

# GEOTECHNICAL REPORT

## SR 160 CABLE BARRIER INSTALLATION MILEPOST CL 21.89 TO NY 0.97

PAHRUMP, NEVADA

AUGUST 2010



MATERIALS DIVISION

**STATE OF NEVADA  
DEPARTMENT OF TRANSPORTATION  
MATERIALS DIVISION  
GEOTECHNICAL SECTION**

**GEOTECHNICAL REPORT  
SR 160 CABLE BARRIER INSTALLATION  
MILEPOST CL 21.89 TO NY 0.97  
PAHRUMP, NEVADA  
AUGUST 2010**

**E.A. 73587**

**CLARK COUNTY/NYE COUNTY, NEVADA**

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## **INTRODUCTION**

### **General**

This geotechnical summary has been produced for the traffic cable barrier installation project to be constructed on SR 160 between Mileposts Clark 21.89 and Nye 0.97, between Mountain Springs and Pahrump, Nevada. The purpose of this summary is to provide information regarding the subsurface soil and groundwater conditions along the project alignment. Specific recommendations concerning the design parameters and constructability of the cable barrier foundation elements are also provided. A site specific geotechnical investigation was conducted specifically for the traffic cable barrier project.

### **Other Reports and Investigations**

Original construction of this section of SR 160 as a two lane highway (existing westbound lanes) was constructed in 1954 under NDOT Contract 890. Existing eastbound lanes, widening of westbound lanes, and other improvements were constructed under NDOT Contracts 2785 (1996) and 2883 (1998), making SR 160 a four lane divided highway separated by a 56 foot wide median.

## **PROJECT DESCRIPTION**

The Nevada Department of Transportation has determined that a traffic cable barrier is to be constructed along the center median of SR 160 between Mileposts CL 21.89 and NY 0.97, stations “X2” 1140+67 and “P” 374+58 respectively. Approximately 22 miles of traffic cable barrier will be constructed in 19 discreet segments. The center median slopes throughout the project are scheduled to be flattened to a 6:1 (H:V) slope geometry using NDOT Borrow Material.

## **LOCAL GEOLOGY**<sup>1</sup>

The majority of the project alignment is located in the Pahrump Valley. The eastern segment of the project alignment from Station is located in the foothills of the Spring Mountains. According to available geologic references the portion of the project site located within the Pahrump Valley

is founded on Quaternary aged alluvium consisting of coarse, gravelly deposits spread by sporadic sheet floods on wide slopes bordering high ranges and bouldery deposits in alluvial fans built up by ephemeral streams that flow from narrow canyons. Granular deposits grade down to sands and silts in valley bottoms. Gravel firmly cemented with calcium carbonate (caliche) is widely exposed in bluffs along major washes that traverse the alluviated slopes bordering high ranges. This gravel consists chiefly of limestone and dolomite fragments, and the carbonate cement was dissolved either from the fragments or from the parent bedrock in the range. Available geologic references also indicate that the eastern portion of the project that climbs out of the Pahrump Valley into the Spring Mountains is founded mainly on Quaternary alluvium with small sections of the project alignment founded on the Bird Spring, Kaibab, Toroweap and Moenkipi Formations.

## **FIELD INVESTIGATION**

A geotechnical field investigation was conducted June 7<sup>th</sup> through June 10<sup>th</sup>, 2010. The subsurface soil conditions were explored by drilling twelve boreholes, identified as CBP1 through CBP12. Boring locations were selected at various proposed cable barrier rail end terminals, determined from preliminary drawings provided by NDOT Roadway Design. Boring Location Map sheets depicting approximate boring locations are included in Appendix A. A Key to Boring Logs and details of borings shown in Exploration Logs can be found in Appendix B. Boring elevations provided on the logs were determined by aerial mapping collected approximately 6 years ago. Surface elevations are accurate to approximately 2 feet according to NDOT Roadway Design.

Logs of the subsurface conditions, as encountered during the field investigation, were recorded by NDOT Geotechnical Engineering staff. All soil samples were examined and identified in the field in accordance with ASTM D2488. Additional soil classification was subsequently performed on soil samples using the Unified Soil Classification System (USCS) in accordance with ASTM D2487 upon completion of laboratory testing. Where soil tests are not listed in the appropriate column of the Exploration Logs, the USCS symbols and terminology are based solely on visual-manual identification (ASTM D2488) rather than laboratory classification.

Drilling was performed using an NDOT Diedrich D-120 drill rig (Drill Rig Unit #1627) equipped with an automatic hammer. Hollow Stem Continuous Flight Augering (HSA) methods were used to explore all borings. Representative bulk soil samples were obtained from auger cuttings at depths indicated on the Exploration Logs. Drive samples were obtained using both a Standard Penetration Testing (SPT, ASTM D1586) sampler and a California Modified (CMS) sampler at locations noted on the Exploration Logs. The drive samples were advanced using a 140-pound hammer with a drop of 30 inches. The energy transfer from the automatic hammer into the drill string is 72% (SPT energy calibration by Gregg Drilling and Testing, Inc., June 11, 2009) with an approximate energy correction factor of 1.2. Sampler driving resistance (N-value), expressed as blows per one foot of penetration, is presented on the Exploration Logs at the respective depth. The N-value is an indication of the apparent density of coarse-grained soils and the consistency of fine-grained soils. The blow counts presented on the Exploration Logs have not been corrected for hammer efficiency, overburden pressure, rod length, etc.

## **LABORATORY ANALYSES**

Soil samples were tested at the NDOT Materials and Testing Laboratory in Carson City, Nevada. Soils were classified using the Unified Soil Classification System (USCS) in accordance with ASTM D 2487.

Particle size gradations through No. 200 sieve (NV T 206), Atterberg Limits (AASHTO T 89 and T 90), Resistance Value (R-value, NV T 115), Natural Moisture Content (AASHTO T 265), Unit Weight, Direct Shear (AASHTO T 236), Chlorides (AASHTO T 291 A), Sulfates (AASHTO T 290 B), pH (AASHTO T 289), and Resistivity (AASHTO T 288) tests were completed to assist in sample identification, classification, and evaluation.

Individual laboratory test results for soil samples can be found in Appendix C of this report.

## **DISCUSSION AND RECOMMENDATIONS**

### **Subsurface Conditions**

Existing roadway cuts and embankment fill are generally shallow along the cable barrier rail project alignment with most of the project alignment located close to original ground, with intermittent transitions from cuts to fills along the project alignment. Existing roadway embankment fills of approximately up to 10 feet can be expected along the cable barrier project alignment. Existing roadway embankment can be generally classified as dense to very dense silty and clayey sands and gravels with varying amounts of cobbles. Roadway cuts of approximately up to 10 feet exist along the project alignment. Native material in cut sections and below fills can be generally classified as very dense silty and clayey sands and gravels with varying amounts of cobbles, boulders, and caliche.

Based on project files for referenced previous NDOT contracts and laboratory testing for this project, Resistance Values (R-values) of the top 10 feet of native ground range from 50 to 88. The R-value test is used by NDOT to measure subgrade strength and expansion potential, and is used in the design of flexible pavements.

Based on project files for referenced previous NDOT contracts and laboratory testing for this project, chemical analyses indicate that native soils range from noncorrosive to corrosive intermittently along the project alignment. Chemical analyses results performed on samples of native soils obtained from the recent field investigation are included in Appendix C.

Sample B from boring CBP1 was the only relatively undisturbed sample successfully collected during the recent field investigation. Direct shear test results for sample B1 from boring CBP1 indicate a residual angle of internal friction of 30 degrees and cohesion equal to 1.23 psi. This sample is not representative of the granular material found in all other borings during the recent field investigation and in past field investigations. An angle of internal friction on 34 degrees and cohesion equal to zero is recommended for use in foundation design for this project.

## **Groundwater**

No ground water was encountered in any of the borings drilled along this project alignment during the subsurface investigation. Groundwater level is estimated to be at 2810 feet above sea level in the Pahrump Valley. <sup>2</sup>

## **Frost Depth**

Assume a frost depth of 1 foot for foundation design. <sup>3</sup>

## **Excavations** <sup>4</sup>

Native materials underlying the project site can generally be classified as OSHA Class B soils defined as granular cohesionless soils including: angular gravel (similar to crushed rock), silt, silt loam, sandy loam and, in some cases, silty clay loam and sandy clay loam. OSHA limits excavation cut slopes in Class B soils to 1H:1V. These limits and soil classifications may change based on the soil conditions exposed during construction. The working area in the median of SR 160 may require the contractor to shore foundation excavations to avoid interfering with the existing roadway pavement. Static and dynamic surcharges should be kept a minimum horizontal distance equal to 50% of the total excavations depth from shored excavation walls. Otherwise, these surcharges shall be accounted for in shoring designs. All excavation work shall conform in accordance with NDOT's 2001 Edition of Standard Specifications for Road and Bridge Construction and current OSHA Excavation Standards.

## **Constructability**

Cobbles, boulders, bedrock, and caliche may be encountered during cable barrier rail installation. The contractor should consider the effect that cobbles, boulders, bedrock, and caliche might have on construction operations. Such effects may include, but not be limited to, longer drilling times, more difficult drilling conditions, etc. Specialized equipment or techniques may also be required. The contractor should note that drilled shafts or anchor block foundations must conform to the requirements presented in Section's 502 and 509 of NDOT's 2001 Edition of Standard Specifications for Road and Bridge Construction.



## **REFERENCES**

1. Longwell, C.R., Pampeyan, E.H., and Bower, Ben, *Geology and Mineral Deposits of Clark County, Nevada*, Bulletin 62, Nevada Bureau of Mines prepared cooperatively by the U.S. Geological Survey. Mackey School of Mines, University of Nevada–Reno, 1965.
2. U.S. Geological Survey, Water Resources Data, Nevada, Water Year 2001. Report NV-01-1, 2002, page 492.
3. Barker, R.M., Duncan, J.M., Rojiani, K.B., Ooi, P.S.K., Tan, C.K., and Kim, S.G., *Manuals for the Design of Bridge Foundations*, Transportation Research Board National Research Council, Washington D.C., December 1991. National Cooperative Highway Research Program Report 343, page 7.
4. OSHA, 29 CFR, Part 1926, Subpart P.
5. NDOT, Contract 890 As-built Construction Plans, 1954.
6. NDOT, Contract 2785 As-built Construction Plans and project files, 1996.
7. NDOT, Contract 2883 As-built Construction Plans and project files, 1998.
8. NDOT, *Geotechnical Policies and Procedures Manual*, 2005.
9. NDOT, *Standard Specifications for Road and Bridge Construction*, 2001.

