anchors or battered piles. <sup>(3)</sup> Active pressures assume a vertical wall face and an interface friction angle of 17 degrees and the pressure resultant is oriented at the interface friction angle above horizontal.

TABLE 3 - LRFD LATERAL EARTH PRESSURE COEFFICIENTS FOR MSE WALLS											
Embankment Fill, Granular Backfill and MSE Backfill											
Unit Weight		$20 \text{ kN/m}^3$									
Sliding Coefficient <sup>(1)</sup> , μ:		0.60 (soi	il on soil) <sup>(1)</sup>								
Bearing Resistance	(Walls H3 and H4	if built on existing embankment soils	, use Plate 3, if partly on new emb	bankment fill, use Plate 4)							
<b>Slope Condition</b> (and assumed friction angle)	Active Coefficient, K <sub>a</sub>	Active Earthquake Coefficient, K <sub>ae</sub>	At-Rest Coefficient, K <sub>0</sub> <sup>(2)</sup>	Passive Coefficient, K <sub>p</sub> <sup>(1)</sup>							
MSE Reinforced Zone and MSE Backfill (level, "Rankine" condition	0.28	0.33	0.44	NA							
[vertical wall, δ=0], φ=34°)											
	Dense	Native Gravel to Clayey Sand v	vith Gravel								
Unit Weight		20	$kN/m^3$								
Sliding Coefficient <sup>(1)</sup> , μ:		0.60 (soi	il on soil) <sup>(1)</sup>								
Bearing Resistance	(Walls H	1 and H2 if reinforced zone on native	soil 2m below lowest adjacent gra	ade, use Plate 2)							
Passive Pressure Coefficient K <sub>p</sub> <sup>(1)</sup> (level, φ=37°)	5.5										
		tance factors of 0.50 and 0.90, respectively ditions, use the greater of the at-rest or ac									

<sup>(3)</sup> For "Rankine" conditions, earth pressure is parallel to the upper ground surface, or horizontal in this case. Rankine pressures determined using the Coulomb equation with interface angle  $\beta$ = slope angle  $\beta$ , wall back-face angle  $\theta$ = 0.

### 7.6 Slope Stability

Global slope stability analyses were performed for the abutment fills, fills with MSE walls, and at Pier 9 where the bridge footing will be founded in the existing embankment above lowest adjacent grade. As requested by the NDOT Materials Division, for global stability purposes only, we have selected a friction angle of 32 degrees for the new and planned embankment fill. Mechanically Stabilized Earth walls with a minimum strap length of 70 percent of the wall height FHWA (2002) were found to provide adequate global slope stability. Mechanically Stabilized Earth wall internal and external stability (overturning, sliding, bearing resistance checks) was analyzed by PBS&J and is not included in this report.

Slope stability cross sections and resulting factors of safety are summarized on Table 4. These slopes have adequate factor of safety for seismic and static conditions. No seismic slope deformations are predicted to occur in the event of the design earthquake (peak ground acceleration of approximately 0.15g).

TABLE 4 - RESULTS OF GLOBAL SLOPE STABILITY ANALYSES										
Slope Stability Analysis Section <sup>(1)</sup>	Static Factor of Safety (FS >1.53 for slopes associated with structures including retaining walls, FS > 1.33 for other slopes) <sup>(3)</sup>	Seismic Factor of Safety at 0.075 g (FS > 1.1)								
Wall H1 & H2 Maximum Section 5.5 m High <sup>(2)</sup>	2.37 (circular) 1.82 (block)	1.92 (circular) 1.59 (block)								
Wall H3 Maximum Section	Less stringent than H1	and H2 by inspection								
Wall H4 Maximum Section 11m High <sup>(2)</sup>	1.84 (circular) 1.99 (block)	1.58 (circular)								
Abutment 2 in 2H:1V Embankment	1.92 (circular) 2.62 (block)	2.19 (block)								
Pier 9 in 2H:1V Embankment	1.80 (circular) 1.50 (block)	1.49 (circular) 1.34 (block)								
2H:1V Embankment Maximum Section	1.43 (shallow circular)	1.12 (shallow circular) 1.47 (block)								

<sup>(1)</sup> Fill materials assigned zero cohesion and friction angle of 32 degrees for global stability ONLY, native material zero cohesion and friction angle of 37 degrees.

<sup>(2)</sup> MSE reinforcement length 70 percent of wall height.

<sup>(3)</sup> Factor of safety corresponds to  $\Phi$  x Nominal Resistance/Service I Load, where minimum factor of safety of 1.53 corresponds to  $\Phi$  of 0.55, minimum factor of safety of 1.33 corresponds to  $\Phi$  of 0.65 according to AASHTO (2007 11.6.2.3), and seismic factor of safety of 1.1 corresponds to  $\Phi$  of 0.90.

## 7.7 Corrosion Potential

Soils typically have concentrations of up to 1,400 parts per million (ppm) of soluble sulfate and up to 240 ppm of soluble chloride, pH between 7.5 and 10.6, and soil resistivity between 1,200 and 7,500 ohm-cm.

## 7.8 Earthwork and Grading Recommendations

#### 7.8.1 Clearing, Grubbing, and Removals

Clearing, grubbing, and removal of obstructions shall be performed in accordance with *NDOT Standard Specifications* Sections 201 and 202 (NDOT, 2001). Except for limited plantings, there is no vegetation on the site, a stripping depth of 0 to 10 cm is expected.

#### 7.8.2 Excavations and Embankment

Excavations and embankment should be prepared in accordance with *NDOT Standard Specifications* Sections 203 (NDOT, 2001). Assuming that embankment will be constructed of sound, gravelly fill similar to the native soil, settlement of the embankment and its foundation should be minor. Settlement of embankment materials should be completed as the fill is raised.

Excavated soils will be slightly-cemented gravelly soils. These soils were drilled without difficulty with hollow-stem augers to a depth of 30 m, indicating that the site materials can generally be excavated with standard excavation equipment, and blasting is not expected to be required. Subsidence and shrinkage of native ground or existing fills excavated and recompacted as embankment fill is expected to be negligible.

#### 7.8.3 Structure Excavation

Structure excavations and backfill should be performed in accordance with *NDOT Standard Specifications* Sections 206, 207 and 208 (NDOT, 2001). All trenching should be performed and stabilized in accordance with OSHA standards. Regardless of excavation soil type or required trench slopes or shoring, pavement quantities should be determined per the *Standard Plans for Road and Bridge Construction* (NDOT, 2007).

As noted above, utilities which lie below a 1.5H:1V plane projected downward from the edge of footings should be completely removed. For footings designed for bearing resistance on native soils, backfill the excavation under the footing area of influence with cement slurry.

Significantly shored or sloped excavations will be required for footing excavations for Piers 9 through 11 and Abutment 2 due to the height of adjacent ramp 11.

The ground water under the project alignment is at a considerable depth and should have no impact on construction

#### 7.8.4 Settlement Monitoring

We recommend that permanent settlement monuments be established and used during construction of the Summerlin HOV Flyover bridge. This will enable NDOT to measure the actual amount of ground settlement that occurs under piers and abutments during and after construction. This type of information has not been gathered by NDOT very often in the past but it is becoming more important as construction pace increased in major urban areas.

Temporary settlement points would be established after the footing is poured and before the columns are formed. The relative elevations would be transferred to a one permanently-accessible location on each column and two locations on the left and right side of the abutments after the forms are removed, and maintained until the end of construction. Settlements would be monitored approximately every 2 months, before and after major increases in bridge loading (column pour, superstructure erection, footing backfill, bridge opening) and once immediately before project completion, whichever is less frequent.

## 8.0 STANDARD LIMITATIONS CLAUSE

This report has been prepared in accordance with generally accepted geotechnical practices. The analyses and recommendations submitted are based on field exploration performed at the locations shown on Plate 1 - Plot Plan and previous on-site investigations for existing structures. This report does not reflect soils variations that may become evident during the construction period, at which time re-evaluation of the recommendations may be necessary. This report has been prepared to provide information allowing the engineer to design the project. In the event of changes in the design or location of the project from the time of this report, recommendations should be reviewed and possibly modified by the geotechnical engineer. If the geotechnical

engineer is not granted an opportunity to make this recommended review, he can assume no responsibility for misinterpretation or misapplication of these recommendations or their validity in the event changes have been made in the original design concept without his prior review. The geotechnical engineer makes no other warranties, expressed or implied, as to the professional recommendations provided under the terms of this agreement and included in this report.

#### **9.0 REFERENCES**

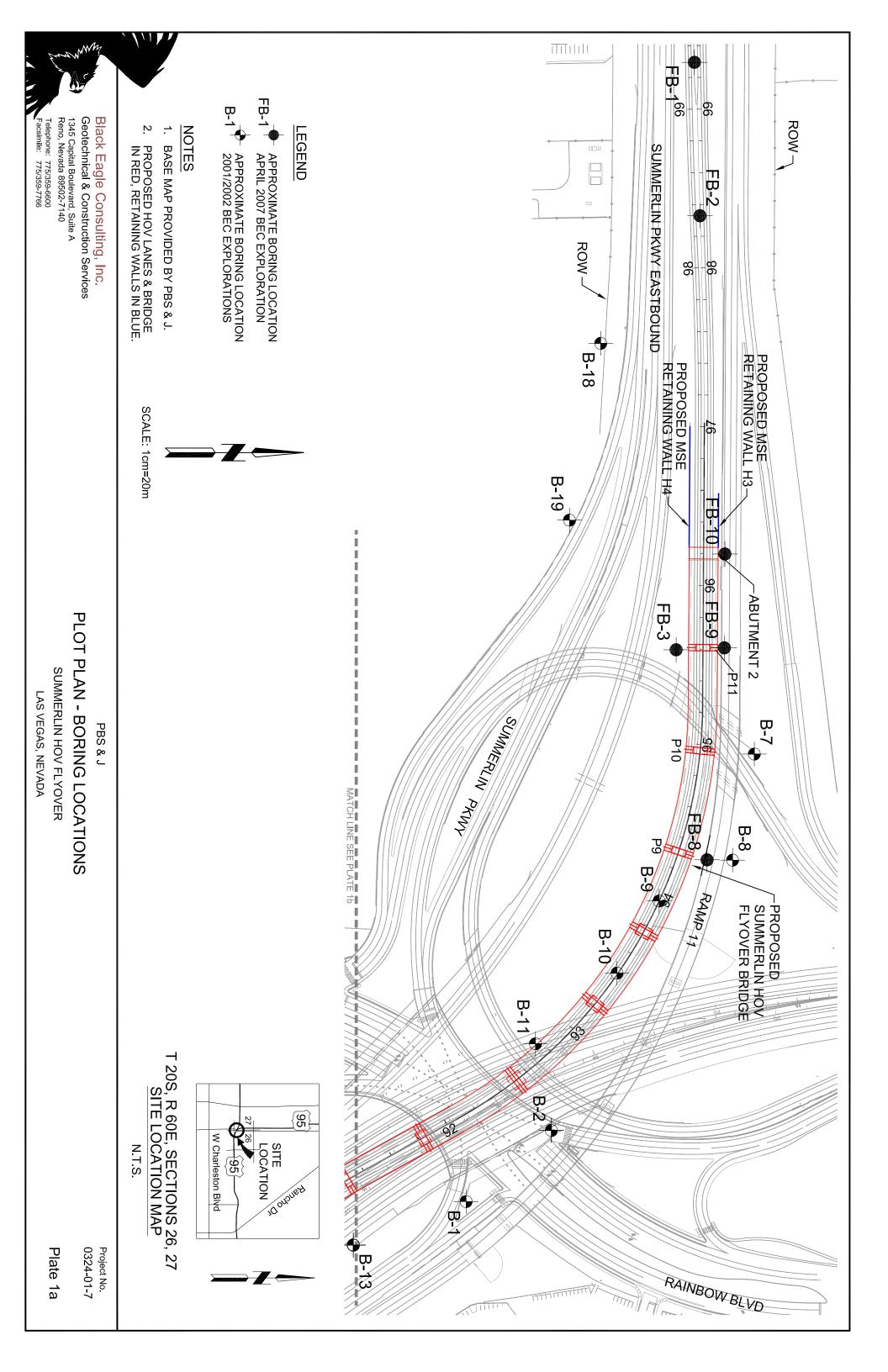
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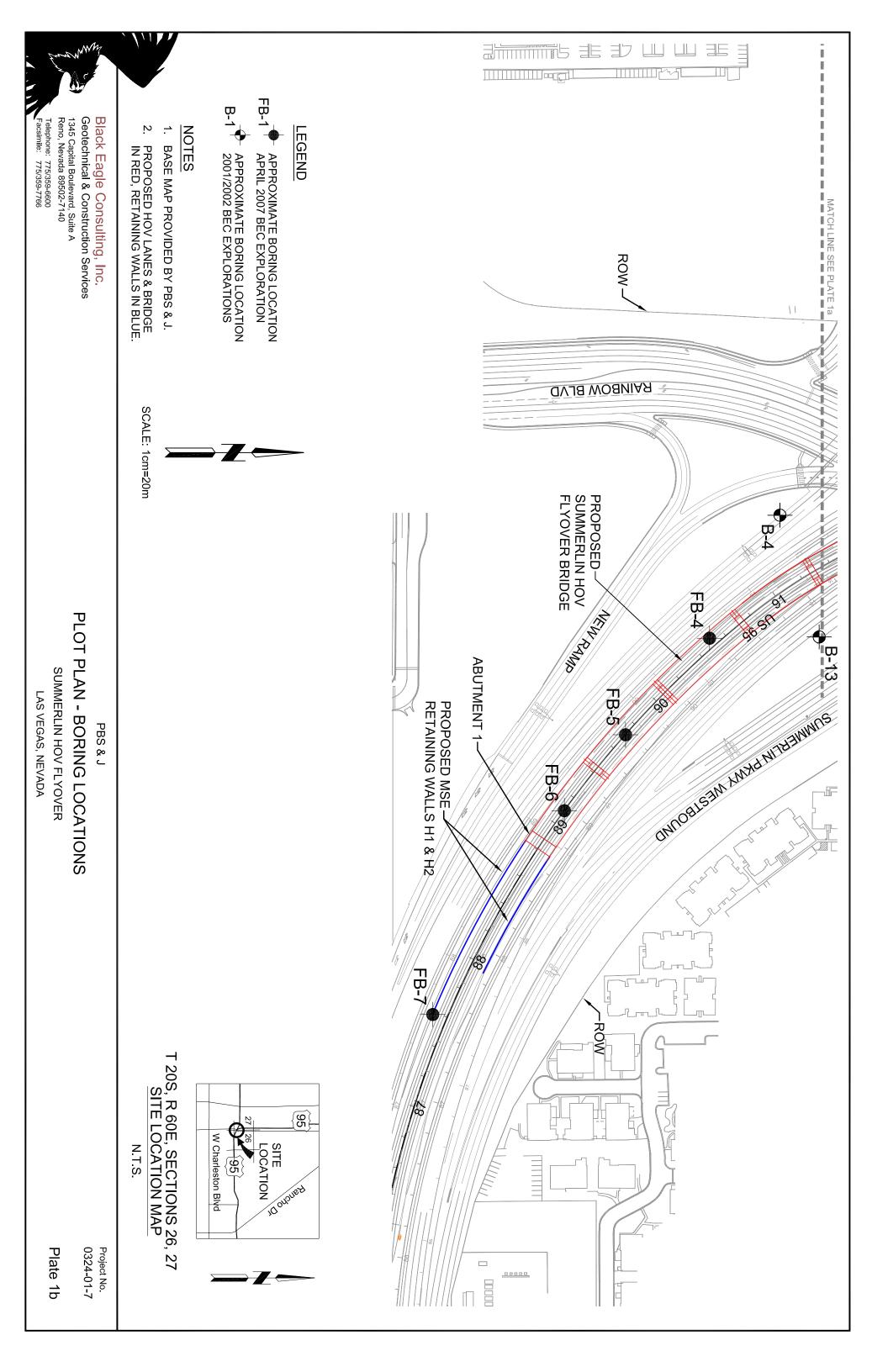
**Black Eagle Consulting, Inc.** 

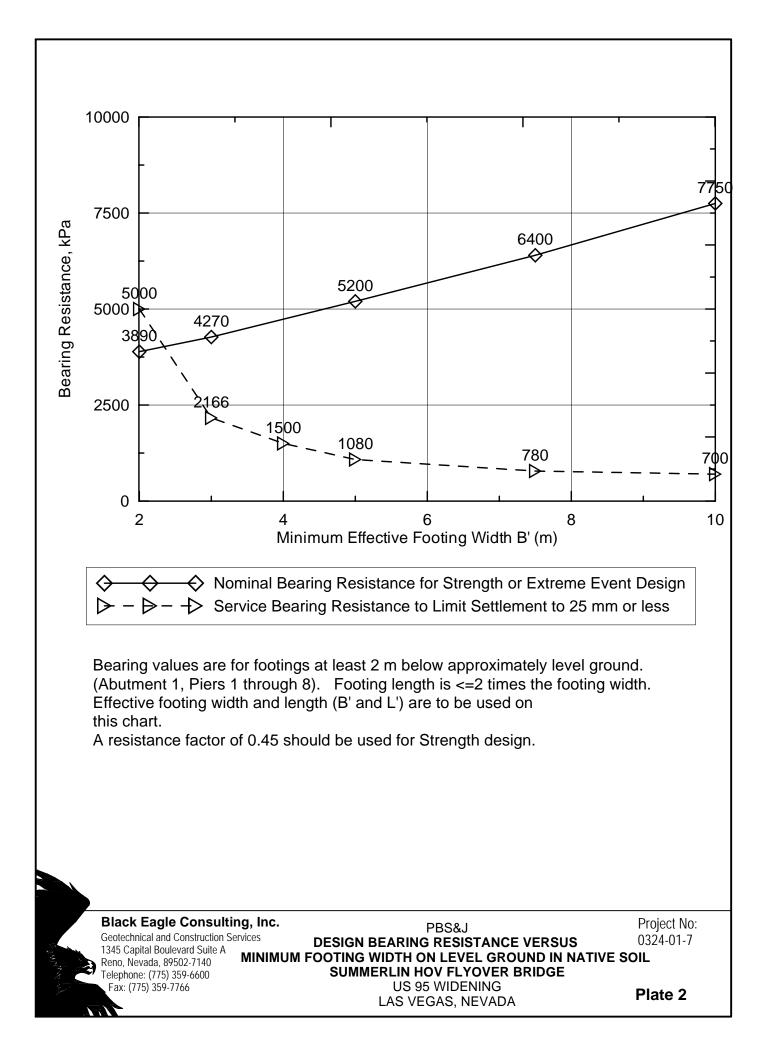
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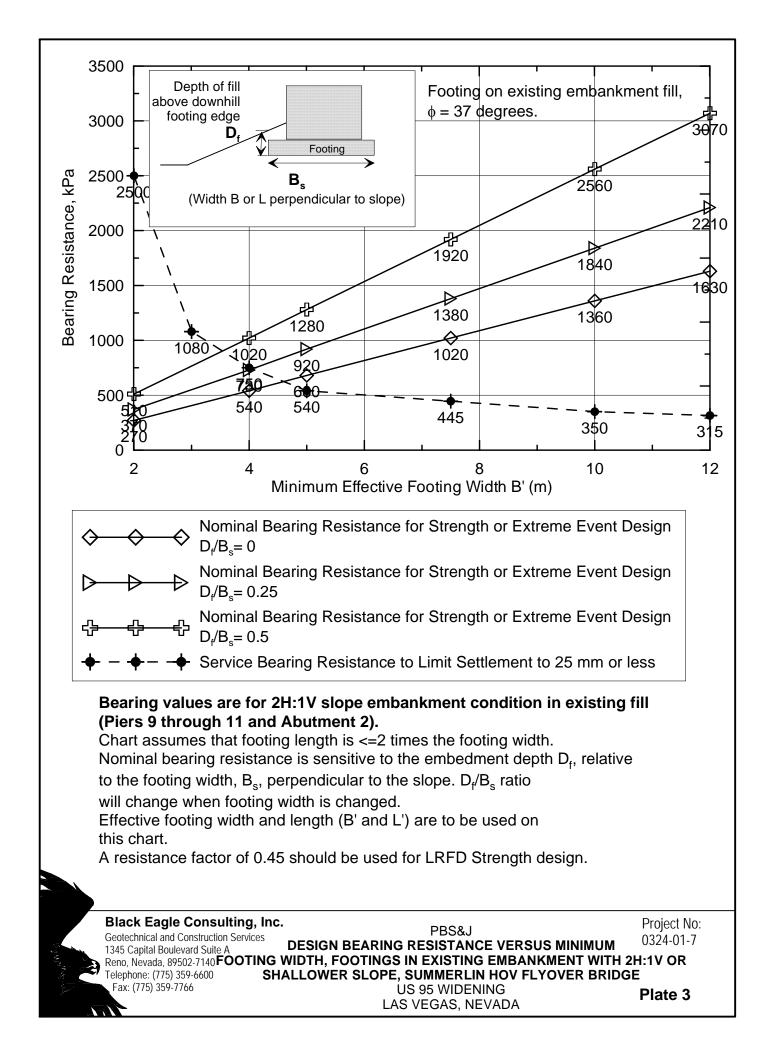
## **APPENDIX** A

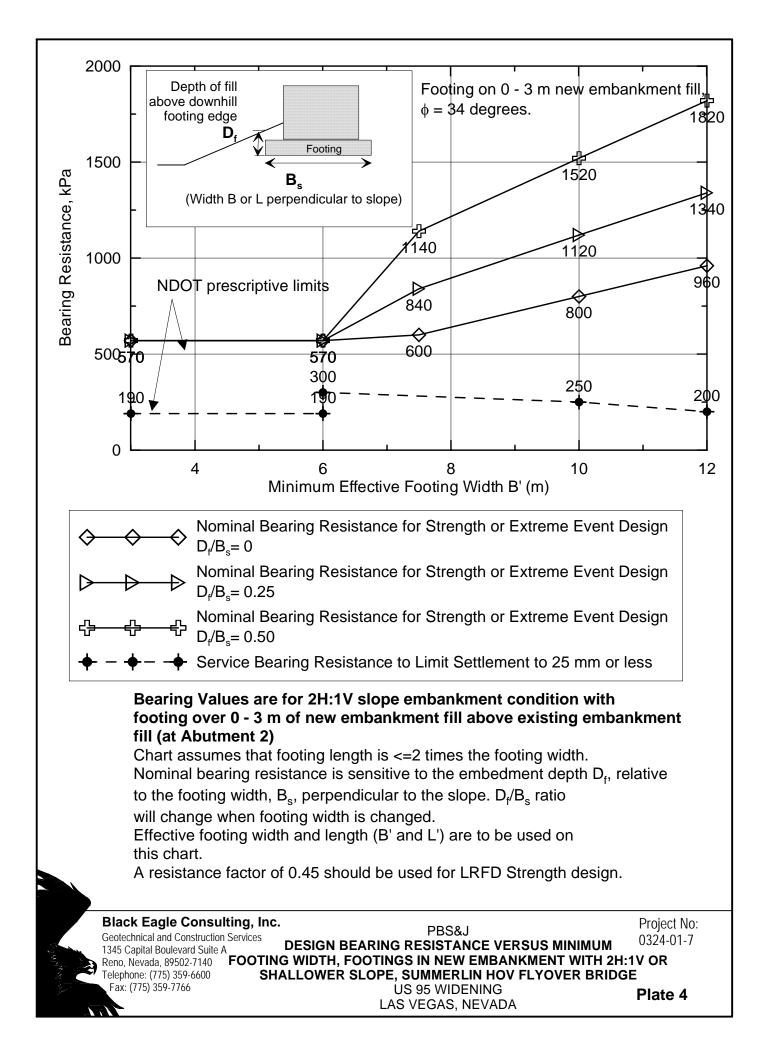
## **FIGURES**











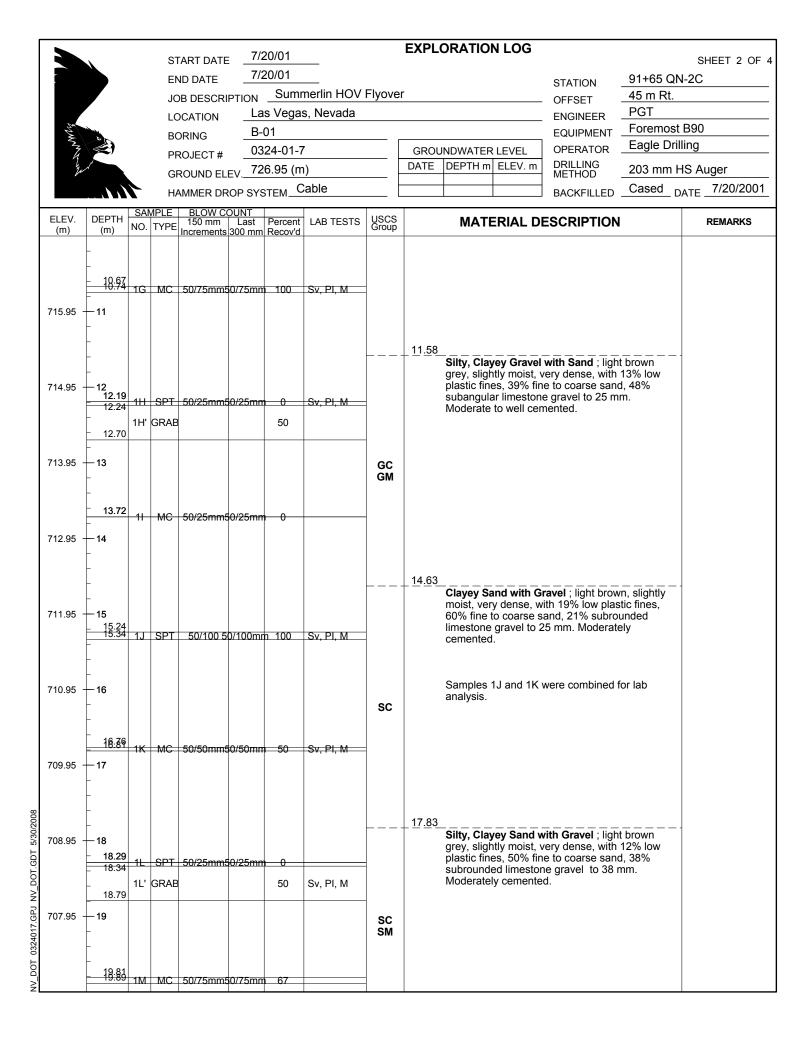
## **APPENDIX B**

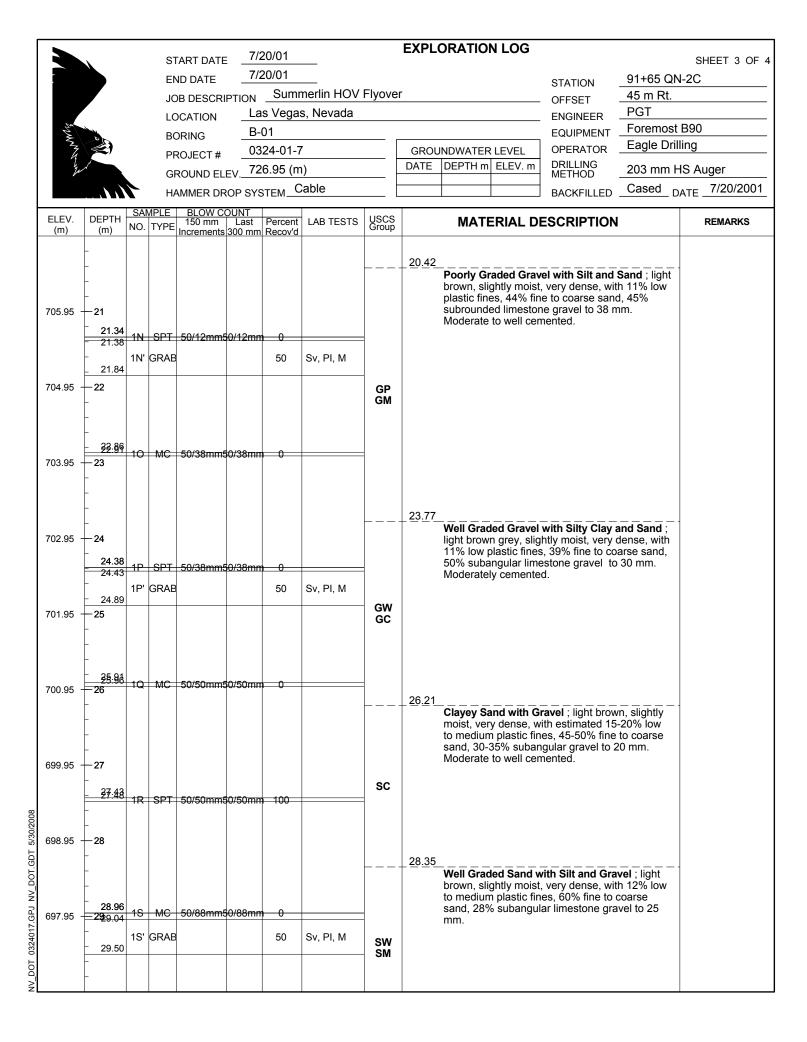
## SUBSURFACE EXPLORATION DATA

## **B-1**

## **BORING LOGS**

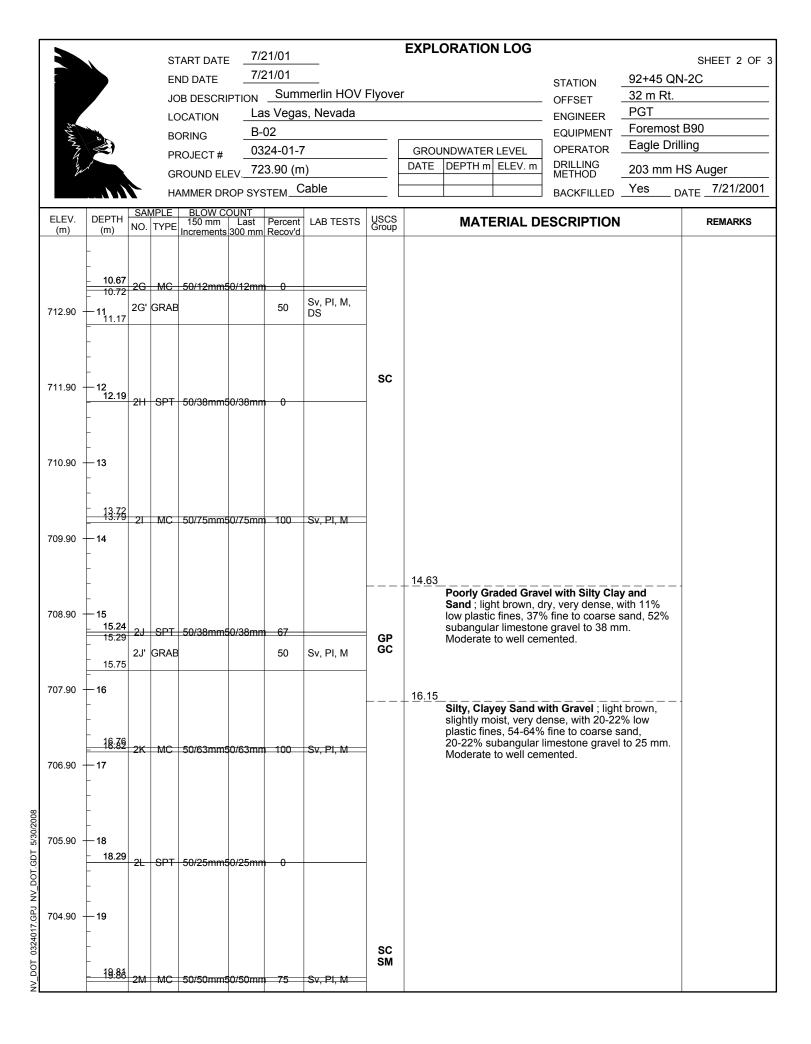
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		-									light brown 8-10% low	n grey, sligl / to mediun	ntly moist, very o n plastic fines, 3	dense, with 0-39% fine	
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		-									Lithology i	s based on	auger cuttings	which is	
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		3.1	Ŭ	GRAE		0/301111	50	DS							
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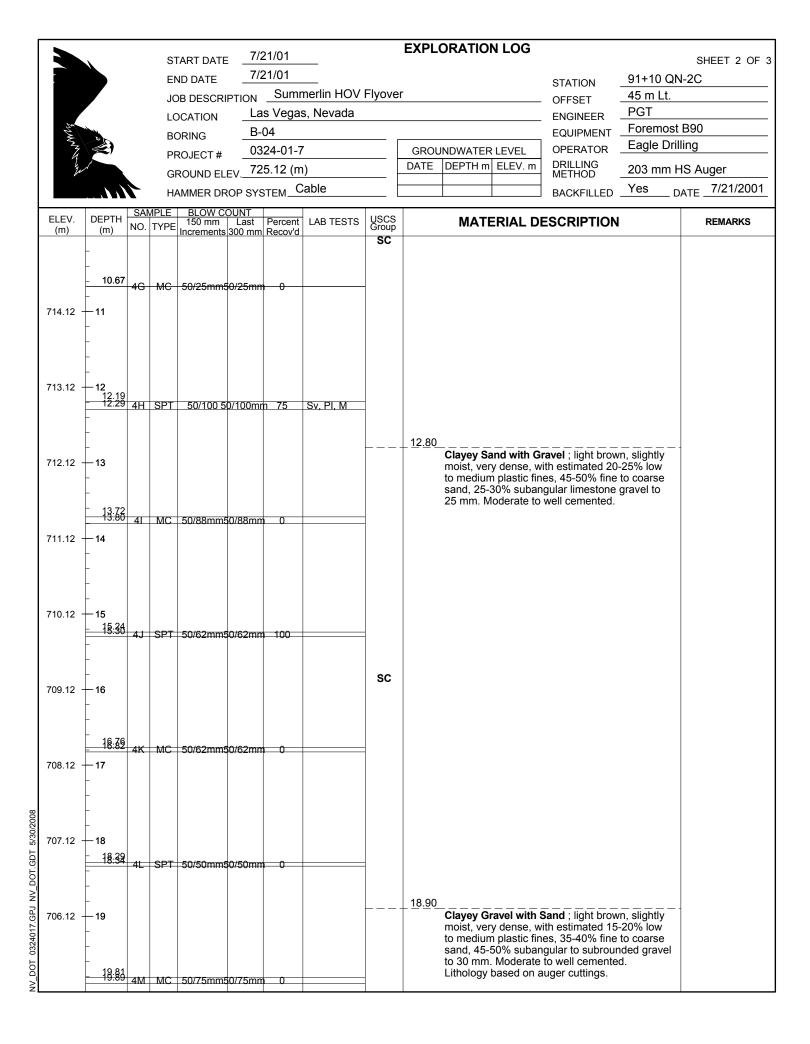
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		-									<b>Clayey Gravel with S</b> dry, very dense, with fines, 42% fine to coa	13% low to med rse sand, 45%	lium plastic subangular	-
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	Že			G	ROUND EL	EV. 72	25.12 (m	1)		DATE	DEPTH m	ELEV. M	METHOD	203 mm H	HS Auger
			1	H	AMMER DF	ROP SYS	STEM_C	Cable	[				BACKFILLED	Yes [	DATE 7/21/2001
ľ	ELEV. (m)	DEPTH (m)		MPLE TYPE	BLOW C 150 mm Increments	Last	Percent		USCS Group		MATE	ERIAL DI	ESCRIPTION		REMARKS
·	(111)				Increments	<u>300 mm</u>	Recova				Silty Grav	el with Sa	nd ; light grey br	own, dry,	
		-							GM	0.91	fines, 35% limestone 125-mm-d total soil n	6 fine to co gravel to 7 liameter co nass. Mode	mated 15% low arse sand, 40% 75 mm. Cobbles omprise about 10 erately cemented	subangular to )% of the	
	724.12 -	-1								T	caliche co	ating on co	bbles. with Silt and G	í	
		F									grey brow	n, dry, very	/ dense, with 10°	% low	
		1.52 <u>1.57</u>	4A	мс	50/50mm	<del>0/50mn</del>	100		SP		plastic fine	es, 49% fin ar limeston	e to coarse sand e gravel to 50 m	d, 41%	
		-	4A'	GRAE	3		50	Sv, PI, M	SM		Moderate	to well cen	nented. Gravel is	s sheared	
	723.12 -	<b>2</b> 2.03							_		by the aug hole.	ger. Calcar	eous cement thr	oughout	
		-								0.44					
		-								2.44	Well Grad	led Gravel	with Silt and Sa	nd ; light	-
											grey brow	n, dry, very	/ dense, with 9% arse sand, 48%	low plastic	
	722.12 -		4B	SPT	50/25mm	0/25mn					limestone	gravel to 5	50 mm. Moderate	e to well	
		3.10				<u>10/2</u> 51111					cemented				
		3.55	4B'	GRAE	9		50	Sv, PI, M							
		-							GW						
	721.12 -	-4							GM						
	721.12	4													
		-													
		4.57	4C	MC	50/25mm	<del>0/25mn</del>	n 0		-						
		-													
	720.12 -	-5								5.18					
		Ľ								Τ			nd ; light brown g		
		-									fines, 35-4	e, with esti 40% fine to	mated 15-20% lo coarse sand, 45	5-50%	
		-										ar to subrou loderately o	unded limestone	gravel to	
	719.12 -	6 6.10	4D	SPT	50/62mm	0/62mn	0				_0 mm. W		somenteu.		
		- 6.16					E0	20	GM						
		6.61	40	GRAE	1		50	DS							
		-													
	718.12 -	-7													
		F								7.32					
		- - <del>7</del> :6 <del>7</del>											<b>ravel</b> ; light brow 19% medium pla		
2008			4E	мс	50/50mm	50/50mn	n 100	Sv, Pl, M	-		48-55% fir	ne to coars	e sand, 27-33%		
5/30/;	717.12 -	-8										ar limeston to well cen	e gravel to 25 m nented	m.	
DT		-									moderate		lienteu.		
OT.G		+													
		F													
PJ N	716.12 -	Ĺ													
117.G	/10.12 -	-9 <u>8:1</u> 4	4F	SPT	50/62mm	0/62mn	n 60	Sv, Pl, M							
3240		-						, ,							
NV_DOT 0324017.GPJ NV_DOT.GDT 5/30/2008		-									Samples 4 analysis.	4F and 4H	were combined	for the lab	
z		I			L		1		1	1					1



						- 7/	21/01			EXPL	ORATIO	N LOG			
							21/01							01 10 01	SHEET 3 OF 3
					ND DATE			 merlin HOV	Flyove	r			STATION	91+10 QN 45 m Lt.	1-20
					DB DESCRI			s, Nevada	i iyovo				OFFSET	PGT	
	2m						04	o, 1101000					ENGINEER EQUIPMENT	Foremost	B90
	E Sala						324-01-7	7	[	GROU	INDWATEF		OPERATOR	Eagle Dril	ling
				PI	ROJECT # ROUND EL					DATE	DEPTH m		DRILLING METHOD	203 mm F	IS Auger
					ROUND EL AMMER DF			able	[						
									l				BACKFILLED	D	ATE 7/21/2001
	ELEV. (m)	DEPTH (m)	NO.	MPLE TYPE	BLOW C 150 mm Increments	Last	Percent	LAB TESTS	USCS Group		MAT	ERIAL DE	ESCRIPTION	l	REMARKS
	(11)				Increments	300 mm	Recova								
		F													
		-													
	704.12 -	-21							GC						
	101.12	21:34													
		21:40	4N	SPT	50/62mm	0/62mr	n 60		-						
		-													
	703.12 -	-22													
	100.12	-													
		-													
		-													
	702.12 -	- <u>22</u> :99	40	мс	50/38mm	50/38mr	n 0		-	22.91					
	102.12	- 20													
		-													
		-													
	701.12 -														
	701.12	- 24													
		-													
		-													
	700.12 -														
	700.12	- 20													
		-													
		-													
	699.12 -	- 26													
	099.12	- 20													
		-													
		-													
	698.12 -	- 27													
	090.12	- 21													
		-													
008		-													
/30/2	607 12 -	- 28													
DT 5,	697.12 -	- 28													
DT.GI		-													
N Ldi	606 12	20													
NV_DOT 0324017.GPJ NV_DOT.GDT 5/30/2008	696.12 -	- 29													
03240		-													
) T (		F													
N		-													

				61	FART DATE	. 11	/9/01			EXPLORATION LOG			SHEET 1 OF 4
							/9/01				07171011	14+50 R	SHEET I OF 4
					DESCRI		Sum	merlin HOV	Flyove	r	STATION OFFSET	90 m Rt.	
								s, Nevada			ENGINEER	MAM	
	M					B-	•				EQUIPMENT	Diedrich D	-50 turbo
	2 May				ROJECT #	03	24-01-7	7		GROUNDWATER LEVEL	OPERATOR	Eagle Drill	ing
	the second				ROUND EL	70	3.04 (n			DATE DEPTH m ELEV. m	DRILLING METHOD	152 mm H	IS Auger
					AMMER DR	L V		, Cable			BACKFILLED	Mar	ATE 11/9/2001
ļ								1		1	DACKFILLED	Di	
	ELEV. (m)	DEPTH (m)		MPLE TYPE	BLOW Co 150 mm Increments	Last	Percent Recov'd		USCS Group	MATERIAL DE WELL GRADED GRA			<b>REMARKS</b> Previously
	732.04 -	- - - - <u>1 1.07</u>		SPT	-50/25mm5	<del>0/25mn</del>			GW	and SAND tan, slight with 11% nonplastic to fine to coarse sand an gravel to 1-1/2". Mod cementation.	ly moist, very de o low plasticity f nd 47% fine sub	ense, ïnes, 42% prounded	reported as boring B-C.
		1.57		GRAB			50	Sv, PI, M, DS, Chem	GC				
	731.04 -	- 2								2.29			
		2.59								POORLY GRADED S			
		2.64		MC	75/25mm7	<del>5/25mn</del>	5		-	and GRAVEL light br dense, with 11% low	plasticity fines,	51% fine to	
	730.04 -	- 		GRAB			50	Sv, PI, M	SP	coarse sand and 38% subrounded gravels to			
	700.04	- 0 3.10							SC	cementation.	o i i modorato		
		-											
		-								3.66 SILTY, CLAYEY SAN			
	729.04 -	- 								slightly moist, very de	nse, with 13% I	ow	
	729.04	4 4.11		SPT	50/25mm5	0/25mm	5		-	plasticity fines, 44% fi 43% fine subrounded			
		-		GRAB			50	Sv, PI, M	SC SM	Moderate cementatio			
		4.62	2						SIVI				
	728.04 -	5											
	720.04 -									<u>5.18</u>			
		-								SILTY, CLAYEY GRA tan-grey, slightly mois	t, very dense, v	<b>)</b> vith 13%	
		5.64		мс	75/25mm7	5/25mm	5		=	low plasticity fines, 40 47% fine subrounded	% fine to coars	e sand and	
	707.04		D'	GRAB			50	Sv, PI, M		Moderate to strong ce		eis lu 1/2 .	
	727.04 -	6 6.14							_				
		-							GC GM				
		-							Givi				
		-											
	726.04 -	7 7.16	E	ST	50/25mm5	<del>0/25mn</del>	5						
		_					-						
08		-											
30/20		-								SILTY SAND with GR to moist, very dense,	with 16% nonpl	astic to low	
T 5/3	725.04 -	-8								plasticity fines, 46% fi 38% fine subrounded			
T.GD										1-1/2". Moderate cen		0.10	
		- 8.69		мс	75/25mm7	5/25mm	5						
N L		-			75/25mm7	Jazann							
NV_DOT 0324017.GPJ NV_DOT.GDT 5/30/2008	724.04 -	-9											
32401													
)Т 0,		-											
		-											
z		1	1	L	i	I	L	1	1	1			

						- 11	/9/01			EXPLORATION LOG			
					TART DATE		/9/01						SHEET 2 OF 4
					ND DATE			 merlin HOV	Elvove	.r	STATION	14+50 R 90 m Rt.	
					OB DESCRI			s, Nevada	TIYOVE		OFFSET	MAM	
	- Sur				OCATION		.07	S, NEVaua			ENGINEER	Diedrich D-	-50 turbo
	× N				ORING		.07 324-01-7	7		GROUNDWATER LEVEL	EQUIPMENT OPERATOR	Eagle Drilli	
	4				ROJECT #					DATE DEPTH m ELEV. m	DRILLING		
	be				ROUND EL	L V	33.04 (m	Cable			METHOD	152 mm HS Yes	1.1.10.10.0.0.1
				H.	AMMER DR		STEM_C	Jable			BACKFILLED	DA	TE 11/9/2001
	ELEV. (m)	DEPTH (m)	NO.	MPLE TYPE	BLOW CO 150 mm Increments	OUNT Last 300 mm	Percent Recov'd	LAB TESTS	USCS Group	MATERIAL DE	SCRIPTION		REMARKS
		- <u>10:2</u> 8	G	SPT	50/50mm5	0/50mn	n 11		-				
		-											
									SM				
	722.04	-11											
		-											
		-											
		11.73 - 11.78	н	мс	75/25mm7	<del>5/25mn</del>	n <u>5</u>						
	721.04	-12		GRAE	3		50	Sv, PI, M					
		_ 12.24							_				
		-											
	720.04	-13											
		-											
		+											
		-								13.72			
	719.04	-14								CLAYEY SAND with slightly moist to moist	, verv dense, w	ith 14-22%	
	713.04	-								low to medium plastic coarse sand and 24-4	city fines, 46-54	% fine to	
		-								limestone gravel to 1	". Moderate cal	careous	
		14.78			50/05-mm	0/05				cementation.			
	718.04	- 15		511	50/25mm5	<del>0/25mn</del>	n <u>5</u>	Sv, PI, M					
	/ 10.04	- 15											
		+											
		-											
	747.04												
	717.04	+ 16											
		-											
		-											
		- -											
	716.04	- 17											
		Ļ											
80		-											
30/20		17.83 17.88 18	J	мс	75/25mm7	5/25mn	n <u>5</u>		=				
T 5/3	715.04		J'	GRAE	3		50	Sv, PI, M					
T.GD		18.34							sc				
DO_/		F											
NV_DOT 0324017.GPJ NV_DOT.GDT 5/30/2008		+											
7.GF	714.04	- 19											
32401													
DT 00		F											
		F											
z		1	1	1	I		1	1	1				

						. 11	/9/01			EXPLORATION LOG			
							/9/01						SHEET 3 OF 4
					ND DATE			 merlin HOV	Flyove	r		14+50 R 90 m Rt.	
					DB DESCRI		-	s, Nevada	TIYOVE			<u>90 m rti.</u> MAM	
	An	· · · ·			DCATION		07	5, Nevaua				Diedrich D-5	50 turbo
	N. W.				ORING		324-01-7	7		GROUNDWATER LEVEL		Eagle Drillin	
	1 miles				ROJECT #					DATE DEPTH m ELEV. m	DRILLING	152 mm HS	
	Ve				ROUND EL	_ •	3.04 (m	able			METHOD		4.4.10.10.0.0.4
					AMMER DR		STEM_C	ane			BACKFILLED	Yes DAT	E
	ELEV. (m)	DEPTH (m)		MPLE TYPE	BLOW CO 150 mm Increments	Last	Percent Recovid	LAB TESTS	USCS Group	MATERIAL DI	ESCRIPTION		REMARKS
	712.04 -	- 20.88 - 21 			- 50/25mmS				-	22.86			
	710.04 -	- 23 - - - - - - - 23.93 - - 24 - - -	- <u>L</u> -	- MC-	<del>- 75/25mm7</del>	<del>'5/25mn</del>	<del>a 5</del>	<del>- Sv, Pl, M</del>	-	SILTY SAND with Gi slightly moist to mois nonplastic fines, 59% 26% fine subrounded	fine to coarse sar	nd and	
	708.04 -	- 25 - -											
	707.04 -	- 26 							SM				
DOT.GDT 5/30/2008	706.04 -	<del>29.83</del> - - - - - - - - - - - -	M	SPT	-50/50mm	\$0/50mn	- 11						
NV_DOT 0324017.GPJ NV_DOT.GDT 5/30/2008	704.04 -	- - - - - - - - - - - - - - - - - - -											

ſ						_ 11	/9/01			EXPL	ORATIO	N LOG			
					ART DAT		/9/01							44.50 0	SHEET 4 OF 4
					ID DATE			merlin HOV	Flyova	r			STATION	14+50 R 90 m Rt.	
					B DESCR		-	s, Nevada	TIYOVC	1			OFFSET	MAM	
	An				CATION	_	07	5, Nevaua					ENGINEER EQUIPMENT	Diedrich D	-50 turbo
	×.				RING		67 624-01-7	7	[		INDWATER		OPERATOR	Eagle Drill	
	T				ROJECT #		3.04 (m			DATE	DEPTH m		DRILLING METHOD	152 mm H	
					ROUND EL		-	able	[						
				HA	MMER DI		STEM_C		[				BACKFILLED	D.	ATE 11/9/2001
	ELEV.	DEPTH		/PLE TYPE	BLOW C 150 mm	Loot	Percent	LAB TESTS	USCS Group		MATE	ERIAL DE	SCRIPTION		REMARKS
ŀ	(m)	(m) 30.07	7 N	MC	Increments 75/50mm	<u>300 mm 75/50mn</u>	Recov'd		Group	30.07					
		-													
		-													
	702.04 -	-31													
		_													
		-													
	701.04 -	-32													
		-													
		-													
	700.04 -	-33													
		_													
		-													
		-													
	699.04 -	- 34													
		-													
		-													
	698.04 -	- 35													
		-													
		-													
	007.04	-													
	697.04 -	- 36													
		-													
		-													
	000.04														
	696.04 -	- 37													
		-													
308		-													
/30/2(	695.04 -														
DT 5	095.04 -	- 30													
NV_DOT 0324017.GPJ NV_DOT.GDT 5/30/2008		-													
		-													
N Ld;	694.04 -														
017.G	004.04 -	- 39													
0324(		F													
DOT		+													
		-													

				_		_ 11	/6/01			EXPL	ORATIO	N LOG			0
					TART DATE		/6/01							02.00.0	SHEET 1 OF 3
								merlin HOV	Flyove	r			STATION	93+88 Q 103 m Lt.	
								s, Nevada	1 190 90	'I			OFFSET	MAM	
	An.					 B-		5, NCVAUA					ENGINEER EQUIPMENT	Diedrich D	0-50 turbo
	× v				ORING		24-01-7	7		GPOL	NDWATER		OPERATOR	Eagle Dril	
	T				ROJECT #		7.62 (m			DATE	DEPTH m		DRILLING	152 mm H	
					ROUND EL	v		able					METHOD	Mar	11/0/0001
					AMMER DF		STEM	able					BACKFILLED	D	ATE <u>11/6/2001</u>
	ELEV. (m)	DEPTH (m)		MPLE TYPE	BLOW C 150 mm Increments	Last	Percent Recov'd		USCS Group				SCRIPTION		REMARKS
		-									brown, sli 14% low p	ghtly moist, plasticity fin	AVEL with SANE dense to very d es, 43% fine to o ubangular limest	ense, with coarse	Previously reported as boring B-D.
	736.62 -	<u>1 1.07</u> 1.27		SPT	46 50/50mm	50/50mm	44	Sv, PI, M	-						
		- 1.27			50/50mm`	J0/ J0/ IIII		00,11,10	-						
	735.62 -	- - 2							GC GM						
		-													
		2.59			==// 0.0 =										
		- 2.09	B	MC	75/100 7	5/100mr	n 0		-						
	734.62 -	-3													
		-								3.35					
	733.62 -	- - - - - - 4 4.11 - 4.29	-	SPT	49 ( 50/75mm	50/75mm	1 50	Sv, PI, M	GC		slightly me medium p sand and	oist, very de plasticity fine	ith SAND light tense, with 14% lense, with 14% les, 42% fine to cubangular limestentation.	ow to oarse	
	732.62 -	- - 5 -								5.18_	SILTY GR moist, ver	AVEL with y dense, wi	SAND light bro th 14% low plas	————— wn, ticity fines,	
		5.64		MC	50/125 5	0/125mr	n 28	Sv, PI, M,	-		42% fine 1	to coarse sa	and and 44% fin	е	
		-			00/1200	0/12011	1 20	DS, Chem	GM		cemented		e gravel to 1-1/2	. weii	
	731.62 -	6													
	730.62 -	- - - 7.16 - 7.26	E	SPT	50/100 5	0/100mr	n 22		SC SM	<u>6.40</u>	brown, mo plasticity f 32% fine s	oist, very de fines, 50% f subangular cementatio	ID with GRAVEI ense, with 18% le ine to coarse sa limestone grave n. Well cemente	ow Ind and el to 1-1/2".	
		-								7.62					
OT.GDT 5/30/2008	729.62 -	- - - 8 -							 		brown, sli 18% low p sand and	ghtly moist plasticity fin 44% fine to	AVEL with SANI to moist, very de es, 37% fine to o coarse subrour ". Moderate cer	ense, with coarse nded	
NV_DOT 0324017.GPJ NV_DOT.GDT 5/30/2008	728.62 -	- 8.69 8.79 - 9 - - -	F	MC	50/100 5	0/100mr	n 100	Sv, Pl, M	-						
z		1	1	1	1	1			1	1					L

						. 11	/6/01			EXPLORATION LOG			
					TART DATE		/6/01					00.000	SHEET 2 OF 3
	i				ND DATE			merlin HOV	Elvovo	r	STATION	93+88 Q	
					OB DESCRI		-		FlyOve	1	OFFSET	<u>103 m Lt.</u> MAM	
	- Am				OCATION		•	s, Nevada			ENGINEER	Diedrich D	-50 turbo
	No.				ORING		08	7			EQUIPMENT OPERATOR	Eagle Drilli	
	T.				ROJECT #		324-01-7			GROUNDWATER LEVEL DATE DEPTH m ELEV. m	DRILLING		
	le				ROUND EL	_ v	87.62 (m				METHOD	152 mm H	-
				H	AMMER DR	OP SYS	STEM_C	Cable			BACKFILLED	Yes DA	TE 11/6/2001
	ELEV. (m)	DEPTH (m)	SAI NO.	MPLE TYPE	BLOW CO 150 mm Increments	OUNT Last 300 mm	Percent Recov'd	LAB TESTS	USCS Group	MATERIAL DE	SCRIPTION		REMARKS
		10.21	G	SPT	50/25mm5	<del>0/25mn</del>	n <u>5</u>						
		-											
		F							GC GM				
	726.62 -	-11											
	120.02	F											
		-											
		11.73	н	MC	75/40mm7	5/40mm	10						
		-		INIC	75/4011117	<del>3/40mm</del>	10						
	725.62 -	12											
		-											
		-											
	724.62 -	-13											
		-								13.72			
		-								CLAYEY SAND with	GRAVEL light l	orown,	
	723.62 -	- 14								slightly moist to moist plasticity fines, 65% f	, very dense wi	th 14% low	
		F								21% fine to coarse su	ibrounded limes	stone	
										gravel to 2".			
		14.78	- 1	SPT	50/25mm5	<del>0/25mn</del>	n <u>5</u>		_				
	722.62 -	- 15											
		-											
		-											
		Ĺ											
	721.62	-16											
		F											
		F											
		Ĺ											
	720.62 -	<u> </u>											
		+											
		F											
908		17.83											
/30/2(	710 62		J	MC	75/25mm7	<del>'5/25mn</del>	n 5		1				
JT 5,	719.62 -								SC				
JT.GL		F											
		F											
رم ۲		F											
17.GF	718.62 -	19											
3240		L											
NV_DOT 0324017.GPJ NV_DOT.GDT 5/30/2008		F											
		F											
z		1	1	1	i		I	1	1	1			

						11	/6/01			EXPL	ORATIO	N LOG			
					FART DATE		/6/01								SHEET 3 OF 3
					ND DATE								STATION	93+88 Q	
					DB DESCRI			merlin HOV	Fiyove	ſ			OFFSET	103 m Lt.	
	- the	、 、		LC	OCATION			s, Nevada					ENGINEER	MAM Diedrich D	50 turbo
	2MM			BC	ORING		08	_	— r				EQUIPMENT	Eagle Drill	
	Z.			PF	ROJECT #		24-01-7					ELEVEL			
	Ĩ.			GI	ROUND EL	<sub>EV.</sub> _73	7.62 (m	1)	ł	DATE	DEPTHM	ELEV. M	DRILLING METHOD	152 mm H	
		<u> </u>		HA	AMMER DR	OP SYS	STEM_C	Cable	[				BACKFILLED	Yes D	ATE 11/6/2001
ľ	ELEV.	DEPTH		MPLE TYPE	BLOW CO	OUNT Last	Percent	LAB TESTS	USCS Group		ΜΑΤ	ERIAL DE	SCRIPTION		REMARKS
-	(m)	(m)	NO.	TTPE	150 mm Increments	<u>300 mm</u>	Recov'd		Group						
		_													
		- 20.88													
	716.62 -	-21	ĸ	ST	50/25mm5	<del>0/25mn</del>	5	Sv, Pl, M	-						
		-													
		_													
		_													
	715.62 -	-22													
		_													
		22.40	L	мс	75/25mm7	<del>5/25mn</del>	5			22.40					
		_													
	714.62 -														
	/ 14.02 -	-23													
		_													
		_													
		-													
	713.62 -	-24													
		-													
		-													
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SHATE TARE         11/7/01         SHEET 1 OF           END DATE         11/7/01         STATON         13/35 R.           UOD DESCRIPTION         Las Vogas, Novada         STATON         13/35 R.           BORNIG         8/09         Control         13/35 R.           PROJECT #         03/2401-7         Control         Edge Dolling           CROUNDERUTE         CONVERT         Convert         Convert           CROUNDERUTE         CONVERT         Convert         Convert         Convert           CROUNDERUTE         Convert         Convert         Convert         Convert         Convert           CROUNDERUTE         Convert         Convert <td< th=""><th>[</th><th></th><th></th><th></th><th></th><th></th><th>_ 11</th><th>/7/01</th><th></th><th></th><th>EXPLORATION LOG</th><th></th></td<>	[						_ 11	/7/01			EXPLORATION LOG		
END UATE         Summerlin HOV Flyover         STATUS         Status           JOB DESCRIPTION         Los Vegas, Nevada         OFFSET         STATUS         JOB DESCRIPTION         IS m RL           DOR DESCRIPTION         Los Vegas, Nevada         Diedrich D-50 turbo         Diedrich D-50 turbo         Cegarator         Eagle Drilling           PROJECT #         0224-01-7         GROUND ELEV. 723.90 (m)         DATE         DATE         DIEdrich D-50 turbo         OPERATOR         Eagle Drilling           MATERIA DESCRIPTION         ISB MMER DROP SYSTEM         Cable         DATE         DATE         DIEdrich D-50 turbo         OPERATOR         Eagle Drilling           MATERIA DESCRIPTION         ISB MMER DROP SYSTEM         Cable         MATERIA DESCRIPTION         REMARKS           T22.80         1.07         A SPT 50/25mm50/25mm         Set Status         Set Status         POORLY GRAPE GRAVEL with CLAY and SAND light brown, slightly most, very dense, with 1% by or medium plasticity lines, slightly most, very dense, with 1% by or medium plasticity lines, slightly most, very dense, with 1% by or medium plasticity lines, slightly most, very dense, with 1% by or medium plasticity lines, slightly most, very dense, with 1% by or medium plasticity lines, slightly most, very dense, with 1% by or medium plasticity lines, slightly most, very dense, with 1% by or medium plasticity lines, slightly most, very dense, with 1% by or medium plasticity lines, slightly most, very dense, with 1% by oremediu 2% line to coarse sand											40.05	SHEET 1 OF 3	
MAM         MAM         BORING       BORING       MAM         BORING       MAM         BORING       MAM         BORING       MAM         BORING       MAM         GROUNDWATER LEVEL       OPERATOR       Called         MAM         Colspan="2">OPERATOR       Called         MAM       Colspan="2">OPERATOR       Called         MAM       Colspan="2">OPERATOR       Called         MAM       Colspan="2">Colspan="2">Colspan="2">Colspan="2">Colspan="2">Colspan="2">Colspan="2">Colspan="2">Colspan="2">Colspan="2">Colspan="2">Colspan="2">Colspan="2">Colspan="2">Colspan="2">Colspan="2">Colspan="2">Colspan="2">Colspan="2">Colspan="2">Colspan="2">Colspan="2">Colspan="2">Colspan="2">Colspan="2">Colspan="2">Colspan="2">Colspan="2">Colspan="2">Colspan="2">Colspan="2">Colspan="2">Colspan="2">Colspan="2">Colspan="2">Colspan="2">Colspan="2">Colspan="2">Colspan="2">Colspan="2">Colspan="2">Colspan="2">Colspan="2">Colspan="2">Colspan="2">Colspan="2">Colspan="2">Colspan="2">Colspan="2">Colspan="2">Colspan="2">Colspan="2">Colspan="2">Colspan="2">Colspan="2">Colspan="2">Colspan="2">Colspan="2">Colspan="2">Colspan="2">Colspan="2">Colspan="2">Colspan="2">Colspan="2">Colspan="2">Colspan="2"         Colspan="2" <th cols<="" td=""><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td>-</td><td> morlin ⊔∩\/</td><td>Flyovo</td><td>r</td><td></td></th>	<td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td>-</td> <td> morlin ⊔∩\/</td> <td>Flyovo</td> <td>r</td> <td></td>								-	 morlin ⊔∩\/	Flyovo	r	
DUCKTION     BC 00       BORNO     B-00       PROLECT #     0324-01-7       GROUNDWATER LEVEL     DEFINING       BORNO     B-00       MAMER DROP SYSTEM     Cable       DATE     DEPTIN       DATE     DEPTIN       MAMER DROP SYSTEM     Cable       DEPTIN     BCONCOUNT       MAMER DROP SYSTEM     Cable       DATE     DEPTIN       DEPTIN     BCONCOUNT       MATERIAL DESCRIPTION     REMARKS       POORLY GRADED GRAVEL With BLT     Providually most differences and and 55% fine to coarse and and 45% fine to c										FIYOVE	OFF3ET	.[	
BOKING     C.S.       PROLICIT #     0324-01-7 GROUND ELEY_723.90 (m) HAMMER DROP SYSTEM_Cable     GROUNDWATER LEVEL DATE DEPTH     OPERATOR BELLNG METHOD     152 mm HS Auger       ELEV     DEPTH     SAME     SOME CASL     MATERIAL DESCRIPTION     Eagle Drilling       ELEV     DEPTH     SAME SUM COUNT     Lab TEST     Visco     Visco     MATERIAL DESCRIPTION     REMARKS       722.90     1.07     A GRA     SO SV, PI, M     GP     GP     Cobles to 4° comprise an estimated 15% of the to coarse sand and d55% file to coarse sine and difference and		, pm							S, INCVAUA		ENGINEER	h D-50 turbo	
PROJECT #     OUZHOF       GROUND ELEV_72300 (m) HAMMER DROP SYSTEM_Cable     DEI     DEI <td></td> <td>2 Mrs</td> <td></td> <td></td> <td>BC</td> <td>ORING</td> <td></td> <td></td> <td>7</td> <td> [</td> <td></td> <td></td>		2 Mrs			BC	ORING			7	[			
Image: construct of the construction of the constrest on the constructing of the construction of the construction o		4						-					
ELEV.     DEPTH (m)     BAMPLE (m)		le			GI	ROUND EL	EV72		,		METHOD 152 III		
ELEX     DEPTH (m)     No     TYPE     150 mm     Last     Percent     Last     Percent     Last     Percent     Percent       (m)					HA	AMMER DR	ROP SYS	STEM_C	able		BACKFILLED Yes	_ DATE11/7/2001	
722.00       1.07 1.12 1.12 1.12 1.12 1.12 1.12 1.12 1.1	Ī					150 mm	Last	Percent Recov'd	LAB TESTS	USCS Group		REMARKS	
721.90       2       1.98         721.90       2.59       0       MC       75/100 75/100mn - 11         720.90       3       3.15       B'       GRAB       50       Sv, PI, M, DS, Chem         719.90       4       4.11       C       SPT       50/15mm50/15mm - 3       status         719.90       4       4.11       C       SPT       50/15mm50/15mm - 3       status         719.90       4       4.11       C       SPT       50/15mm50/15mm - 3       status         719.90       4       4.11       C       SPT       50/15mm50/15mm - 3       status         718.90       5       50       Sv, PI, M       Sc       Sc       3.81         718.90       5       5       Sv, PI, M       Sc       Sc       St         718.90       5       5       Sv, PI, M       Sc       Sc       St       St         718.90       5       5       Sv, PI, M       Sc       Sc       St		722.90	1.12				<del>50/25mn</del>		Sv, PI, M		CLAY and SAND light brown, slightly moist, dense to very dense, with 10% low to medium plasticity fines, 35% fine to coarse sand and 55% fine to coarse limestone gravel to 3". Cobbles to 4" comprise an estimated 15% of	reported as	
721.90       -2       -2       -2       -2       -2       -2       -3       -2       -3       -2       -3       -2       -3       -2       -3       -2       -3       -2       -3       -2       -3       -2       -3       -2       -3       -3       -3       -3       -3       -3       -3       -3       -3       -3       -3       -3       -3       -3       -3       -3       -3       -3       -3       -3       -3       -3       -3       -3       -3       -3       -3       -3       -3       -3       -3       -3       -3       -3       -3       -3       -3       -3       -3       -3       -3       -3       -3       -3       -3       -3       -3       -3       -3       -3       -3       -3       -3       -3       -3       -3       -3       -3       -3       -3       -3       -3       -3       -3       -3       -3       -3       -3       -3       -3       -3       -3       -3       -3       -3       -3       -3       -3       -3       -3       -3       -3       -3       -3       -3       -3       -3			1.57										
721.90       -2       -2       WELL GRADED GRAVEL with CLAY and SAND light brown, slightly moist, very dense, with 11% low plasticity lines, 43% fine to coarse and and 40% fine subangular limestone gravel to 3/4". Moderate cementation.         720.90       -3       3.15       B'       GRAB       50       Sv, PI, M, DS, Chem         719.90       -4       4.11       C       SPT       50/15mm50/15mm       3         719.90       -4       4.11       C       SPT       50/15mm50/15mm       3         719.90       -4       4.11       C       SPT       50/15mm50/15mm       3         719.90       -5       C       GRAB       50       Sv, PI, M       Sc         718.90       -5       -5       -5       -5       -5       -5         718.90       -5       -5       -5       -5       -5       -5         718.90       -5       -5       -5       -5       -5       -5         718.90       -5       -5       -5       -5       -5       -5         718.90       -5       -5       -5       -5       -5       -5       -5         717.90       -6       6.14       D'       GRAB       50       Sv, PI, M       5			-								1.98		
- 2.59       - MC       75/100 75/100 mm       11         720.90       -3       3.15       B' GRAB       50       Sv. Pl. M. DS, Chem       Sw         719.90       -4       4.11       -       -       -       -       -         719.90       -4       4.11       -       -       -       -       -       -         719.90       -4       4.11       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -		721.90	<u>_</u> 2								WELL GRADED GRAVEL with CLAY and		
720.90       3       3.15       B       GRAB       50       Sv. PI, M. DS, Chem       GW       GW       3.81         719.90       4       4.11       50       Sv. PI, M. DS, Chem       3.81       3.81         719.90       4       4.11       50       Sv. PI, M. So, Chem       3.81       3.81         719.90       4       4.11       50       Sv. PI, M. So, Chem       3.81       3.81         719.90       4       6.12       GRAB       50       Sv. PI, M. So, PI, M. So, Chem       SC       3.81         718.90       -5       -5       -5       -50       Sv. PI, M. So, PI, M. So, PI, M. So, So, PI, PI, M. So, PI, PI, M. So, PI, PI,											with 11% low plasticity fines, 43% fine to coars	e	
720.90       -3       3.15       B' GRAB       50       Sv, Pl, M, DS, Chem       GC         719.90       -4       4.11       C       SPT       50/15mm50/15mm       3.81         719.90       -4       4.11       C       SPT       50/15mm50/15mm       3.81         719.90       -4       4.11       C       SPT       50/15mm50/15mm       3.81         719.90       -4.62       GRAB       50       Sv, Pl, M       Sc       3.81         718.90       -5       -       -       -       -       -         718.90       -5       -       -       -       -       -         718.90       -5       -       -       -       -       -       -         718.90       -5       -       -       -       -       -       -       -         717.90       -6       6.14       C       50/25mm50/25mm       5       Sv, Pl, M       SM       SILTY, CLAYEY SAND with GRAVEL       -         717.90       -6       6.14       C       GRAB       50       Sv, Pl, M       SM       SILTY, CLAYEY SAND with GRAVEL       -         716.90       -7       7.16       -				в	мс	75/100 7	<del>5/100mi</del>	n 11				əl	
720.90       -3       3.15       JS, Chellin       JS, Chellin       JS         719.90       -4       4.11       C       SPT       50/15mm50/15mm       3.81         719.90       -4       4.11       C       SPT       50/15mm50/15mm       3.81         719.90       -4       4.11       C       SPT       50/15mm50/15mm       3.81         719.90       -4       4.12       C       GRAB       50       Sv, PI, M       Sc         718.90       -5       -5       -5       -5       -5       -5       -5       -5         718.90       -5       -5       -5       -5       -5       -5       -5       -5         717.90       -6       6.14       -50/25mm50/25mm       -5       Sv, PI, M       Sc       St       St       St         717.90       -6       6.14       -50/25mm50/25mm       -5       Sv, PI, M       Sc       St       St       St       St       St       -5.18       St       St       -5.18       St       -5.18       St       -5.18       St       -5.40%       For coarse sand and 27% fine to coarse sand and			-	В'	GRAB			50		-			
719.90       4 4.11       C       SPT       50/15mm50/15mm       3         4.62       C'       GRAB       50       Sv, PI, M       Sc       Sc       43% fine to coarse sand and 42% fine to coarse subangular limestone gravel to 3". Unit contains an estimated 10% subangular cobbles to 4". Moderate cementation.         718.90       5       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -<		720.90	3 3.15	_					DS, Chem	GC			
719.90       4 4.11       C       SPT       50/15mm50/15mm       3         4.62       C'       GRAB       50       Sv, PI, M       Sc       Sc       43% fine to coarse sand and 42% fine to coarse subangular limestone gravel to 3". Unit contains an estimated 10% subangular cobbles to 4". Moderate cementation.         718.90       5       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -<			-										
719.90       4 4.11       c       SPT 50/15mm50/15mm 3         4.62       50       Sv, PI, M       Sc         718.90       -       -       -         -       4.62       50       Sv, PI, M         718.90       -       -       -         -       -       -       -         -       -       -       -         -       -       -       -         -       -       -       -         -       -       -       -         -       -       -       -         -       -       -       -         -       -       -       -         -       -       -       -         -       -       -       -         -       -       -       -       -         -       -       -       -       -       -         -       -       -       -       -       -       -         -       -       -       -       -       -       -         -       -       -       -       -       -       -         -			[										
4.16       0       01       0       0       4.16       0       01       0       0       4.62       C       GRAB       50       Sv, Pl, M       Sc       sc       4.3% fine to coarse sand and 42% fine to coarse sand and 50% fine to coarse sand and 50% fine to coarse sand and 27% fine to coarse sand and 29.33% fine to coarse gravel to 3". Moderate cementation.		719.90	4 4.11		0.007						CLAYEY SAND with GRAVEL light brown-arey slightly moist very dense with		
4.62     0     0     0     0     0     0     0     0     0     0     0     0     0     0     0     0     0     0     0     0     0     0     0     0     0     0     0     0     0     0     0     0     0     0     0     0     0     0     0     0     0     0     0     0     0     0     0     0     0     0     0     0     0     0     0     0     0     0     0     0     0     0     0     0     0     0     0     0     0     0     0     0     0     0     0     0     0     0     0     0     0     0     0     0     0     0     0     0     0     0     0     0     0     0     0     0     0     0     0     0     0     0     0     0     0     0     0     0     0     0     0     0     0     0     0     0     0     0     0     0     0     0     0     0     0     0     0     0     0     0     0     0     0     0     0				C			<del>)0/15mn</del>				estimated 14% medium to high plasticity fines		
718.90       -5       an estimated 10% subangular cobbles to 4". Moderate cementation.         718.90       -5       -5         -5       -5       -5         -5,64       -5,69       -5,69         -5,69       -6,6,14       -5         -6       -6,14       -50/25mm50/25mm -5         -50       SV, PI, M       SC         SM       SILTY, CLAYEY SAND with GRAVEL tan-light brown, slightly moist to moist, very dense, with 13% low plasticity fines, 60% fine to coarse sand and 27% fine to coarse subrounded limestone gravel to 2". Moderate calcareous cementation.         716.90       -7       -7         -7       -7       -6         -7       -7       -6         -7       -7       -6         -7       -7       -6         -7       -7       -6         -7       -7       -6         -7       -7       -6         -7       -7       -6         -7       -7       -6         -7       -7       -6         -7       -7       -6         -7       -7       -6         -7       -7       -7         -7       -7       -6         -7			4.62	C'	GRAB			50	Sv, PI, M	SC	subangular limestone gravel to 3". Unit contain		
718.90       -5       5.64       5.64       5.64       5.64       5.64       5.64       5.64       5.64       5.64       5.64       5.64       5.64       5.64       5.64       5.64       5.64       5.64       5.64       5.64       5.64       5.64       5.64       5.64       5.64       5.64       5.64       5.64       5.64       5.64       5.64       5.64       5.64       5.64       5.64       5.64       5.64       5.64       5.64       5.64       5.64       5.64       5.64       5.64       5.64       5.64       5.64       5.64       5.64       5.64       5.64       5.64       5.64       5.64       5.64       5.64       5.64       5.64       5.64       5.64       5.64       5.64       5.64       5.64       5.64       5.64       5.64       5.64       5.64       5.64       5.64       5.64       5.64       5.64       5.64       5.64       5.64       5.64       5.64       5.64       5.64       5.64       5.64       5.64       5.64       5.64       5.64       5.64       5.64       5.64       5.64       5.64       5.64       5.64       5.64       5.64       5.64       5.64       5.64       5.64											an estimated 10% subangular cobbles to 4".		
717.90       -5.64       D       MC       50/25mm50/25mm       5         717.90       -6       6.14       D'       GRAB       50       Sv, PI, M       SC       SM         717.90       -6       6.14       D'       GRAB       50       Sv, PI, M       SC       SM       SM       Subrounded limestone gravel to 2". Moderate calcareous cementation.         716.90       -7       7.16       -       -       -       -       -         716.90       -7       7.16       -       -       -       -       -         716.90       -7       7.16       -       -       -       -       -         716.90       -7       7.16       -       -       -       -       -         -       -       -       -       -       -       -       -       -         -       -       -       -       -       -       -       -       -       -       -         -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       - <td></td> <td>718.90</td> <td>-5</td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td>		718.90	-5										
717.90       -       5.64       D       MC       50/25mm50/25mm       5         717.90       -       6       6.14       D'       GRAB       50       Sv, PI, M       SC       SM         717.90       -       6       6.14       D'       GRAB       50       Sv, PI, M       SC       SM       SM       tan-light brown, slightly moist to moist, very dense, with 13% low plasticity fines, 60% fine to coarse subrounded limestone gravel to 2". Moderate calcareous cementation.         716.90       -       -       -       -       -       -       -         716.90       -       7       -       -       -       -       -       -         716.90       -       7       -       -       -       -       -       -         -       -       -       -       -       -       -       -       -         -       -       -       -       -       -       -       -       -       -         -       -       -       -       -       -       -       -       -       -         -       -       -       -       -       -       -       -       -       -			F										
717.90     0     MC     50/25mm 50/25mm 50/25mm 5       6     0'     GRAB     50     Sv, PI, M       717.90     6     6.14     50     Sv, PI, M       717.90     6     6.14     6.40       716.90     7     7.16     6.40       716.90     7     7.16     6.40			- 								tan-light brown, slightly moist to moist, very	_	
717.90       6       6.14       D'       GRAB       50       Sv, PI, M       SM       subrounded limestone gravel to 2". Moderate calcareous cementation.         717.90       6       6.14       D'       GRAB       6       6.40         716.90       7       7.16       6       6.40       6.40         716.90       7       7.16       6       6.40         716.90       7       7.16       6       6.40         716.90       7       7.16       7       6         716.90       7       7.16       7       7.16         7       7.29       E       SPT       50/125 50/125mm 28       Sv, PI, M         7       7.29       E       SPT       50/125 50/125mm 28       Sv, PI, M				D	MC	50/25mm5	<del>0/25mn</del>	15			coarse sand and 27% fine to coarse	0	
716.90     7     7.16       6.40     6.40       6.40       6.40       6.40       716.90       7       7.16       7.16       7.29       E       SPT       50/125       50/125       50/125       50/125       50/125       50/125       50/125       50/125       50/125       50/125       50/125       50/125       50/125       50/125       50/125       50/125       50/125       50/125       50/125       50/125       50/125       50/125       50/125       50/125       50/125       50/125       50/125       50/125       50/125       50/125       50/125       50/125       50/125       50/125       50/125       50/125       50/125       50/125       50/125       50/125       50/125       50/125       50/125       50/125       50/125       50/125 <td></td> <td>717.90</td> <td>- 6</td> <td>D'</td> <td>GRAB</td> <td>8</td> <td></td> <td>50</td> <td>Sv, PI, M</td> <td>SM</td> <td>subrounded limestone gravel to 2". Moderate</td> <td></td>		717.90	- 6	D'	GRAB	8		50	Sv, PI, M	SM	subrounded limestone gravel to 2". Moderate		
716.90       7       7.16         716.90       7       7.16         716.90       7       7.16         7       7.29       E       SPT         50/125       50/125       50/125       50/125         50/125       50/125       50/125       50/125         7       7.16       7.16       7.16         7       7.29       E       SPT       50/125         50/125       50/125       50/125       Sv. PI, M       Moderate cementation.			- 0.14										
715.90       -8         -8       -         -8       -         -8       -         -8       -         -8       -         -8       -         -8       -         -8       -         -8       -         -8       -         -8       -         -7       -         -9       -         -       -         -       -         -       -         -       -         -       -         -       -         -       -         -       -         -       -         -       -         -       -         -       -         -       -         -       -         -       -         -       -         -       -         -       -         -       -         -       -         -       -         -       -         -       -         -       -	008	716.90	/.16	E	SPT	50/125 5	0/125mr	n 28	Sv, Pl, M	-	CLAYEY SAND with GRAVEL light brown-tan, slightly moist to moist, very dense, with 17-21% medium plasticity fines, 49-50% fine to coarse sand and 29-33% fine to coarse		
Image: 1000 million     Image: 1000 million     Image: 1000 million     Image: 1000 million       Image: 1000 million     Image: 1000 million     Image: 1000 million     Image: 1000 million       Image: 1000 million     Image: 1000 million     Image: 1000 million     Image: 1000 million       Image: 1000 million     Image: 1000 million     Image: 1000 million     Image: 1000 million       Image: 1000 million     Image: 1000 million     Image: 1000 million     Image: 1000 million       Image: 1000 million     Image: 1000 million     Image: 1000 million     Image: 1000 million       Image: 1000 million     Image: 1000 million     Image: 1000 million     Image: 1000 million       Image: 1000 million     Image: 1000 million     Image: 1000 million     Image: 1000 million       Image: 1000 million     Image: 1000 million     Image: 1000 million     Image: 1000 million       Image: 1000 million     Image: 1000 million     Image: 1000 million     Image: 1000 million       Image: 1000 million     Image: 1000 million     Image: 1000 million     Image: 1000 million       Image: 1000 million     Image: 1000 million     Image: 1000 million     Image: 1000 million       Image: 1000 million     Image: 1000 million     Image: 1000 million     Image: 1000 million       Image: 1000 million     Image: 1000 million     Image: 1000 million     Image: 1000 million	/30/2	715.00	L a										
0100 N     8.69     F     MC     75/50mm75/50mm     11       714.90     -9     -     -     SC	DT 5,	115.90	-0										
No.     8:92     F     MC     75/50mm75/50mm     11       714.90     -9     -     -     -     -       -     -     -     -     -     -       -     -     -     -     -     -       -     -     -     -     -     -       -     -     -     -     -     -	DT.GL		F										
714.90     -9     -     -     SC	V_DC		8.62	F	MC	75/50mm		11					
9     714.90     -9       10     -       10     -       10     -       10     -       10     -       10     -       10     -       10     -       10     -       10     -       10     -       10     -       10     -       10     -       10     -       10     -       10     -       10     -       10     -       10     -       10     -       10     -       10     -       10     -       10     -       10     -       10     -       10     -       10     -       10     -       10     -       10     -       10     -       10     -       10     -       10     -       10     -       10     -       10     -       10     -       10     -       10     -       10     -	N N	744.00	6				0,001111						
	17.GI	/14.90	_9										
	3240		F							SC			
	OT 0		F										
z			<u> </u>										

ſ				0.		- 11	1/7/01			EXPLORATION LOG			
					TART DATE		1/7/01				12 1		SHEET 2 OF 3
	Ì				ND DATE			 merlin HOV	Flyove	-	15 ATION	35 R n Rt.	
								s, Nevada	i iyovo				
	2m				OCATION		-09	0, 1107000				drich D-50	) turbo
	× v						324-01-7	7	[			le Drilling	
	1				ROJECT #		23.90 (m			DATE DEPTH m ELEV. m	DRILLING 150	mm HS A	luger
	be				ROUND EL	L V	-	Cable		N			4.4./=/0.0.0.4
ļ					AMMER DF		STEM		l	E	BACKFILLED Yes	DATE	
	ELEV. (m)	DEPTH (m)		MPLE TYPE	BLOW C 150 mm Increments	Last	Percent Recov'd	LAB TESTS	USCS Group	MATERIAL DES	CRIPTION		REMARKS
		- <u>18:21</u>	G	SPT	50/75mm	0/75mr	n 17	Sv. Pl. M	_				
		-											
		-											
	712.90 -	-11											
	112.00												
		-											
		11:78	ш	MO	75/50mm	75/50mr	11						
	711.90 -	- 12		IVIC	7.0/0011111	3/30111							
	711.00	- '2								12.19			
		-								WELL GRADED SAND GRAVEL light brown, n	noist, very dense, w	vith	
		-								12% low plasticity fines, sand and 35% fine to co	, 53% fine to coarse	•	
	710.90 -	- 13								limestone gravel to 2'.	Moderate cementat	ion.	
	710.00												
		-											
		-											
	700.00	-											
	709.90 -	- 14							SW				
		Ļ							SC				
		14.78											
		14.98		SPT	49 50/50mm	50/50mr	n 44	Sv, PI, M					
	708.90 -	- 15			- 50/5011111								
		[											
		-											
		-											
	707.90 -	- 16								16.15			
		Ĺ								CLAYEY GRAVEL with moist, very dense, with	SAND light brown	, low	
		-								plasticity fines, 30-35 fir	ne to coarse sand a	nd	
		-								50% fine to coarse subr	rounded gravel to 3	".	
	706.90 -	- 17											
		-											
~													
/2008		17.83											
5/30	705.90 -	17.96 	J	MC	75/125 7	<u>5/125m</u>	m 28		_				
GDT		-											
DOT.		t											
N		[											
GPJ	704.90 -	- 19											
4017.		+							GC				
NV_DOT 0324017.GPJ NV_DOT.GDT 5/30/2008		F											
DOT		Ľ											
2 Z													