# **APPENDIX A : FIGURES**

Location Map

Boring Location Map

Supporting Photographs

Location Map

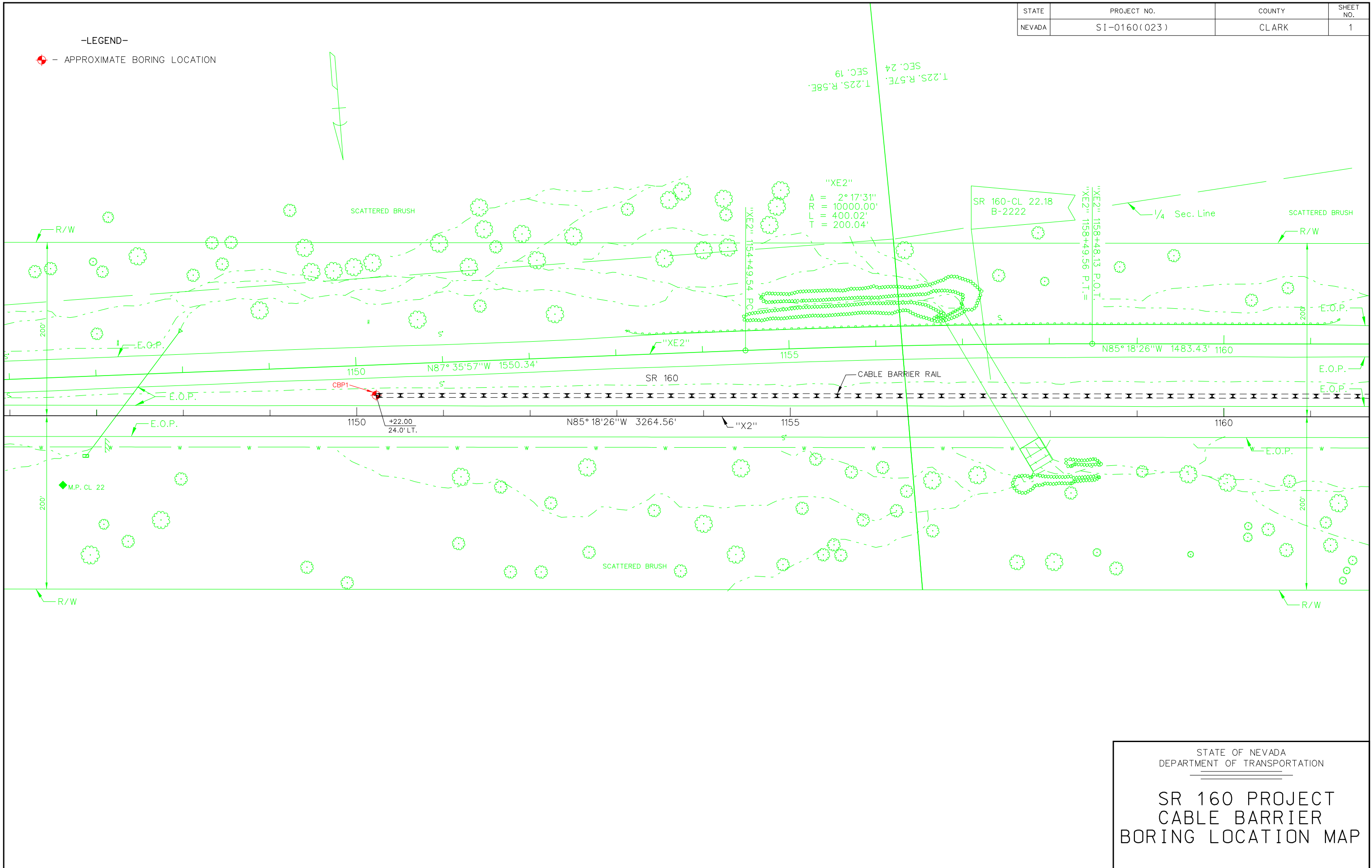


PROJECT LIMITS

STATE	PROJECT NO.	COUNTY	SHEET NO.
NEVADA	SI-0160(023)	CLARK	1

-LEGEND-

◆ - APPROXIMATE BORING LOCATION



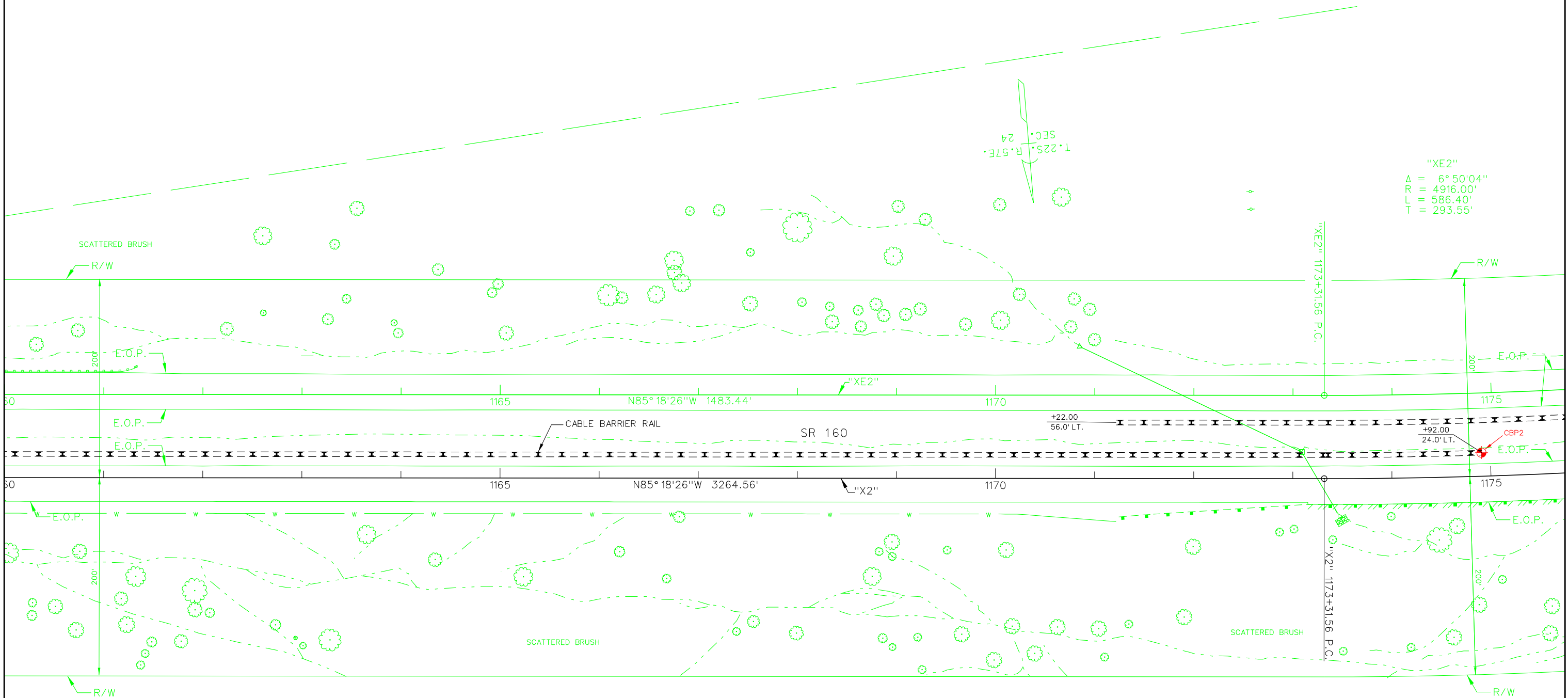
STATE OF NEVADA  
 DEPARTMENT OF TRANSPORTATION

**SR 160 PROJECT  
 CABLE BARRIER  
 BORING LOCATION MAP**

STATE	PROJECT NO.	COUNTY	SHEET NO.
NEVADA	SI-0160(023)	CLARK	2

-LEGEND-

⊕ - APPROXIMATE BORING LOCATION




"XE2"  
 $\Delta = 6^\circ 50'04''$   
 $R = 4916.00'$   
 $L = 586.40'$   
 $T = 293.55'$

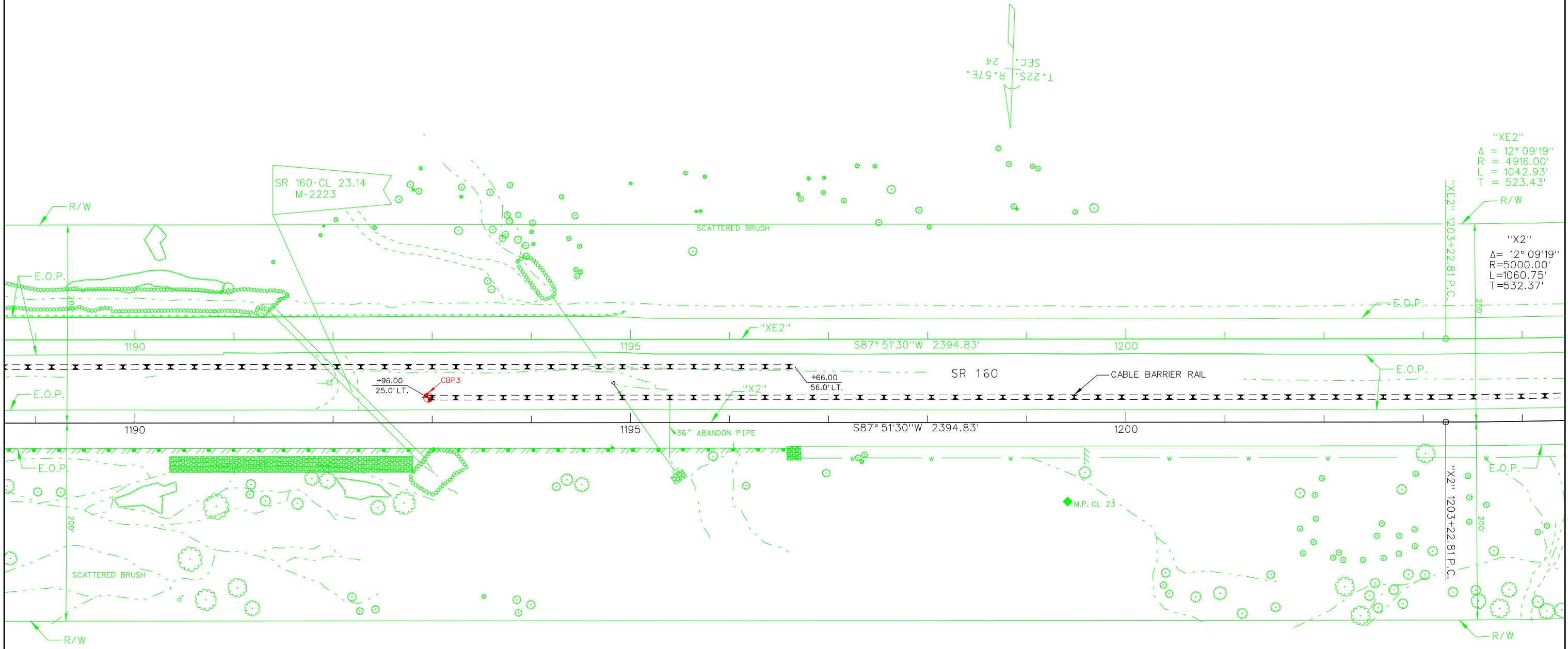
1.22S. R.57E.  
 SEC. 24

STATE OF NEVADA  
 DEPARTMENT OF TRANSPORTATION

**SR 160 PROJECT  
 CABLE BARRIER  
 BORING LOCATION MAP**

STATE	PROJECT NO.	COUNTY	SHEET NO.
NEVADA	SI-0160(023)	CLARK	3

-LEGEND-  
 - APPROXIMATE BORING LOCATION



"XE2"  
 $\Delta = 12^{\circ}09'19''$   
 $R = 4916.00'$   
 $L = 1042.93'$   
 $T = 523.43'$

"XE2"  
 $\Delta = 12^{\circ}09'19''$   
 $R = 5000.00'$   
 $L = 1060.75'$   
 $T = 532.37'$

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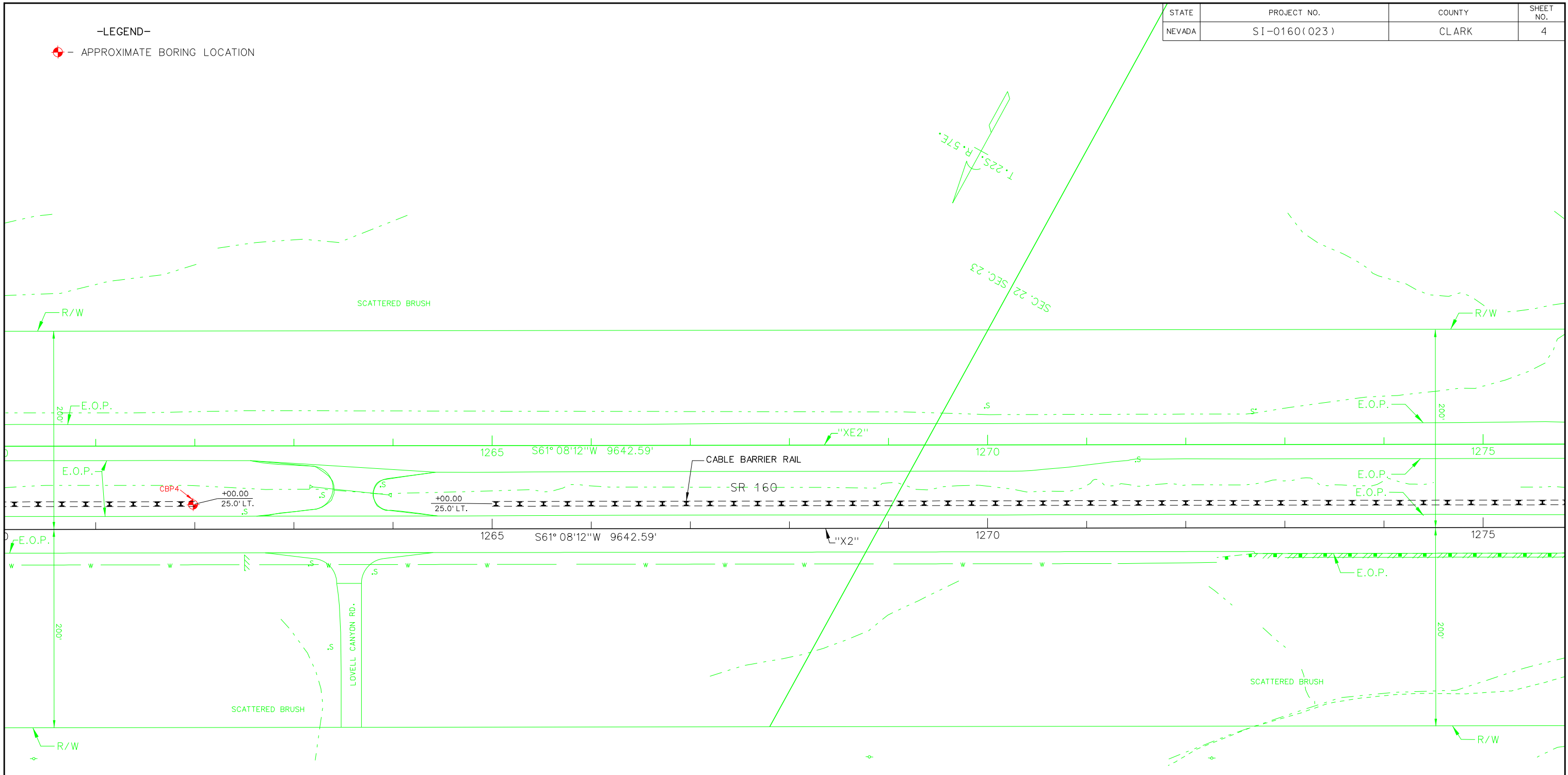
**SR 160 PROJECT  
CABLE BARRIER  
BORING LOCATION MAP**