ſ						11	/7/01			EXPL	ORATIO	N LOG			
					FART DATE		/7/01								SHEET 3 OF 3
					ND DATE					_			STATION	13+35 R	
					DB DESCR		-	merlin HOV	Fiyove				OFFSET	15 m Rt.	
	-she			LC	OCATION			s, Nevada					ENGINEER	MAM Diedrich D	
	2MM	6		BC	ORING		09	_	— r				EQUIPMENT	Eagle Dril	
	Z.			PF	ROJECT #		24-01-7				NDWATEF				
	Ĩ.			GI	ROUND EL	EV72	.3.90 (m	1)	ł	DATE	DEPTHM	ELEV. M	DRILLING METHOD	152 mm H	
			1	HA	AMMER DF	ROP SYS	STEM_C	able	[				BACKFILLED	Yes D	ATE 11/7/2001
ŀ	ELEV.	DEPTH		MPLE	BLOW C	OUNT	Percent	LAB TESTS	USCS Group		МАТ		SCRIPTION		REMARKS
	(m)	(m)	NO.	TYPE	150 mm Increments	300 mm	Recov'd	2.012010	Group						
		-													
		-													
		-													
	702.90 -	- 20.88 20.98 - 21	К	SPT	50/100 5	0/100mr	n 22		-						
	102.00														
		-													
		-													
		-													
	701.90 -	-22													
		22.40	L	мс	75/15mm	75/15mn	3			22.40					
		-													
		-													
	700.90 -	-23													
		-													
		_													
	699.90 -	-24													
		-													
		-													
		-													
	698.90 -	-25													
	000.00														
		-													
		-													
		-													
	697.90 -	-26													
		[													
		-													
		-													
	696.90 -	-27													
		-													
<u>م</u>		Ĺ													
/2008		_													
5/30	695.90 -	-28													
GDT		F													
DOT.		-													
GPJ	694.90 -	-29													
NV_DOT 0324017.GPJ NV_DOT.GDT 5/30/2008		-													
0324		$\vdash$													
ĎŢ		F													

						11	/6/01			EXPL	ORATIO	N LOG			
					TART DATE		/6/01							10.555	SHEET 1 OF 3
					ND DATE			morlin LIOV	Elver	r			STATION	12+55 R	
					OB DESCR			merlin HOV	FIYOVE	1			OFFSET	<u>33 m Rt.</u> MAM	
	-bn				OCATION		is vega 10	s, Nevada					ENGINEER	Diedrich E	)-50 turbo
	a way				ORING		24-01-7	7			NDWATER		EQUIPMENT OPERATOR	Eagle Dril	
					ROJECT #		-			DATE	DEPTH m		DRILLING		-
	Ve				ROUND EL		.9.39 (m	) Cable		5,2			METHOD	152 mm H	44/0/0004
			1	Н	IAMMER DF	ROP SYS	STEM	able					BACKFILLED	Yes D	ATE 11/6/2001
	ELEV. (m)	DEPTH (m)	I NO	MPLE	BLOW C 150 mm Increments 50/50mm	Last	Percent Recov'd	LAB TESTS	USCS Group				SCRIPTION		REMARKS
		0.0		GRA		J0/J0/IIII	50				brown, sli	ghtly moist,	ID with GRAVEI very dense, wit	h 14% low	Previously reported as
		- 0.5									plasticity f	ines, 50% f	ine to coarse sa ubangular limesi	nd and	boring B-F.
											gravels to	1-1/2". Mo	oderate calcareo		
	728.39	+1 1.0	7 B	ent	50/25mm	E0/25mm			sc		cementati	on.			
		1.1	2 -			<del>90/25mi</del>		Sv, PI, M,	SM						
		1.5	8' 7	GRAI	В		50	DS, Chem							
	727.39	2								2.13					
		-								2.13	WELL GR		ID with CLAY a	nd	
		2.5	9								GRAVEL	light brown th 9% low t	n, slightly moist, o medium plastic	very city fines	
		2:6	5 C	МС	75/50mm	/5/50mn	11		sw		61% fine t	to coarse s	and and 30% fin	e to coarse	
	726.39	-3 <sub>3.1</sub>	2 C'	GRA	В		50	Sv, PI, M	SC			l; difficult dr	e gravels to 1-1/2 illing.	2°. vveli	
		-											-		
		-								2.00					
										3.66	WELL GR	ADED GR	AVEL with SILT	and	
	725.39	4 4 1	1								SAND tar	n, slightly m	oist, very dense es, 42% fine to o	, with	
		4:1	₿D.	SPT	50/50mm	50/50mn	11		GW		sand and	48% fine to	coarse subang	ular	
		4.6	D'	GRA	В		50	Sv, PI, M	GM		limestone drilling.	gravels to	2". Well cement	ted; difficult	
		- 4.0	0						-	4.88	5				
	724.39	-5								4.00			VEL with SILT		
		-											vn, slightly moist		
		- 56									fine to coa	arse sand a	nd 47% fine to c	coarse	
		<u>5.7</u>	4 4 E	МС	75/75mm	75/75mn	17		GW				e gravels to 1-1/2 cementation.	ζ.	
	723.39	-6	E'	GRA	в		50	Sv, PI, M	GC						
		6.2	:0						-						
		-													
		Ĺ								6.71			RAVEL with SI	LT and	
	722.39	+7 7.1	6								SAND lig	ht brown, m	noist, very dense	, with	
		- 7.1	F	ST	50/15mm	<del>50/15mn</del>	1 2	Sv, Pl, M	-		sand and	47% fine s	es, 43% fine to o ubangular limest	coarse tone gravel	
		-									to 1-1/2".	Moderate	cementation.	-	
2008															
5/30/	721.39	-8													
3DT		-													
DOT.(		<u> </u>													
NV_E		- 8.6	<sup>9</sup> G	мс	75/15mm	75/15mn	2		-						
GPJ	720.39	9													
NV_DOT 0324017.GPJ NV_DOT.GDT 5/30/2008		-													
0324		+													
DOT		Ĺ													
Ž															

						. 11	/6/01			EXPLO	ORATIO	N LOG			
					TART DATE		/6/01							10	SHEET 2 OF 3
					ND DATE			 merlin HOV	Elver	r			STATION	12+55 R	
					OB DESCRI		-	s, Nevada	FIYOVE	I			OFFSET	<u>33 m Rt.</u> MAM	
	- An				OCATION		10	S, Nevaua					ENGINEER	Diedrich D	-50 turbo
	K.				ORING		324-01-7	7	[	CROU	NDWATER		EQUIPMENT OPERATOR	Eagle Drill	
	1				ROJECT #		24-01-7 29.39 (m			DATE	DEPTH m		DRILLING METHOD	152 mm H	S Auger
					ROUND EL	_ •	-	Cable							ATE 11/6/2001
ļ					AMMER DR		STEM		I				BACKFILLED	DA	ATE
	ELEV. (m)	DEPTH (m)		MPLE TYPE	BLOW CO 150 mm Increments	OUNT Last	Percent	LAB TESTS	USCS Group		MATE	ERIAL DE	ESCRIPTION		REMARKS
·	(11)	10.21													
		_	H	51	50/25mm5	<del>0/25mn</del>	n <u>5</u>								
		-													
		-							GP						
	718.39 -	-11							GM						
		11.73					_								
		-		MC	75/25mm7	<del>(5/25mn</del>	n 5								
	717.39 -	- 12													
		-													
		-													
	716.39 -	- 13													
		_													
		-													
		-													
	715.39 -	- 14													
		-													
	744.00		J	SPT	50/50mm5	0/50mn	n 11		-						
	714.39 -	- 15								15.24					
		-										SAND with	GRAVEL light	brown,	
		-									plasticity f	fines, 54-60	th 13-16% low the fine to coarse	e sand and	
	740.00	-											e subrounded li te cementation.		
	713.39 -										9				
		-													
		-													
	712.39 -														
	/12.39 -														
		-													
908		17.83													
/30/2	711.39 -	- 1 <del>7</del> :88 - 18	к	MC	75/50mm7	5/50mn	11	Sv, Pl, M	-						
DT 5.	111.38 -	-													
OT.G		-													
NV_DOT 0324017.GPJ NV_DOT.GDT 5/30/2008		F							_						
A LAS	710.39 -								SC						
017.6	110.00														
0324		-													
DOT		Ľ													
N		Ĺ													

ſ						11	/6/01			EXPL	ORATIO	N LOG			
					FART DATE		/6/01								SHEET 3 OF 3
					ND DATE								STATION	12+55 R	
					DB DESCRI		-	merlin HOV	Fiyove	ſ			OFFSET	33 m Rt.	
	.tre	``````````````````````````````````````		LC	OCATION		-	s, Nevada					ENGINEER	MAM Diedrich D	
	2Mm	e s		B	ORING		10	_	— r				EQUIPMENT	Eagle Dril	
	Z.			PF	ROJECT #		24-01-7					R LEVEL ELEV. m			
	Ű.			G	ROUND EL	EV72	.9.39 (m	1)	ł	DATE		ELEV. M	DRILLING METHOD	152 mm H	
			1	H	AMMER DF	ROP SYS	STEM_C	able	[				BACKFILLED	Yes D	ATE 11/6/2001
ľ	ELEV.	DEPTH		MPLE TYPE	BLOW C 150 mm Increments	OUNT Last	Percent	LAB TESTS	USCS Group		MAT	ERIAL DE	SCRIPTION		REMARKS
-	(m)	(m)	NO.	1156	Increments	300 mm	Recov'd		Group						
		-													
		Ľ													
		- 20:98													
	708.39 -	-20.93	L	SPT	50/50mm	50/50mn	11								
		-													
		-													
		Ē													
	707.39 -	-22													
		F													
		22.40	М	мс	75/25mm7	<del>/5/25mn</del>	1 5	Sv, Pl, M		22.40					
		-													
	706.39 -														
	700.39 -	23													
		Ļ													
		F													
		-													
	705.39 -	-24													
		Ľ													
		_													
		-													
	704.39 -	-25													
		-													
		-													
		[													
	703.39 -	-26													
		-													
		-													
		-													
	702.39 -	-27													
	102.00														
		-													
908		-													
30/2(	704 00														
T 5/	701.39 -	-28													
T.GL		Ļ													
_ D0		F													
N N		F													
7.GP	700.39 -	-29													
(2401															
NV_DOT 0324017.GPJ NV_DOT.GDT 5/30/2008		Ļ													
		-													
ź		1	1			1	1		1	1					

						. 11	/7/01			EXPLORATION LOG			
							/7/01					04.00.0	SHEET 1 OF 3
					ND DATE		-	 merlin HOV	Flyove		STATION	91+68 Q 31 m Rt.	
					DB DESCRI			s, Nevada	TIYOVE		OFFSET	MAM	
	An	· · · ·			DCATION	B-	•	5, Nevaua			ENGINEER EQUIPMENT	Diedrich D	-50 turbo
	× v				ORING		24-01-7	7			OPERATOR	Eagle Drill	
	The second se				ROJECT #		24-01-7 3.90 (m			DATE DEPTH m ELEV m	DRILLING	152 mm H	S Auger
	Ve				ROUND EL	L V		Cable			METHOD	Vee	44/7/0004
				H	AMMER DR		STEM	Jable			BACKFILLED	D/	ATE 11/7/2001
	ELEV. (m)	DEPTH (m)	SAI NO.	MPLE TYPE	BLOW CO 150 mm Increments	Last	Percent Recov'd	LAB TESTS	USCS Group	MATERIAL DE			REMARKS
	722.90 -	- - - - <u>1 1.93</u>		ST	-50/25mm <sup>e</sup>	0/2Emm	Ē		GP GM	POORLY GRADED GF SAND light brown-tan dense, with 6% nonpla coarse sand and 50% limestone gravel to 1".	, slightly moist, astic fines, 44% fine subangula	, very 5 fine to ar	Previously reported as boring B-G.
		1.12		0.	00.20	<del>)0/201111</del>							
		1.57	A	GRAE	8		50	Sv, PI, M		1.00			
	721.90 -	2	В	MG	-75/25mm7	75/25mm	5		- SW	1.68 WELL GRADED SANE SAND light brown, slig with 11% low plasticity sand and 35% fine sub gravel to 3/4". Modera cementation.	ghtly moist, ver fines, 54% fin prounded limes	y dense e to coarse	
		2.64				5/251111		Sv, PI, M,	30				
	720.90 -	<b>3</b> 3.10	В'	GRAE	3		50	DS, Chem					
		-								3.35			
	719.90 -	- - - - - - - - - - - - - - - - - - -		ST GRAE	50/50mm5	50/50mn	<u>11</u> 50	Sv, PI, M	GP GC	P OORLY GRADED G CLAY and SAND light with 10% low plasticity sand and 51% fine sut to 1/2". Moderate cerr	t tan, moist, ve fines, 39% fin pangular limes	ry dense, e to coarse	
		_											
	718.90 -	-5								5.18			
		-								WELL GRADED SAND			
		- 5.64								GRAVEL light brown, 21% medium plasticity			
		<u>- 3:40</u> - 5:70			75/38mm7	5/38mn			SW SC	sand and 18% fine sub	bangular limes		
	717.90 -	-6 <sub>6.16</sub>	D'	GRAE	8		50	Sv, PI, M	30	to 3/4". Moderate cem	ientation.		
		-								6.40			
5/30/2008	716.90 - 715.90 -	- - - - - - - - - - - 8	E	ST	50/100 5	0/100mr	n 22	Sv, Pl, M	-	CLAYEY SAND with G moist, very dense, with fines, 44-65% fine to c fine subrounded limes Moderate cementation	n 14-21% low p coarse sand an tone gravel to	blasticity d 18-42%	
DT (		-											
NV_DOT 0324017.GPJ NV_DOT.GDT 5/30/2008	714.90 -	- 8.69 - 8.81 - 9 - -		MC	75/125 7	5/125mr	n 28	Sv, PI, M	_				
		-											

						11	/7/01			EXPLO	ORATIO	N LOG			
					TART DATE		/7/01							04 00 0	SHEET 2 OF 3
					ND DATE			 merlin HOV	Elvore	r			STATION	91+68 Q	
					OB DESCRI		-		riyove				OFFSET	<u>31 m Rt.</u> MAM	
	- Ary				OCATION		is vega 11	s, Nevada					ENGINEER	Diedrich D	-50 turbo
	1 Mar				ORING			7	[				EQUIPMENT OPERATOR	Eagle Drilli	
	The second se				ROJECT #						NDWATEF	ELEVEL	DRILLING		
	le				ROUND EL	_ •	23.90 (m			BATE				152 mm H	
				H	AMMER DR	ROP SY	STEM_C	able	[				BACKFILLED	Yes DA	TE 11/7/2001
	ELEV. (m)	DEPTH (m)		MPLE TYPE	BLOW CO 150 mm Increments	OUNT Last	Percent Recovid	LAB TESTS	USCS Group		MAT	ERIAL DE	ESCRIPTION		REMARKS
		10.21	G	ет	50/25mm										
		_	0	51	50/25mm	0/25111									
		-													
		-													
	712.90 -	-11													
		-													
		11:78							SC						
		-	H	MC	75/50mm7	5/50mr	n 11								
	711.90 -	- 12													
		-													
		-													
	710.90 -	-13													
		-													
		-													
	709.90 -	- 14													
		14:88 14:83		SPT	50/50mm	0/50mr	n 11		-						
	708.90 -	- 15		GRAE	3		50								
		- 15.29							-						
		F													
		-													
	707.90 -	- 16													
		-													
		Ļ								40.55					
		-								16.76	SILTY. CI	LAYEY GRA	AVEL with SANI	D light	
	706.90 -	- 17									brown, m	oist, very de	ense, with 16% I	ow	
		-									44% fine	subrounded	fine to coarse sa d gravel to 1". N	loderate	
											cementat	ion.	-		
0/200		- 17:83		MC	75/63mm7	5/63mr	n 17		_						
5/30	705.90 -	-18			, 5,0511111/	5,55111			1						
GDT		-													
DOT		Ľ													
N		F													
7.GPJ	704.90 -	- 19													
24017		F							GC						
NV_DOT 0324017.GPJ NV_DOT.GDT 5/30/2008									GM						
		F													
ź															

						11	/7/01			EXPLORATION LOG			
				S	TART DATE								SHEET 3 OF 3
	i i				ND DATE		/7/01		-		STATION	91+68 Q	
				JC	DB DESCR		-	merlin HOV	Flyove	r	OFFSET	31 m Rt.	
	.tm			LC	OCATION			s, Nevada			ENGINEER	MAM Diadriah D	FO turb o
	10m			B	ORING		11		ı		EQUIPMENT	Diedrich D Eagle Drilli	
	Ä	<b>1</b>		PI	ROJECT #		824-01-7			GROUNDWATER LEVEL	OPERATOR		
	n n			G	ROUND EL	EV72	23.90 (m	ו)		DATE DEPTH m ELEV. m	DRILLING METHOD	152 mm H	S Auger
			1	H	AMMER DF	ROP SYS	STEM_C	able	[		BACKFILLED	_Yes	ATE 11/7/2001
	ELEV.	DEPTH		MPLE	BLOW C	OUNT	<b>D</b> (						
	(m)	(m)	NO.	TYPE	150 mm Increments	Last 300 mm	Percent Recov'd	LAB TESTS	USCS Group	MATERIAL DE	SCRIPTION		REMARKS
		_											
		-											
		-											
		- 20.88	к	ST	50/25mm	<del>0/25mn</del>	n <u>5</u>		-				
	702.90 -	-21											
		21.38											
		_	K'	GRAE	3		50	Sv, PI, M					
		_ 21.84							_	21.95			
	701.90 -	-22							SC	SILTY, CLAYEY SAN	D tan-light brow	wn, moist,	
		<u>-</u> <u>22:49</u>							SM	very dense, with 23% 22.45 fine to coarse sand an	low plasticity find 10% fine to c	nes, 67%	
		22.43	L	MC	75/50mm	75/50mn	n 11	Sv, Pl, M		∖subangular gravel to 2	2". Moderate c	alcareous	
		_								cementation.		/	
	700.90 -	-23											
		-											
		-											
		-											
	699.90 -	-24											
	033.30	-											
		_											
		-											
		-											
	698.90 -	-25											
		_											
		-											
	697.90 -	-26											
		-											
		-											
	696.90 -	-27											
		-											
		-											
008		-											
/30/2	695.90 -												
NV_DOT 0324017.GPJ NV_DOT.GDT 5/30/2008	030.30	20											
DT.GI		F											
		-											
Ň		+											
17.GF	694.90 -	-29											
32401		[											
)T 0(		F											
		F											
ź		1	1		1		1						

						_ 11	/7/01			EXPLORATION LOG			
							/7/01					02:45 0	SHEET 1 OF 3
								 merlin HOV	Flyove	r	STATION	93+15 Q 60 m Rt.	
					DB DESCR			s, Nevada	TIYOVC	1	OFFSET	MAM	
	2m				DCATION	 B-	•	0, 1107000			ENGINEER EQUIPMENT	Diedrich D	-50 turbo
	× v				ORING		24-01-7	7		GROUNDWATER LEVEL	OPERATOR	Eagle Drill	ing
					ROJECT #		0.24 (m			DATE DEPTH m ELEV. m	DRILLING	152 mm H	S Auger
					ROUND EL	v		Dable			METHOD	Mark	44/7/0004
					AMMER DF		STEM				BACKFILLED	D/	ATE 11/7/2001
	ELEV. (m)	DEPTH (m)		MPLE TYPE	BLOW C 150 mm Increments	Last	Percent Recov'd	LAB TESTS	USCS Group				REMARKS
	719.24	- - - <u>1 1.07</u> <u>1.14</u>		SPT	-50/75mm	<del>50/75mm</del>	<del>1 16 1</del>		_	SILTY, CLAYEY SA brown, dry to slightly 13-15% low plasticit sand and 34-36% fin limestone gravel to cementation.	/ moist, very dens y fines, 52% fine ne to coarse subr	se, with to coarse rounded	Previously reported as boring B-I.
		-		GRAB	3		50	Sv, PI, M					
	718.24	<u>    1.60</u> - - - 2							SC SM				
		2.59		мс	75/25mm	75/25mm	5		_				
		2.64	В'	GRAB	2		50	Sv, PI, M,					
	717.24	-3 3.10			1			SH, Chem	_				
										3.35			
	716.24	- - - - - 4 4.11 - - 4.16 -	C'	SPT GRAB	<del>50/25mm</del>	<del>50/25mm</del>	<u>5</u>	Sv, PI, M	sc	CLAYEY SAND with slightly moist, very of plasticity fines, 51% 34% fine subangula Strong calcareous c	ense, with 15% l fine to coarse sa r limestone grave	ow nd and	
	715.24	<u>4.62</u> - 5 -								5.18 SILTY, CLAYEY SA		light	
		-								brown, slightly moist	, very dense, wit	h 14% low	
		_ <b>5.64</b> 5.79		MC	75/0mm	75/0mm	0		-	plasticity to nonplast sand and 37% fine s	tic fines, 49% fine Subangular limest	e to coarse tone gravel	
	714.24	<b>6</b> 		GRAB	3		50	Sv, PI, M	SC SM	to 1-1/2". Moderate			
		F								6.71			
	713.24	- 	E	SPT	50/100 5	0/100mr	n 22	Sv, Pl, M	_	CLAYEY GRAVEL v slightly moist to mois low plasticity fines, 3 and 47-48% fine sub	st, very dense, wi 84-37% fine to co	ith 15-19% arse sand	
		-								Moderate calcareou		.~	
/2008													
5/30/	712.24	-8											
GDT		-											
DOT.		0.00							GC				
N		- <u>8</u> .69	F	мс	75/75mm	/5/75mm	17	Sv, Pl, M	-				
GPJ	711.24	-9											
NV_DOT 0324017.GPJ NV_DOT.GDT 5/30/2008		+											
r 032		Ĺ											
		Ĺ											
≥													

						. 11	/7/01			EXPLORATION LOG			
					TART DATE		/7/01					<b></b>	SHEET 2 OF 3
					ND DATE				Elverit	<b>-</b>	STATION	93+15 Q	
					OB DESCRI		-	merlin HOV	гуоче	l	OFFSET	60 m Rt.	
	100			L	OCATION			s, Nevada			ENGINEER	MAM Diedrich D	50 turbo
	2MM	6		В	ORING		13	_	i		EQUIPMENT	Eagle Drilli	
	Ĩ,			Ρ	ROJECT #		324-01-7				OPERATOR DRILLING		
	Ĩ.			G	ROUND EL	<sub>EV.</sub> _72	20.24 (m			DATE DEPTH m ELEV. m	METHOD	152 mm H	
				H	AMMER DR	OP SYS	STEM_C	able	[		BACKFILLED	Yes DA	TE 11/7/2001
	ELEV. (m)	DEPTH (m)		MPLE TYPE	BLOW CO 150 mm Increments	Last	Percent Recov'd	LAB TESTS	USCS Group	MATERIAL DE	SCRIPTION		REMARKS
		10.21	G	SPT	50/25mm5	<del>0/25mn</del>	n <u>5</u>			10.06 SILTY, CLAYEY SAN	D with GRAVE	L tan,	
	709.24 -	- - -								slightly moist to moist low plasticity fines, 49 and 29-34% fine to co limestone gravel to 3	9-58% fine to co parse subround	arse sand	
	705.24	_											
		11.73											
		- 11.78	H	MC	75/50mm7	<del>5/50mn</del>	n 11		=				
	708.24 -	+ <b>12</b> _ 12.24		GRAE	3		50	Sv, PI, M					
		_ 12.24											
		-							SC				
		-							SM				
	707.24 -	-13											
		-											
		-											
	706.24 -	- 14											
		_											
		14.78	- 1-	SPT	50/25mm5	<del>0/25mn</del>	n <u>5</u>	Sv, Pl, M	-				
	705.24 -	- 15								15.24			
										CLAYEY GRAVEL w			
		-								slightly moist to moist estimated 25% low pl	, very dense, w asticity fines 3!	ith 5% fine to	
		-								coarse sand and 45%	fine subround	ed gravel to	
	704.24 -	- 16								1-1/2". Moderate cer	nentation.		
		-											
		$\vdash$											
	703.24 -	- 17											
		-											
~													
0/200		_ 17.83	J	мс	75/38mm7	<del>'5/38mn</del>	n 4						
5/30	702.24 -	- 18											
GDT		-											
NV_DOT 0324017.GPJ NV_DOT.GDT 5/30/2008		Ľ											
N		F							GC				
7.GPJ	701.24 -	- 19											
24017		-											
T 032		Ľ											
DO		Ļ											
ź													

ſ						11	/7/01			EXPL	ORATIO	N LOG			
					FART DATE		/7/01								SHEET 3 OF 3
					ND DATE					-			STATION	93+15 Q	
					DB DESCRI		-	merlin HOV	Fiyove	ſ			OFFSET	60 m Rt.	
				LC	OCATION			s, Nevada					ENGINEER	MAM Diedrich E	
	2Mm	6		B	ORING		13	_	r				EQUIPMENT	Eagle Dril	
	Z.			PF	ROJECT #		24-01-7								
	Ť.			G	ROUND EL	EV72	20.24 (m	1)	ł	DATE	DEPTHM	ELEV. m	DRILLING METHOD	152 mm H	
			•	H	AMMER DF	ROP SYS	STEM_C	able	[				BACKFILLED	Yes D	ATE 11/7/2001
ľ	ELEV.	DEPTH		MPLE TYPE	BLOW C 150 mm Increments	OUNT Last	Percent	LAB TESTS	USCS Group		MAT	ERIAL DE	SCRIPTION		REMARKS
·	(m)	(m)	NO.	TTPE	Increments	300 mm	Recov'd		Group						
		-													
		20.88													
	699.24 -	-21	ĸ	SPT	50/25mm	<del>50/25mn</del>	1 5								
		-													
		-													
		-													
	698.24 -	-22													
	000.21	_													
		<u> 22</u> :48	L	мс	75/63mm7	5/63mn	1 12			22.46					
		-													
	697.24 -														
	097.24 -	-23													
		_													
		-													
		-													
	696.24 -	-24													
		-													
		-													
	695.24 -	-25													
		-													
		Ľ													
		_													
	694.24 -	-26													
		-													
		-													
	693.24 -	-27													
		-													
908		-													
/30/2(	602.24	-													
JT 5/	692.24 -	-28													
JT.GL		-													
/ DC		$\vdash$													
N L		F													
7.GF	691.24 -	-29													
32401															
NV_DOT 0324017.GPJ NV_DOT.GDT 5/30/2008		-													
/ DC		F													
ź		1	1				1		1	1					

					~		- 1/·	15/02			EXPL	ORATIO	N LOG			
								15/02							DE 12:00	SHEET 1 OF 1
									 merlin HOV	Flyove	r			STATION	R5 13+90 20 m Rt.	
						DB DESCR			s, Nevada	119000				OFFSET	PGT	
	22					DCATION		18	5, 140 Vada					ENGINEER EQUIPMENT	Diedrich D	0-50
	E SA	لا م	5					24-01-7	7	[	GROU	NDWATER			Eagle Dril	ling
						ROJECT #	70	6.40 (m			DATE	DEPTH m		DRILLING	152 mm F	IS Auger
						ROUND EL	. E V		able					METHOD	Vee	4/44/0000
						AMMER DF		STEM	able					BACKFILLED	D	ATE
	ELEV. (m)		PTH m)	SAN NO.	MPLE TYPE	BLOW C 150 mm Increments	Last	Percent Recov'd	LAB TESTS	USCS Group				SCRIPTION		REMARKS
		-	0.76							SC SM		brown, dr fines, 50% limestone	y, very dens 6 fine to coa gravel to 3	<b>ith Gravel</b> ; ligh se, with 16% low arse sand, 34% ". Cobbles to	v plastic subangular	Boring located in field south of NDOT right-of-way at
		-	0.76	IBA		50/25mm	50/25mn			511		at the surf	imeter com face. Mode	prise 15% of the rately cemented	with weak	west end of Summerlin
	735.40	+1	1.12	18A'	GRAB			50	Sv, PI, M	_	1.22	caliche co	pating on co			eastbound off-ramp. The
		F	1.52									Poorly Gr	raded Grav	el with Silt and dense, with 9%	Sand ; light	westernmost of five holes.
		-	1.57	100	SPT GRAB	50/25mm	<del>50/25mn</del>	0 50	Sv, Pl, M			fines, 36%	6 fine to coa	arse sand, 55%	subangular	
		-	1.88	ТОВ	GRAD			50	3V, FI, IVI	GP			nded limest to well cem	one gravel to >2 nented.	2.5".	
	734.40	<b>+</b> 2								GM						
		-									2.74					Grab samples are from auger
		-									+			el with Silty Cla		cuttings at
	733.40	-3	3.05	18C	мс	50/65mm	50/65mn	100		-		Sand ; ligi	ht grey brov astic fines	vn, dry, very der 39% fine to coa	nse, with rse sand	surface. They are much larger
												52% suba	angular lime	stone gravel to	3".	in volume than
														mented. Actual aded gravel with		the limited volume SPT
		-										and Sand			only only	and MC
	732.40	+4								GP						samples.
		-								GC						
		-	4.57		SPT											
			4:87	18D	SPT	50/100 5	0/100mr	n 0								
	731.40	-5	5.13	18D'	GRAB			50	Sv, PI, M							
		-	5.15							-						
		-									<u>5.4</u> 9					
		F												ith Silty Clay ar very dense, wit		Hard, cemented gravel
	730.40	1	0.40									plastic fine	es, 46% fin	e to coarse sand	d, 44%	throughout
	730.40	-	6.10	18E	MC	50/25mm	<del>50/25mn</del>	100	Sv, PI, M	SW SC		subangula	ar limestone	e gravel to 3".		hole. Auger bit produces
		-														angular sand
		-									6.71					and fine gravel fragments from
		$\vdash$									T			with Silty Clay		originally larger
	729.40	+7	7.16							SP				own, dry, very do , 51% fine to co		gravel and possibly cobble
		-		105	GRAB			50	Sv, PI, M	SC		39% suba	ingular lime	stone gravel to	3".	clasts.
~			7.62				0/50		5V, PI, IVI		7.67					
/2008		_	7.67	181	SPI	50/50mm	50/50mn	0			1.01					
5/30/2008	728.40	+8														
5DT		-														
DOT.(		-														
		Ē														
I LA	727.40	9														
NV_DOT 0324017.GPJ NV_DOT.GDT	, .+0	Ļ														
3324(		-														
DT (		F														
		<u> </u>														

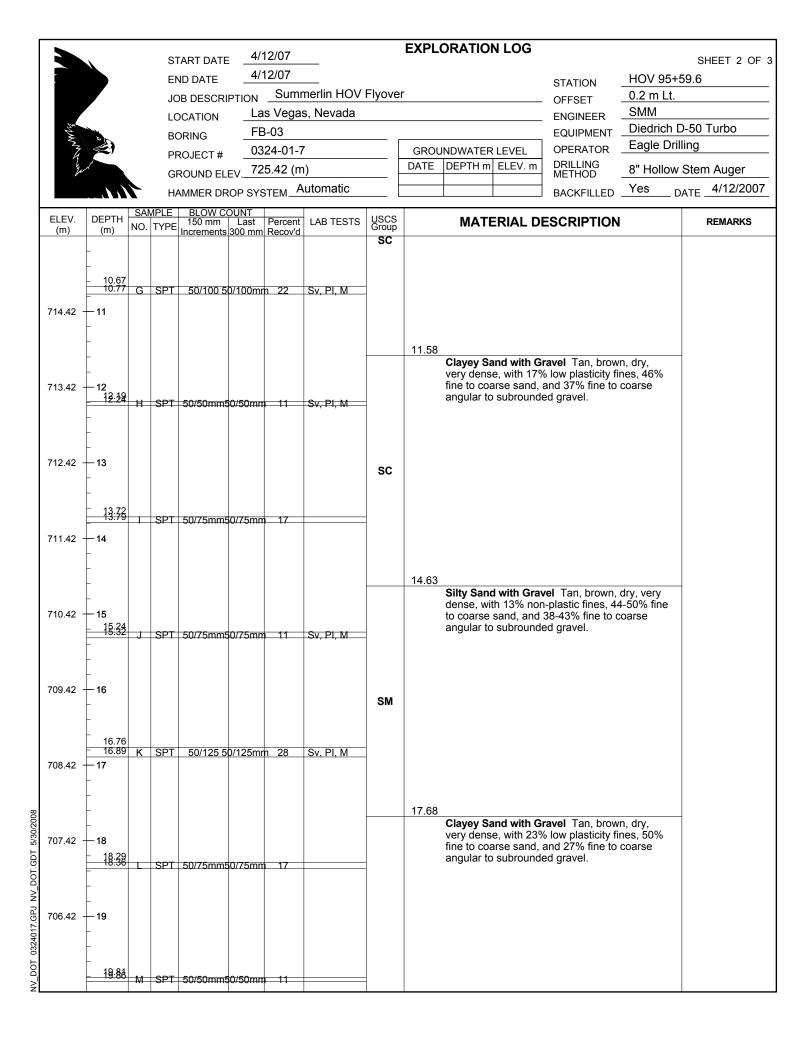
[							_ 1/	15/02			EXPL	ORATIO	N LOG			
						TART DATE		15/02								SHEET 1 OF 2
						ND DATE			 marlin LIOV		-			STATION	R5 12+90	
					JC	DB DESCR			merlin HOV	FIYOVE	er			OFFSET	<u>10 m Rt.</u>	
	24				LC	OCATION			s, Nevada					ENGINEER	PGT Diedrich D	50
	2MM	Contraction of the second	•		B	ORING		19	_					EQUIPMENT	Eagle Drill	
	4		7		PI	ROJECT #		324-01-7				NDWATER		OPERATOR DRILLING		
	ž.	1			G	ROUND EL	.EV73	85.48 (m	,		DATE	DEPTH m	ELEV. M	METHOD	152 mm H	ý –
			7)	1	H.	AMMER DF	ROP SYS	STEM_C	able					BACKFILLED	Yes D	ATE 1/14/2002
	ELEV. (m)	DEP (m		SAN NO.	<u>/PLE</u> TYPE	BLOW C 150 mm Increments	Last	Percent Recov'd	LAB TESTS	USCS Group		MATE	ERIAL DE	SCRIPTION		REMARKS
			0.76	194	SPT	- 50/15mm				SM		very dense to coarse gravel to 3 comprise	e, with 14% sand, 42% 3". Cobbles 15% of the	el; light grey br low plastic fine subangular lime to 6-inch-diame total mass. Moc	s, 43% fine estone eter lerately	Boring located in field south of NDOT right-of-way at west end of
	734.48 -		).81		GRAE			50	Sv, PI, M			cemented cobbles.	with weak	caliche coating	on	Summerlin eastbound
	734.40		1.12		0.0.2				0.1, 1.1, 1.1		1.22					off-ramp.
		- 1	1.52									Poorly Gr	aded Sand	with Silt and G dense, with 119	ravel ; light	
			1.57	-		50/10mm	<del>50/10mn</del>		0.01.11	SP		plastic fine	es, 47% fin	e to coarse sand	d, 42%	
		- 1	1.88	19B'	GRAE	\$		50	Sv, PI, M	SM		subangula	ar limestone	e gravel to 3".		
	733.48 -	-2									2.13					
	732.48		3.05 3.12	19C	SPT	50/75mm	50/75mn	n 100	Sv, PI, M	GW GM	3.66	grey brow fines, 43%	n, dry, very	with Silt and Sa dense, with 9% arse sand, 48% ".	low plastic	Grab samples are from auger cuttings at surface. They are much larger in volume than the limited volume SPT
	731.48 -		4.57 1.65	<del>19D</del>	MC	- 50/75mm	<del>50/75mn</del>	<del>n 67</del>		GW		light grey	brown, dry, es, 40% fin	with Silty Clay a very dense, wit e to coarse sance gravel to 3".	h 10% low	and MC samples.
		-	1.00	יחמו	GRAE	2		50	Sv, PI, M	GC						
	730.48	-5 5	5.11	150		1			00, 11, 10							
		-														
	729.48		6.10 5.17	<del>19E</del>	SPT	<del>50/75mm</del>	<del>50/75mn</del>	<del>1 67</del>		 GW	5.49_	grey brow plastic fine	n, dry, very es, 43% fin	with Silt and Sa dense, with 10 <sup>0</sup> e to coarse sand gravel to 3".	% low	Hard, cemented gravel throughout hole. Auger bit produces
		+		19E'	GRAE	3		50	Sv, PI, M	GM						angular sand and fine gravel
		- 6	5.63							_						fragments from
		F _									7.01					originally larger gravel and
DT 5/30/2008	728.48 - 727.48 -	-7 - - - - - 8 -	7.62	<del>-19F</del>	- <del>MC</del> -	<del>- 50/25mm!</del>	<del>50/25mn</del>	<del>n 100 -</del>	<del>- Sv, Pl, M</del>	 SC		dry, very c 50% fine t limestone	lense, with o coarse sa gravel to 3	avel ; light grey 13% medium pl and, 37% suban ". Moderately c ating on cobbles	astic fines, gular emented	possibly cobble clasts.
T.GL		F									0.50					
NV_DOT 0324017.GPJ NV_DOT.GDT 5/30/2008	726.48 -		9.14 9.19	<del>19G</del>	SPT	50/50mm!	<del>30/50mn</del>	<del>- 50</del>		 - SP	8.53_	grey brow fines, 48%	n, dry, very	with Silt and G dense, with 9% arse sand, 42% ".	low plastic	
0324(		+	-	19G'	GRAE	3		50	Sv, PI, M	SM						
OT C		- 9	9.65	-						-						
		F														

ſ						1/	1 5/00			EXPLORATION LOG		
				S	TART DATE		15/02					SHEET 2 OF 2
	Ĩ			Eľ	ND DATE	1/*	15/02			STATIO		
				JC	DB DESCRI	PTION	Sum	merlin HOV	Flyove	r OFFSET	10 m Rt.	
				LC	OCATION	La	is Vega	s, Nevada		ENGINE	ER <u>PGT</u>	
	W			B	ORING	B-	19			EQUIPM		
	No.				ROJECT #	03	24-01-7	7		GROUNDWATER LEVEL OPERAT		lling
	n n				ROUND EL	EV 73	5.48 (m	ו)		DATE DEPTH m ELEV. m DRILLIN METHOL	G 152 mm H	IS Auger
						L V		Cable				
				H/	AMMER DF		SIEM_S			BACKFI		ATE
	ELEV. (m)	DEPTH (m)	SAI NO.	MPLE TYPE	BLOW C 150 mm Increments	Last	Percent Recov'd	LAB TESTS	USCS Group	MATERIAL DESCRIP	ΓΙΟΝ	REMARKS
	724.48 -	- - - <u>18:67</u> - - - 11	19H	МС	50/75mm	30/75mn	1 100	Sv, Pl, M	sc	10.06 Clayey Sand with Gravel ; ligh dry, very dense, with 13% low fines, 52% fine to coarse sand, limestone gravel to 3".	to medium plastic	
	723.48 -	- - - - - 12.19 -	191	SPT	-50/25mm{	<del>0/25mn</del>	0			12.50 Poorly Graded Gravel with Sil	ty Clay and	
	722.48 -	- - 13 - - <u>13.72</u> - <u>13.76</u>	19J	MC	-50/25mm	<del>:0/25mn</del>	0		GP	Sand ; light grey brown, dry, ve 10% low plastic fines, 35% fine 55% subangular limestone gra Moderately to well cemented.	to coarse sand,	
	721.48 -	14 14.22  		GRAE	3		50	Sv, Pl, M	GC			
	720.48 -	- 15 - 15.24 - 15.37 - -	19K	SPT	50/125 5	0/125mr	n 40	Sv, PI, M	-	15.24		-
	719.48 -	- 16 - - -										
7/2008	718.48 -	- 17 - - -										
NV_DOT.GDT 5/3(	717.48 -	- 18 - - -										
NV_DOT 0324017.GPJ NV_DOT.GDT 5/30/2008	716.48 -											

ſ					~		4/*	12/07			EXPL	ORATIO	N LOG			
						TART DATE		12/07								SHEET 1 OF 1
						ND DATE			merlin HOV	Flyove	r			STATION	SW 99+29 3.8 m Lt.	9.5
						OB DESCRI			s, Nevada	TIYOVE						
	12 Mar					OCATION		<u>is vega</u> 3-01	5, Nevaua					ENGINEER EQUIPMENT	0-50 Turbo	
	× v	e M	5			ORING		5-01 524-01-7	7		GPOI	NDWATER		OPERATOR	Eagle Dril	
	Level .	X				ROJECT #		0.95 (m			DATE	DEPTH m		DRILLING		Stem Auger
	Ve					ROUND EL	L V		utomatic					METHOD	Vee	4/40/0007
						AMMER DF		STEM_	utomatic					BACKFILLED	D	ATE
	ELEV. (m)		PTH m)	-	MPLE TYPE	BLOW C 150 mm Increments	Last	Percent Recov'd	LAB TESTS	USCS Group				SCRIPTION		REMARKS
	739.95 -	- - - - - 1 -	1.52		SPT	44 ( -50/25mm	0/25mn	1 39	Sv, PI, M	_		dense, wit	th 14% non- ind, and 40%	el Tan, brown, -plastic fines, 44 % fine to coarse	5% fine to	Drive hammer had an efficiency of 69 percent. Blowcounts can be converted to Standard SPT N60 by multiplying by 1.15.
		-				00/201111				SM						
	738.95 -	+-2 -														
	737.95 -	- - - - -	3.98	В	SPT	-50/50mm(	50/50mn	11		_						
		F									3.66_	Silty San	d Tan brow			
	736.95	- - -	4.57									15% non-	plastic fines	vn, dry, very den s, 71% fine to co se angular to su	barse sand.	
		-	4.57		SPT	43 50/25mm	0/25mn	1 39	Sv, PI, M							
		-				-50/25mm				SM						
	735.95	-5														
	734.95 -	6	8:14	D	SPT	-50/50mm	50/50mn				6.14					
8	733.95 -	- - - - -														
DOT.GDT 5/30/200	732.95 -	- 8														
NV_DOT 0324017.GPJ NV_DOT.GDT 5/30/2008	731.95 -	- - - - -														
≥																