STATE	PROJECT NO.	COUNTY	SHEET NO.
NEVADA	SI-0160(023)	CLARK	4

-LEGEND-

◆ - APPROXIMATE BORING LOCATION



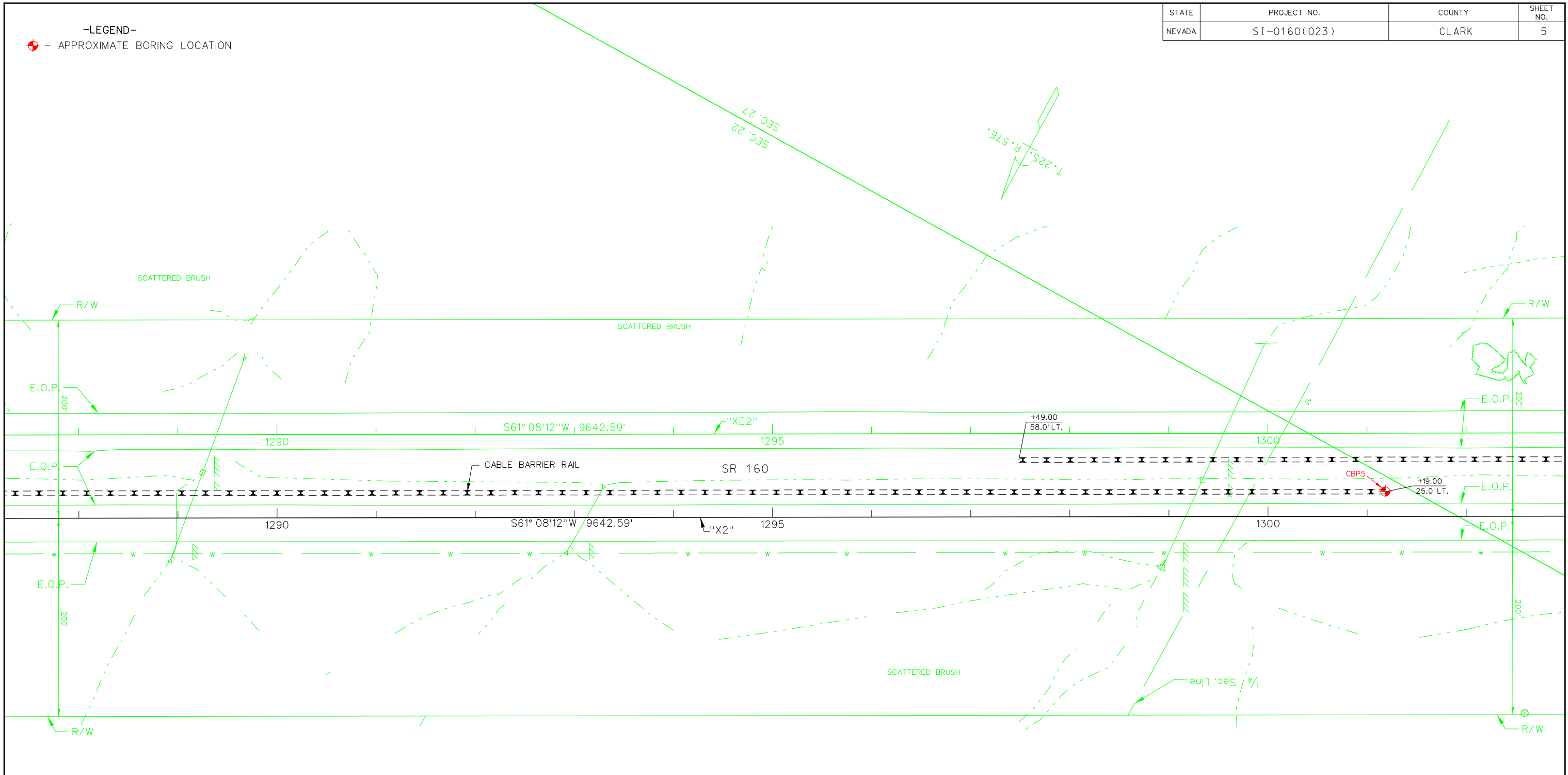
STATE OF NEVADA  
 DEPARTMENT OF TRANSPORTATION

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SR 160 PROJECT  
 CABLE BARRIER  
 BORING LOCATION MAP

STATE	PROJECT NO.	COUNTY	SHEET NO.
NEVADA	SI-0160(023)	CLARK	5

-LEGEND-  
 - APPROXIMATE BORING LOCATION



STATE OF NEVADA  
DEPARTMENT OF TRANSPORTATION

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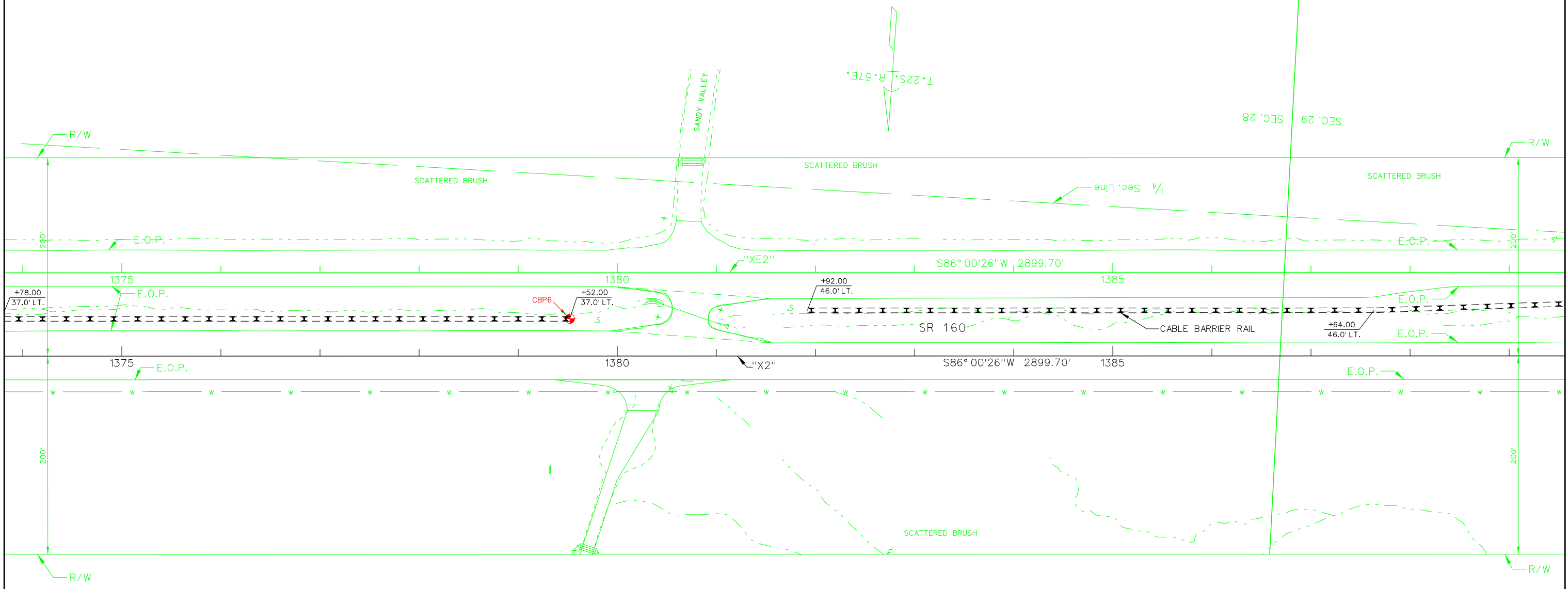
SR 160 PROJECT  
CABLE BARRIER  
BORING LOCATION MAP