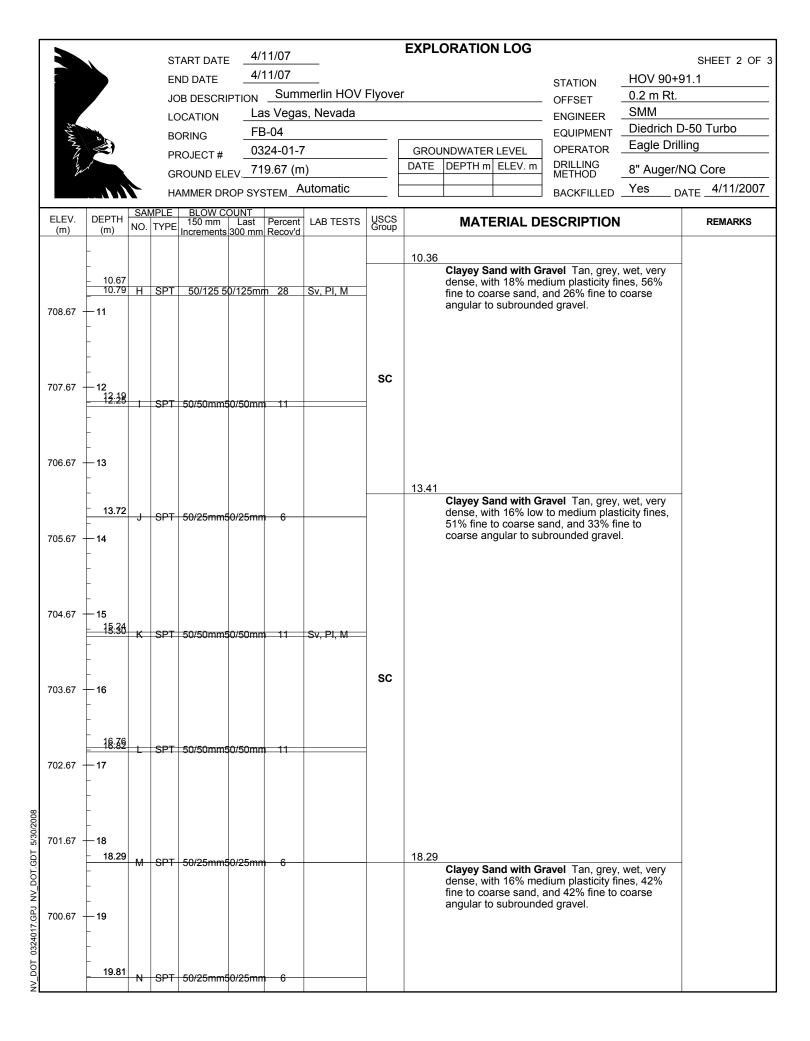
ſ							1/-	12/07			EXPLO	ORATIO	N LOG			
						TART DATE		12/07								SHEET 1 OF 1
						ND DATE					-			STATION	SW 98+32	2.9
					JC	DB DESCR			merlin HOV	гіуоче	[			OFFSET	2.4 m Lt. SMM	
	, by	<b>.</b>				DCATION			s, Nevada					ENGINEER		0-50 Turbo
	2 May					ORING		3-02	7	[	00011			EQUIPMENT OPERATOR	Eagle Dril	
	T.	$\sum$				ROJECT #		24-01-7				NDWATER		DRILLING		
	h	,				ROUND EL	L V	8.47 (m	-		BATE			METHOD	Vee	Stem Auger
			J/		H	AMMER DF	ROP SYS	STEM_P	utomatic					BACKFILLED	Yes D	ATE 4/12/2007
	ELEV. (m)	DEPT (m)			/PLE TYPE	BLOW C 150 mm Increments	Last	Percent Recov'd	LAB TESTS	USCS Group				SCRIPTION		REMARKS
	737.47 - 736.47 -	- - - - - - - - - - - - 2 -	:56	-A	SPT	50/75mm	30/75mn	1 17	Sv, Pl, M	SM	2.44	dense, wit	th 21% non nd, and 25%	el Tan, brown, -plastic fines, 54 % fine to coarse	4% fine to	Drive hammer had an efficiency of 69 percent. Blowcounts can be converted to Standard SPT N60 by multiplying by 1.15.
		[									2.77	Silty Sand	d with Grav	el Tan, brown,	dry, very	
		F										dense, wit	th 14% non nd and 299	-plastic fines, 57%	7% fine to	
	735.47 -	<u>-3 3</u>	.05 .15	B	SPT	50/100 5	0/100mr	n 22	Sv. PI, M	SM		subrounde	ed gravel.		ungului to	
		-		0		00/1000										
		E									3.66					
	734.47 -	- 4 	.57									brown, dry fines, 53%	, very dens fine to coal	with Silt and G ee, with 10% not arse sand, and 3 prounded grave	n-plastic 37% fine to	
		- 4	.70	С	SPT	50/125 5	0/125mr	n 28	Sv, PI, M	00						
	733.47 -	-5								SP SM						
	732.47 -	- - -	10													
	/32.4/ -	-6 6 6	.10 .20	D	SPT	50/100 5	0/100mr	n 22			6.20					
	731.47 -	- - - <b>7</b> -														
908		-														
NV_DOT 0324017.GPJ NV_DOT.GDT 5/30/2008	720 47	-														
JT 5/	730.47 -	-8														
DT.GL		ŀ														
		F														
PJ N	720 17	-9														
017.G	729.47 -	-9														
0324(		F														
DOT		F														
		-														

						Δ/	12/07			EXPLO	ORATION	N LOG			
					FART DATE		12/07								SHEET 1 OF 3
								 merlin HOV	Flyove	r			STATION	HOV 95+5	0.80
					DB DESCRI			s, Nevada	,010	•				SMM	
	2m				DCATION DRING		3-03	-,					ENGINEER EQUIPMENT		D-50 Turbo
	E S				ROJECT #		24-01-7	7	[	GROU	NDWATER	LEVEL	OPERATOR	Eagle Dril	ling
	1 miles						25.42 (m				DEPTH m		DRILLING	8" Hollow	Stem Auger
					ROUND EL AMMER DR	L V		utomatic					METHOD BACKFILLED		ATE 4/12/2007
					BLOW C				I	1			BACKFILLED	D	ATE
	ELEV. (m)	DEPTH (m)		MPLE TYPE	450	Last	Percent Recov'd	LAB TESTS	USCS Group				SCRIPTION		REMARKS Drive hammer
	724.42 - 723.42 -	- - - - - - - - - - - - - - - - - - -	A	SPT	50/125 5	<u>0/125m</u>	n 28	<del>Sv, PI, M, Chem</del>	GP GM	2.44	brown, dry fines, 37%	, very dens fine to coa	e, with 9% non- rse sand, and 5 brounded grav	-plastic 54% fine to	had an efficiency of 69 percent. Blowcounts can be converted to Standard SPT N60 by multiplying by 1.15.
	722.42 - 721.42 -	- - - <u>3</u> 3.05 - <u>3.18</u> - - -		SPT	50/125 5	0/125mi	n 28	Sv, Pl, M	sc	2.44	very dense 43% fine t	e, with 14% o coarse sa	avel Tan, brow medium plastic and, and 43% fii ibrounded grav	city fines, ne to	
	721.42 -	- - - - - - 5	с	SPT	50/75mm5	0/75mn	17	Sv, PI, M	sc	4.27	very dense fines, 43%	e, with 16% fine to coa	<b>avel</b> Tan, brow low to medium arse sand, and 4 brounded grave	plasticity 41% fine to	
	719.42 -	- - - - - 6 <u>6.19</u>	D	SPT	50/75mm5	0/75mn	17	Sv. Pl. M		5.79	very dense	e, with 15%	avel Tan, brow medium plastic	city fines,	
	718.42 -	- - - - 7 - - 7							sc				and, and 41% fil		
308		7.62 7.72	E	SPT	50/100 5	0/100mi	n 22								
<sup>-</sup> .GDT 5/30/2(	717.42 -														
DOT		Ļ								8.53	Clavey Sa	nd with Gr	avel Tan, brow	n. drv	
NV_DOT 0324017.GPJ NV_DOT.GDT 5/30/2008	716.42 -	- -9 <u>8.14</u> - -	F	SPT	_50/75mm5	0/75mn	1 6		-		very dense 54% fine t	e, with 22% o coarse sa	medium plastic and, and 24% fi prounded grave	city fines, ne to	
NN_DC		_													



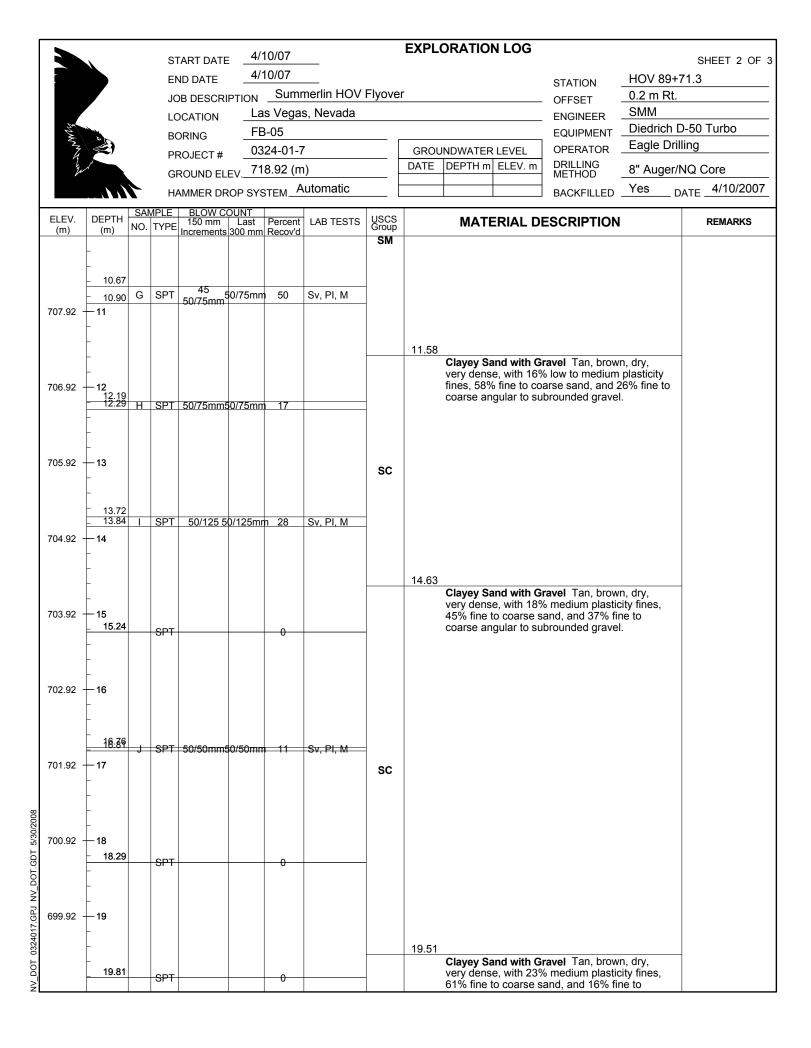
				01		- 4/	12/07			EXPLORATION LOG			
					TART DATE		12/07						SHEET 3 OF 3
					ND DATE			 merlin HOV	Flyove	r	STATION	HOV 95+5 0.2 m Lt.	9.0
		/			DB DESCRI		-	s, Nevada		·	OFFSET	SMM	
	24				DCATION		3-03	0, 1101000			ENGINEER EQUIPMENT		-50 Turbo
	× v				ORING		324-01-7	7	[	GROUNDWATER LEVEL	OPERATOR	Eagle Drill	
	The second se				ROJECT #		2 <u>4-01-</u> 25.42 (m			DATE DEPTH m ELEV. m	DRILLING	8" Hollow	Stem Auger
	he				ROUND EL	L V		utomatic	[		METHOD		-
				HA	AMMER DR		STEM	atomatic	[		BACKFILLED	D.	ATE 4/12/2007
	ELEV. (m)	DEPTH (m)	NO.	MPLE TYPE	BLOW CO 150 mm	Last	Percent	LAB TESTS	USCS Group	MATERIAL DE	SCRIPTION		REMARKS
	(11)				Increments	300 mm	Recova		SC				
		F											
		-											
	704.42	-21											
		21.34	N	SPT	50/125 5	0/125m	m 29	Sv, PI, M	_				
		- 21.40		JE I	50/1255	0/12511	11 20	3V, FI, IVI	-				
		-											
	703.42	-22											
		F								22.25 Clayey Sand with Gr	avel Tan brown	n drv	
										very dense, with 19%	medium plastic	ity fines,	
		22.86								42% fine to coarse sa coarse angular to sub	ind, and 39% fir prounded gravel	ie to	
	702.42 -	-23 23.14	0	SPT	14 50/125 <sup>5</sup>	0/125m	m 61	Sv, PI, M	SC		Ū		
		-			00/120				-				
		F											
										23.77			
	701.42 -	-24								Clayey Sand with Gr very dense, with 20%	avel Tan, brown medium plastic	n, dry, itv fines	
		24.39							SC	50% fine to coarse sa	nd and 30% fir	ne to	
		<u>24:38</u> 24:46	Р	SPT	50/75mm5	0/75mr	17	Sv, Pl, M	-	24.46 coarse angular to sub	rounded gravel		
		F											
	700.42	-25											
		-											
		-											
		-											
	699.42	-26											
	033.42	20											
		-											
		F											
	698.42 -												
	090.42	-27											
		F											
08		-											
30/20		-											
T 5/:	697.42 -	-28											
T.GD		Ĺ											
_ DO		F											
NN C		F											
7.GP	696.42	-29											
12401		Ľ											
NV_DOT 0324017.GPJ NV_DOT.GDT 5/30/2008		Ļ											
/_DC		F											
ź		1					1						

				-		/	11/07			EXPL	ORATIO	N LOG			
							11/07								SHEET 1 OF 3
							-	merlin HOV	Flyove	r			STATION	HOV 90+9 0.2 m Rt.	91.1
					DESCRI			s, Nevada	1 190 00	•			OFFSET	SMM	
	22						3-04	0, 1101000					ENGINEER EQUIPMENT		0-50 Turbo
	N.M.						24-01-7	7		GROU	NDWATER		OPERATOR	Eagle Dril	
	1				ROJECT #		9.67 (m			DATE	DEPTH m		DRILLING	8" Auger/I	NO Core
						L V		vutomatic					METHOD	Vee	4/44/0007
					AMMER DR		SIEM_						BACKFILLED	D	ATE
	ELEV. (m)	DEPTH (m)		MPLE TYPE	BLOW C 150 mm Increments	Last	Percent Recov'd		USCS Group						<b>REMARKS</b> Drive hammer
	718.67 -	- - -  1 - - - - 1.52							sc		very dens 52% fine t	e, with 20% to coarse sa	avel Tan, brow medium plastic and, and 28% fir prounded grave	city fines, ne to	had an efficiency of 69 percent. Blowcounts can be converted to Standard SPT N60 by multiplying by
		_ 1.65	Α	SPT	50/125 5	0/125mi	n 28	Sv, PI, M							1.15.
		_ 1.83							0.44	1.83	Well Grad	ded Gravel	Tan, wet, very	dense	Drilled NC core
	717.67 -	-2							GW	2.13	with 2% m	nedium plas	sticity fines, 7% to 6, 7% to 6, 7% to 7, 7% to	fine to	from 1.5 to 4.5
	716.67 -	- - 		CORE			15	Sv, PI, M			subrounde 4.5 meters 1.8 to 2 m consisting subrounde	ed gravel. s. Only cor leters interv g of clean m ed volcanic	Attempted to co e recovery was al with the rema edium to coarse gravel.	re 1.5 to from the ainder e	meters. Poor recovery.
		3.32	С	SPT	9 50/125 <sup>5</sup>	0/125mi	n 61	Sv, PI, M	GW GM				with Silt and Sa e, with 6% non-p		
	715.67 -	- - 	C'	CORE			0			4.27	fines, 37% coarse an	6 fine to coa gular to sul	arse sand, and sorounded grave	57% fine to	
		4.57		SPT	50/125 5	0/125m	n 28	Sv, PI, M	_		dense, wit	th 16% med	lium plasticity fi	nes, 45%	
		- 4.03		JSF I	50/1255	0/12511	11 20		sc		angular to	arse sand, a subrounde	and 39% fine to ed gravel.	coarse	
	714.67 -	-5							00						
		-								F 40					
	713.67 -	- - - - - 6.19 - -	E	SPT	_50/75mm	0/75mn	17	Sv, PI, M	-	5.49	dense, wit 46% fine t	th 12% low to coarse sa	avel Tan, grey, to medium plas and, and 42% fi prounded grave	ticity fines, ne to	
38	712.67 -	- - - - - - - - - - - - - - - - - - -	F	SPT	50/50mm	0/50mm	11		SC						
0/200		-							1						
Γ 5/3(	711.67 -	-8													
-GD1		t													
NV_DOT 0324017.GPJ NV_DOT.GDT 5/30/2008	710.67 -	- - - 9 <u>9.14</u> - 9.42	6	SPT	26 50/125 <sup>5</sup>	0/125mi	n 61	Sv, Pl, M	sc	8.53	dense, wit fine to coa	th 19% med	<b>avel</b> Tan, grey, lium plasticity fir and 35% fine to ad gravel.	nes, 46%	
		F													



						A/*	11/07			EXPL	ORATIO	N LOG			
					TART DATE		11/07								SHEET 3 OF 3
					ND DATE			 merlin HOV	Flyova	r			STATION	HOV 90+9	91.1
					DB DESCRI		-	s, Nevada	TIYOVC				OFFSET	SMM	
	24				DCATION		3-04	3, NCVAUA					ENGINEER EQUIPMENT	Diedrich D	-50 Turbo
	× vy				ORING		3-04 324-01-7	7	[	GROU	NDWATE		OPERATOR	Eagle Dril	
	1				ROJECT #		9.67 (m			DATE		ELEV.	DRILLING	8" Auger/I	
					ROUND EL	L V		utomatic	— [				METHOD		ATE _ 4/11/2007
					AMMER DR		STEM	atomatic	[				BACKFILLED	D	ATE
	ELEV. (m)	DEPTH (m)		MPLE TYPE	BLOW CO 150 mm Increments	Last	Percent Recov'd	LAB TESTS	USCS Group		MAT	ERIAL DE	ESCRIPTION		REMARKS
	698.67 - 697.67 -	- - - - - - - - - - - - - - - - - - -	0	SPT	-50/50mm5	0/50mn	<u> </u>		SC						
	696.67 -	- 22.86 -2 <b>3</b> 3.07 - -		SPT	49 50/ 50mm	0/50mn	n 44	Sv, PI, M	-	23.77					
	695.67 -	-24							GC		dense, wi	ith 14% mee arse sand, a	Sand Tan, grey dium plasticity fi and 50% fine to	nes, 36%	
		24.38 - 24.48	Q	SPT	50/100 5	0/100mr	n 22	Sv, Pl, M	_	24.48	angular to	o subrounde	ed gravel.		
	694.67 -	- 25 													
	693.67 -	- 26 													
38	692.67 -	- 27 - -													
V_DOT.GDT 5/30/200	691.67 -	- 28 - -													
NV_DOT 0324017.GPJ NV_DOT.GDT 5/30/2008	690.67 -	- 29 - - -													

ſ				~		⊿/	10/07			EXPLO	DRATION LOG			
					TART DATE	·	10/07							SHEET 1 OF 3
					ND DATE			 merlin HOV	Flyove	۰r		STATION	HOV 89+7	/1.3
					DB DESCRI		-	s, Nevada	iyuve	1		OFFSET	<u>0.2 m Rt.</u> SMM	
	2 m				DCATION		3-05	o, merdua				ENGINEER EQUIPMENT		0-50 Turbo
	Xnn 4				ORING		5-05 524-01-7	7		GROUI	NDWATER LEVEL	OPERATOR	Eagle Dril	
	The second se				ROJECT #		8.92 (m				DEPTH m ELEV. m	DRILLING	8" Auger/I	
	le				ROUND EL	L V		utomatic				METHOD		
				H	AMMER DR		STEM_A	utomatic				BACKFILLED	D	ATE 4/10/2007
	ELEV. (m)	DEPTH (m)		MPLE TYPE	BLOW CO 150 mm Increments	Last	Percent Recov'd	LAB TESTS	USCS Group		MATERIAL DE			REMARKS
	717.92 -	- - - - - 1 - - - - 1.52									Silty Sand with Grave moist, very dense, wit fines, 51% fine to coa coarse angular to sub	h 13% medium rse sand, and 3	plasticity 36% fine to	Drive hammer had an efficiency of 69 percent. Blowcounts can be converted to Standard SPT N60 by multiplying by 1.15.
		1.78	•	SPT	43 50/100 <sup>50</sup>	0/100mi	n 56	Sv, PI, M			Attempt NC core run	from 1.8 to 3 n	n and 3.5	
	740.00				50/100						to 4.5 meters. Very p of 0.5 meter of clean	oor recovery o coarse volcani	composed	
	716.92 -	2 - - - - - - - - - -	В	CORE			15		SM					
	714.92 -	- 4								4.27				
		4.57									Poorly Graded Grave Tan, brown, slightly m	I with Clay and	l Sand	
		- 4.70	С	SPT	50/125 5	0/125mi	n 28	Sv, PI, M	-		11% medium plasticity	y fines, 40% fin	e to coarse	
	713.92 -	- 4.88 - 5							-		sand, and 49% fine to subrounded volcanic	o coarse angula gravel.	ir to	
	/13.92 -	-										gravon		
	712.92 -	- - - - -	D	SPT	50/75mm5	0/75mn	17		GP GC					
	711.92 -	7												
		-												
		7:67												
2008		- 7:07	E	SPT	50/50mm5	0/50mn	11							
5/30/	710.92 -	-8												
3DT		-												
0T.C		+								8.53				
NV_DOT 0324017.GPJ NV_DOT.GDT 5/30/2008	709.92 -	- - - 9 <u>9:19</u> -	F	SPT	-50/50mm5	i0/50mn	11				Silty Sand Tan, sligh with 19% non-plastic t sand, and 12% fine to subrounded gravel.	fines, 69% fine	to coarse	
D_NN		_												



				-			10/07			EXPLORATION L	OG			
					TART DATE		10/07							SHEET 3 OF 3
					ND DATE			morlin LIOV	Elver	r		STATION	HOV 89+7	/1.3
					DB DESCRI			merlin HOV	Fiyove	ſ		OFFSET	0.2 m Rt.	
	24			LC	OCATION		-	s, Nevada				ENGINEER	SMM Diedrich D	
	2MM	6		BC	ORING		3-05	_				EQUIPMENT	Eagle Drill	
	Z.			PF	ROJECT #		24-01-7			GROUNDWATER LEV				
	Ž.			GI	ROUND EL	EV71	8.92 (n			DATE DEPTH m ELE	EV. M	DRILLING METHOD	8" Auger/N	
			1	HA	AMMER DF	ROP SYS	STEM_A	utomatic				BACKFILLED	Yes D	ATE 4/10/2007
	ELEV.	DEPTH		MPLE TYPE	BLOW C 150 mm Increments	OUNT Last	Percent	LAB TESTS	USCS Group	MATERI	AL DE	SCRIPTION		REMARKS
	(m)	(m)	110.		Increments	300 mm	Recov'd		Group	coarse angula	ar to sub	orounded gravel		
		Ľ										-		
		-												
	697.92 -	-21												
		21.34		SPT			0							
		F												
	696.92 -	-22							SC					
		+												
		F												
		- 22.89												
	695.92 -	-23	K	SPT	50/50mm	50/50mn	11	Sv, Pl, M	-					
		+												
		+												
		-												
	694.92 -	-24												
	034.92													
		24:48		SPT	50/50mm	50/50mn	n 0		_	24.43				
		-												
	000.00	-												
	693.92 -	-25												
		-												
		-												
		-												
	692.92 -	-26												
		-												
		+												
	691.92 -	-27												
		Ľ												
		Ļ												
)/200.		F												
5/30	690.92 -	-28												
GDT		F												
DOT.		Ľ												
N														
GPJ	689.92 -	-29												
NV_DOT 0324017.GPJ NV_DOT.GDT 5/30/2008		F												
0324		F												
DOT		t												
N														

						. 4/	9/07			EXPL	ORATIO	N LOG			
					TART DATE		9/07								SHEET 1 OF 3
								 merlin HOV	Flyove	r			STATION	HOV 89+0 0.1 m Rt.	19.9
					DB DESCRI			s, Nevada	1 190 10	'			OFFSET	SMM	
	22				DCATION		3-06	0, 140 4000					ENGINEER EQUIPMENT		0-50 Turbo
	× v						324-01-7	7		GROU	INDWATER		OPERATOR	Eagle Dril	ling
	1 miles				ROJECT #		8.38 (m				DEPTH m		DRILLING	8" Hollow	Stem Auger
						L V		utomatic					METHOD	Vaa	4/0/2007
					AMMER DF		STEM		I				BACKFILLED	<u> </u>	ATE 4/9/2007
	ELEV. (m)	DEPTH (m)		MPLE TYPE	BLOW C 150 mm Increments	Last	Percent Recov'd		USCS Group						REMARKS
	717.38 -	- - - - - - - - - - - - - - - - - - -	A	SPT	50/50mm	30/50mn	- 11	Sv. Pl. M	SC SM		dry, very o 44% fine t subrounde	dense, with to coarse sa ed fine to m sts are com	ith Gravel Tan, 12% low plastic and, and 44% an edium gravel. \$ posed of calicher	ity fines, ngular to Some	Drive hammer had an efficiency of 69 percent. Blowcounts can be converted to Standard SPT N60 by multiplying by 1.15.
	716.38 -	-2													
	715.38 -	- - - <u>3</u> 3.05 <u>3.18</u>	В	SPT	50/125 5	0/125mr	m 28	<del>Sv, PI, M, Chem</del>	SP	2.44	brown, dry fines, 62% angular to Some gra	y, very dens 6 fine to coa 9 subrounde	with Silt and G se, with 9% non- arse sand, and 2 ed fine to mediuu re composed of origin.	plastic 29% n gravel.	
	714.38 -	- - - - - - - - - - - - - - - - - - -								4.27			r <b>ith Gravel</b> Tan, 13% low plastic		
	713.38 -	4.57 - 4.67 - 	С	SPT	50/100 5	0/100mr	n 22	Sv, Pl, M	SC SM	5.49	53% fine t	to coarse sa	and, and 34% and and and gravel.	ngular to	
	712.38 -	- - - 6 6.10 - 6.22		SPT	50/125 5	0/125mr	n 28	Sv, PI, M	sc	0.40	very dens 59% fine t	e, with 14% to coarse sa	avel Tan, brow b medium plastic and, and 27% an ledium gravel.	ty fines,	
	711.38 -	- 7 7								7.32	Clavey Sa	and with Gr	<b>avel</b> Tan, brow	n drv	
JT.GDT 5/30/2008	710.38 -	- 7.62 - 7.72 - 8 -	E	SPT	50/100 5	0/100mr	n 22	Sv, Pl, M			very dens fines, 46-	e, with 19-2 51% fine to	25% medium pla coarse sand, ar ed fine to mediur	sticity nd 25-34%	
NV_DOT 0324017.GPJ NV_DOT.GDT 5/30/2008	709.38 -	- - - 9 9.14 - 9.24 -	F	SPT	50/100 5	0/100mr	n 22	Sv, PI, M	-						
		_													

				0		- 4/9	9/07			EXPL	ORATION LOG			
					TART DATE ND DATE		9/07						HOV 89+	SHEET 2 OF 3
		7			ND DATE			merlin HOV	Flvove	r		STATION .	0.1 m Rt.	09.9
					DESCRI			s, Nevada		<u> </u>		OFFSET	SMM	
	44				ORING		3-06	-,				EQUIPMENT .	Diedrich	D-50 Turbo
	S.				ROJECT #		24-01-7	7	[	GROL	INDWATER LEVEL	OPERATOR .	Eagle Dri	lling
	1 miles					74	8.38 (n			DATE	DEPTH m ELEV. m	DRILLING	8" Hollow	Stem Auger
						v	Ì	Automatic				METHOD .	Vee	DATE 4/9/2007
								1	L			BACKFILLED		
	ELEV. (m)	DEPTH (m)		MPLE TYPE	BLOW C 150 mm Increments	Last	Percent Recovid	LAB TESTS	USCS Group		MATERIAL DE	SCRIPTION		REMARKS
	()	()			Increments	300 1111	Recovu							
									sc					
		- 10.67												
		10.81	G	SPT	50/137 5	0/137mr	n 30	Sv, PI, M	-					
	707.38 -	-11												
		[												
		-												
		-												
	706.38 -	- 12 1 <u>3</u> .29		0.007										
			н	SPI	50/50mm	50/50mn	1 11							
		-												
		-												
	705.38 -	- 13												
										13.41				
		13 72									Silty, Clayey Sand wi dry, very dense, with	th Gravel Tan,	brown, ty fines	
		13:79		SPT	50/75mm	50/75mm	17	Sv, Pl, M			56% fine to coarse sa	ind, and 22% ar	igular to	
	704.38 -	- 14							SC SM		subrounded fine to m	edium gravel.		
		-												
										14.63				
		-									Clayey Sand with Gravery dense, with 24%	avel Tan, brown	n, dry, itv fines	
	703.38 -	- 15									46% fine to coarse sa	ind, and 30% ar	igular to	
		_ <u>15.24</u> - 15.47		SPT	37 50/75mm	50/75mm	n 50	Sv, PI, M	-		subrounded fine to co	arse gravel.		
		- 15.47			50/75mm`				sc					
		-												
	702.38 -	- 16												
		-								10.40				
										16.46	Silty Sand with Grave	el Tan, brown, o	dry, very	_
		16:84	к	SPT	50/75 5	50/75mn	1 17	Sv, PI, M			dense, with 14% low coarse sand, and 34%	plasticity fines, 5	52% fine to	
	701.38 -	- 17									fine to coarse gravel.	o angular to suc	ioundeu	
		-												
		-												
2008														
5/30	700.38 -	-18							SM					
NV_DOT 0324017.GPJ NV_DOT.GDT 5/30/2008		- 18.29			10									
).TOC		40.07	L	SPT		4/225mr	n 83							
NV_E		- 18.67			50/75mm				-					
GPJ	699.38 -	- 19												
4017.		-												
0324		F								19.51	<u> </u>			_
DOT		- 19.81 - 19.91									Clayey Sand with Gravery dense, with 18%			
N		19.91	М	SPT	50/100 5	0/100mr	n 22	Sv, PI, M			42% fine to coarse sa			

ſ						A/	9/07			EXPL	ORATIO	N LOG			
					TART DATE		9/07								SHEET 3 OF 3
					ND DATE			 merlin HOV	Elvovo	-			STATION	HOV 89+0	9.9
					DB DESCRI				riyove				OFFSET	0.1 m Rt. SMM	
	- Ann	<b></b>			OCATION		3-06	s, Nevada					ENGINEER	Diedrich D	)-50 Turbo
	×.				ORING		5-00 524-01-7	7	[				EQUIPMENT OPERATOR	Eagle Drill	
	and the second se				ROJECT #					DATE	NDWATER		DRILLING		
	Ve				ROUND EL	L V	8.38 (m	,	— t				METHOD		Stem Auger
				H	AMMER DR	ROP SYS	STEM_	utomatic	L				BACKFILLED	Yes D	ATE 4/9/2007
	ELEV. (m)	DEPTH (m)		MPLE TYPE	BLOW CO 150 mm Increments	Last	Percent Recov'd	LAB TESTS	USCS Group				ESCRIPTION	l	REMARKS
		-									subround	ed fine to c	oarse gravel.		
	697.38 -	- - 21							sc						
		21.34 - 21.46	N	SPT	50/125 5	0/125mi	n 28		-						
		-													
	696.38 -	-22													
		-								22.56					
		- <u>22.</u> 89									Clayey Sa verv dens	and with Gr e. with 31%	avel Tan, brow medium plastic	n, dry, citv fines.	
	695.38 -	-23	0	SPT	50/50mm5	50/50mn	11	Sv, Pl, M			47% fine	to coarse sa	and, and 22% a	ngular to	
		-							SC		are prima	rily compos	oarse gravel. G ed of caliche.	ravel clasts	
		-								23.77	Clayey Sa	and with Gr	avel Tan, brow	n, dry,	
	694.38 -	- 24							sc		very dens 43% fine	e, with 22% to coarse s	6 medium plastic and, and 35% a	city fines, ngular to	
		24.38 24.59	-	SPT	42 50/50mm <sup>8</sup>	0/50mn	1 44	Sv, PI, M	-	24 50	subround	ed fine to co	oarse gravel. G ed of caliche.	ravel clasts	
					50/50mm`	0,001111				24.55	ure prind				
	693.38 -	-25													
		-													
		-													
		F													
	692.38 -	-26													
		-													
		F													
	691.38 -	-27													
		[													
80		-													
30/20	000.05	-													
JT 5/;	690.38 -	- 28													
DT.GL		F													
		-													
N Lď	689.38 -														
017.G	003.00														
NV_DOT 0324017.GPJ NV_DOT.GDT 5/30/2008		-													
DOT		Ĺ													
Ž															

ſ							1/	11/07			EXPL	ORATIO	N LOG			
						FART DATE		11/07								SHEET 1 OF 1
						ND DATE			 mortin LIOV		-			STATION	HOV 87+	58.1
					JC	DB DESCR			merlin HOV	FIYOVE	1			OFFSET	9.0 m Lt. SMM	
	- An	<b>.</b>			LC	DCATION			s, Nevada					ENGINEER		D-50 Turbo
	2 Mary					ORING		3-07	7		0001			EQUIPMENT OPERATOR	Eagle Dril	
	1 miles	$\geq$				ROJECT #		24-01-7			DATE	NDWATER		DRILLING		
	Ve					ROUND EL	L V	8.52 (m	,		DITE			METHOD	Vee	Stem Auger
					HA	AMMER DF	ROP SYS	STEM_	utomatic					BACKFILLED	Yes D	ATE 4/11/2007
	ELEV. (m)	DEPT (m)	нГ		<u>IPLE</u> TYPE	BLOW C 150 mm Increments	Last	Percent Recov'd	LAB TESTS	USCS Group				SCRIPTION		REMARKS
	717.52 -	- - - - - - - - - -	52	A	SPT	50/50mm	:0/50mn	11	Sv, Pl, M	SC		very dense 44% fine t subrounde	e, with 18% to coarse sa ed fine to m sts are com	avel Tan, brow medium plastic and, and 38% a ledium gravel. S posed of calich	city fines, ngular to Some	Drive hammer had an efficiency of 69 percent. Blowcounts can be converted to Standard SPT N60 by multiplying by 1.15.
	716.52 -	-2 - -														
		-									2.74					-
	715.52 -	- <u>3</u> 3. - - 3.		в	SPT	23 49 9 50/75mm	9/225mi	n 83	Sv, Pl, M	sc		dense, wit fine to coa	th 18% med	ravel Tan, dry, v dium plasticity fi and 35% fine to ed gravel.	nes, 47%	
	714.52 -	- 4 -									4.27	Clavov Sa	and with Gr	avel Tan, dry, v		
		4	57	~	CDT	50/75mm	0/75mn	1 17	Sv, PI, M			dense, wit	th 17% med	dium plasticity fi	nes, 47%	
	713.52 -	5		U		<u> </u>	JOT SIIII		- 3V, F 1, W	sc			arse sand, a subrounde	and 36% fine to ad gravel.	coarse	
		F									5.49					
	712.52 -	- - - 6 6. - 6.	10	D	SPT	50/100 5	0/100mr	n 22	Sv, Pl, M	GC	6.19	dense, wit fine to coa	th 15% mea	and Tan, dry, v dium plasticity fi and 43% fine to ed gravel.	nes, 42%	-
08	711.52 -	- - - 7 - -														
OT.GDT 5/30/200	710.52 -	- 8 -														
NV_DOT 0324017.GPJ NV_DOT.GDT 5/30/2008	709.52 -	- - - - -														
NV_D(		_														

BIN CHIE         10/17/07           JOB DESCRIPTION         SUMMERIN NUMBER           JOB DESCRIPTION         STATION           JOB DESCRIPTION         STATION           BOB NA         FP-08           PROJECT #         S324:01-7           CROUNDWATER LEVE         DATE DEFINIT           PROJECT #         S324:01-7           CROUNDWATER LEVE         DATE DEFINIT           PROJECT #         S324:01-7           CROUNDWATER LEVE         DATE DEFINIT           DATE DEFINIT         ELEV.           OPENTIN         BATE DEFINIT           ELEV.         DEFINIT           NAME REAL DESCRIPTION         ERECTOR           1.52         STATION           1.52         STA						10	/16/07			EXPLO	ORATIO	N LOG			
END DATE         OB DESCRIPTION         Summerlin HOV Flyover         OFFSET         INON PERSON           JOB DESCRIPTION         Las Vegas, Nevada         SMM         SMM         Diedrich D-S0 Tu           BORING         FB-08         SMM         Diedrich D-S0 Tu         Diedrich D-S0 Tu         Diedrich D-S0 Tu           PROLICIT #         0324-01-7         GROUNDWATER LEVL         OPERATOR         Bage Dnilling           PROLICIT #         0324-01-7         GROUNDWATER LEVL         Diedrich D-S0 Tu         Diedrich D-S0 Tu           GROUND ELEV_737.71 (m)         HAMMER DROP SYSTEM_Automatic         GROUNDWATER LEVL         Diedrich D-S0 Tu         Person full Marker DROP SYSTEM Automatic         Person full Marker DROP SYSTEM Automatic         Diedrich D-S0 Tu         Person full Marker DROP SYSTEM Automatic         Porty Graded Gravel with Sity Clay and Sand (FLL) Tan, brown, efficient parameter Balon million fine to coarse angular to subrounded gravel.         Porty Graded Gravel with Sity Clay and Sand (FLL) Tan, brown, efficient parameter Balon million fine to coarse angular to subrounded gravel.         Fill for elevated freeway.         Bulk si collect G-1.50 Tu           736.71         1         1.52         21         78         78         Fill Marker Balon															SHEET 1 OF 2
Location         Las Vegas, Nevada         BNMM           BORING         FE-08         BORING         FB-08           PROJECT #         0324-01-7         Operation         Definition         Definition           GROUND ELEV 737.71 (m)         Definition         Definition         BORING         Eleven           MAMMER DROP SYSTEM         Automatic         DATE         DEPTH         ELEV.         Percent         Provided         Provide							-			-			STATION		
LOANNON         FB-08         Converting         Dedrich         <				JC	DB DESCRI				FIYOVE	1					<u>t.</u>
BORNS         1.500         0.224-01-7         GROUND ELEV         0.322-01-7         GROUNDWATER LEVE         DOFENTION         Eagle Drilling           PROJUCT #         0.322-01-7         GROUND ELEV         0.327-11         DATE         DATE <t< td=""><td>Du</td><td></td><td></td><td>LC</td><td>OCATION</td><td></td><td></td><td>s, nevada</td><td></td><td></td><td></td><td></td><td></td><td></td><td></td></t<>	Du			LC	OCATION			s, nevada							
PROJECT #         USERVIT         ORCENT #         USERVIT         OPENANCE         DELLING         PENANCE	W.			BC	ORING			,	(						
GROUND ELEV.         Participation         METHOD         Induction         METHOD         Induction         Method         Second Second         Method         Second Second Second         Method         Second Second Second         Method         Second Sec				PF	ROJECT #						-				
ELEW         DEPTH         MATERIAL DESCRIPTION         Recover           (m)         (m) <td></td> <td>n -</td> <td></td> <td>Gl</td> <td>ROUND EL</td> <td>EV73</td> <td></td> <td>,</td> <td></td> <td>DATE</td> <td></td> <td></td> <td>METHOD</td> <td></td> <td></td>		n -		Gl	ROUND EL	EV73		,		DATE			METHOD		
LEV.         DEPTH         No.         TYPE         150 mm         Last         Percent         LAB TESTS         Strong         MATERIAL DESCRIPTION         Record           (m)				HA	AMMER DR	OP SYS	STEM_A	utomatic					BACKFILLED	Yes [	DATE 10/17/2007
736.71         1         0.21         Asphal Concrete Pavement           736.71         1         1         0.21         Asphal Concrete Pavement         Drive 1 had a method           736.71         1         1         1         0.21         Asphal Concrete Pavement         Drive 1 had a method           736.71         1         1         1         1         Drive 1 had a method         Drive					150 mm	Last		LAB TESTS	USCS Group		MATI	ERIAL DE	SCRIPTION		REMARKS
738.71       1       1       SPT       21       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1		_								0.21					
736.71       -2       collect         736.71       -2       collect         736.71       -3       -3       -5         737.71       -3       -3       -5         737.71       -4       -4       -4         -4       -4       -5       -7         -7       -5       -5       -7         -7       -5       -7       -7         -7       -7       -7       -7         -7       -7       -7       -7         -7       -7       -7       -7         -7       -7       -7       -7         -7       -7       -7       -7         -7       -7       -7       -7         -7       -7       -7       -7         -7       -7       -7       -7         -7       -7       -7       -7         -7       -7       -7       -7         -7       -7       -7       -7         -7       -7       -7       -7         -7       -7       -7       -7         -7       -7       -7       -7         -7       -	736.71	- - <u>1.52</u> -		SPT	38	76	78	(BULK) Sv, PI, M	_		Sand (FIL dense, wi fines, 30-3 fine to coa	L) Tan, bro th 10-11% I 33% fine to arse angula	own, slightly moi ow to medium p coarse sand, ar r to subrounded	st, very lasticity nd 56-60%	Drive hammer had an efficiency of 69 percent. Blowcounts can be converted to Standard SPT N60 by multiplying by 1.15. Bulk sample
733.71     -4     -4     -4     -4     -4     -4     -4.5r       733.71     -4     -4     -4     -4     -4     -4.5r       732.71     -5     -5     -5     -6     -6     -6       731.71     -6     §.19     -5     -6     -6     -7       731.71     -6     §.19     -7     -7     -7     -7       730.71     -7     -7     -7     -7     -7       730.71     -7     -7     -7     -7     -7       700.71     -7     -7     -7     -7     -7		1 <del>2</del> - - -		SPT		0/102mr	n 25		-						sample collected, 0-1.5m. Bulk sample collected, 1.5-3m. Bulk sample
731.71       6       6.10	733.71	_	С	SPT	50/76mm5	0/76mm	1 33								collected, 3-4.5m.
6:17     SPT     50/76mm50/76mm     0     (BULK) SV, PI, M       -     -     -     -       -     -     -     -       -     -     -     -       -     -     -     -       -     -     -     -       -     -     -     -       -     -     -     -       -     -     -     -       -     -     -     -       -     -     -     -       -     -     -     -       -     -     -     -       -     -     -     -       -     -     -     -       -     -     -     -       -     -     -     -       -     -     -     -       -     -     -     -       -     -     -     -       -     -     -     -       -     -     -     -       -     -     -     -       -     -     -     -       -     -     -     -       -     -     -     -       -     -     -	732.71	- - - - - -													Bulk sample collected, 4.5-6m.
730.71 7 7 62 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7	731.71	1 <b>+6</b> <u>6.10</u>		007	F0/70	0/70		(BULK) Sv.							
7.62 Clayey Sand with Gravel (FILL) Tan, brown,	730.71	- - - <u>8:89</u>	D						-	7 32					Bulk sample collected, 6-7.6m.
7.62 slightly moist very dense with an estimated		F								1.52					1
729.71       -8       -8.38       -9.91       -9.91       -9.91       -9.91       -0.076 mm 50/76 mm 33       BULK) Sv,       -8.84       -8.84       Bulk site to coarse angular to subrounded gravel.       Bulk site to coarse angular to subrounded gravel.       -0.016 mm 50/76 mm 33       Bulk site to coarse angular to subrounded gravel.       -0.016 mm 50/76 mm 33       Bulk site to coarse angular to subrounded gravel.       -0.016 mm 50/76 mm 50/76 mm 50/76 mm 50/76 mm 50/76 mm 33       Bulk site to coarse angular to subrounded gravel.       -0.016 mm 50/76	800	7.62 7.72	E	SPT	50/102.5	0/102mr	n 13				slightly m	oist, very de	ense, with an es	timated	
728.71 9 9.14 9 9.14	729.71 729.71	1 -8							SC		coarse sa	and, and 30	35% fine to coa		Bulk sample collected, 7.6-9m.
************************************	Z R	F								8.84	Poorly C	radad Sand	with Silty Class	and	-
9.91 9.91 9.10.7 9.10.7	V_DOT 0324017.GP.	1 -9 <u>9:14</u> - <u>9:22</u> - - - <u>9.91</u>	G	SPT		0/76mn	1 33		GC GM		Gravel (F dense, wi coarse sa	ILL) Tan, b th 10% low and, and 66	rown, slightly m plasticity fines, 2	oist, very 24% fine to	Bulk sample collected, 9-10.7m.

					10	/16/07			EXPL	ORATIO	N LOG			
						/17/07								SHEET 2 OF 2
				ND DATE			merlin HOV	Flyove	r			STATION	HOV 94+3 18.5 m Rt	
				DB DESCR			s, Nevada	TIYOVC	1			OFFSET	SMM	·
- An-				DCATION		3-08	S, Nevaua							D-50 Turbo
S. Mary			B	ORING			7		000			EQUIPMENT OPERATOR	Eagle Dril	
4			PF	ROJECT #		24-01-7			DATE	INDWATER		DRILLING		•
te			G	ROUND EL	.EV73	57.71 (m	,		DATE			METHOD		Stem Auger
	)		H	AMMER DF	ROP SYS	STEM_A	utomatic					BACKFILLED	Yes D	ATE
ELEV. (m)	DEPTH (m)		IPLE TYPE	BLOW C 150 mm Increments 50/76mm	Last	Percent Recov'd	LAB TESTS	USCS Group		MATE	ERIAL D	ESCRIPTION		REMARKS
	10.13	п	351	50/7011116	J0/701111	1 50		-						
	-								40.07					
	- 10.67 10.77		SPT	50/102 5	0/102mr	n 75	(BULK) Sv, PI, M,	_	10.67	Silty, Clay	/ev Gravel	with Sand (FILL	<b>?)</b> Tan.	
726.71	-11						Chem			brown, sli	ghtly moist	, very dense, wit	h 14%	Dull comple
	-									and 44%	fine to coa	11% fine to coars	e sand, Ibrounded	Bulk sample collected,
	- 11: <del>4</del> 3	J	SPT	50/76mm!	50/76mn	1 67		GC GM		gravel.		-		10.7-12.2m.
	-	-						Givi						
705 74	- 10													
725.71	- 12 12.19 - 12.29		ODT	50/102 5	0/400	- 75	(BULK) Sv.		12.19					4
	-	ĸ	SPT	50/102 5	0/102mr	n 75	PI, M, DS	_		Poorly Gr	aded Grav	vel with Silty Cla	y and	
	-									dense, wi	th 11% low	/ plasticity fines, 3	33% fine to	Bulk sample
	12.95							GP		coarse sa subrounde		5% fine to coarse	angular to	collected, 12.2-13.7m.
724.71	-13 13.17	L	SPT	48 50/76mm	50/76mn	n 89		GC		Subround	eu gruvei.			12.2 10.711.
	-			00/7011111										
									1.0 -0					
	- 13.72 - 13.90	М	SPT	40 50/25mm	50/25mn	1 85			13.72	Poorly Gr	aded San	d with Silt, Clay,	and	-
723.71	-14	IVI	011	50/25mm`	50/251111	00		SW		Gravel (Fl	LL?) Tan,	brown, slightly m	noist, very	
	-							SC	14 33	dense, wit	th an estim	nated 10-15% no 5-50% fine to co	n-plastic to	Bulk sample collected.
	- 14:48	N	ерт	50/76mm	0/76mn	1 33				and 35-40	)% fine to a	coarse angular to	) [	13.7-15.2m.
	-	-11	- JF I	50/7011116						subround	ed gravel. aded Grav	el with Silt and	Sand Tan	Hard, slow
700 74	45									grey, sligh	ntly moist to	o dry, very dense	e, with an	drilling below
722.71	- 15 - 15.24									estimated to coarse	5-10% no sand, and	n-plastic fines, 2 65-70% fine to c	0-25% fine	14.3m.
	15.34	0	SPT	50/102 5	0/102mr	n 25				angular to	subround	ed gravel. Samp	oles consist	
	-							GP GM			or fines re	oken gravel clast coverv.	s with very	Level Ground at
	-													Toe of
721.71	- 1 <b>6</b> 6:88	Р	SPT	50/76mm	0/76mn	33		_						Embankment 722 m. Bulk
	-													sample
														collected, 15.2-16.8m.
	18:39	0	SPT	50/76mm!	0/76mn	1 67			16.82					
720.71	-17													
	-													
	-													
719.71	19													
19.71	- 10													
	-													
718.71	19													
8														
	[													
1								1	1					

NV\_DOT 0324017.GPJ NV\_DOT.GDT 5/30/2008

						10	)/15/07			EXPL	ORATIO	N LOG			
				S	TART DATE										SHEET 1 OF 2
				El	ND DATE	10	0/16/07			_			STATION	HOV 95-	
				JC	DB DESCRI			merlin HOV	Flyove	r			OFFSET	<u>13.3 m F</u>	
	.tw			LC	OCATION			s, Nevada					ENGINEER	SMM	
	M	C In		B	ORING		3-09		r				EQUIPMENT	Eagle Dr	D-50 Turbo
	N.			PI	ROJECT #	03	824-01-7	7			NDWATER		OPERATOR		
	n de			G	ROUND EL	<sub>EV.</sub> _73	86.49 (m	ı)		DATE	DEPTH m	ELEV. m	DRILLING METHOD	8" Hollov	V Stem Auger
				H	AMMER DR		STEM_A	utomatic					BACKFILLED	Yes	DATE 10/16/2007
	ELEV. (m)	DEPTH (m)	SAN NO.	MPLE TYPE	450	Last		LAB TESTS	USCS Group		MATE	ERIAL DE	SCRIPTION		REMARKS
										0.21	•	oncrete Pa			Drive hammer had an
	735.49	- - - - - 1 - - - - - - - - - - - - - -							SP SC	1.52	Gravel (FI very dens non-plasti coarse sa	ILL) Tan, g e, with an e c to low pla nd, and 30-	with Silt, Clay, rey, brown, sligl stimated 5-10% sticity fines, 55- 35% fine to coa ad gravel. Fill fo	htly moist, 60% fine to Irse	efficiency of 69 percent. Blowcounts can be converted to Standard SPT N60 by multiplying by 1.15.
		1.52 1.62	Α	SPT	50/102 5	0/102mr	n 75	(BULK) Sv, PI, M	_	1.52	Poorly Gr	aded Grav	el with Clay and	d Sand	
	734.49	- 2 - -							GP GC		slightly mo plasticity f	oist, very de ines, 30% f	reddish brown, t ense, with 10% l ine to coarse sa ngular to subrou	ow and, and	Bulk sample collected, 1.5-3m.
	733.49	3 3.05								3.05					
	733.49	- 3.29	В	SPT	22 50/102 <sup>5</sup>	0/102mr	m 70	(BULK) Sv, PI. M		0.00			ith Gravel (FILL		-
	732.49	- - - - - 4									12-16% lo coarse sa	w plasticity	noist, very dens fines, 45-56% f 42% fine to coa d gravel.	fine to	Bulk sample collected, 3-4.5m.
		4.57	-		5			(BULK) Sv,	_						
	731.49 ·	<u>4.80</u> - 5 -	С	MC	5 ب 50/76mm	0/76mn	n 33	PI, M	-						Bulk sample collected, 4.5-6m.
	730.49	- - - 6 <u>6.19</u>	—D—	SPT	<del>50/25 {</del>	<del>i0/25mn</del>	100	(BULK) Sv, PI, M	SC SM						
		F													Bulk sample collected,
		- 6.86							_						6-7.6m.
	729.49	<b>7</b> 7.32	Е	SPT	5 5 30	35	44								
		7.62													
2008		7.74	F	SPT	50/127 5	0/127mr	n 80	(BULK) Sv, PI, M	-						
T.GDT 5/30/2008	728.49		G	MC	- 50/25mm	<del>i0/25m</del> n	n 100		=						Bulk sample collected, 7.6-9m.
/_DC		-													Grab sample
7.GPJ NV	727.49	- 9 <u>8:14</u>				<u></u>		(BULK) Sv,		8.84			vith Sand (FILL) very dense, wit		collected due to poor sample recovery.
NV_DOT 0324017.GPJ NV_DOT.GDT		- - - 9.91		SPT	50/76mm5	00/76mn		PI, M, DS, Chem	-		medium p	lasticity fine	es, 43% fine to c o coarse angula	coarse	Bulk sample collected, 9-10.7m.
ź					21										

				S	TART DATE	. 10	/15/07			EXPL	ORATIO	N LOG			SHEET 2 OF 2
	F						/16/07						STATION	HOV 95+6	
					DB DESCRI		Sumr	merlin HOV	Flyove	r			OFFSET	13.3 m Rt	
							s Vega	s, Nevada					ENGINEER	SMM	
	A.						3-09						EQUIPMENT	Diedrich D	-50 Turbo
	E Sala						24-01-7	7		GROU	NDWATER		OPERATOR	Eagle Dril	ling
					ROJECT #		6.49 (m			DATE	DEPTH m		DRILLING METHOD	8" Hollow	Stem Auger
					ROUND EL	L V		,	[					Max	40/40/0007
					AMMER DR		STEM	atomatic					BACKFILLED	D	ATE 10/16/2007
	ELEV. (m)	DEPTH (m)	NO.	MPLE TYPE	Increments	Last	Percent Recov'd	LAB TESTS	Group		MATE	ERIAL DE	SCRIPTION	1	REMARKS
		10.09	Н	SPT	50/25mm5	0/25mn	5/		GC GM						Hard, slow drilling
		-													beginning at
		- 10.67		SPT	50/102 5	0/102mr	n 0		_						9m.
	705 40	-			00/102 0				_						
	725.49 -	-11								11.28					Bulk sample collected,
		_ 11.43								11.20	Poorly Gr	aded Grav	el with Clay and	d Sand	10.7-12.2m.
		- 11.67	Т	SPT	4 50/102 <sup>50</sup>	0/102mr	n 30		GP GC		Tan, brow	n, slightly r	noist, very dens edium plasticity	e, with an	
		-								11.89	30-35% fii	ne to coars	e sand, and 50-	55% fine to _	
	724.49 -	- <b>12</b> 12.19									Clavey Gr	gular to sul	brounded grey g and Tan, brown	gravel. /	
				SPT	4 8 5	8/279mr	n 47		GC		moist, ver	y dense, w	th an estimated	l 15-20%	
		12.62	J	5P1	50/127	6/27900	n 47			12.62	medium p	lasticity fine	es, 25-30% fine to coarse and	to coarse	Level Ground at
		_										ed grey gra		<b>J</b>	Toe of
	723.49 -	- 13													Embankment 724 m.
		-													
		-													
		-													
	722.49 -	-14													
	122.45	-													
		-													
		-													
		-													
	721.49 -	- 15													
		_													
		-													
	720.49 -	- 16													
		-													
		-													
	719.49 -	-17													
	110.10														
		-													
908		-													
30/20		-													
NT 5/.	718.49 -	- 18													
T.GD															
DO		F													
^N ∩		F													
7.GP,	717.49 -	- 19													
NV_DOT 0324017.GPJ NV_DOT.GDT 5/30/2008		Ĺ													
T 03.		Ĺ													
DO		-													
2															

						. 10	/18/07			EXPL	ORATIO	N LOG			
					TART DATE		/19/07								SHEET 1 OF 2
					ND DATE			 nerlin HOV	Elvovo	r			STATION	HOV 96+	
					DB DESCR				FIyOve	1			OFFSET	<u>13.3 m R</u> SMM	l
	- An			LC	DCATION		•	s, Nevada					ENGINEER		D-50 Turbo
	2Mm			B	ORING		3-10	,	[	000			EQUIPMENT OPERATOR	Eagle Dri	
	4				ROJECT #		24-01-7			DATE	NDWATER		DRILLING		
	he			G	ROUND EL	EV73	5.70 (m	,		DATE			METHOD		Stem Auger
				H	AMMER DF	OP SYS	STEM_A	utomatic	[				BACKFILLED	Yes [	DATE
	ELEV. (m)	DEPTH (m)	SAI NO.	MPLE TYPE	BLOW C 150 mm Increments	Last	Percent Recov'd	LAB TESTS	USCS Group		MATE	RIAL DE	ESCRIPTION		REMARKS
		-								0.21	Asphalt C				Drive hammer had an
	734.70	- - - - 1 - - - - - - - - - - - - - - -									brown, gre 13% low p sand, and	ey, slightly i lasticity fin	with Sand (FILL moist, very dens es, 39% fine to o coarse angula vel.	se, with coarse	efficiency of 69 percent. Blowcounts can be converted to Standard SPT N60 by multiplying by 1.15. Bulk
		-	А	SPT		8/203mr	n 71	(BULK) Sv, PI, M	GC GM						sample collected,
	733.70	- 1.89 - 2 - -			_50/51mm										0-1.5m. Bulk sample collected, 1.5-3m. Hard drillling, 2.1-2.4m.
	732.70	3 3.05						(BULK) Sv,		3.05					2.1 2.3111.
	731.70	<u>3.20</u>	B	SPT	50/152 5	0/152mr	n 34	PI, M, DS, Chem	-		and Sand moist, very 36-39% fir coarse and Poor same	(FILL) Tar y dense, wi ne to coars gular to sul ple return w	ed Gravel with \$ n, brown, grey, s th 10% low plas e sand, and 51- brounded grey g vith samples cor rravel chips. Ha	slightly sticity fines, 54% fine to gravel. mposed	Bulk sample collected, 3-4.5m.
	730.70	- <u>4:67</u> - 5	С	SPT	50/76mm	0/76mn	17	<u>(BULK) Sv,</u> PI, M	GW GC				m classifies as ell graded.	poorly	Bulk sample collected, 4.5-6m.
	729.70	6 6.10 6.20						(BULK) Sv.		6.10					
	728.70	- 6.86 - 6.96 - 7 - 7		SPT	50/102 5			PI, M	-		(FILL?) W to dry, ver 33-40% fir	/hite, grey, y dense, w ne to coars	el with Silt and tan, brown, slig ith 9% non-plas e sand, and 51- brounded grave	htly moist tic fines, 58% fine to	Bulk sample collected, 6-7.6m.
		7.62							0.5						
DT 5/30/2008	727.70	- 7.85 - 7.85	F	SPT	45 50/76mm	0/76mn	100	(BULK) Sv, Pl, M	GP GM						Bulk sample collected, 7.6-9m.
JT.GL		8.38	<u> </u>	0.07	12	0/70			_						
		8.61	G	SPT	, 12 50/76mm	0/76mn	1 56		-						
N N		F.													
17.Gł	726.70	9 9.14		0.57	38 ,	0/05		(BULK) Sv,		9.14	Decit: C	adad C		1 Oor -1	
NV_DOT 0324017.GPJ NV_DOT.GDT 5/30/2008		9.33 - - - 9.91 - 10.01		SPT	50/25mm	0/25mn	1 58	PI, M, DS, HM			(FILL?) T dense, wit	an, brown, h 9% low p nd, and 579	el with Clay and slightly moist, v plasticity fines, 3 % fine to coarse	ery 4% fine to	Bulk sample collected, 9-10.7m. HM is Harvard Minature
z		10.01	1	1	1	I	1	1	1	1					1

	>			S	TART DATE	_ 10	)/18/07			EXPL	ORATIO	N LOG			SHEET 2 OF 2
	Ē			E	ND DATE	10	)/19/07						STATION	HOV 96+2	
					DB DESCR		Sum	merlin HOV	Flyove	r			OFFSET	13.3 m Rt	
							is Vega	s, Nevada					ENGINEER	SMM	
	Ary V				ORING	FE	3-10						EQUIPMENT	Diedrich D	)-50 Turbo
	Zar.				ROJECT #	03	324-01-7	7	[	GROU	NDWATER	RLEVEL	OPERATOR	Eagle Dril	ling
	1 m				ROUND EL		85.70 (m			DATE	DEPTH m		DRILLING METHOD	8" Hollow	Stem Auger
					AMMER DF			,	— F				BACKFILLED		ATE 10/19/2007
ļ									L			I]	BACKFILLED	D	ATE
	ELEV.	DEPTH (m)	NO.	MPLE TYPE	BLOW C 150 mm	Last	Percent	LAB TESTS	USCS Group		MATE		ESCRIPTION	l	REMARKS
ŀ	(m)	(11)		SPT	Increments 50/102 5	0/102mi	n 50								compaction test
		-													(NDOT T101E)
		- 10.67	,						GC						
		10.90		SPT	13 50/76mm	50/76mn	n 33								Ditch at Toe of
	724.70 -	-11			- 50/7011111										Embankment 725.0 m. Bulk
		-													sample
		- 11:43	ĸ	SPT	50/51mm	50/51mn	1 26								collected, 10.7-12.2m.
										11.89					
	723.70 -	-12							GC	11.00	Clayey G	ravel with S	Sand Tan, brow	n, slightly	
		- 12 12.19 - 12.29	) ' L	SPT	50/102 5	0/102mi	m 50		GC	12.28	moist, ver	y dense, w	th an estimated es, 30-35% fine	15-20%	
		-									sand, and	l 45-50% fir	ne to coarse ang	gular to	
											subround	ed gravel.			
	722.70 -	-13													
		-													
		-													
		-													
	704 70	-													
	721.70 -	- 14													
		_													
		-													
		-													
	720.70 -	- 15													
		_													
		-													
	719.70 -	- 16													
		-													
		Ľ													
	718.70 -	-17													
		-													
		-													
2008		-													
6/30/2	717.70 -	- 18													
DT 5	0	- '													
DT.G		-													
		-													
N G	740	-													
17.GI	716.70 -	- 19													
3240		L													
NV_DOT 0324017.GPJ NV_DOT.GDT 5/30/2008		-													
		-													
z		1	1	1	1	1	1		1	1					

**B-2** 

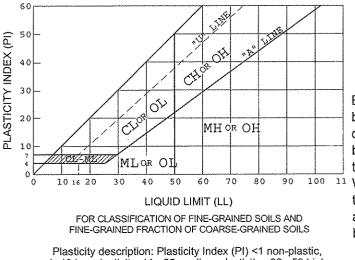
# **KEY TO BORING LOGS**

### SOIL CLASSIFICATION CHART

		SYM	SOLS	TYPICAL	
MAJOR DIVISIONS		GRAPH	LETTER	DESCRIPTIONS	
	GRAVEL	CLEAN GRAVELS		GW	WELL-GRADED GRAVELS, GRAVEL - SAND MIXTURES, LITTLE OR NO FINES
COARSE GRAINED SOILS	AND GRAVELLY SOILS MORE THAN 50% OF COARSE FRACTION RETAINED ON NO. 4 SIEVE	(LITTLE OR NO FINES)		GP	POORLY-GRADED GRAVELS, GRAVEL - SAND MIXTURES, LITTLE OR NO FINES
		GRAVELS WITH FINES		GM	SILTY GRAVELS, GRAVEL - SAND - SILT MIXTURES
		(APPRECIABLE AMOUNT OF FINES)		GC	CLAYEY GRAVELS, GRAVEL - SAND - CLAY MIXTURES
MORE THAN 50% SAND OF MATERIAL IS LARGER THAN NO, 200 SIEVE SIZE SOILS MORE THAN 50% OF COARSE FRACTION PASSING ON NO. 4 SIEVE	AND SANDY	CLEAN SANDS (LITTLE OR NO FINES)		SW	WELL-GRADED SANDS, GRAVELLY SANDS, LITTLE OR NO FINES
				SP	POORLY-GRADED SANDS, GRAVELLY SAND, LITTLE OR NO FINES
	OF COARSE	SANDS WITH FINES		SM	SILTY SANDS, SAND · SILT MIXTURES
	PASSING ON	(APPRECIABLE AMOUNT OF FINES)		SC	CLAYEY SANDS, SAND - CLAY MIXTURES
FINE GRAINED SOILS OF MATERIAL IS SMALLER THAN NO. 200 SIEVE SIZE	SILTS AND CLAYS	LIQUID LIMIT LESS THAN 50		ML	INORGANIC SILTS AND VERY FINE SANDS, ROCK FLOUR, SILTY OR CLAYEY FINE SANDS OR CLAYEY SILTS WITH SLIGHT PLASTICITY
				CL	INORGANIC CLAYS OF LOW TO MEDIUM PLASTICITY, GRAVELLY CLAYS, SANDY CLAYS, SILTY CLAYS, LEAN CLAYS
				OL	ORGANIC SILTS AND ORGANIC SILTY CLAYS OF LOW PLASTICITY
	SILTS AND CLAYS	LIQUID LIMIT GREATER THAN 50		MH	INORGANIC SILTS, MICACEOUS OR DIATOMACEOUS FINE SAND OR SILTY SOILS
				СН	INORGANIC CLAYS OF HIGH PLASTICITY
				он	ORGANIC CLAYS OF MEDIUM TO HIGH PLASTICITY, ORGANIC SILTS
CEMENTED SOILS				CE	CALICHE OR HIGHLY CEMENTED LAYERS
FILL MATERIAL					FILL MATERIAL, NON-NATIVE

NOTE: DUAL SYMBOLS ARE USED TO INDICATE BORDERLINE SOIL CLASSIFICATIONS.

### PLASTICITY CHART



1- 10 low plasticity, 11 - 25 medium plasticity, 26 - 50 high plasticity, >50 very high plasticity



Black Eagle Consulting, Inc. 1345 Capital Blvd., Suite A Reno, Nevada 89502-7140 Telephone: (775) 359-6600 Fax: (775) 359-7766

### EXPLORATION SAMPLE TERMINOLOGY

Sample Type	Sample Code		
Auger Cuttings	Auger		
Bulk (Grab) Sample	Grab		
Modified California Sampler	MC		
Shelby Tube	SH or ST		
Standard Penetration Test	SPT		
Split Spoon	SS		
Field SPT blowcounts with automatic hammer can be corrected to N60 by multiplying by 1.33, unless stated otherwise in log remarks.			

#### MOISTURE CONDITION

Dry	Absence of moisture.
Moist	Damp, no visible water
Wet	Visible Free Water

#### LABORATORY TEST ABBREVIATIONS

Consol=Consolidation; DS = Direct Shear; E=Expansion; HYD = Hydrometer; MD= Moisture and Density PI= Atterberg Limits; R = R-value; Sv = Sieve Analysis; TXCU= Consolidated Undrained Triaxial; TXUU= Unconsolidated Undrained Triaxial; SpGr = Specific Gravity; Chem = Chemical Testing; M = Moisture Content

GRAIN SIZE TE Component of Sample	RMINOLOGY Size Range			
Boulders	Over 12 in. (300mm)			
Cobbles	12 in. to 3 in. (300mm to 75mm)			
Gravel	3 in. to #4 sieve (75mm to 2mm)			
Sand	# 4 to #200 sieve (2mm to 0.074mm)			
Silt or Clay	Passing #200 sieve (0.074mm)			
RELATIVE DENSITY OF GRANULAR SOILS				

### N - Blows/ft Relative Density

Nº DIOWS/IC	ridiario bonony
0 - 4	Very Loose
5 - 10	Loose
11 - 30	Medium Dense
31 - 50	Dense
greater than 50	Very Dense

Blowcounts on this site typically resulted in refusal before achieving all three 150-mm-increments of driving. The blowcount for each increment and the blowcount sum for the last 300 mm of driving is therefore recorded as 50[blows]/25mm typically. Where the last increment of driving exceeds 100mm, there is not enough room between the log columns, and the "mm" label may be omitted (50[blows]/125[mm] being written as 50/125).

### Key to Boring Logs

Project: Summerlin HOV Flyover

Location: Las Vegas, Nevada

Project Number: 0324-01-7 Plate Number:

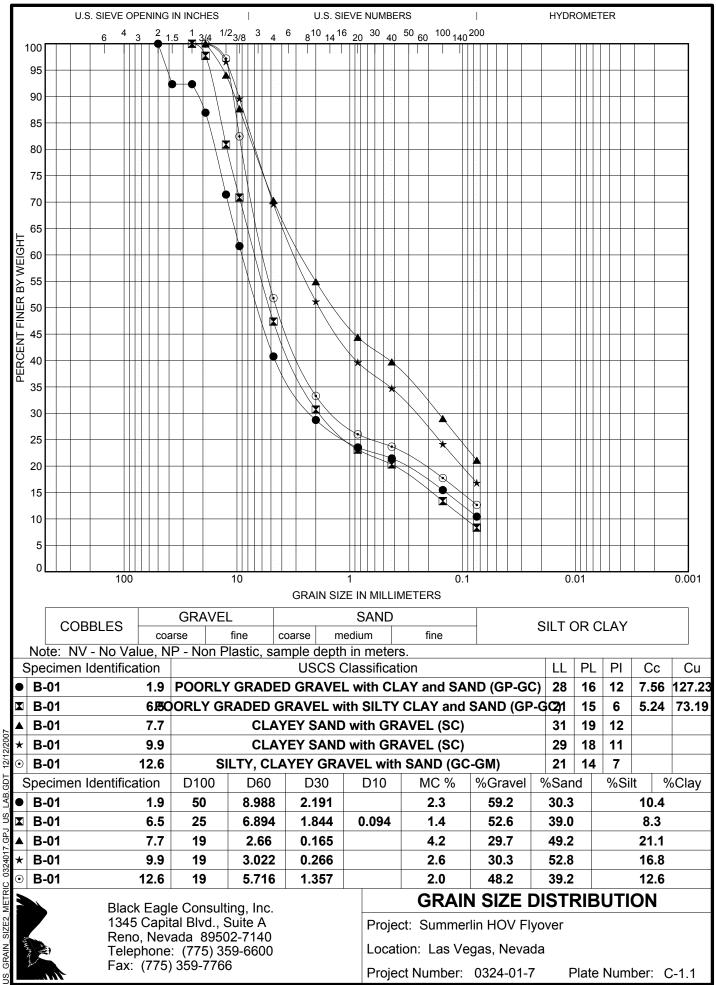
B-2

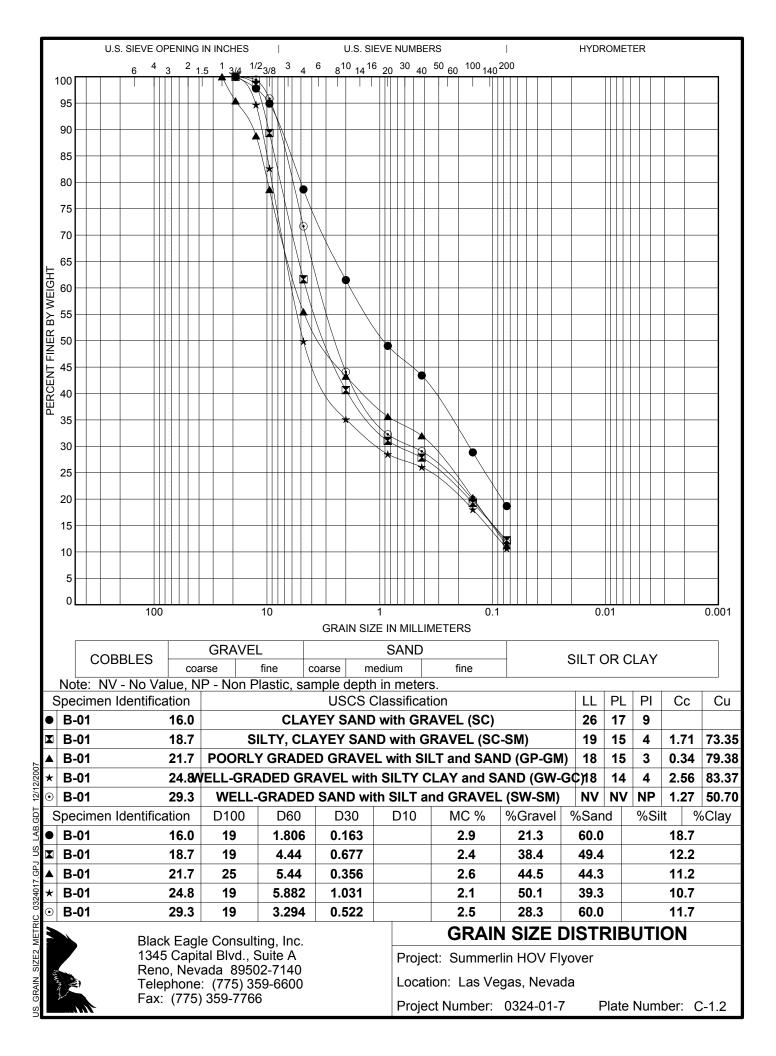
# **APPENDIX C**

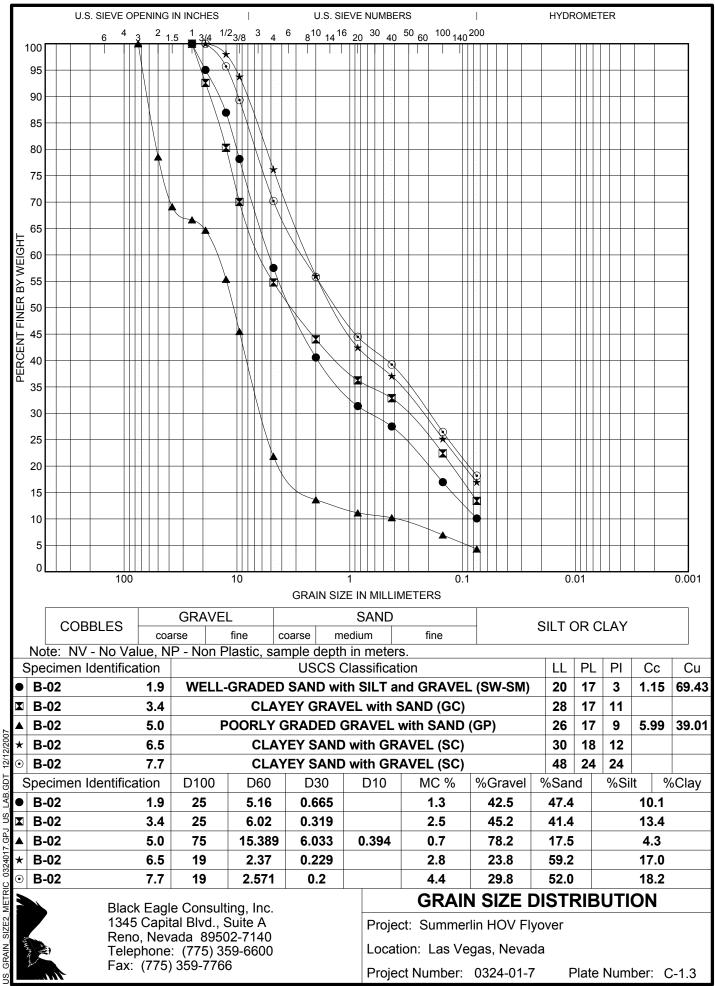
## LABORATORY TEST RESULTS

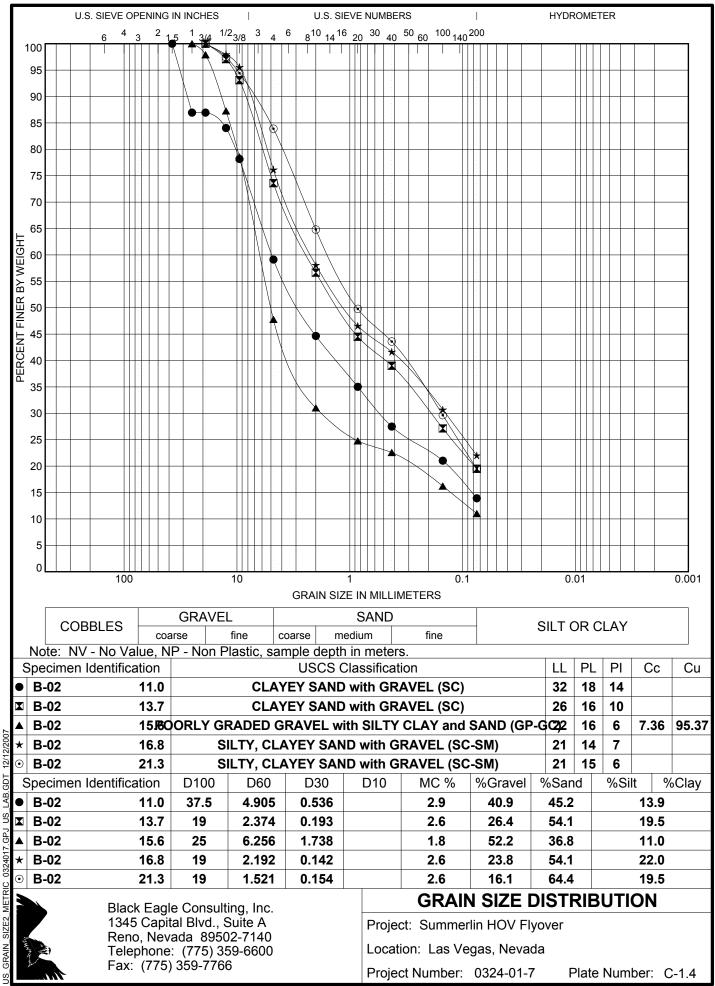
# **C-1**

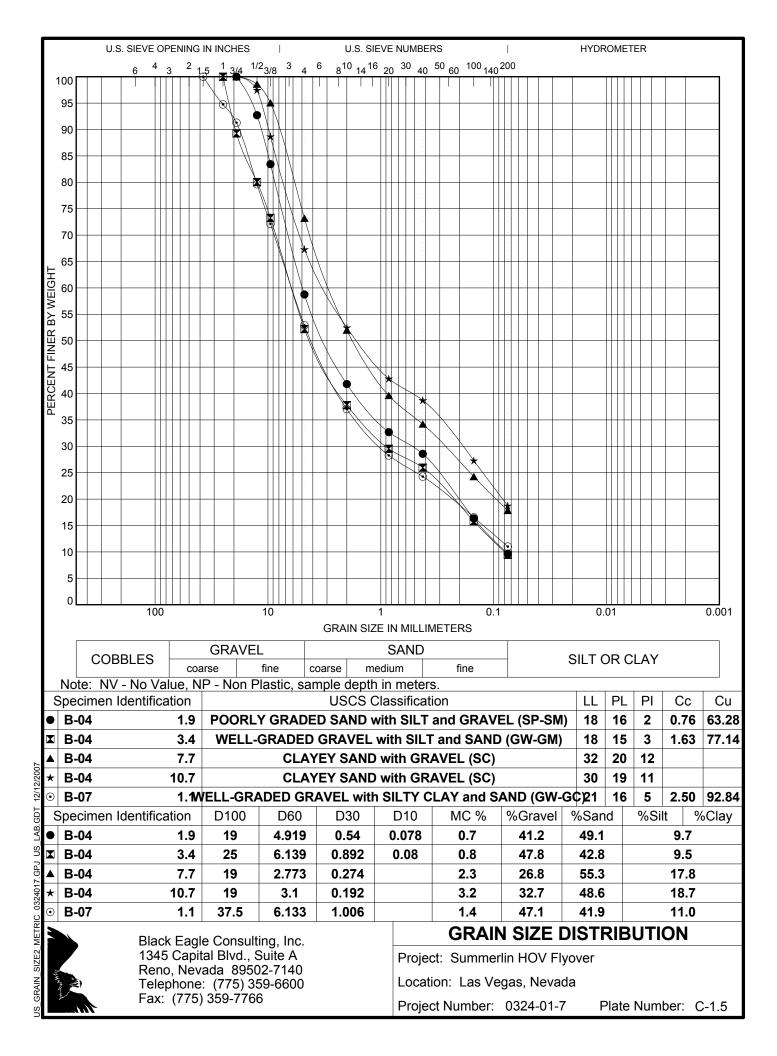
# **INDEX TEST RESULTS**

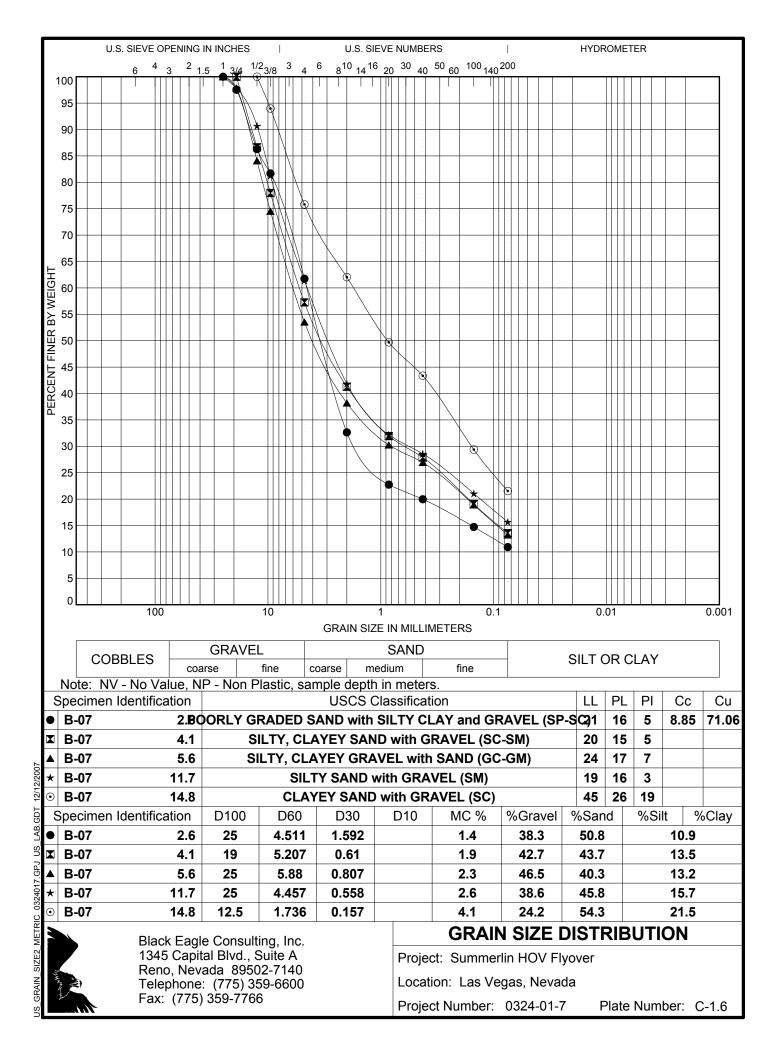


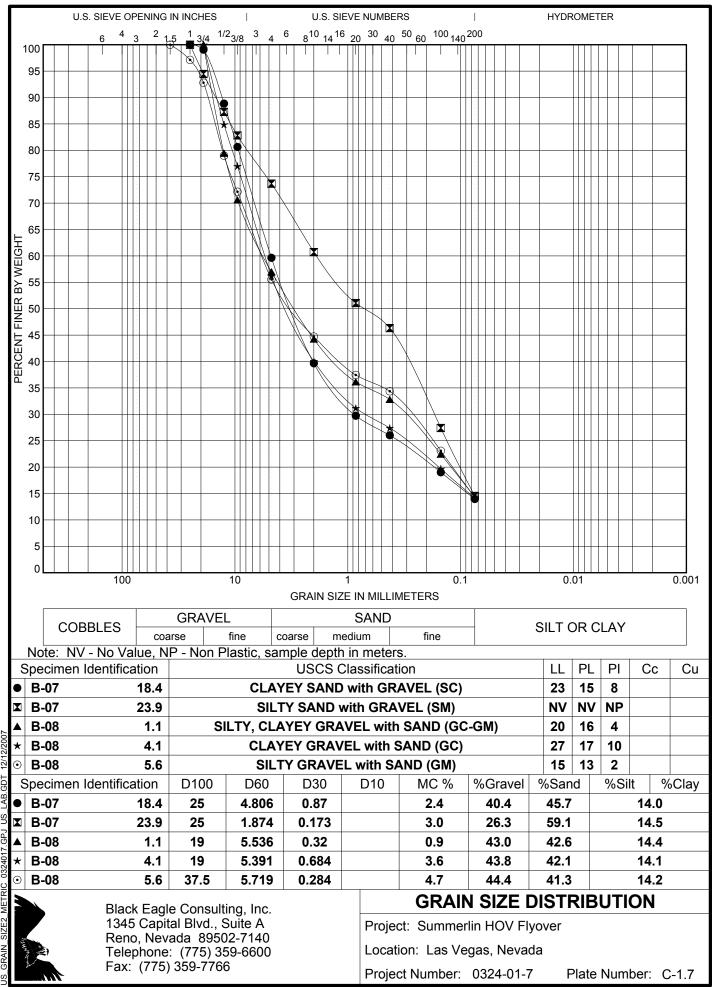




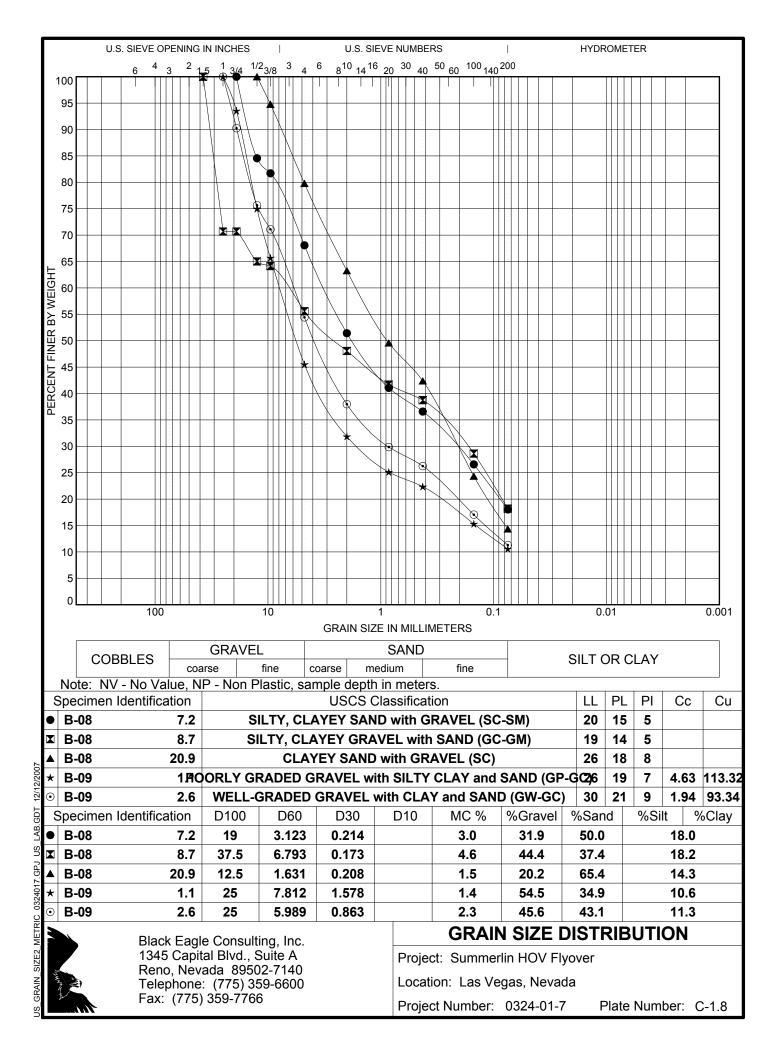


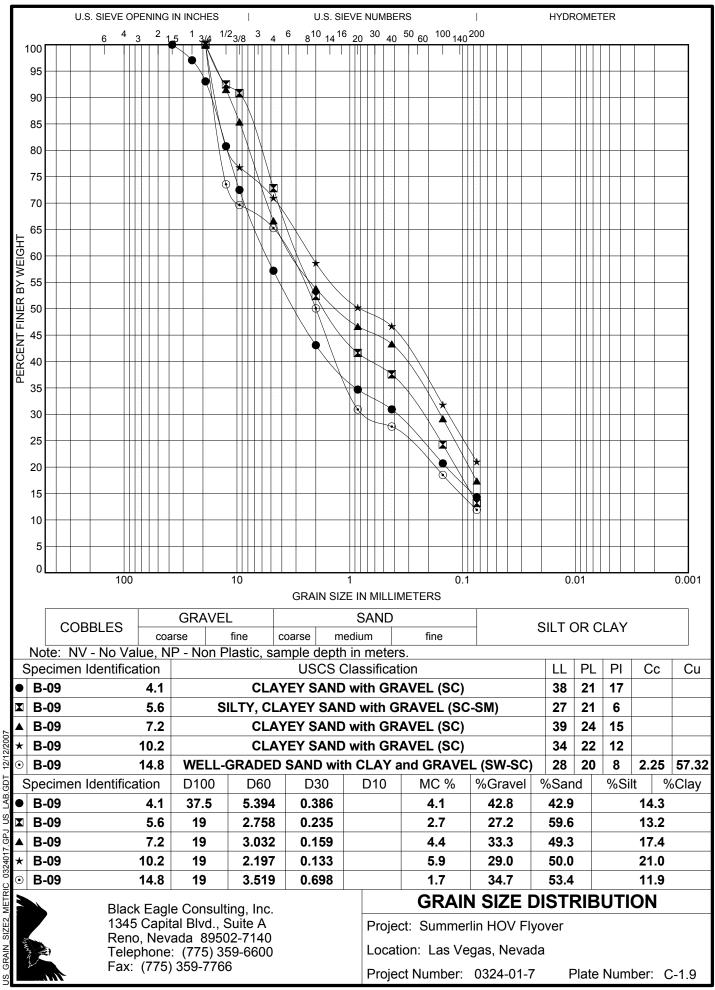


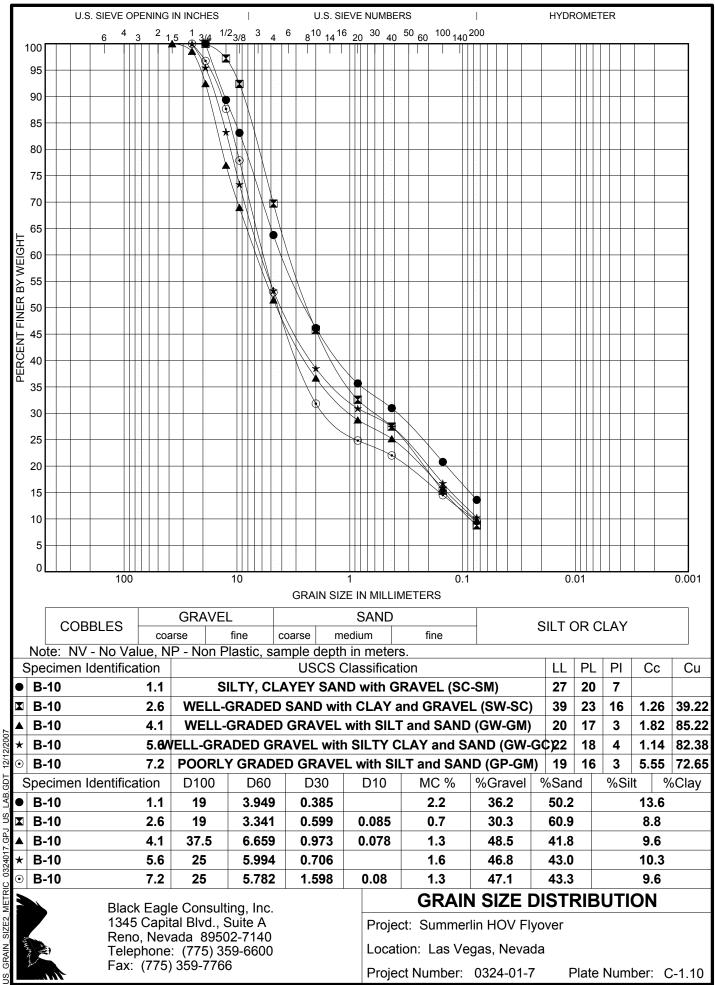




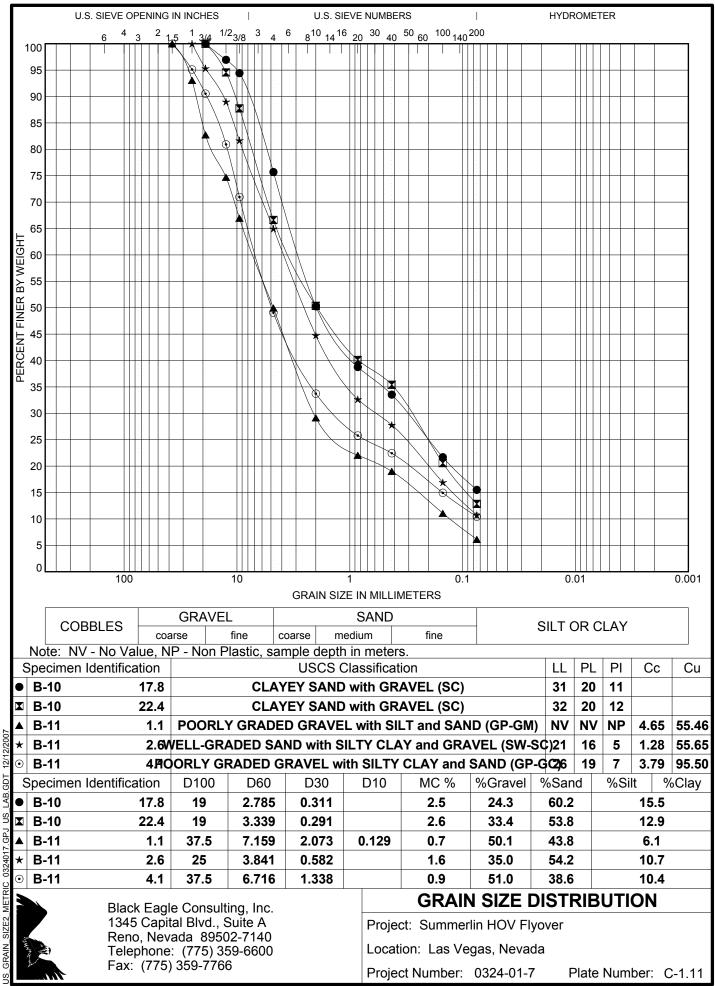
12/1 GDT S 0324017.GPJ 

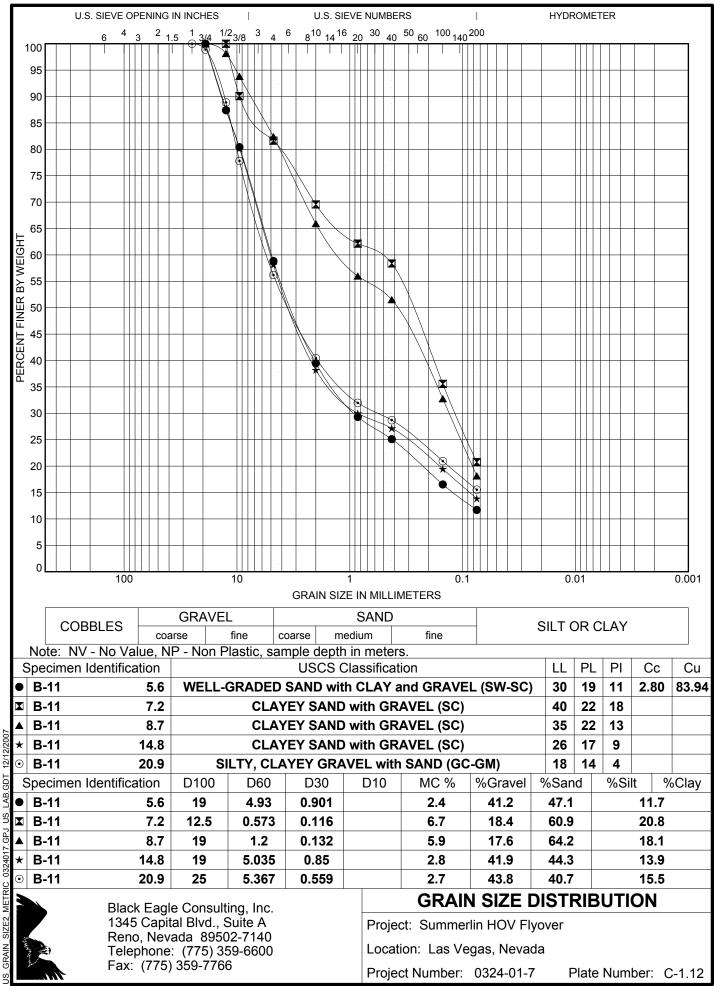


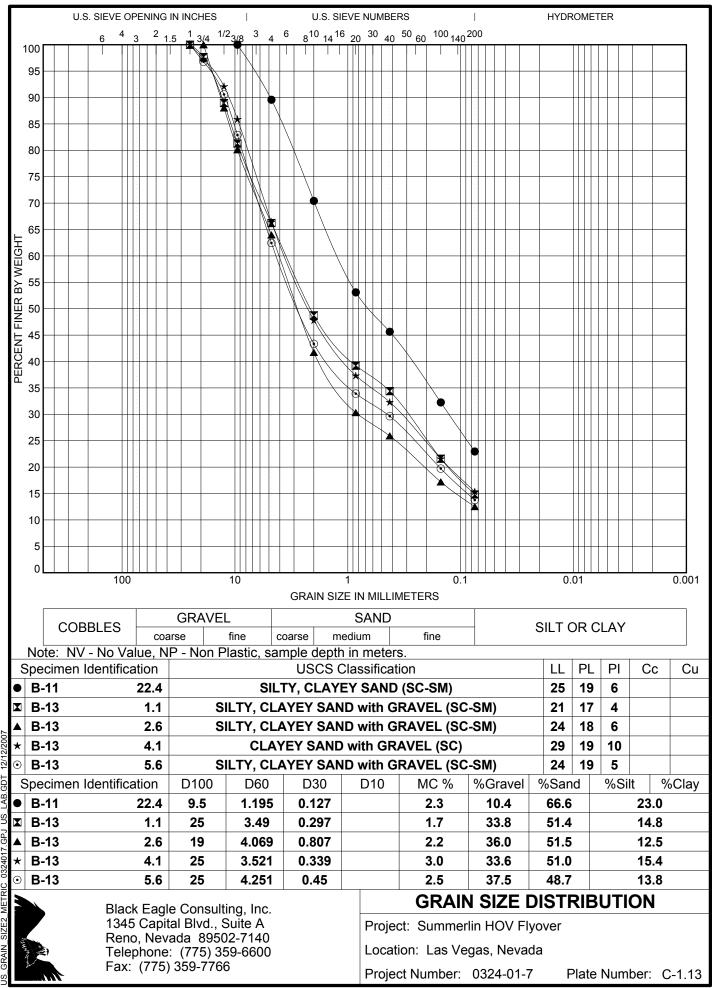


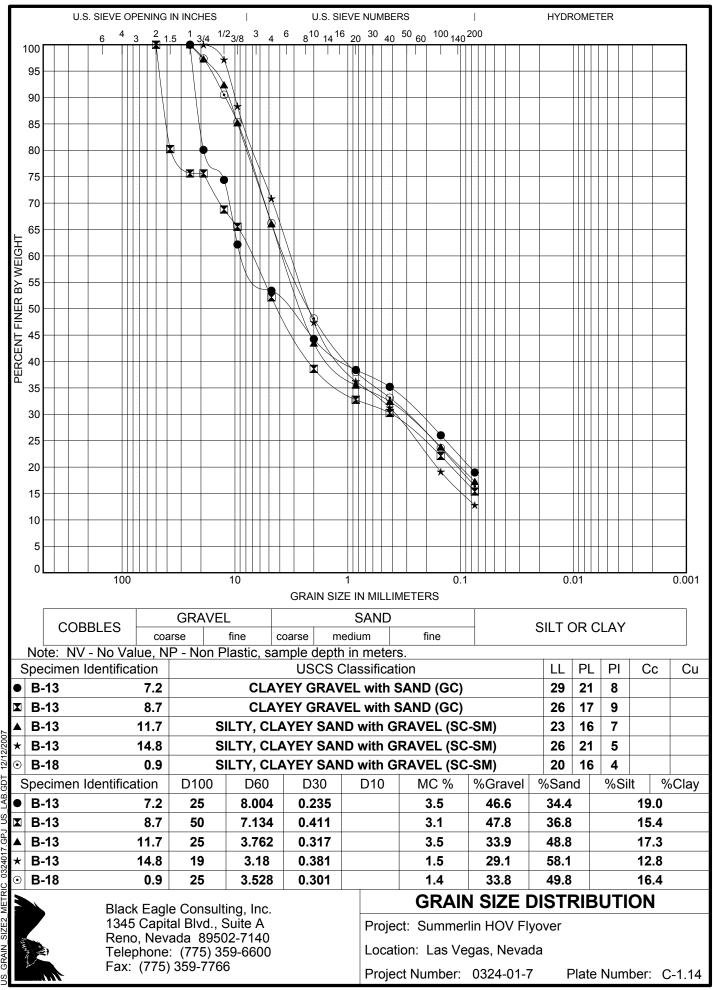


GDT 2 S GP. 324017 

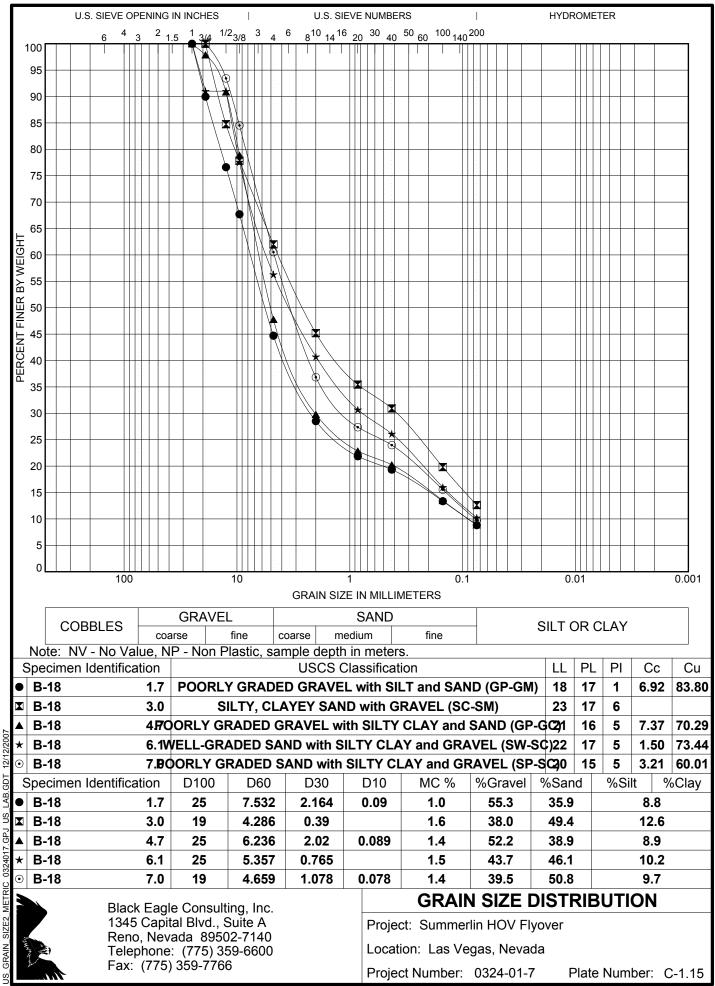




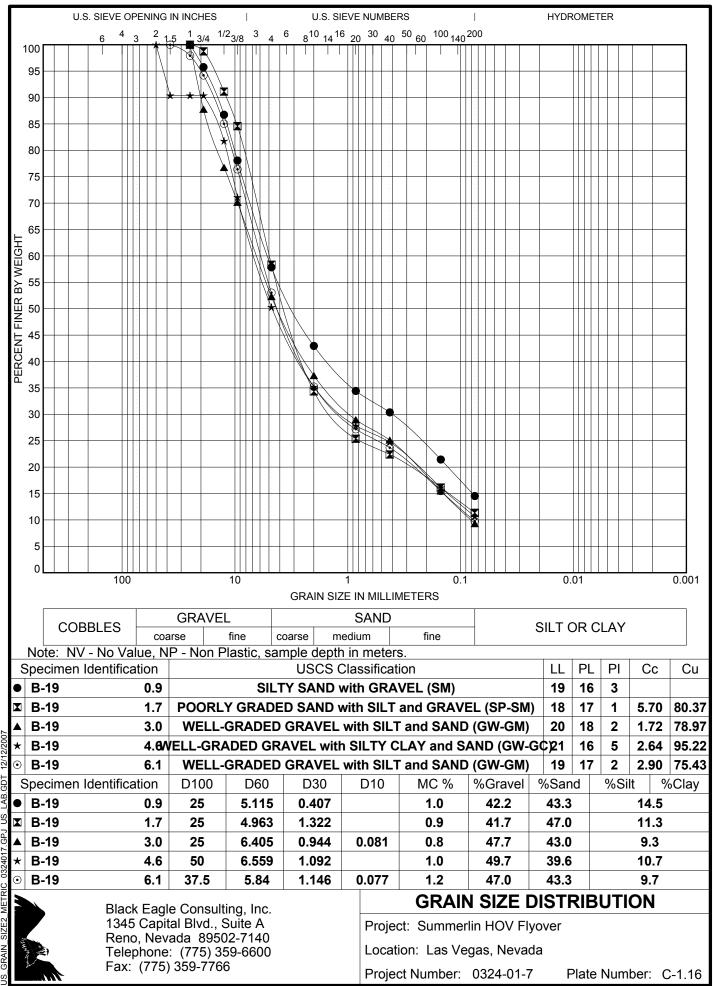




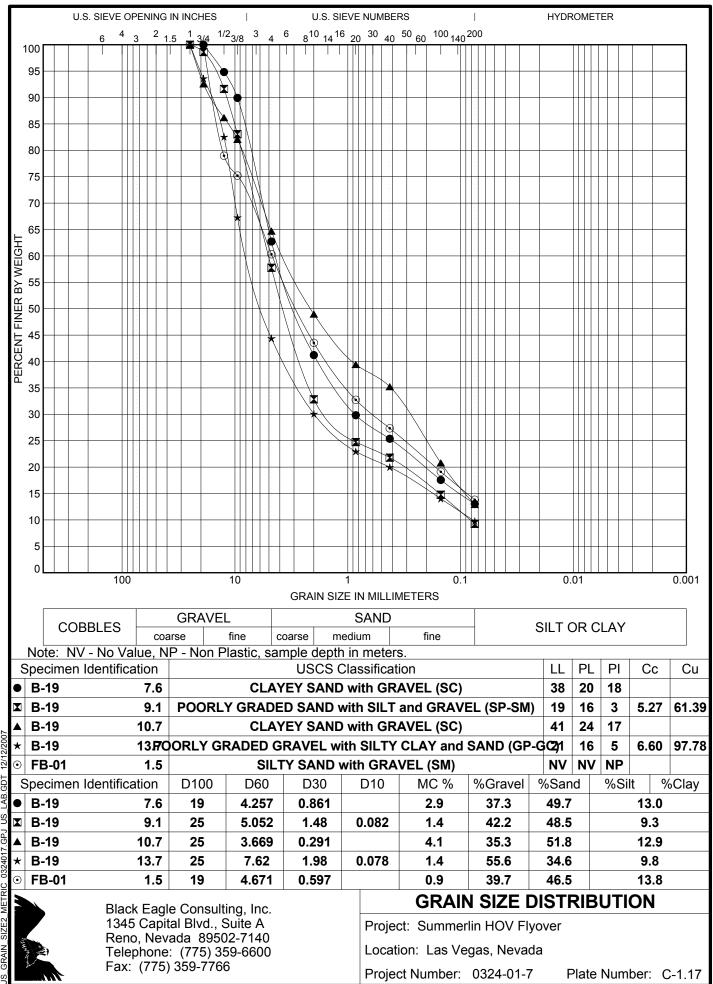
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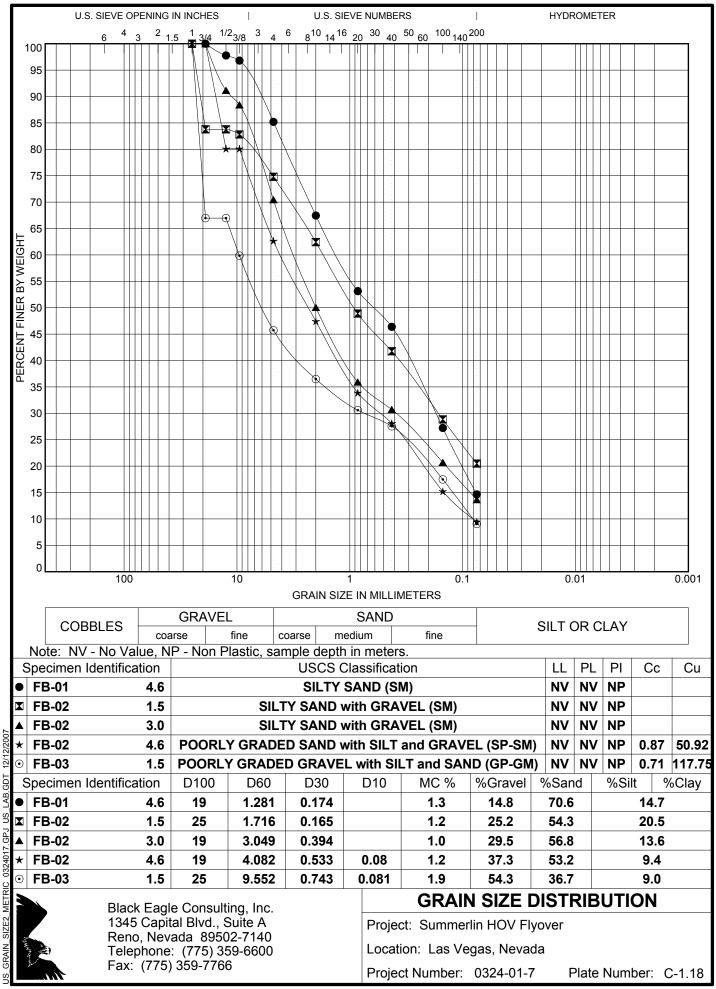
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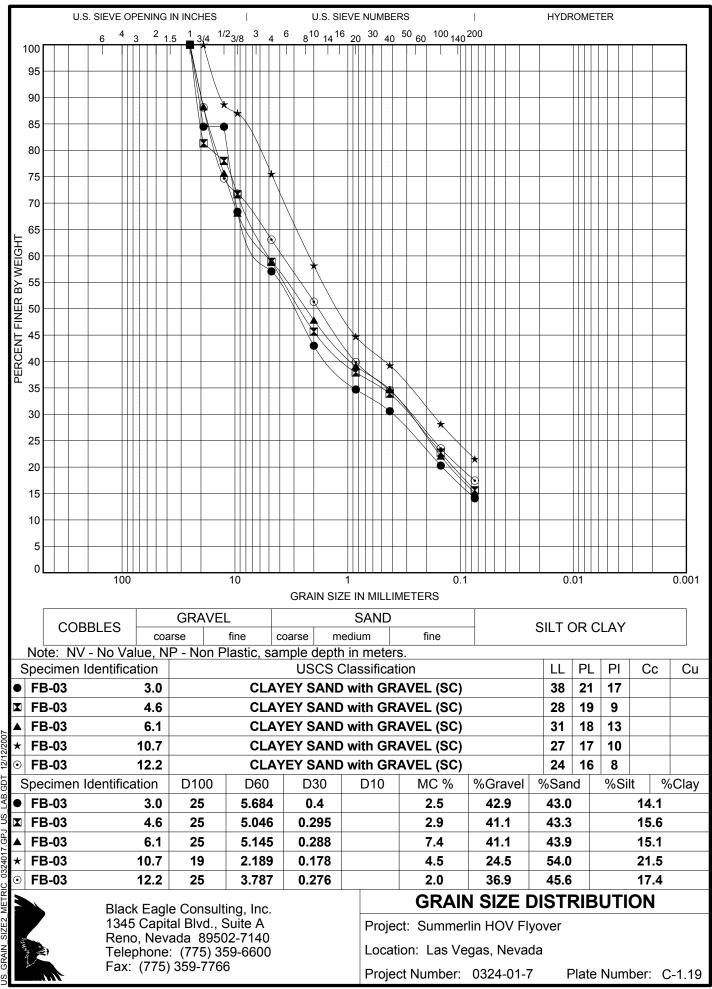
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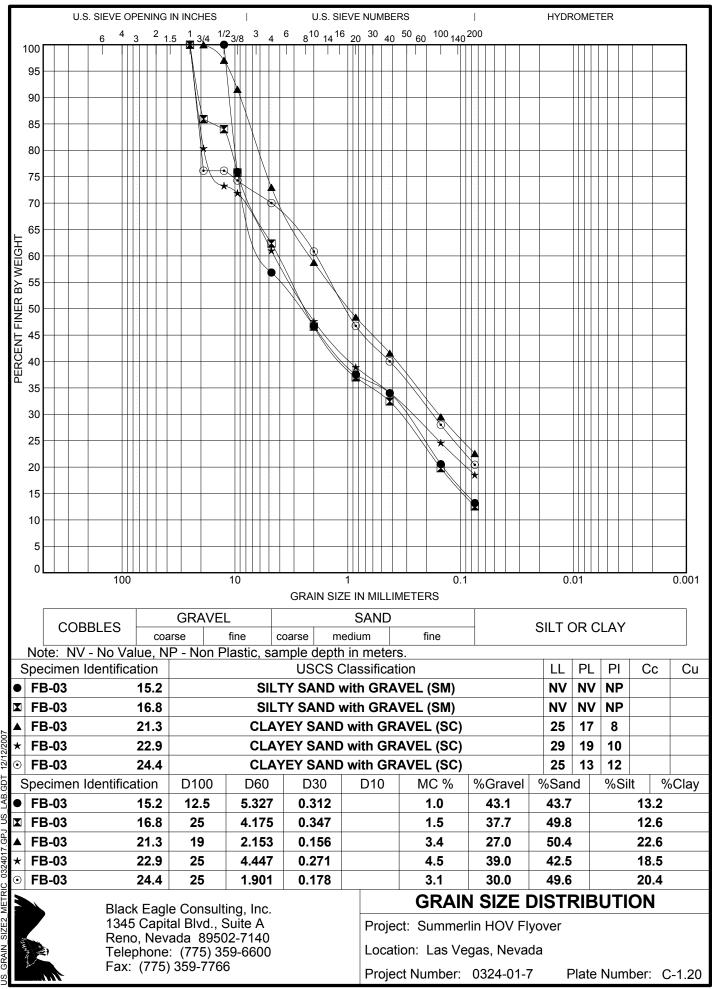