STATE	PROJECT NO.	COUNTY	SHEET NO.
NEVADA	SI-0160(023)	CLARK	6

-LEGEND-


📍 - APPROXIMATE BORING LOCATION



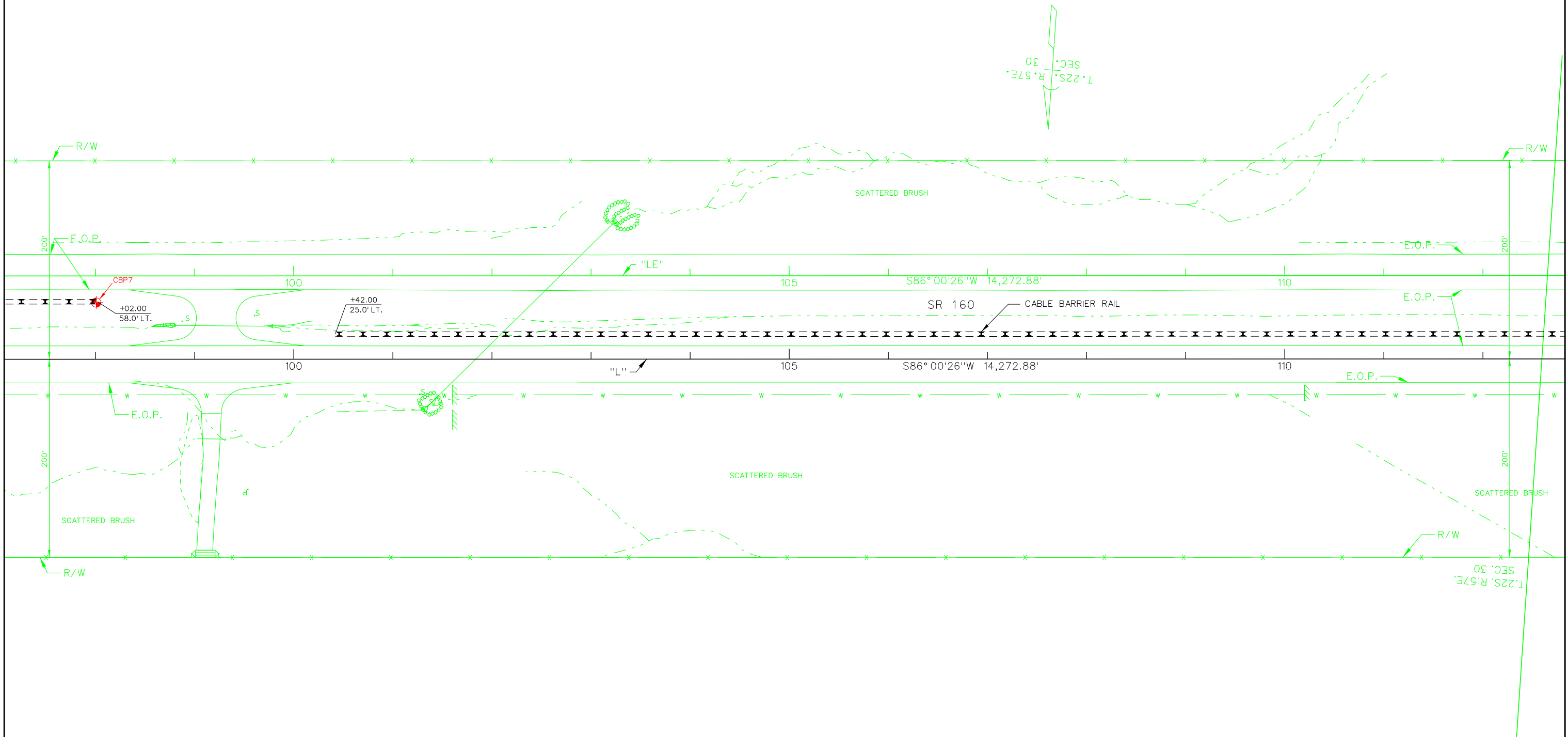
STATE OF NEVADA  
 DEPARTMENT OF TRANSPORTATION

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SR 160 PROJECT  
 CABLE BARRIER  
 BORING LOCATION MAP

-LEGEND-  
 - APPROXIMATE BORING LOCATION

STATE	PROJECT NO.	COUNTY	SHEET NO.
NEVADA	SI-0160(023)	CLARK	7



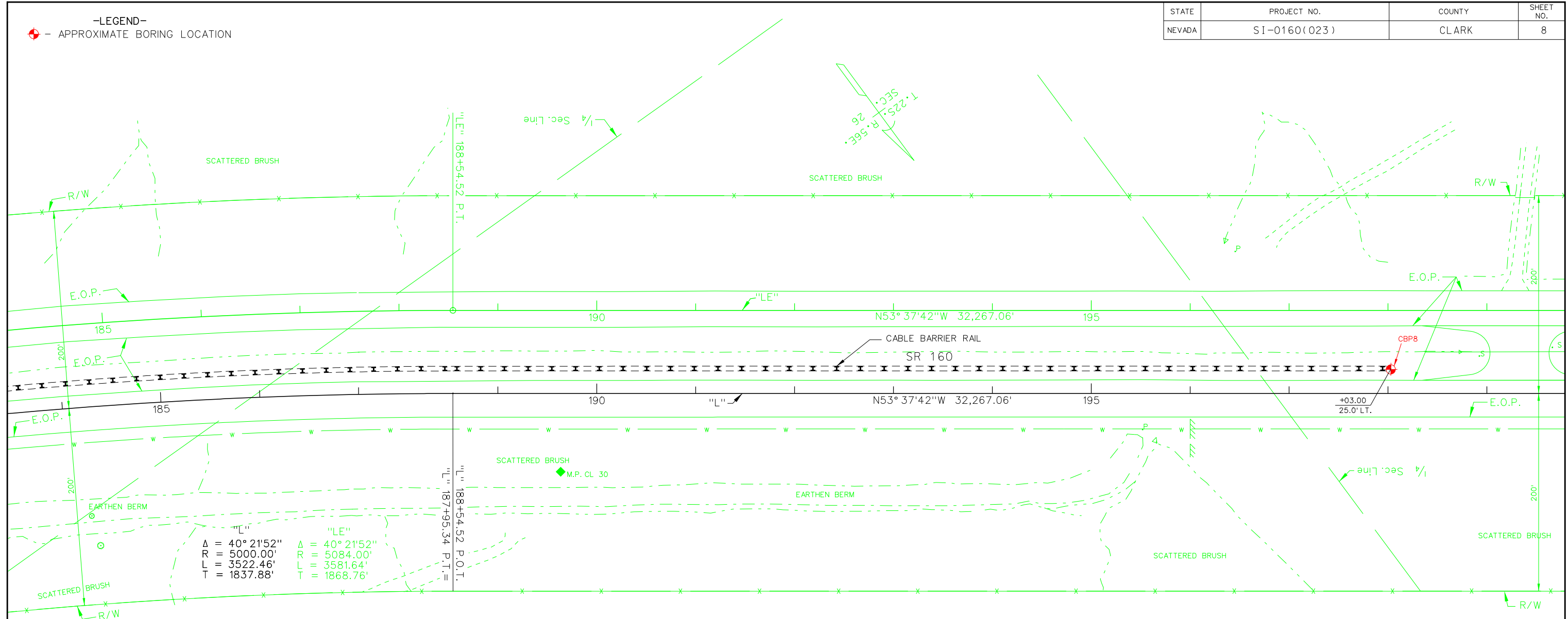
STATE OF NEVADA  
 DEPARTMENT OF TRANSPORTATION

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SR 160 PROJECT  
 CABLE BARRIER  
 BORING LOCATION MAP

-LEGEND-  
 - APPROXIMATE BORING LOCATION

STATE	PROJECT NO.	COUNTY	SHEET NO.
NEVADA	SI-0160(023)	CLARK	8




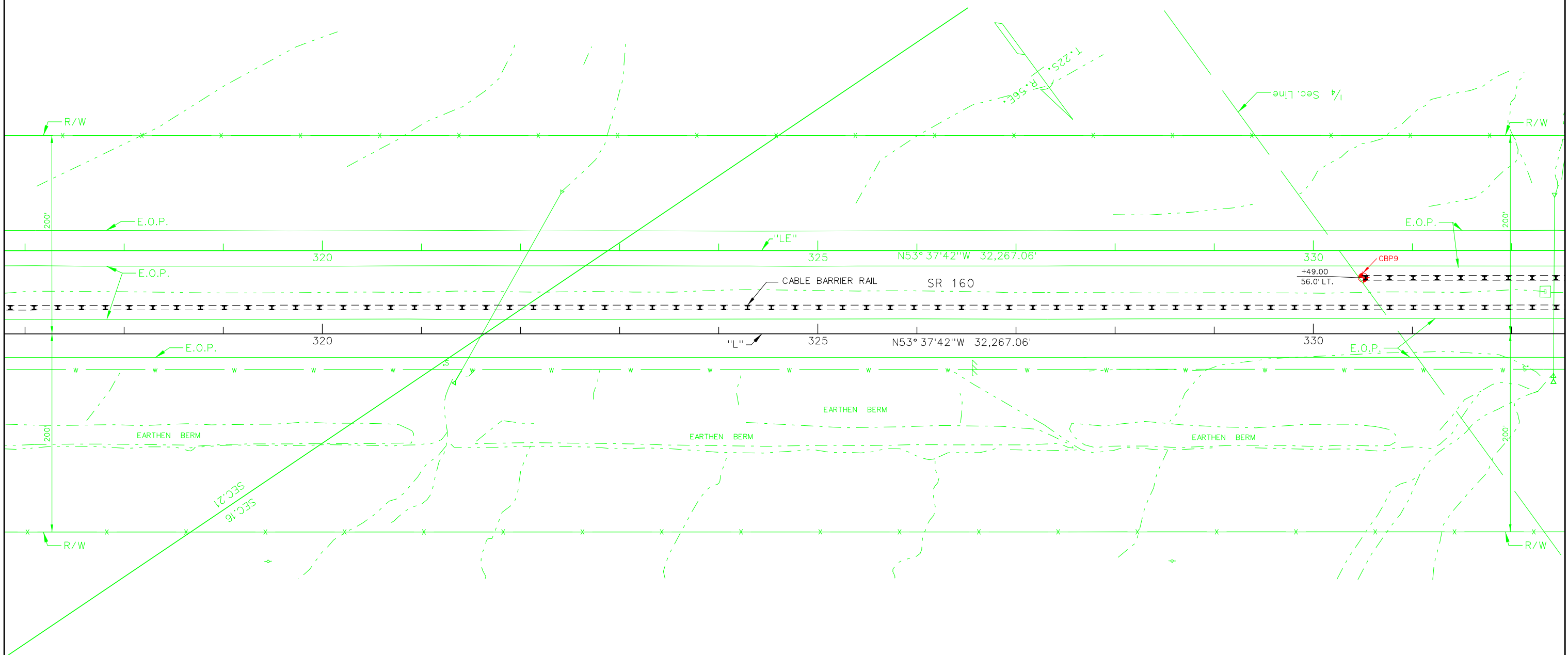
STATE OF NEVADA  
 DEPARTMENT OF TRANSPORTATION

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SR 160 PROJECT  
 CABLE BARRIER  
 BORING LOCATION MAP

STATE	PROJECT NO.	COUNTY	SHEET NO.
NEVADA	SI-0160(023)	CLARK	9

-LEGEND-  
 - APPROXIMATE BORING LOCATION



STATE OF NEVADA  
 DEPARTMENT OF TRANSPORTATION

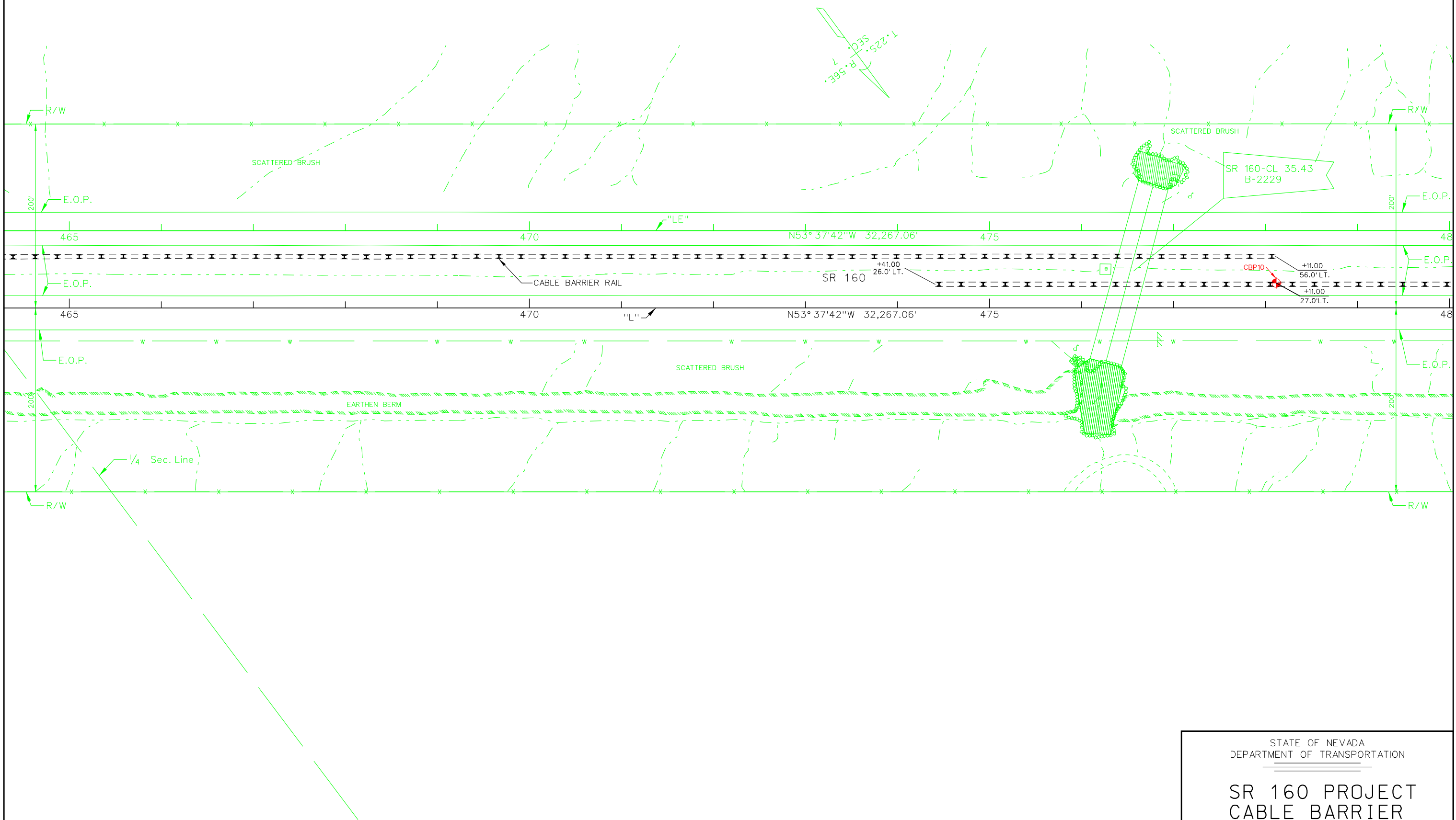
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SR 160 PROJECT  
 CABLE BARRIER  
 BORING LOCATION MAP

STATE	PROJECT NO.	COUNTY	SHEET NO.
NEVADA	SI-0160(023)	CLARK	10

-LEGEND-

⊕ - APPROXIMATE BORING LOCATION



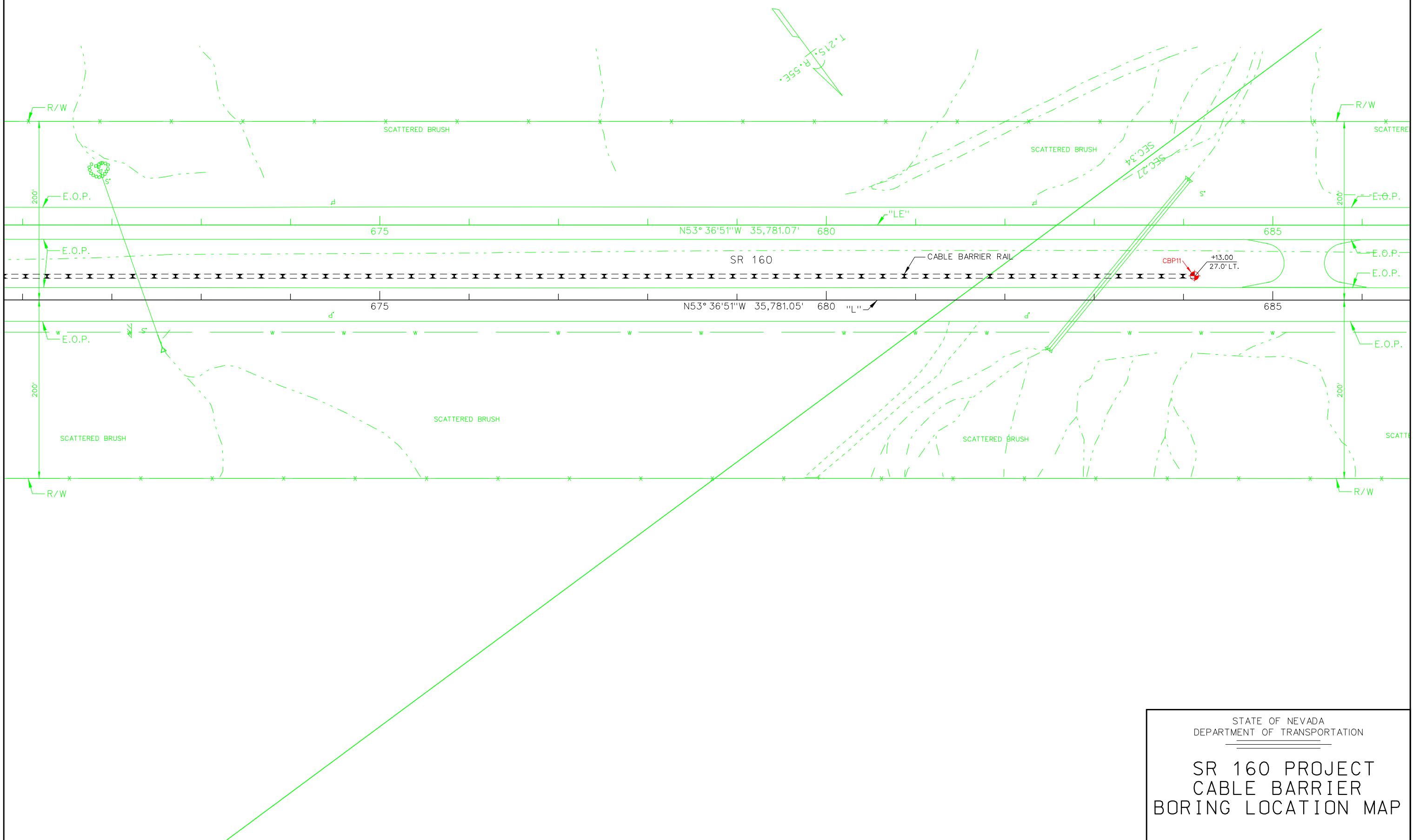
STATE OF NEVADA  
 DEPARTMENT OF TRANSPORTATION

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SR 160 PROJECT  
 CABLE BARRIER  
 BORING LOCATION MAP

STATE	PROJECT NO.	COUNTY	SHEET NO.
NEVADA	SI-0160(023)	CLARK	11


-LEGEND-  
 - APPROXIMATE BORING LOCATION



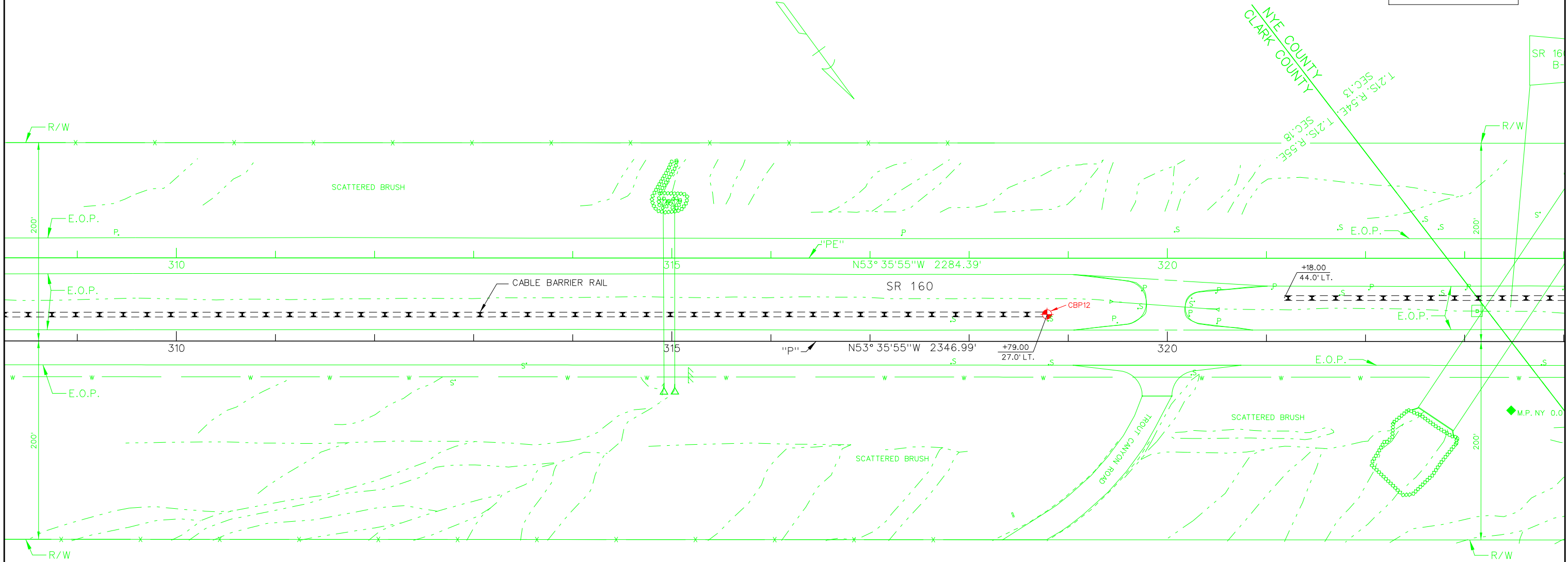
STATE OF NEVADA  
 DEPARTMENT OF TRANSPORTATION

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SR 160 PROJECT  
 CABLE BARRIER  
 BORING LOCATION MAP

-LEGEND-  
 - APPROXIMATE BORING LOCATION

STATE	PROJECT NO.	COUNTY	SHEET NO.
NEVADA	SI-0160(023)	CLARK	12
		NYE	



STATE OF NEVADA  
 DEPARTMENT OF TRANSPORTATION

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SR 160 PROJECT  
 CABLE BARRIER  
 BORING LOCATION MAP



## Supporting Photographs



Beginning of project at station "X2" 1140+67 looking east towards Mountain Springs.



Beginning of project at station "X2" 1140+67 at CBP1 looking west along project alignment.



CBP1, SPT Sample A, 2.5'-4.0' depth.



CBP1, SPT Sample C, 7.5'-8.0' depth.



CBP1, SPT Sample D, 10.0'-11.5' depth.



CBP1, SPT Sample E, 12.5'-14.0' depth.



# Supporting Photographs



CBP1, SPT Sample F, 15.0'-16.5' depth.



At station "X2" 1170 looking west near CBP2. Note cut on left and fill on right.



Drilling at CBP2.



Cut on left at CBP2.



CBP2, SPT Sample A, 0.5'-1.85' depth.



CBP2, SPT Sample B, 2.5'-2.85' depth.



# Supporting Photographs



CBP3, SPT Sample A, 2.5'-4.0' depth.



CBP3, CMS Sample B, 5.0'-5.6' depth.



CBP3, SPT Sample C, 7.5'-9.0' depth.



CBP3, SPT Sample E, 12.5'-12.9' depth.



CBP3, CMS Sample F, 15.0'-16.5' depth.



CBP1, SPT Sample G, 16.5'-17.5' depth.



# Supporting Photographs



At station "X2" 1262+00 at CBP4 looking west.



CBP4, SPT Sample A, 2.0'-2.9' depth.



CBP4, SPT Sample B, 5.0'-6.5' depth.



CBP4, SPT Sample D, 15.0'-16.5' depth.



CBP5, SPT Sample A, 1.0'-2.5' depth.



CBP5, SPT Sample B, 5.0'-6.4' depth.



## Supporting Photographs



CBP5, SPT Sample C, 7.5'-9.0' depth.



CBP5, SPT Sample D, 10.0'-11.5' depth.



CBP5, SPT Sample E, 15.0'-15.4' depth.



At station "X2" 1301+19 at CBP5 looking north.



Cuttings at CBP5.



CBP6, SPT Sample A, 5.0'-6.5' depth.



# Supporting Photographs



CBP6, SPT Sample B, 10.0'-11.3' depth.



At station "X2" 1379+52 at CBP6 looking west.



CBP7, SPT Sample A, 2.5'-2.95' depth.



CBP7, SPT Sample B, 5.0'-6.5' depth.



CBP8, SPT Sample A, 2.5'-3.0' depth.



CBP8, SPT Sample B, 5.0'-6.5' depth.



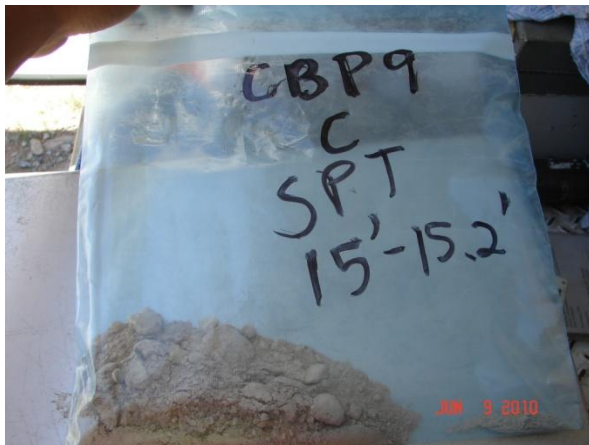
# Supporting Photographs



CBP8, SPT Sample C, 10.0'-11.5' depth.