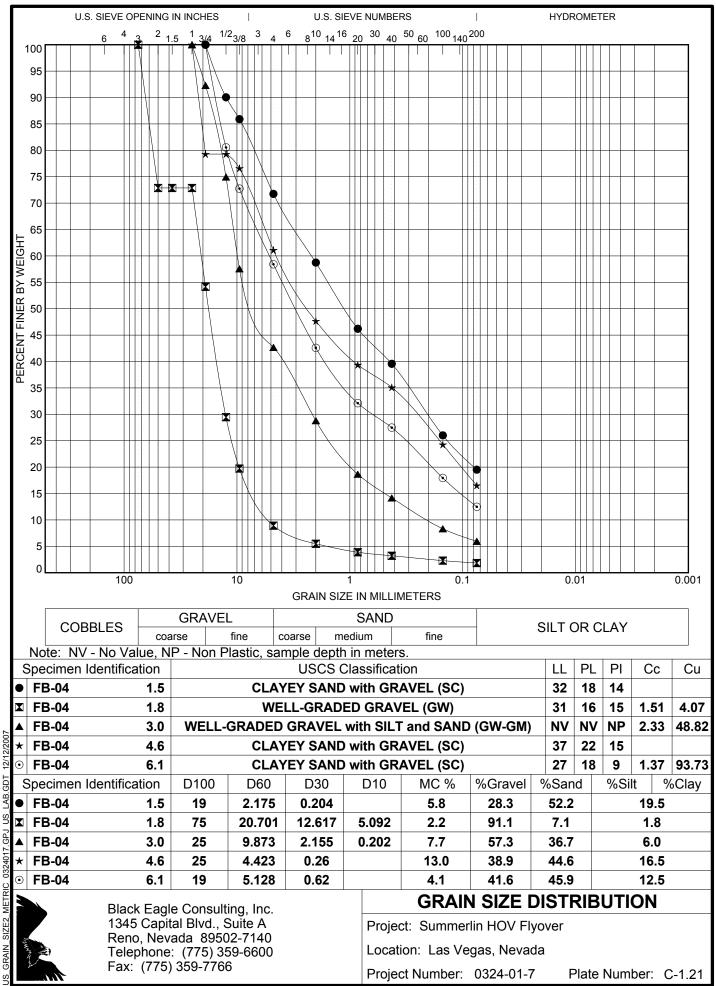
12/1 GDT 2 S GР 324017 



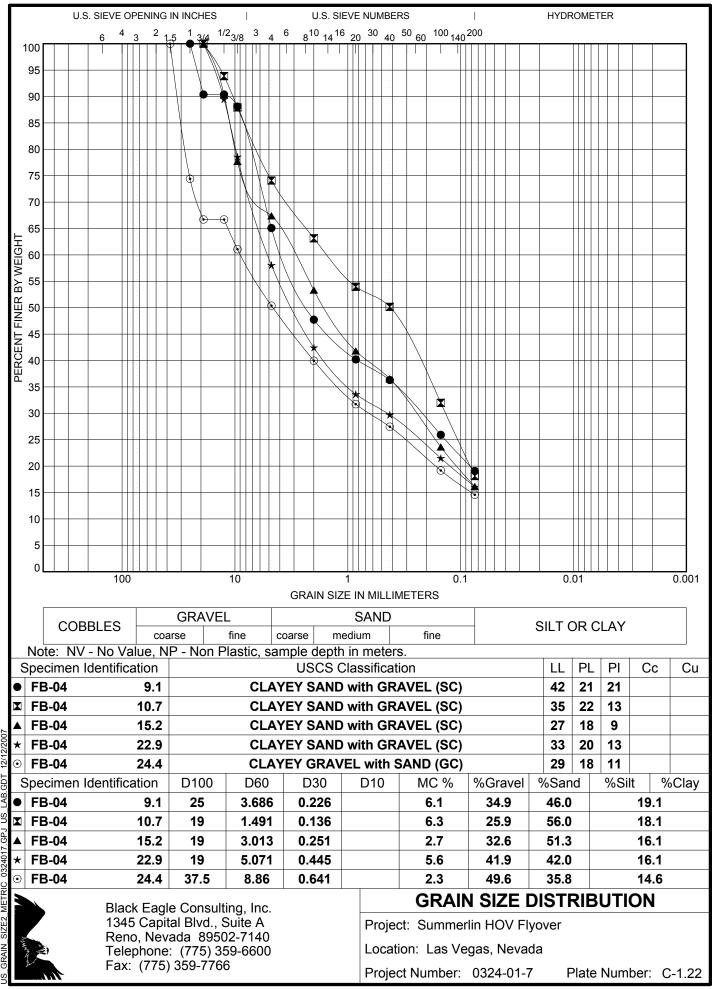
GDT 2 S GP. 324017 

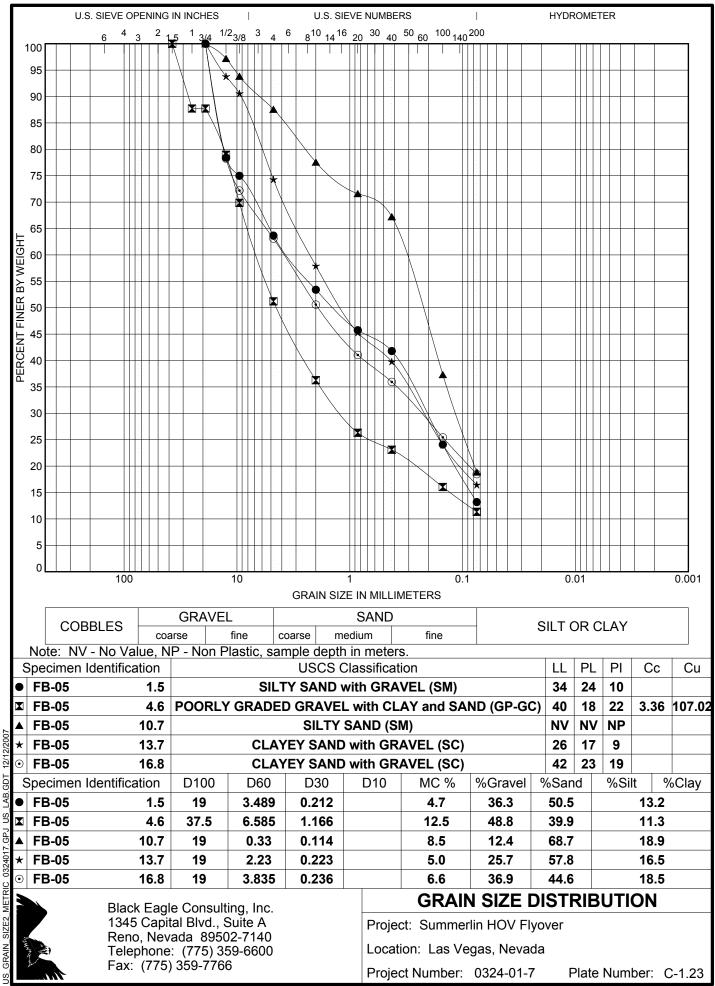




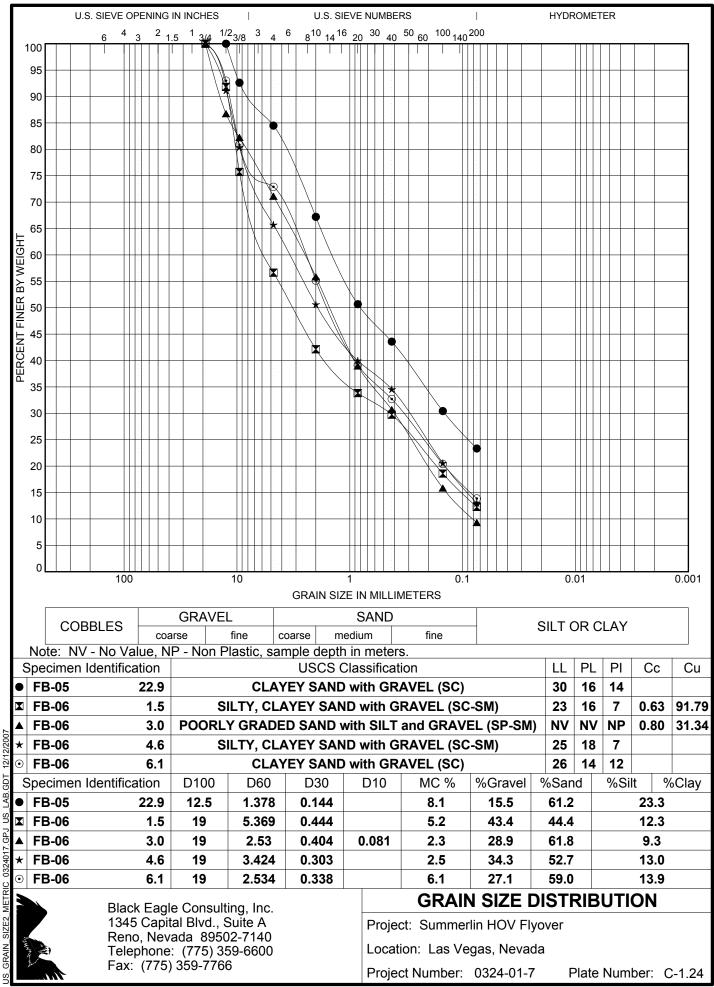


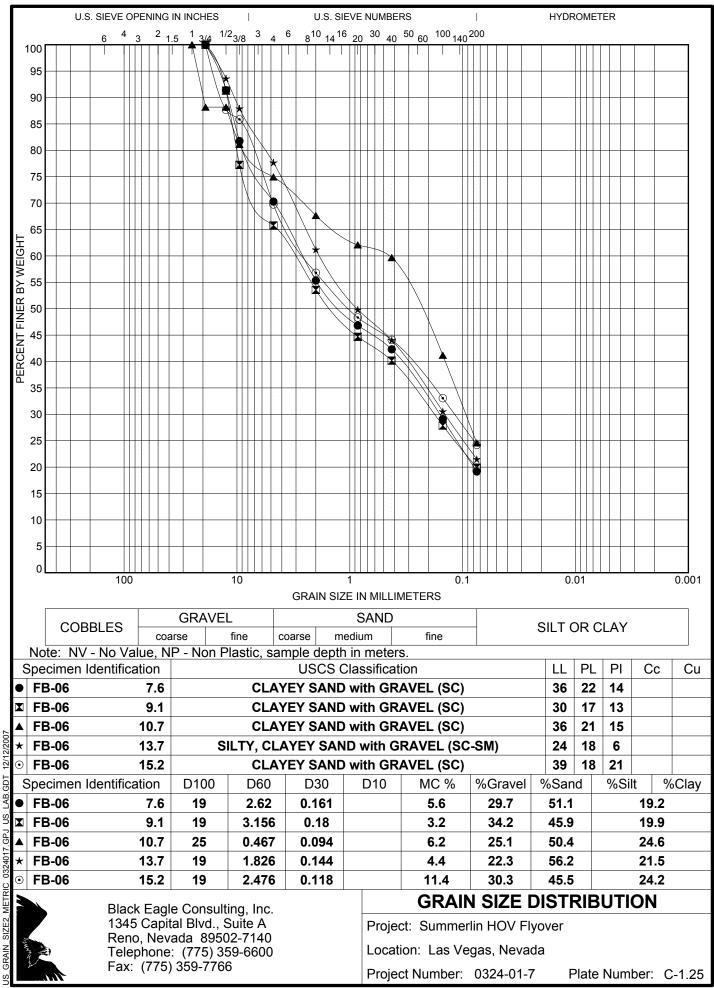
GDT 2 SU GР 324017 

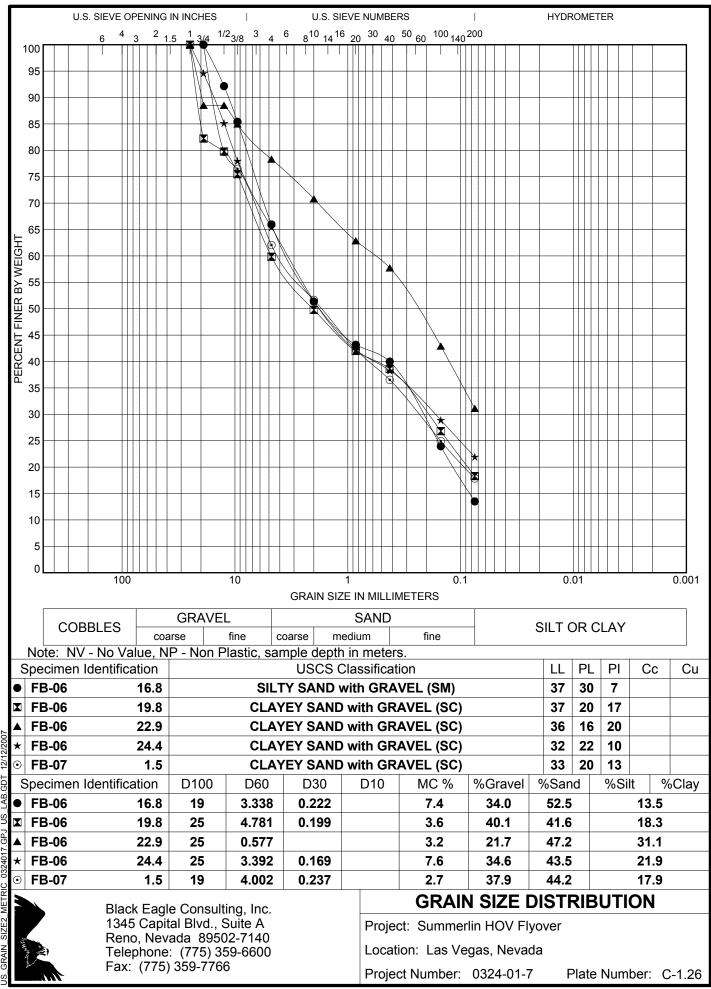


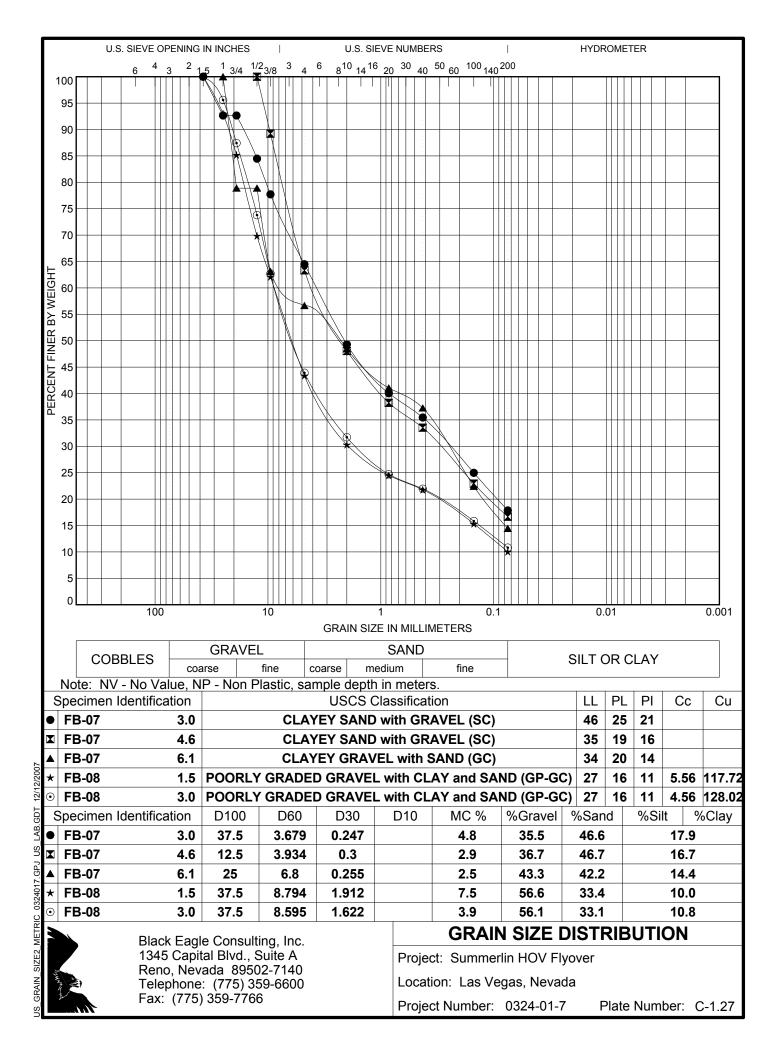


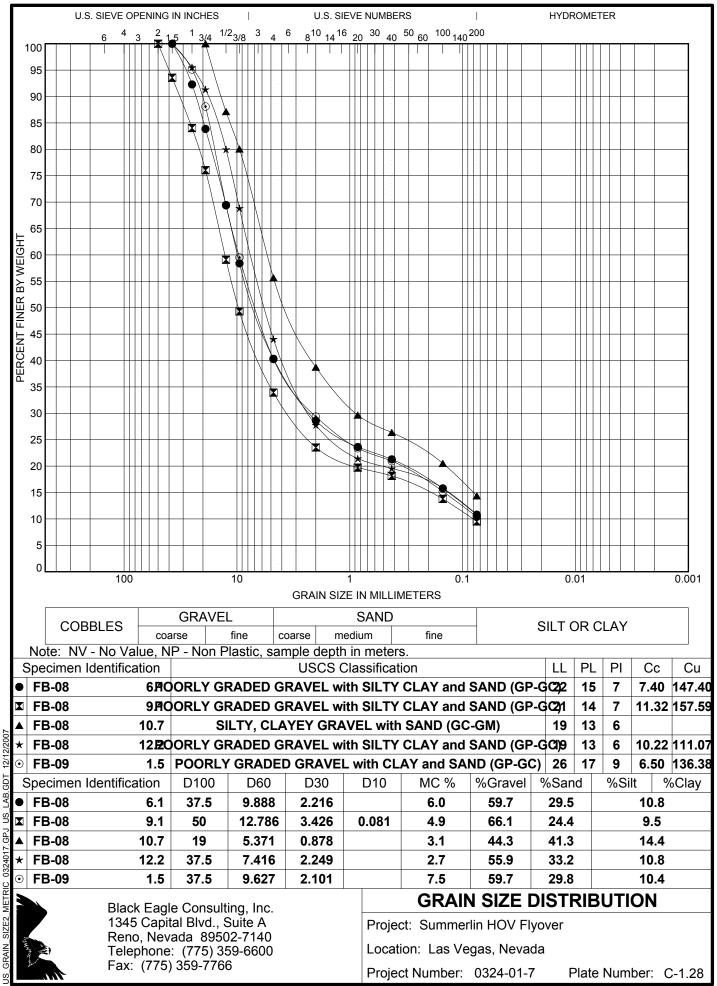
GDT 2 S GP. 324017



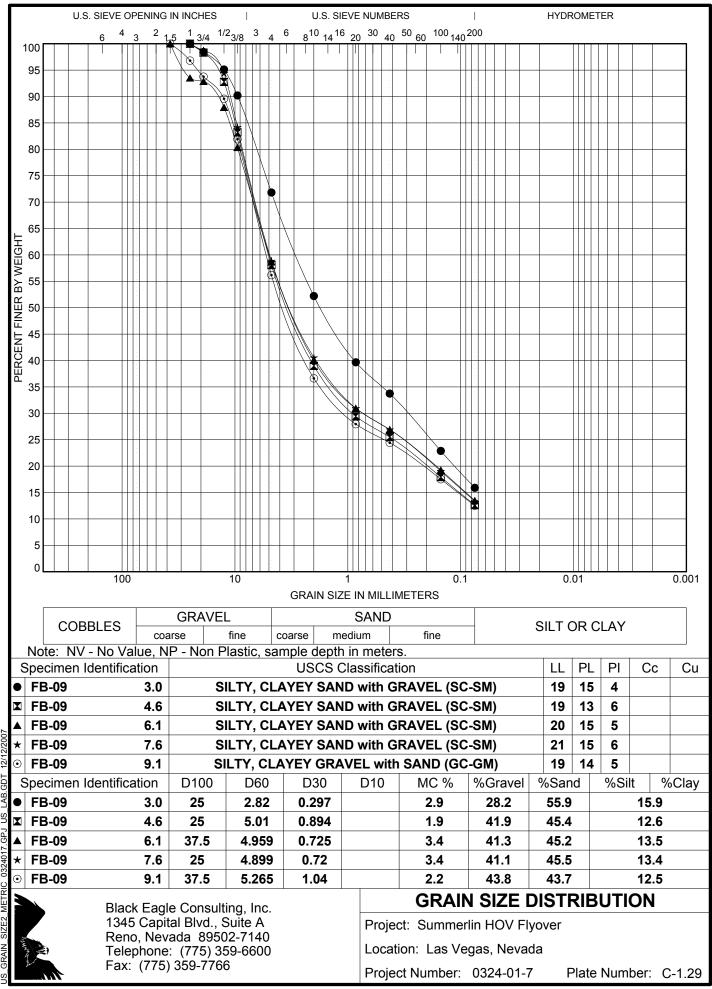




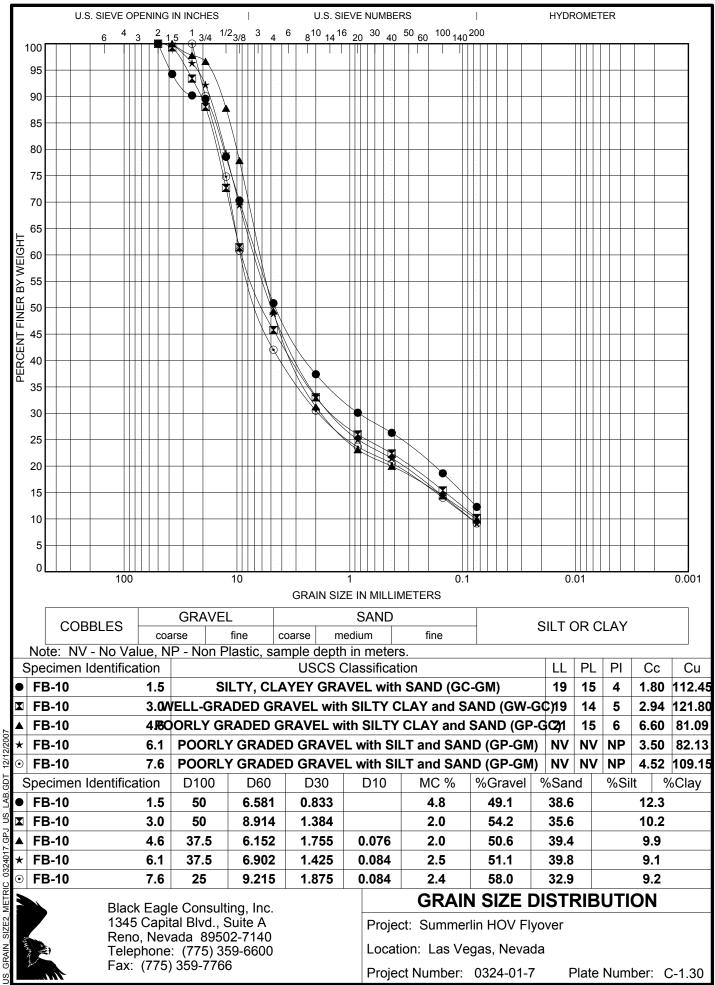




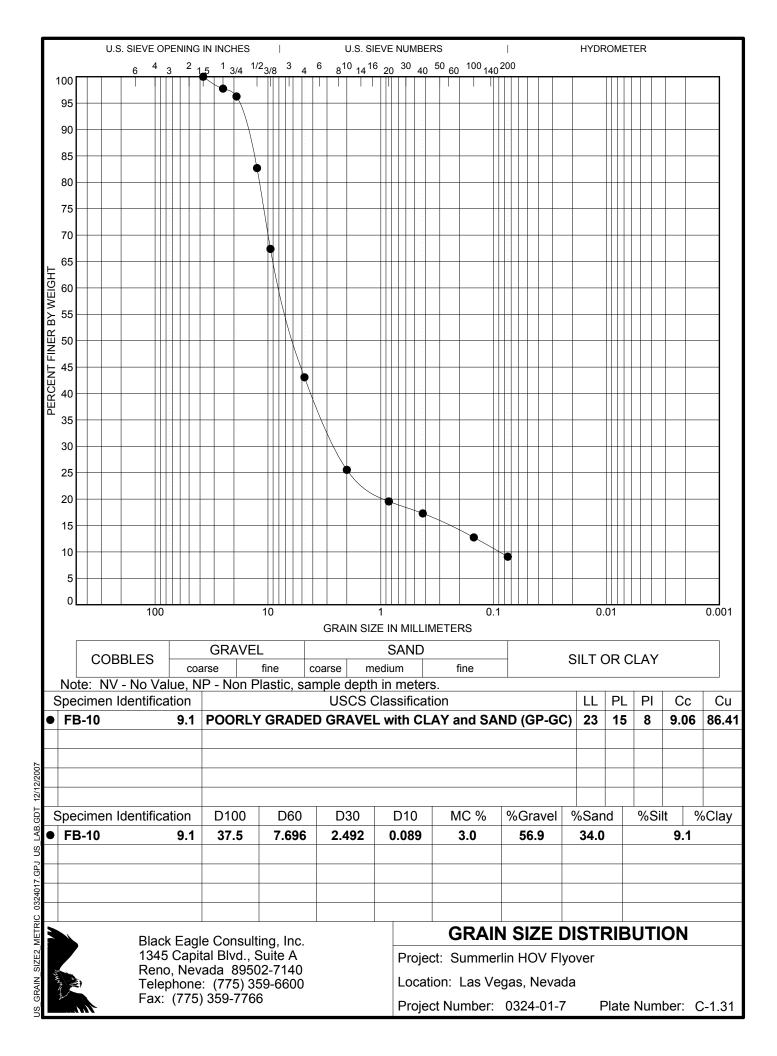
GDT 2 SU GP. 324017 

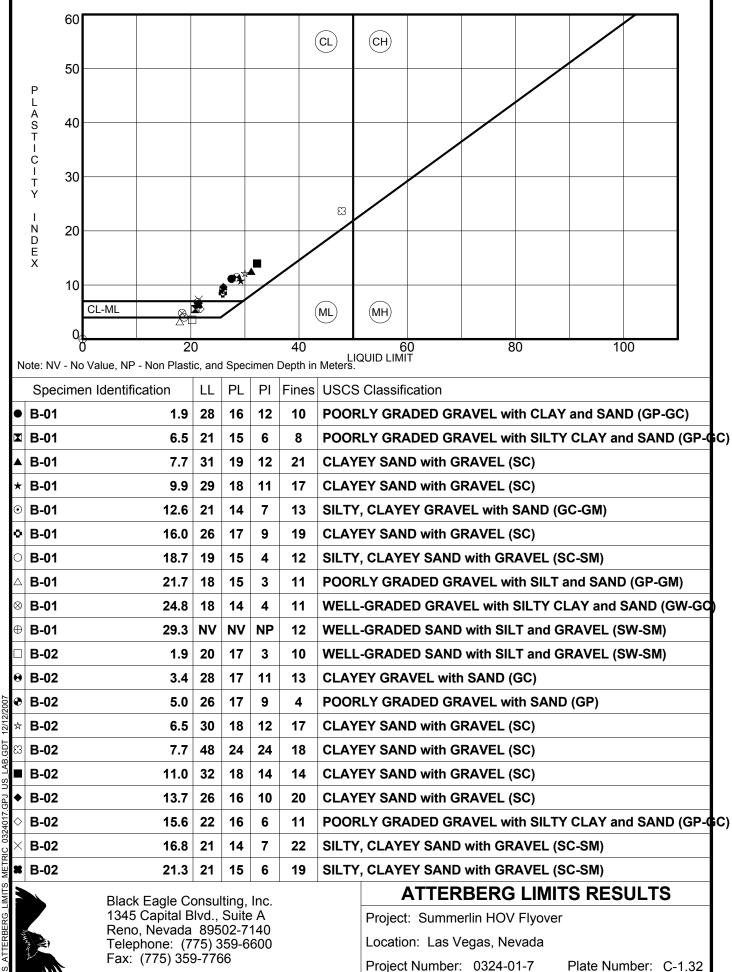


12/1 GDT 2 S GР 324017 

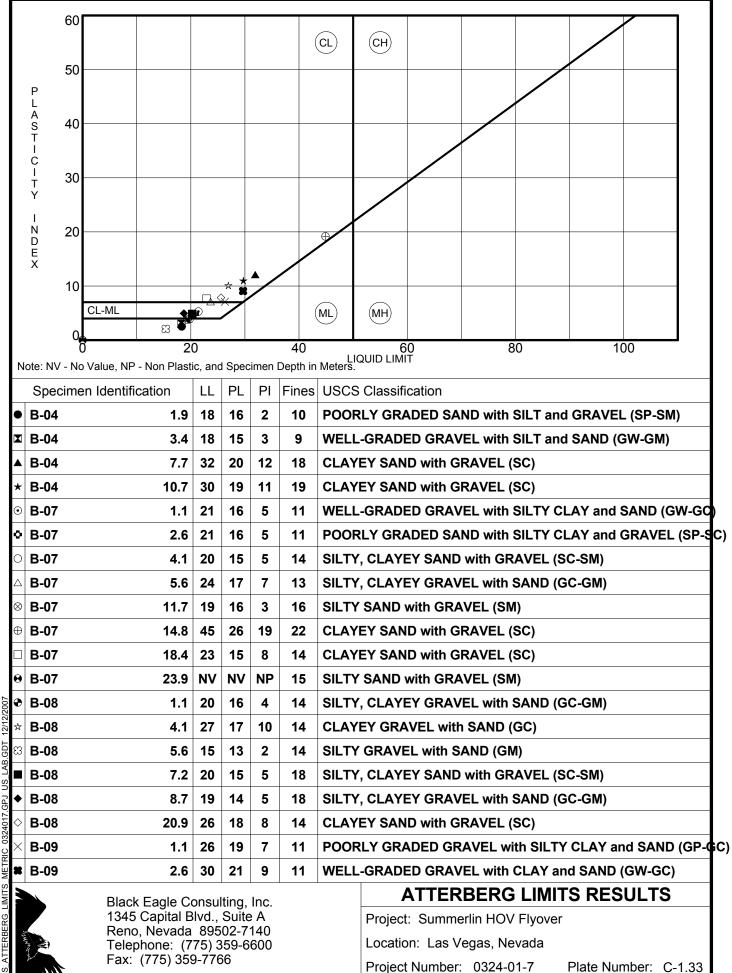


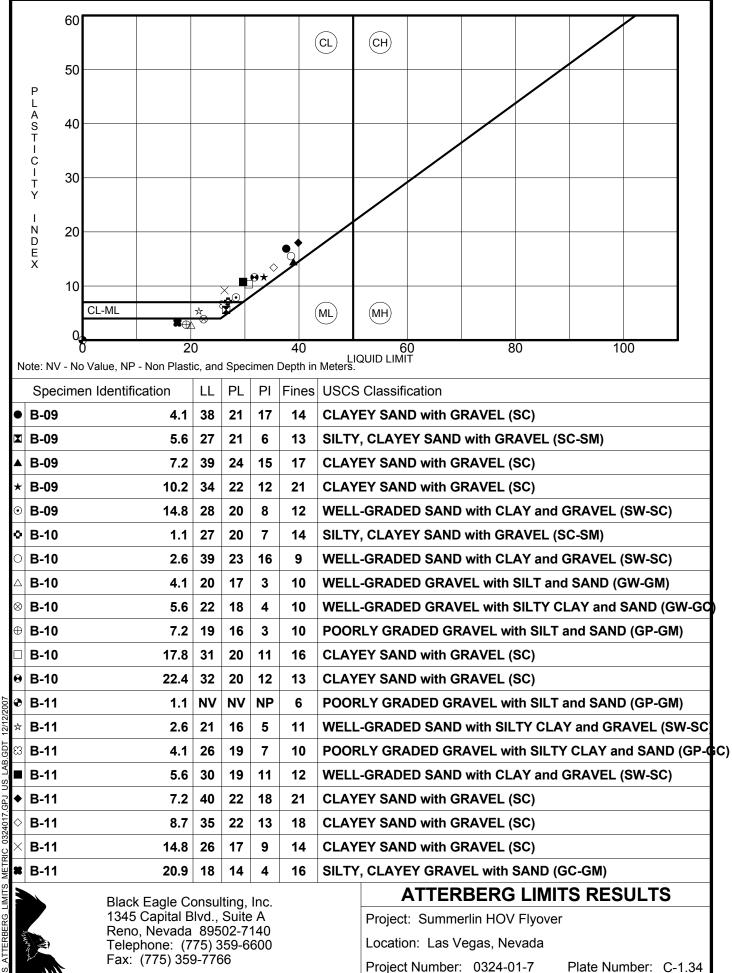
GDT 2 SU GP. 324017 

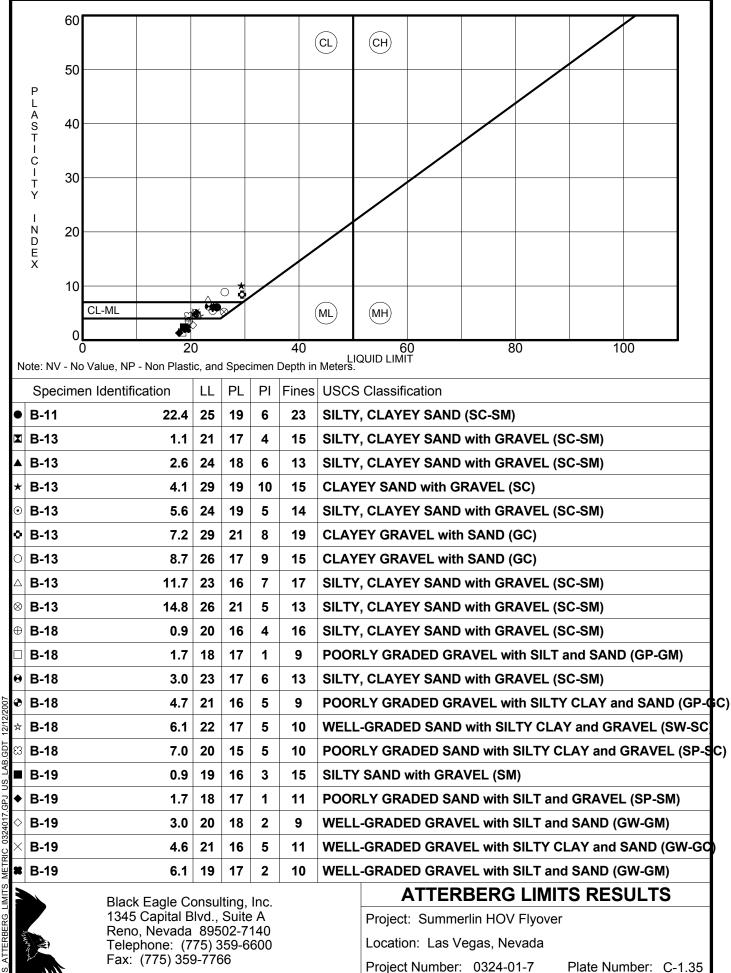


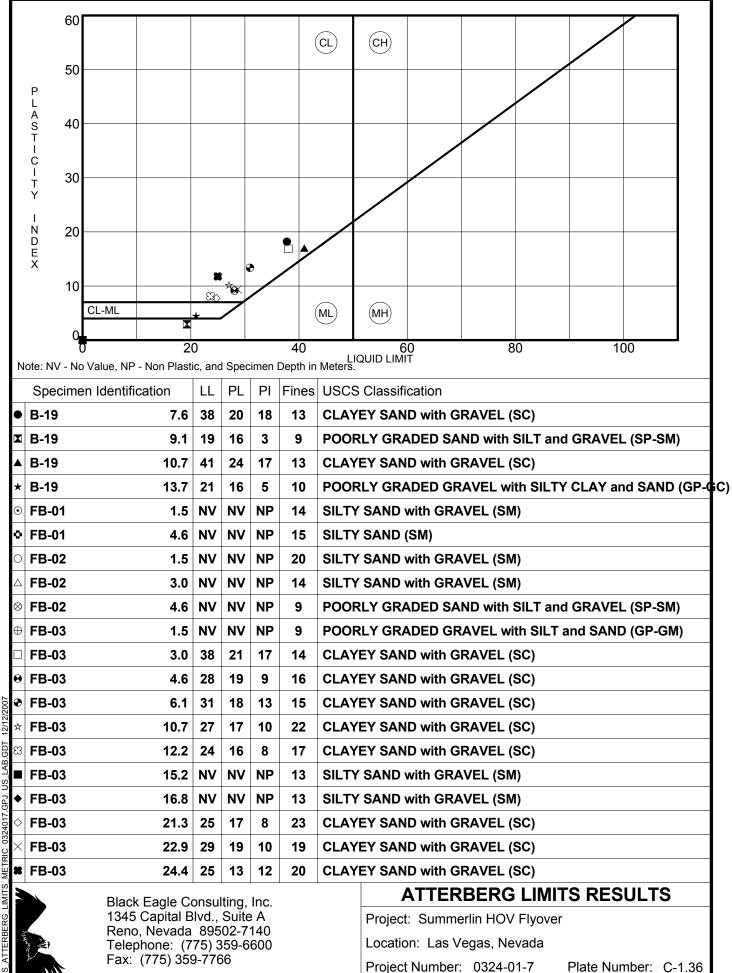


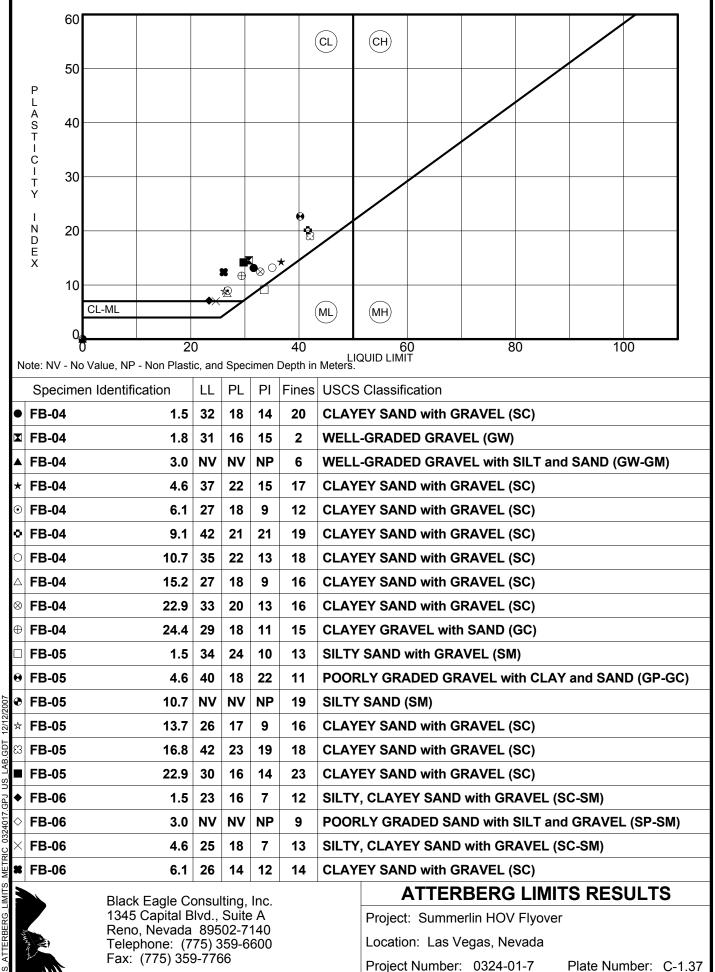
GDT AP 20 324017 Ŭ



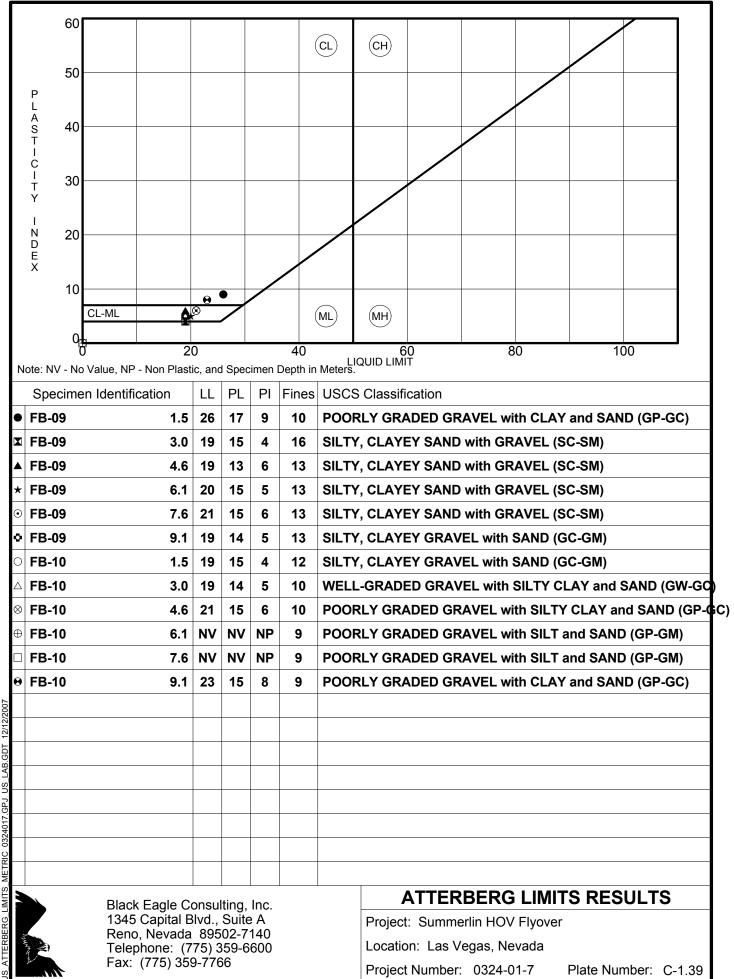








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	С	L-ML	<b>⇔</b>			•	(ML) (MH)					
	٥	2	0			40						
N	U ote: NV - No V	∠ alue, NP - Non Plast	u ic, and	l Spec	imen l	40 Depth in	60 80 100 LIQUID LIMIT Meters.					
		Identification	LL	PL	PI	Fines						
•	FB-06	7.6	36	22	14	19	CLAYEY SAND with GRAVEL (SC)					
X	FB-06	9.1	30	17	13	20	CLAYEY SAND with GRAVEL (SC)					
	FB-06	10.7	36	21	15	25	CLAYEY SAND with GRAVEL (SC)					
*	FB-06	13.7	24	18	6	22	SILTY, CLAYEY SAND with GRAVEL (SC-SM)					
•	FB-06	15.2	39	18	21	24	CLAYEY SAND with GRAVEL (SC)					
٥	FB-06	16.8	37	30	7	14	SILTY SAND with GRAVEL (SM)					
0	FB-06	19.8	37	20	17	18	CLAYEY SAND with GRAVEL (SC)					
	FB-06	22.9	36	16	20	31	CLAYEY SAND with GRAVEL (SC)					
	FB-06	24.4	32	22	10	22	CLAYEY SAND with GRAVEL (SC)					
	FB-07	1.5 3.0	33 46	20 25	13 21	18 18	CLAYEY SAND with GRAVEL (SC)					
	FB-07 FB-07	4.6	46 35	25 19	21 16	18	CLAYEY SAND with GRAVEL (SC) CLAYEY SAND with GRAVEL (SC)					
	FB-07	6.1	34	20	14	14	CLAYEY GRAVEL with SAND (GC)					
_	FB-08	1.5	27	16	11	10	POORLY GRADED GRAVEL with CLAY and SAND (GP-GC)					
ន		3.0	27	16	11	11	POORLY GRADED GRAVEL with CLAY and SAND (GP-GC)					
ස ∎	FB-08	6.1	22	15	7	11	POORLY GRADED GRAVEL with SILTY CLAY and SAND (GP-0					
٠	FB-08	7.6	22	14	8		0					
$\diamond$	FB-08	9.1	21	14	7	10	POORLY GRADED GRAVEL with SILTY CLAY and SAND (GP-					
$\times$	FB-08	10.7	19	13	6	14	SILTY, CLAYEY GRAVEL with SAND (GC-GM)					
	FB-08	12.2	19	13	6	11	POORLY GRADED GRAVEL with SILTY CLAY and SAND (GP-					
		Black Eagle C					ATTERBERG LIMITS RESULTS					
3		1345 Capital E Reno, Nevada					Project: Summerlin HOV Flyover					
ndre		Telephone: (7 Fax: (775) 359	75) 3	359-6			Location: Las Vegas, Nevada					
L			5-110	.0			Project Number: 0324-01-7 Plate Number: C-1.38					