Cuttings at CBP8.



CBP9, SPT Sample C, 15.0'-15.2' depth.



Cuttings at CBP9.



CBP10, SPT Sample A, 5.0'-6.5' depth.



CBP10, SPT Sample B, 7.5'-9.0' depth.



## Supporting Photographs



Drilling at CBP10.



CBP10, SPT Sample C, 10.0'-11.5' depth.



CBP10, SPT Sample D, 12.5'-14.0' depth.



CBP10, SPT Sample E, 15.0'-16.5' depth.



CBP11, SPT Sample B, 5.0'-6.0' depth.



CBP11, SPT Sample E, 15.0'-16.1' depth.

## Supporting Photographs



At station "P" 318+79 at CPB12 looking west towards the end of project.



CBP12, SPT Sample B, 5.0'-6.5' depth.



# **APPENDIX B: SUBSURFACE EXPLORATION DATA**

Key to Boring Logs

Exploration Logs

# KEY TO BORING LOGS

PARTICLE SIZE LIMITS								
CLAY	SILT	SAND			GRAVEL		COBBLES	BOULDERS
		FINE	MEDIUM	COARSE	FINE	COARSE		
.002 mm	#200	#40	#10	#4	¾ inch	3 inch	12 inch	

USCS GROUP	TYPICAL SOIL DESCRIPTION
GW	Well graded gravels, gravel-sand mixtures, little or no fines
GP	Poorly graded gravels, gravel-sand mixtures, little or no fines
GC	Clayey gravels, poorly graded gravel-sand-clay mixtures
SW	Well graded sands, gravelly sands, little or no fines
SP	Poorly graded sands, gravelly sands, little or no fines
SM	Silty sands, poorly graded sand-silt mixtures
SC	Clayey sands, poorly graded sand-clay mixtures
ML	Inorganic silts and very fine sands, rock flour, silty or clayey fine sands with slight plasticity
CL	Inorganic clays of low to medium plasticity, gravelly clays, sandy clays, silty clays, lean clays
OL	Organic silts and organic silt-clays of low plasticity
MH	Inorganic silts, micaceous or diatomaceous fine sandy or silty soils, elastic silts
CH	Inorganic clays of high plasticity, fat clays
OH	Organic clays of medium to high plasticity
PT	Peat and other highly organic soils

### MOISTURE CONDITION CRITERIA

<u>Description</u>	<u>Criteria</u>
Dry	Absence of moisture, dusty, dry to touch.
Moist	Damp, no visible free water.
Wet	Visible free water, usually below groundwater table.

### SOIL CEMENTATION CRITERIA

<u>Description</u>	<u>Criteria</u>
Weak	Crumbles or breaks with handling or little finger pressure.
Moderate	Crumbles or breaks with considerable finger pressure.
Strong	Won't break or crumble w/ finger pressure



Groundwater Elevation Symbols

STANDARD PENETRATION CLASSIFICATION*			
GRANULAR SOIL		CLAYEY SOIL	
BLOWS/FT	DENSITY	BLOWS/FT	CONSISTENCY
0 - 4	VERY LOOSE	0 - 1	VERY SOFT
5 - 10	LOOSE	2 - 4	SOFT
11 - 30	MEDIUM DENSE	5 - 8	MEDIUM STIFF
31 - 50	DENSE	9 - 15	STIFF
OVER 50	VERY DENSE	16 - 30	VERY STIFF
		31 - 60	HARD
		OVER 60	VERY HARD

\*Standard Penetration Test (N) 140 lb hammer  
30 inch free fall on 2 inch O.D. x 1.4 inch I.D. sampler.

Field Blow counts on California Modified Sampler (NCMS) can be converted to NSPT field by:  
(NCMS field)(0.62) = NSPT field

Blow counts from Automatic Hammer can be converted to Standard SPT N<sub>60</sub> by:  
Rig #1627: (NsPT field)(1.2) = N<sub>60</sub>  
Rig #1082: (NsPT field)(1.45) = N<sub>60</sub>

<u>TEST ABBREVIATIONS</u>	<u>SAMPLER NOTATION</u>																						
<table border="0" style="width: 100%;"> <tr> <td>CD CONSOLIDATED DRAINED</td> <td>O ORGANIC CONTENT</td> </tr> <tr> <td>CH CHEMICAL (CORROSIVENESS)</td> <td>OC CONSOLIDATION</td> </tr> <tr> <td>CM COMPACTION</td> <td>PI PLASTICITY INDEX</td> </tr> <tr> <td>CU CONSOLIDATED UNDRAINED</td> <td>RQD ROCK QUALITY DESIGNATION</td> </tr> <tr> <td>D DISPERSIVE SOILS</td> <td>RV R-VALUE</td> </tr> <tr> <td>DS DIRECT SHEAR</td> <td>S SIEVE ANALYSIS</td> </tr> <tr> <td>E EXPANSIVE SOIL</td> <td>SL SHRINKAGE LIMIT</td> </tr> <tr> <td>G SPECIFIC GRAVITY</td> <td>U UNCONFINED COMPRESSION</td> </tr> <tr> <td>H HYDROMETER</td> <td>UU UNCONSOLIDATED UNDRAINED</td> </tr> <tr> <td>HC HYDRO-COLLAPSE</td> <td>UW UNIT WEIGHT</td> </tr> <tr> <td>K PERMEABILITY</td> <td>W MOISTURE CONTENT</td> </tr> </table>	CD CONSOLIDATED DRAINED	O ORGANIC CONTENT	CH CHEMICAL (CORROSIVENESS)	OC CONSOLIDATION	CM COMPACTION	PI PLASTICITY INDEX	CU CONSOLIDATED UNDRAINED	RQD ROCK QUALITY DESIGNATION	D DISPERSIVE SOILS	RV R-VALUE	DS DIRECT SHEAR	S SIEVE ANALYSIS	E EXPANSIVE SOIL	SL SHRINKAGE LIMIT	G SPECIFIC GRAVITY	U UNCONFINED COMPRESSION	H HYDROMETER	UU UNCONSOLIDATED UNDRAINED	HC HYDRO-COLLAPSE	UW UNIT WEIGHT	K PERMEABILITY	W MOISTURE CONTENT	<p>CMS CALIF. MODIFIED SAMPLER<sup>1</sup></p> <p>CPT CONE PENETRATION TEST</p> <p>CS CONTINUOUS SAMPLER<sup>2</sup></p> <p>PB PITCHER BARREL</p> <p>RC ROCK CORE<sup>3</sup></p> <p>SH SHELBY TUBE<sup>4</sup></p> <p>SPT STANDARD PENETRATION TEST</p> <p>TP TEST PIT</p>
CD CONSOLIDATED DRAINED	O ORGANIC CONTENT																						
CH CHEMICAL (CORROSIVENESS)	OC CONSOLIDATION																						
CM COMPACTION	PI PLASTICITY INDEX																						
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HC HYDRO-COLLAPSE	UW UNIT WEIGHT																						
K PERMEABILITY	W MOISTURE CONTENT																						
<p>SOIL COLOR DESIGNATIONS ARE FROM THE MUNSELL SOIL/ROCK COLOR CHARTS.</p> <p>EXAMPLE: (7.5 YR 5/3) BROWN</p>																							
<p>1- I.D.= 2.421 inch</p> <p>2- I.D.=3.228 inch with tube; 3.50 inch w/o tube</p> <p>3- NXB I.D.= 1.875 inch</p> <p>4- I.D.= 2.875 inch</p>																							



START DATE 6/7/10  
 END DATE 6/7/10  
 JOB DESCRIPTION SR160 Cable Barrier Rail  
 LOCATION SR160 MP CL21.89 to NY0.97  
 BORING CBP1  
 E.A. # 73587-1  
 GROUND ELEV. 5140 +/- (ft)  
 HAMMER DROP SYSTEM Automatic

**EXPLORATION LOG**

STATION "X2" 1150+22  
 OFFSET 24' LT  
 ENGINEER Ablahani  
 EQUIPMENT Diedrich D120, Rig #1627  
 OPERATOR Ford  
 DRILLING METHOD 6" Hollow Stem Auger  
 BACKFILLED Yes DATE 6/7/2010

GROUNDWATER LEVEL		
DATE	DEPTH ft	ELEV. ft
N/A	N/A	0.0

ELEV. (ft)	DEPTH (ft)	SAMPLE		BLOW COUNT			LAB TESTS	USCS Group	MATERIAL DESCRIPTION	REMARKS
		NO.	TYPE	6 inch Increments	Last 1 foot	Percent Recov'd				
5134.6	2.50							SM	<u>Silty Sand with Gravel</u> , medium dense, dry to moist, (10 YR 6/2) pale yellowish brown.  100 psi down pressure entire depth. Easy drilling.  Approx. 5' of fill.	
	4.00	A	SPT	7	17	100	S, PI, W			
	5.00									
5129.6	6.50	B	CMS	10 9 8	17	100	S, PI, W, DS	ML	<u>Sandy Silt</u> stiff, dry to moist, (10YR 6/2) pale yellowish brown.  5'-10' Bulk 1.	
	7.50									
	9.00	C	SPT	4 7 5	12	85	S, PI, W			
5124.6	10.00							SM	F: Fractured rock at 15.4'-15.6' in sampler.	
	11.50	D	SPT	11 12 10	22	80	S, PI, W			
	13.00	E	SPT	7 7 5	12	80	S, PI, W			
	15.00									
	16.50	F	SPT	53 37 27	64	80	S, PI, W			
									<u>B.O.H.</u>  No groundwater encountered.  Backfilled with cuttings after drilling.	