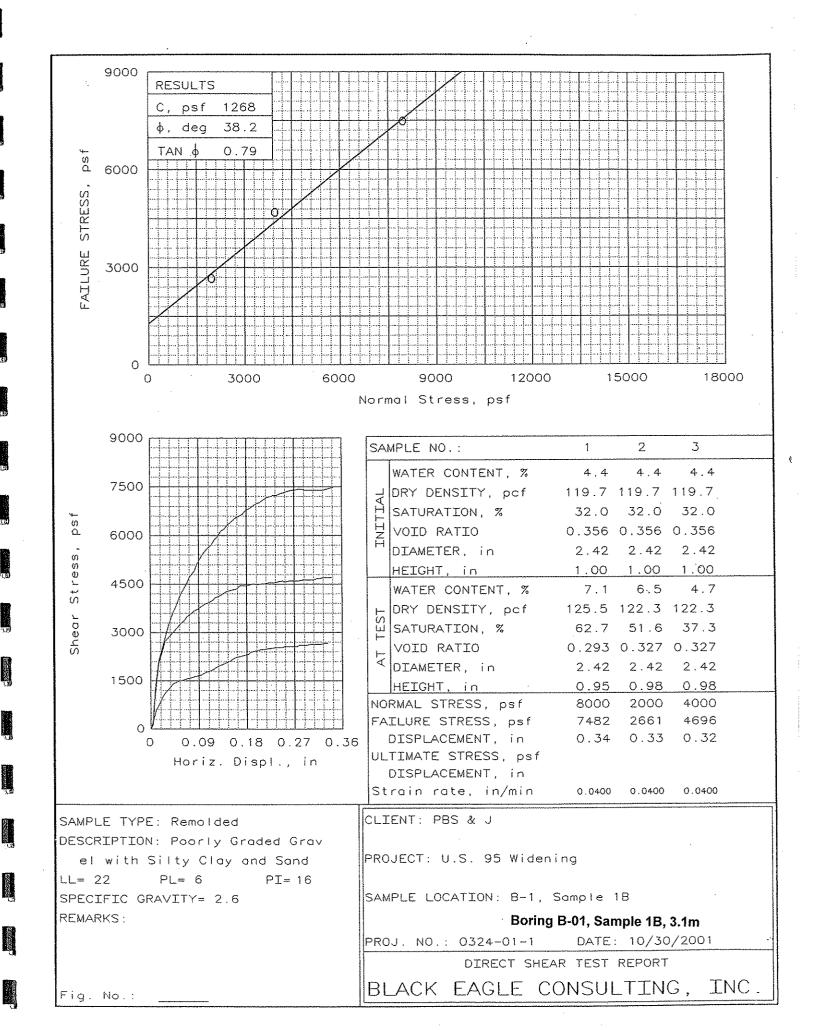
GDT ŝ GP.J 0324017 METRIC STIM

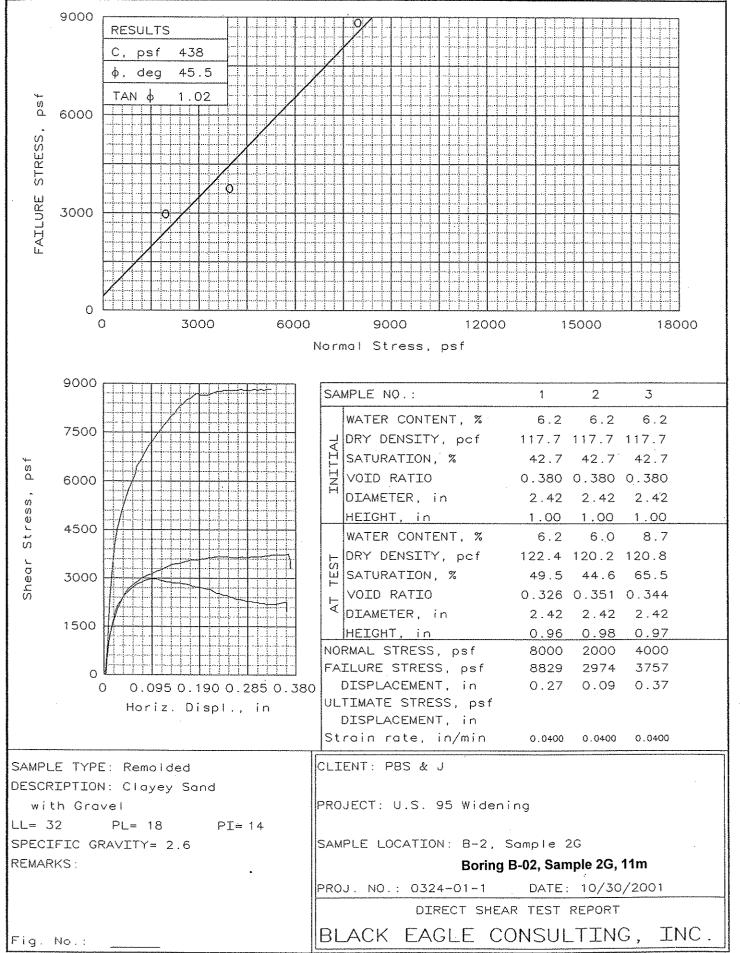
# **C-2**

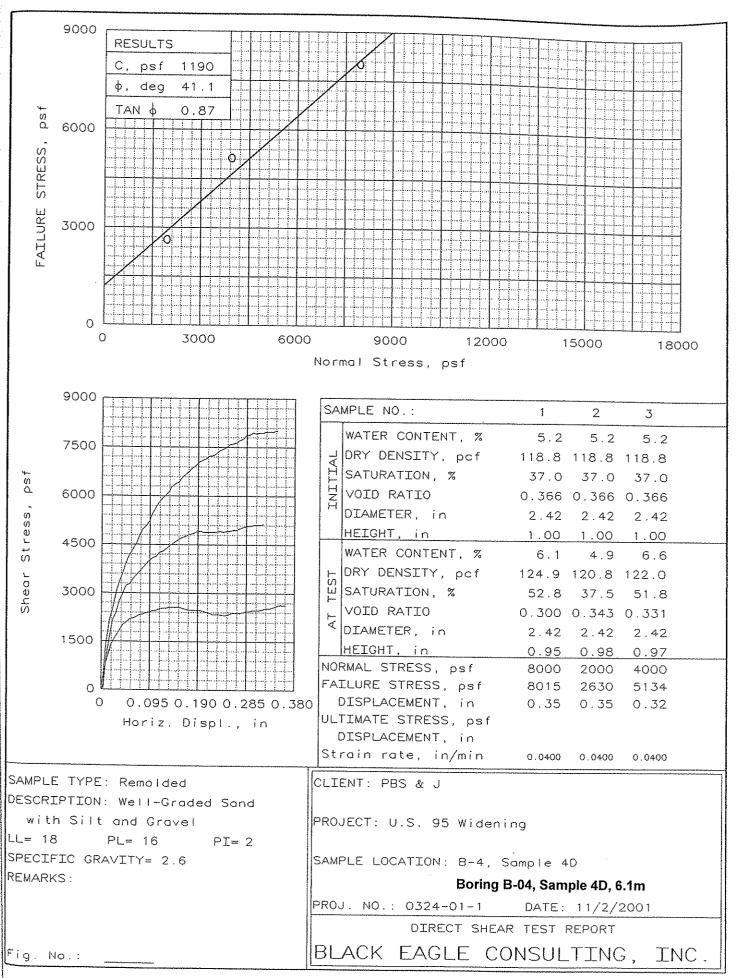
### **STRENGTH TEST RESULTS**

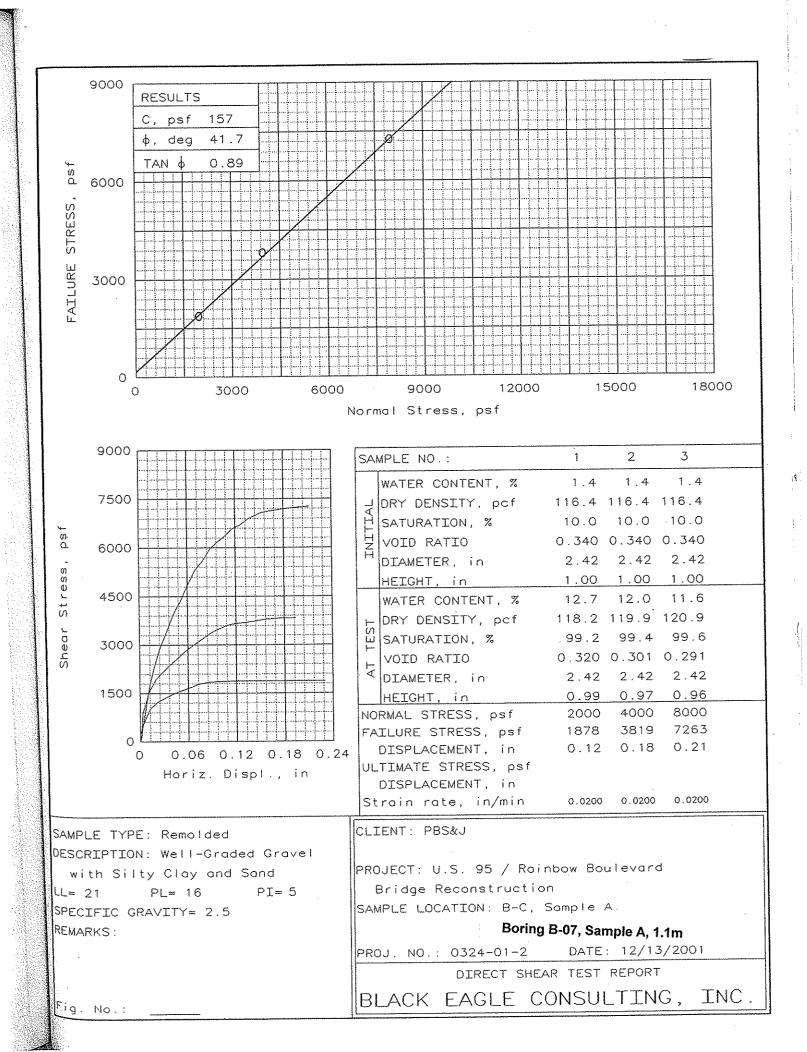
Depth, m	Test type Direct Shear	Sampler Type	Avg Dry Density,		Moisture								
3.1	Direct Shear		kN/m^3	Void Ratio	Content (percent)	USCS	PI	Fines, percent^	Sand, percent^	Gravel, percent^	Cohesion, c (kPa)	Friction Angle, Φ	Comment
	Recompacted	Grab	18.8	0.36	4.4	GP-GC(SC)	2	10 (35)	30 (65)	59 (0)	61.5	38.2	
11	Direct Shear Recompacted	Grab	18.5	0.38	6.2	SC	16	14 (31)	45 (69)	41 (0)	21.2	45.5	
6.1	Direct Shear Recompacted	Grab	18.7	0.37	5.2	SW-SM	2	17 (48)	37* (52)	45 (0)	57.7	41.1	
1.1	Direct Shear Recompacted	Grab	18.3	0.34	12.7	GW-GC(SC)	5	11 (30)	42 (70)	47 (0)	7.6	42.0	
5.65	Direct Shear Recompacted	MC	15.5	0.49	13.6	GC(SC)	2	14 (31)	41 (69)	44 (0)	28.1	34.0	
2.7	Direct Shear Recompacted	Grab	18.1	0.34	12.2	GW-GC(SC)	9	11 (28)	43 (72)	46 (0)	121.5	27.0	
1.1	Direct Shear Recompacted	Grab	18.0	0.36	13.6	SC-SM	7	14 (31)	50 (69)	36 (0)	18.4	26.2	
2.6	Direct Shear Recompacted	Grab	19.3	0.35	11.4	SW-SC(SC)	5	11 (25)	54 (75)	35 (0)	0.0	46.0	
12.2	Direct Shear Recompacted	Grab	19.6	0.35	6.2	GP-GC(SC)	6	11 (11)	33 (91)	56 (0)	0.0	41.7	
9.1	Direct Shear Recompacted	Grab	20.3	0.31	8.3	GC-GM(SC-SM)	5	13 (13)	43 (87)	44 (0)	0.0	40.2	
3	Direct Shear Recompacted	Grab	19.2	0.38	6.2	GW-GC(SW-SC)	5	10 (10)	36 (90)	54 (0)	11.6	36.9	
9.1	Direct Shear Recompacted	Grab	19.6	0.35	9.7	GP-GC(SP-SC)	8	9 (9)	34 (91)	57 (0)	15.7	37.8	Harvard Miniature Compaction of 19.8 kN/m^3 at 9% moisture content.
	1.1         5.65         2.7         1.1         2.6         12.2         9.1         3         9.1         cording to N only	Recompacted1.1Direct Shear Recompacted5.65Direct Shear Recompacted2.7Direct Shear Recompacted1.1Direct Shear Recompacted2.6Direct Shear Recompacted12.2Direct Shear Recompacted9.1Direct Shear Recompacted3Direct Shear Recompacted9.1Direct Shear Recompacted	Recompacted1.1Direct Shear RecompactedGrab5.65Direct Shear RecompactedMC2.7Direct Shear RecompactedGrab1.1Direct Shear RecompactedGrab2.6Direct Shear RecompactedGrab2.6Direct Shear RecompactedGrab12.2Direct Shear RecompactedGrab9.1Direct Shear RecompactedGrab3Direct Shear RecompactedGrab9.1Direct Shear RecompactedGrab9.1Direct Shear RecompactedGrab9.1Direct Shear RecompactedGrab9.1Direct Shear RecompactedGrab9.1Direct Shear RecompactedGrab9.1Direct Shear RecompactedGrab9.1Direct Shear RecompactedGrab0.1Direct Shear RecompactedGrab9.1Direct Shear RecompactedGrab0.1Direct Shear RecompactedGrab	RecompactedGrab18.31.1Direct Shear RecompactedGrab18.35.65Direct Shear RecompactedMC15.52.7Direct Shear RecompactedGrab18.11.1Direct Shear RecompactedGrab18.02.6Direct Shear RecompactedGrab19.32.6Direct Shear RecompactedGrab19.312.2Direct Shear RecompactedGrab19.69.1Direct Shear RecompactedGrab19.29.1Direct Shear RecompactedGrab19.29.1Direct Shear RecompactedGrab19.69.1Direct Shear RecompactedGrab19.69.1Direct Shear RecompactedGrab19.69.1Direct Shear RecompactedGrab19.69.1Direct Shear RecompactedGrab19.60.1Direct Shear RecompactedGrab19.6	RecompactedGrab18.30.341.1Direct Shear RecompactedGrab18.30.345.65Direct Shear RecompactedMC15.50.492.7Direct Shear RecompactedGrab18.10.341.1Direct Shear RecompactedGrab18.00.362.6Direct Shear RecompactedGrab19.30.352.6Direct Shear RecompactedGrab19.30.3512.2Direct Shear RecompactedGrab19.60.359.1Direct Shear RecompactedGrab19.20.389.1Direct Shear RecompactedGrab19.60.359.1Direct Shear RecompactedGrab19.60.359.1Direct Shear RecompactedGrab19.60.359.1Direct Shear RecompactedGrab19.60.359.1Direct Shear RecompactedGrab19.60.359.1Direct Shear RecompactedGrab19.60.359.1Direct Shear RecompactedGrab19.60.3519.1Direct Shear RecompactedGrab19.60.35	RecompactedGrab18.30.3412.71.1Direct Shear RecompactedMC15.50.4913.65.65Direct Shear RecompactedGrab18.10.3412.22.7Direct Shear RecompactedGrab18.10.3412.21.1Direct Shear RecompactedGrab18.00.3613.62.6Direct Shear RecompactedGrab19.30.3511.412.2Direct Shear RecompactedGrab19.60.356.29.1Direct Shear RecompactedGrab19.20.386.29.1Direct Shear RecompactedGrab19.60.359.7cording to NDOT standards, requiring all material be screened through the #10 sieve. Bu only0.1410.1410.14	RecompactedGrab18.30.3412.7GW-GC(SC)5.65Direct Shear RecompactedMC15.50.4913.6GC(SC)2.7Direct Shear RecompactedGrab18.10.3412.2GW-GC(SC)1.1Direct Shear RecompactedGrab18.00.3613.6SC-SM2.6Direct Shear RecompactedGrab19.30.3511.4SW-SC(SC)12.2Direct Shear RecompactedGrab19.60.356.2GP-GC(SC)9.1Direct Shear RecompactedGrab19.20.386.2GW-GC(SW-SC)9.1Direct Shear RecompactedGrab19.60.359.7GP-GC(SP-SC)9.1Direct Shear RecompactedGrab19.60.359.7GP-GC(SP-SC)9.1Direct Shear RecompactedGrab19.60.359.7GP-GC(SP-SC)0.11Direct Shear RecompactedGrab19.60.359.7GP-GC(SP-SC)	RecompactedGrab18.30.3412.7GW-GC(SC)55.65Direct Shear RecompactedMC15.50.4913.6GC(SC)22.7Direct Shear RecompactedGrab18.10.3412.2GW-GC(SC)91.1Direct Shear RecompactedGrab18.00.3613.6SC-SM72.6Direct Shear RecompactedGrab19.30.3511.4SW-SC(SC)512.2Direct Shear RecompactedGrab19.60.356.2GP-GC(SC)69.1Direct Shear RecompactedGrab19.20.386.2GW-GC(SW-SC)53Direct Shear RecompactedGrab19.60.359.7GP-GC(SP-SC)89.1Direct Shear RecompactedGrab19.60.359.7GP-GC(SP-SC)80.1Direct Shear RecompactedGrab19.60.359.7GP-GC(SP-SC)80.1Direct Shear RecompactedGrab19.60.359.7GP-GC(SP-SC)80.1Direct Shear RecompactedGrab19.60.359.7GP-GC(SP-SC)8cording to NDOT standards, requiring all material be screened through the #10 sieve. Bulk gradations shown plain; only000000	RecompactedGrab18.30.3412.7GW-GC(SC)511 (30)5.65Direct Shear RecompactedMC15.50.4913.6GC(SC)214 (31)2.7Direct Shear RecompactedGrab18.10.3412.2GW-GC(SC)911 (28)1.1Direct Shear RecompactedGrab18.00.3613.6SC-SM714 (31)2.6Direct Shear RecompactedGrab19.30.3511.4SW-SC(SC)511 (25)12.2Direct Shear RecompactedGrab19.60.356.2GP-GC(SC)611 (11)9.1Direct Shear RecompactedGrab19.20.386.2GW-GC(SW-SC)513 (13)3Direct Shear RecompactedGrab19.60.359.7GP-GC(SP-SC)89 (9)0.1Direct Shear RecompactedGrab19.60.359.7GP-GC(SP-SC)89 (9)	Neccompacted         Grab         18.3         0.34         12.7         GW-GC(SC)         5         11 (30)         42 (70)           5.65         Direct Shear Recompacted         MC         15.5         0.49         13.6         GC(SC)         2         14 (31)         41 (69)           2.7         Direct Shear Recompacted         Grab         18.1         0.34         12.2         GW-GC(SC)         9         11 (28)         43 (72)           1.1         Direct Shear Recompacted         Grab         18.0         0.36         13.6         SC-SM         7         14 (31)         50 (69)           2.6         Direct Shear Recompacted         Grab         19.3         0.35         11.4         SW-SC(SC)         5         11 (25)         54 (75)           12.2         Direct Shear Recompacted         Grab         19.3         0.35         6.2         GP-GC(SC)         6         11 (11)         33 (91)           9.1         Direct Shear Recompacted         Grab         19.6         0.35         6.2         GP-GC(SC)         6         11 (11)         33 (91)           9.1         Direct Shear Recompacted         Grab         19.6         0.35         6.2         GP-GC(SC)         6         11 (11)	Recompacted         Grab         18.3         0.34         12.7         GW-GC(SC)         5         11 (30)         42 (70)         47 (0)           5.65         Direct Shear Recompacted         MC         15.5         0.49         13.6         GC(SC)         2         14 (31)         41 (69)         44 (0)           2.7         Direct Shear Recompacted         Grab         18.1         0.34         12.2         GW-GC(SC)         9         11 (28)         43 (72)         46 (0)           1.1         Direct Shear Recompacted         Grab         18.0         0.36         13.6         SC-SM         7         14 (31)         50 (69)         36 (0)           2.6         Direct Shear Recompacted         Grab         19.3         0.35         11.4         SW-SC(SC)         5         11 (25)         54 (75)         35 (0)           12.2         Direct Shear Recompacted         Grab         19.6         0.35         6.2         GP-GC(SC)         6         11 (11)         33 (91)         56 (0)           9.1         Direct Shear Recompacted         Grab         19.2         0.38         6.2         GP-GC(SC)         6         11 (11)         33 (91)         56 (0)           9.1         Direct Shear Re	Recompacted         Grab         18.3         0.34         12.7         GW-GC(SC)         5         11 (30)         42 (70)         47 (0)         7.6           5.65         Direct Shear Recompacted         MC         15.5         0.49         13.6         GC(SC)         2         14 (31)         41 (69)         44 (0)         28.1           2.7         Direct Shear Recompacted         Grab         18.1         0.34         12.2         GW-GC(SC)         9         11 (28)         43 (72)         46 (0)         121.5           1.1         Direct Shear Recompacted         Grab         18.0         0.36         13.6         SC-SM         7         14 (31)         50 (69)         36 (0)         18.4           2.6         Direct Shear Recompacted         Grab         19.3         0.35         11.4         SW-SC(SC)         5         11 (25)         54 (75)         35 (0)         0.0           12.2         Direct Shear Recompacted         Grab         19.6         0.35         6.2         GP-GC(SC)         6         11 (11)         33 (91)         56 (0)         0.0           9.1         Direct Shear Recompacted         Grab         20.3         0.31         8.3         GC-GM(SC-SM)         5	Recompacted         Grab         18.3         0.34         12.7         GW-GC(SC)         5         11 (30)         42 (70)         47 (0)         7.6         42.0           5.65         Direct Shear Recompacted         MC         15.5         0.49         13.6         GC(SC)         2         14 (31)         41 (69)         44 (0)         28.1         34.0           2.7         Direct Shear Recompacted         Grab         18.1         0.34         12.2         GW-GC(SC)         9         11 (28)         43 (72)         46 (0)         121.5         27.0           1.1         Direct Shear Recompacted         Grab         18.0         0.36         13.6         SC-SM         7         14 (31)         50 (69)         36 (0)         18.4         26.2           2.6         Recompacted Recompacted         Grab         19.3         0.35         11.4         SW-SC(SC)         5         11 (25)         54 (75)         35 (0)         0.0         46.0           12.2         Direct Shear Recompacted         Grab         19.6         0.35         6.2         GP-GC(SC)         6         11 (11)         33 (91)         56 (0)         0.0         41.7           9.1         Direct Shear Recompacted         G

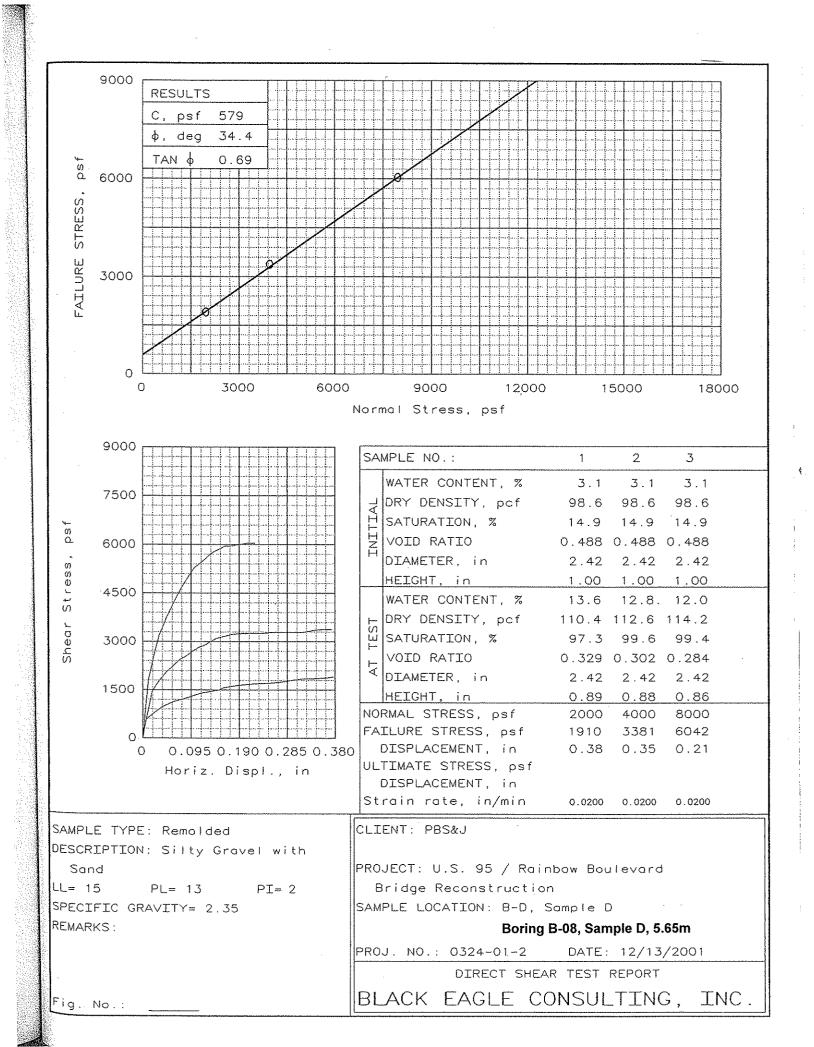
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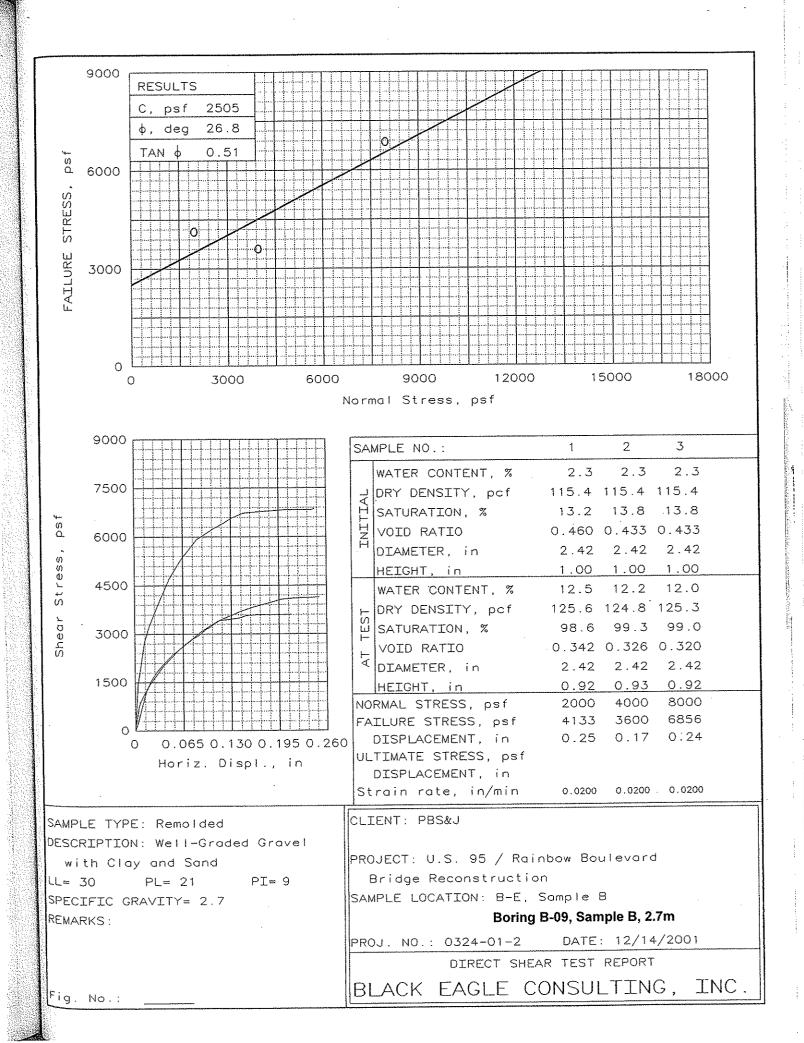


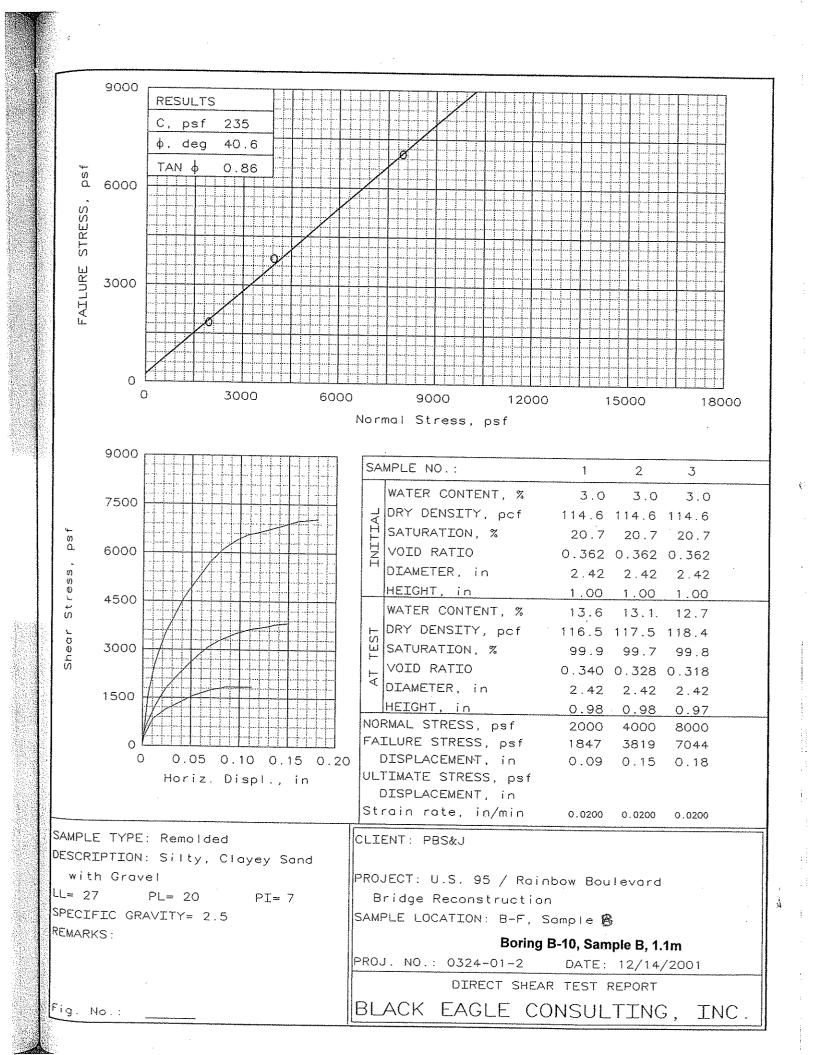


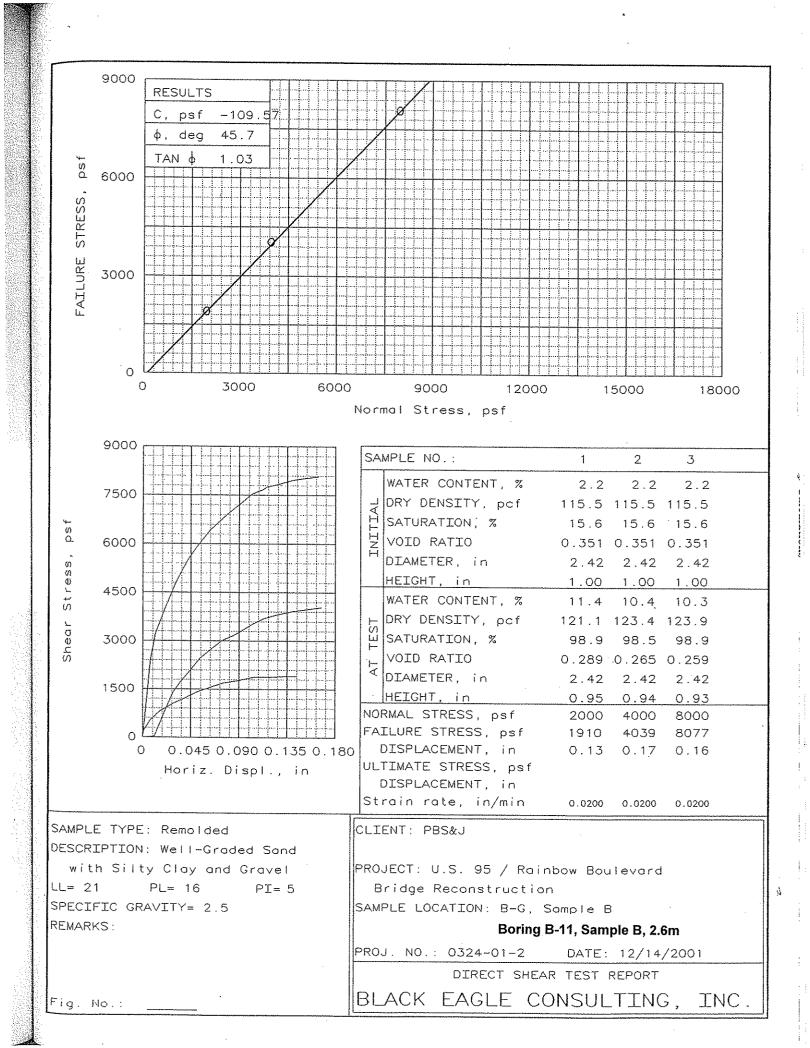


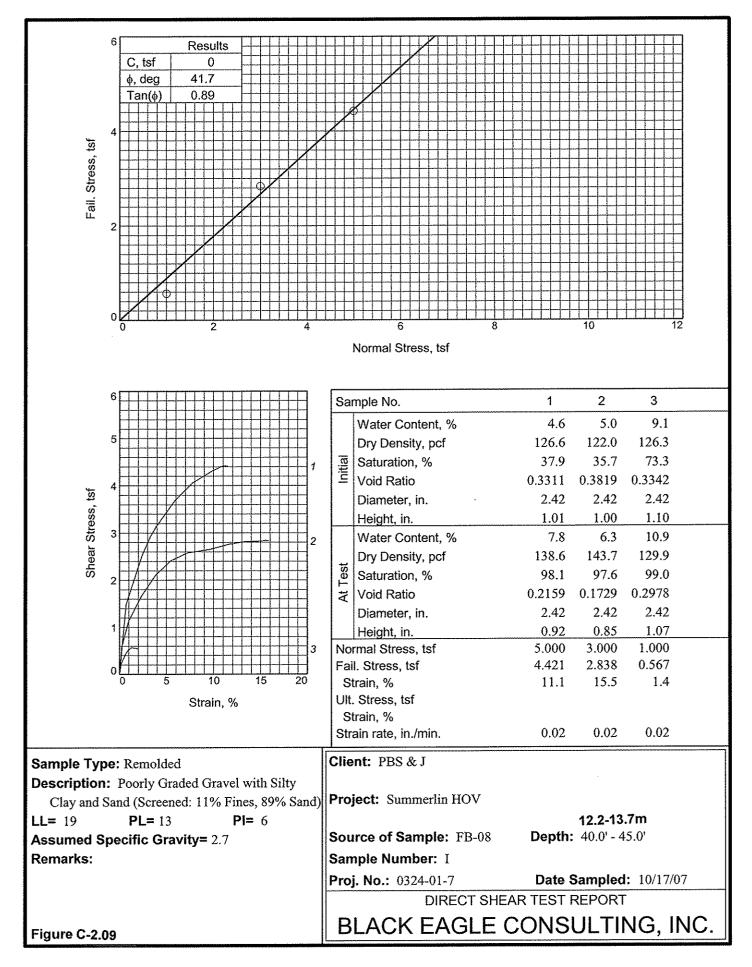


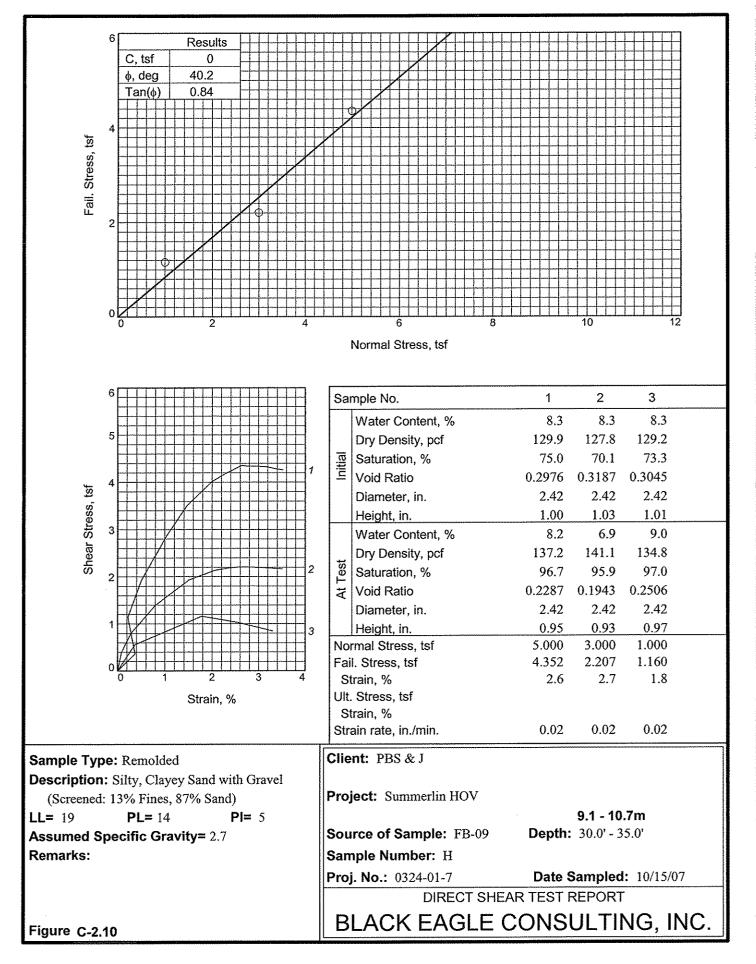


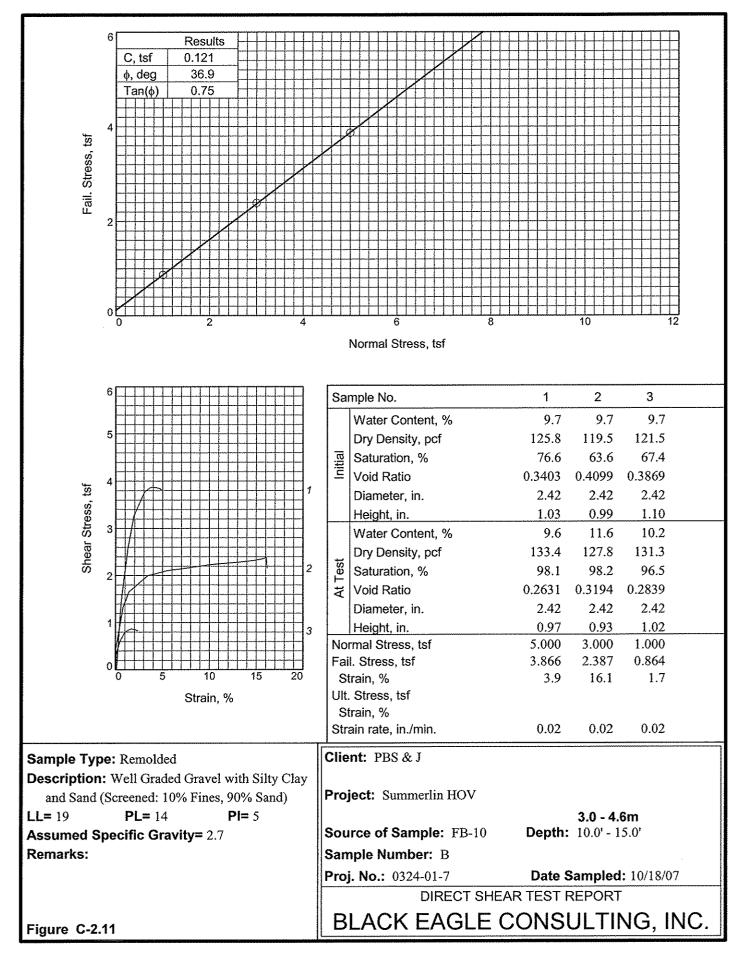


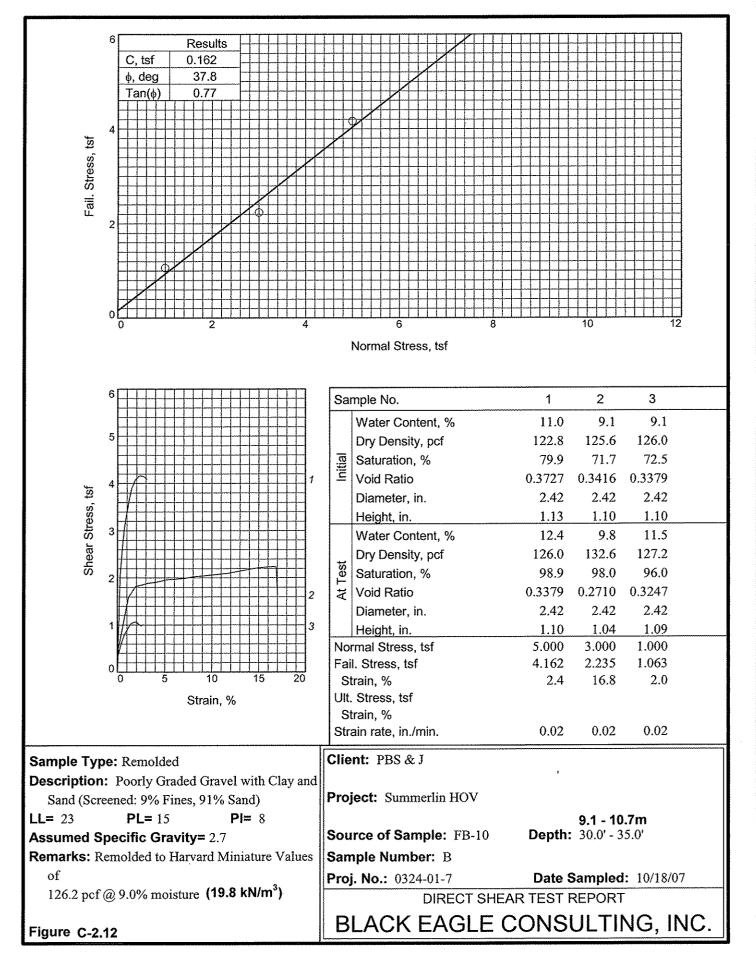












## **C-3**

### **CHEMICAL TEST RESULTS**

Black Eagle Consulting 1345 Capital Blvd, Suite A Reno, NV 89502 Attn: Ron Weber Phone: 359-6600 Fax: 359	۰ ۰		EPA Lab ID: Received: Lab Sample ID: Reported:	NV004 11/20/01 5-111-124 12/04/01	
Project Name/Number: Sample ID: Date/Time Collected: Sampled By:	US 95/Rainbow/0324-01-1 See below 11/20/01 Client				
Parameter	Method	Results	Units	Analyzed	

US95/RAINBOW BORE C-A @ 3.5-5.0' Soluble Chloride pH Resistivity Soluble Sulfate	Boring B-07 300.0 T238A T235B 300.0	10 8.31 6000 56	_ mg/Kg SU Ω.cm mg/Kg	11/26/01 12/03/01 12/03/01 11/26/01
US95/RAINBOW BORE D-D @ 18.5-20' Soluble Chloride pH Resistivity Soluble Sulfate*	Boring B-08 300.0 T238A T235B 300.0	240 8.29 920 440	mg/Kg SU Ω.cm mg/Kg	11/26/01 12/03/01 12/03/01 11/26/01

Comments: Sulfate & Chloride run from a 1:10 extract.

Lance Bell, Lab Manager

Black Eagle Consulting 1345 Capital Blvd, Suite A Reno, NV 89502 Attn: Ron Weber 
 EPA Lab ID:
 NV004

 Received:
 11/20/01

 Lab Sample ID:
 5-111-124

 Reported:
 12/04/01

Calculations

Phone: 359-6600 Fax: 359-7766

US 95/Rainbow/0324-01-1
See below
11/20/01
Client

Method	Results	Units	Analyzed
Boring B-09			
300.0	15	mg/Kg	11/26/01
T238A	8.21	SU	12/03/01
T235B	1200	Ω.cm	12/03/01
300.0	1400	mg/Kg	11/26/01
Boring B-10		· .	
300.0	<10	mg/Kg	11/26/01
T238A	8.60	<u></u> su	12/03/01
	6200	Ω.cm	12/03/01
300.0	<50	mg/Kg	11/26/01
	<ul> <li>Boring B-09         <ul> <li>300.0</li> <li>T238A</li> <li>T235B</li> <li>300.0</li> </ul> </li> <li>Boring B-10         <ul> <li>300.0</li> <li>T238A</li> <li>T235B</li> </ul> </li> </ul>	Boring B-09         300.0         15           T238A         8.21         T235B         1200           300.0         1400         300.0         1400           Boring B-10         300.0         <10         7238A         8.60           T235B         6200         6200         6200	Boring B-09         300.0         15         mg/Kg           T238A         8.21         SU           T235B         1200         Ω.cm           300.0         1400         mg/Kg           Boring B-10         300.0         <10

Comments:

Sulfate & Chloride run from a 1:10 extract.

Lance Bell, Lab Manager

Black Eagle Consulting 1345 Capital Blvd, Suite A Reno, NV 89502 Attn: Ron Weber

EPA Lab ID:	NV004
Received:	11/20/01
Lab Sample ID:	5-111-124
Reported:	12/04/01

Phone: 359-6600 Fax: 359-7766

Project Name/Number:		US 95/Rainbow/0324-01-1
Sample ID:	,	See below
Date/Time Collected:		11/20/01
Sampled By:		Client

Parameter	Method	Results	Units	Analyzed
US 95/RAINBOW BORE G-B @ 8.5-10'	Boring B-11			
Soluble Chloride	300.0	<10	mg/Kg	11/26/01
pH	T238A	8.61	ŠU	12/03/01
Resistivity	T235B	7400	Ω.cm	12/03/01
Soluble Sulfate	300.0	<50	mg/Kg	11/26/01

#### US 95/RAINBOW BORE H-A @ 3.5-5 Boring B-12 (not included in this report)

Soluble Chloride	e 300.0	<10	mg/Kg	11/26/01 `
pH	T238A	, 8.58	SU	12/03/01
Resistivity	- T235B	7200	Ω.cm	12/03/01
Soluble Sulfate	* 300.0	<50	mg/Kg	11/26/01

Comments:

Sulfate & Chloride run from a 1:10 extract.

Lance Bell, Lab Manager

Appendix III Calculations

Black Eagle Consulting 1345 Capital Blvd, Suite A Reno, NV 89502 Attn: Ron Weber 
 EPA Lab ID:
 NV004

 Received:
 11/20/01

 Lab Sample ID:
 5-111-124

 Reported:
 12/04/01

Calculations

Phone: 359-6600 Fax: 359-7766

Project Name/Number:	US 95/Rainbow/0324-01-1
Sample ID:	See below
Date/Time Collected:	11/20/01
Sampled By:	Client

Sampled By:	Client					
P	arameter	Method	Results	Units	Analyzed	
Solu 	BORE I-B @ 8.5-10' ble Chloride pH Resistivity uble Sulfate	Boring B-13 300.0 T238A T235B 300.0	<10 10.64 7500 <50	mg/Kg SU Ω.cm mg/Kg	11/26/01 12/03/01 12/03/01 11/26/01	

Comments:

Sulfate & Chloride run from a 1:10 extract.

Lance Bell, Lab Manager

#### Western Environmental Testing Laboratory Analytical Report

Black Eagle Consulting 1345 Capital Boulevard, Suite A Reno, NV 89502-7140 Attn: Gary Bomberger Phone: (775) 359-6600 Fax: (775) 359-7766 PO\Project: Summerlin HDV / 0324-01-7 
 Date Printed:
 5/29/2007

 OrderID:
 0705087

Collect Date/Time: 4/6/2007

Receive Date: 5/8/2007 16:40

Customer Sample ID:FB-03 A 5WETLAB Sample ID:0705087-001

Parameter	Method	Results	Units	Reporting Limit	Date Analyzed
Sulfide	A21.5-99	negative			5/9/2007
Redox Potential	SM 2580B	350	mV	1	5/10/2007
Sulfate	EPA 300.0	41	mg/kg	15	5/14/2007
Paste pH	SW846 9045B	7.55	pH Units		5/14/2007
Resistivity	SM 2510B	3100	ohms.cm	1	5/10/2007

Customer Sample ID: FB-06 B 10 WETLAB Sample ID: 0705087-002 Collect Date/Time: 4/10/2007 Receive Date: 5/8/2007 16:40

Parar	neter		Method	Results	Units	Reporting Limit	Date Analyzed
Sulfid	e		A21.5-99	negative			5/9/2007
Redo	e Potential		SM 2580B	320	mV	1	5/10/2007
Sulfat	e		EPA 300.0	45	mg/kg	15	5/14/2007
Paste	pH	1.14	SW846 9045B	8.19	pH Units	· · ·	5/14/2007
Resist	tivity		SM 2510B	4300	ohms.cm	1	5/10/2007

#### Western Environmental Testing Laboratory **Analytical Report**

**Black Eagle Consulting** 1345 Capital Boulevard, Suite A Reno, NV 89502-7140 Attn: Gary Bomberger Fax: (775) 359-7766 **Phone:** (775) 359-6600 PO\Project: Summerlin/0324-01-7

10/31/2007 Date Printed: 0710235 OrderID:

Customer Sample ID: WETLAB Sample ID:	FB-08 Bulk 35 0710235-001	-40'	·	•	Collect Date/Time: Receive Date:	
Parameter		Method	Results	Units	Reporting Limit	Date Analyzed
Chloride		EPA 300.0	190	mg/kg	15	10/31/2007
Sulfate		EPA 300.0	200	mg/kg	15	10/31/2007
Paste pH		SW846 9045B	7.92	pH Units		10/31/2007
Resistivity		SM 2510B	670	ohms.cm	1	10/31/2007

FB-09 Bulk 30-35' **Customer Sample ID:** 

WETLAB Sample ID: 0710235-002

Collect Date/Time: 10/19/2007 Receive Date: 10/24/2007 15:15

Parameter	Method	Results	Units	Reporting Limit	Date Analyzed
Chloride	EPA 300.0	77	mg/kg	15	10/31/2007
Sulfate	EPA 300.0	180	mg/kg	15	10/31/2007
Paste pH	SW846 9045B	8.29	pH Units	41 A	10/31/2007
Resistivity	SM 2510B	830	ohms.cm	. <b>1</b>	10/31/2007

**Customer Sample ID:** 0710235-003 WETLAB Sample ID:

FB-10 Bulk 10-15'

Collect Date/Time: 10/19/2007 Receive Date: 10/24/2007 15:15

				Reporting	Date
Parameter	Method	Results	Units	Limit	Analyzed
Chloride	EPA 300.0	36	mg/kg	15	10/31/2007
Sulfate	EPA 300.0	120	mg/kg	15	10/31/2007
Paste pH	SW846 9045B	8.33	pH Units		10/31/2007
Resistivity	SM 2510B	1600	ohms.cm	1	10/31/2007