START DATE 6/9/10  
 END DATE 6/9/10  
 JOB DESCRIPTION SR160 Cable Barrier Rail  
 LOCATION SR160 MP CL21.89 to NY0.97  
 BORING CBP10  
 E.A. # 73587-1  
 GROUND ELEV. 3338 +/- (ft)  
 HAMMER DROP SYSTEM Automatic

**EXPLORATION LOG**

STATION "L" 478+11  
 OFFSET 27' LT  
 ENGINEER Ablahani  
 EQUIPMENT Diedrich D120, Rig #1627  
 OPERATOR White  
 DRILLING METHOD 6" Hollow Stem Auger  
 BACKFILLED Yes DATE 6/9/2010

GROUNDWATER LEVEL		
DATE	DEPTH ft	ELEV. ft
N/A	N/A	0.0

ELEV. (ft)	DEPTH (ft)	SAMPLE		BLOW COUNT			LAB TESTS	USCS Group	MATERIAL DESCRIPTION	REMARKS
		NO.	TYPE	6 inch Increments	Last 1 foot	Percent Recov'd				
3332.9	5							GC GM	<u>Silty Clayey Gravel with Sand and Cobbles</u> very dense, dry to moist, (10 YR 5/4) moderate yellowish brown. Mostly cobbles 0-5'.	Start: 2:15pm Finish: 3:30pm Sunny, breezy and hot.  Approx. OG, no cut/fill.  100 psi down pressure entire depth.  5'-10' Bulk 1.
		A	SPT	9	71	80	S, PI, W			
	6.50			23						
				48						
3327.9	7.50							GC GM	<u>Poorly graded Gravel with Silty Clayey Sand</u> very dense, dry, yellowish brown.	C: Fractured cobble in sampler shoe and at 10.4'-10.7' in sampler.
		B	SPT	15	51	85	S, PI, W			
	9.00			25						
				26						
3322.9	10.00							GP GC	<u>B.O.H.</u> No groundwater encountered.  Backfilled with cuttings after drilling.	
		C	SPT	30	67	100	S, PI, W			
	11.50			30						
				37						
3322.9	12.50							GP GC		
		D	SPT	18	91	95	S, PI, W			
	14.00			38						
3322.9	15							GP GC		
		E	SPT	18	89	80	S, PI, W			
	16.50									



START DATE 6/10/10  
 END DATE 6/10/10  
 JOB DESCRIPTION SR160 Cable Barrier Rail  
 LOCATION SR160 MP CL21.89 to NY0.97  
 BORING CBP11  
 E.A. # 73587-1  
 GROUND ELEV. 3430 +/- (ft)  
 HAMMER DROP SYSTEM Automatic

**EXPLORATION LOG**

STATION "L" 684+13  
 OFFSET 27' LT  
 ENGINEER Ablahani  
 EQUIPMENT Diedrich D120, Rig #1627  
 OPERATOR White  
 DRILLING METHOD 6" Hollow Stem Auger  
 BACKFILLED Yes DATE 6/10/2010

GROUNDWATER LEVEL		
DATE	DEPTH ft	ELEV. ft
N/A	N/A	0.0

ELEV. (ft)	DEPTH (ft)	SAMPLE		BLOW COUNT			LAB TESTS	USCS Group	MATERIAL DESCRIPTION	REMARKS
		NO.	TYPE	6 inch Increments	Last 1 foot	Percent Recov'd				
3425.3	2.50							GW GC	<u>Well-graded Gravel with Silty Clay, Sand and Cobbles</u> very dense, dry, yellowish brown.	Start: 8:45am Finish: 10:00am Sunny and hot.  Approx. OG, <1' cut.  100 psi down pressure unless noted otherwise.
	4.00	A	SPT	15 23 36	59	80	S, PI, W			
	5.00							GP GM	<u>Poorly graded Gravel with Silt, Sand, and Cobbles</u> very dense, dry, yellowish brown.	5'-10' Bulk 1.  C: Refusal, 10 blows with no progress. 7.5'-10' hard drilling, increased down pressure to 500psi. Possible boulder?
	6.00	B	SPT	31 68/0.5'	68/0.5'	90	S, PI, W			
	7.50	C	SPT	25/0.1'	25/0.1'	0				
3420.3	10.00							GM	<u>Silty Gravel with Sand and Cobbles</u> very dense, dry, yellowish brown.	E: Refusal, 10 blows with no progress.
	11.35	D	SPT	46 55 50/0.35'	50/0.35'	90	S, W			
3415.3	15.00							GM	<u>Silty Gravel with Sand and Cobbles</u> very dense, dry, yellowish brown.	E: Refusal, 10 blows with no progress.
	16.10	E	SPT	32 22/0.1'	22/0.1'	100	S, W			
									<u>B.O.H.</u> No groundwater encountered. Backfilled with cuttings after drilling.	



START DATE 6/10/10  
 END DATE 6/10/10  
 JOB DESCRIPTION SR160 Cable Barrier Rail  
 LOCATION SR160 MP CL21.89 to NY0.97  
 BORING CBP12  
 E.A. # 73587-1  
 GROUND ELEV. 3193 +/- (ft)  
 HAMMER DROP SYSTEM Automatic

**EXPLORATION LOG**

STATION "P" 318+79  
 OFFSET 27' LT  
 ENGINEER Ablahani  
 EQUIPMENT Diedrich D120, Rig #1627  
 OPERATOR White  
 DRILLING METHOD 6" Hollow Stem Auger  
 BACKFILLED Yes DATE 6/10/2010

GROUNDWATER LEVEL		
DATE	DEPTH ft	ELEV. ft
N/A	N/A	0.0

ELEV. (ft)	DEPTH (ft)	SAMPLE		BLOW COUNT			LAB TESTS	USCS Group	MATERIAL DESCRIPTION	REMARKS
		NO.	TYPE	6 inch Increments	Last 1 foot	Percent Recov'd				
3187.8	1.00							GC	<u>Clayey Gravel with Sand and Cobbles</u> very dense, dry, yellowish brown.	Start: 10:30am Finish: 11:30am Sunny and hot.  Approx. OG, <1' fill.  100 psi down pressure entire depth.
	2.50	A	SPT	19 23 55	78	80	S, PI, W			
	4.00									
3187.8	5.00							GC GM	<u>Silty Clayey Gravel with Sand and Cobbles</u> very dense, dry, yellowish brown.	5'-10' Bulk 1.
	6.50	B	SPT	24 35 46	81	80	S, PI, W			
	8.00									
3182.8	10.00							SC SM	<u>Silty Clayey Sand with Gravel and Cobbles</u> very dense, dry, yellowish brown.	
	10.50	C	SPT	75/0.5'	75/0.5'	100	PI, W			
	12.00									
3177.8	15.00							GC GM	<u>Silty Clayey Gravel with Sand and Cobbles</u> very dense, dry, yellowish brown.	
	15.80	D	SPT	28 50/0.3'	50/0.3'	90	S, PI, W			
	15.80									
									<u>B.O.H.</u> No groundwater encountered. Backfilled with cuttings after drilling.	



START DATE 6/7/10  
 END DATE 6/7/10  
 JOB DESCRIPTION SR160 Cable Barrier Rail  
 LOCATION SR160 MP CL21.89 to NY0.97  
 BORING CBP2  
 E.A. # 73587-1  
 GROUND ELEV. 5022 +/- (ft)  
 HAMMER DROP SYSTEM Automatic

**EXPLORATION LOG**

STATION "X2" 1174+92  
 OFFSET 24' LT  
 ENGINEER Ablahani  
 EQUIPMENT Diedrich D120, Rig #1627  
 OPERATOR White  
 DRILLING METHOD 6" Hollow Stem Auger  
 BACKFILLED Yes DATE 6/7/2010

GROUNDWATER LEVEL		
DATE	DEPTH ft	ELEV. ft
N/A	N/A	0.0

ELEV. (ft)	DEPTH (ft)	SAMPLE		BLOW COUNT		Percent Recov'd	LAB TESTS	USCS Group	MATERIAL DESCRIPTION	REMARKS
		NO.	TYPE	6 inch Increments	Last 1 foot					
5017.3	0.50							GP GM	<u>Poorly graded Gravel with Silt, Sand, and Cobbles</u> very dense, dry, (10 YR 6/2) pale yellowish brown, with cobbles.	Start: 12:30pm Finish: 3:00pm Sunny and hot. Approx. OG, no cut/fill. 100 psi down pressure unless noted otherwise. 0-4' grinding/slow drilling.
		A	SPT	13						
				19	50/0.35'	75	S, PI, W			
	1.85				50/0.35'					
	2.50									
	2.85	B	SPT	50/0.35'	50/0.35'	170	S, W			
5012.3	5.00							SM	<u>Silty Sand with Gravel and Cobbles/Boulders</u> very dense, dry, (10 YR 6/2) pale yellowish brown.	A: Fractured rock in sampler shoe. Refusal possibly due to cobble. B: 0.3' Slough in sample. 4'-5.5', 400 psi down pressure. Hard drilling, possibly boulder. C: Refusal, 10 blows with no progress. Fractured rock in shoe. 5'-10' Bulk 1. 5.5'-13', 100 psi down pressure. Hard drilling, possibly cobbles/boulders.
	5.80	C	SPT	35/0.3'	35/0.3'	100	PI, W			
	10.00									
	10.30	D	SPT	13/0.3'	13/0.3'	100	S, W			
5007.3	15								<u>B.O.H.</u> No groundwater encountered. Backfilled with cuttings after drilling.	D: Refusal, 10 blows with no progress. Rod not plumb, out about 10 degrees. At 13' increased down pressure to 400 psi, could not penetrate, possibly caliche.



START DATE 6/8/10  
 END DATE 6/8/10  
 JOB DESCRIPTION SR160 Cable Barrier Rail  
 LOCATION SR160 MP CL21.89 to NY0.97  
 BORING CBP3  
 E.A. # 73587-1  
 GROUND ELEV. 4946 +/- (ft)  
 HAMMER DROP SYSTEM Automatic

**EXPLORATION LOG**

STATION "X2" 1192+96  
 OFFSET 25' LT  
 ENGINEER Ablahani  
 EQUIPMENT Diedrich D120, Rig #1627  
 OPERATOR Ford  
 DRILLING METHOD 6" Hollow Stem Auger  
 BACKFILLED Yes DATE 6/8/2010

GROUNDWATER LEVEL		
DATE	DEPTH ft	ELEV. ft
N/A	N/A	0.0

ELEV. (ft)	DEPTH (ft)	SAMPLE		BLOW COUNT			LAB TESTS	USCS Group	MATERIAL DESCRIPTION	REMARKS
		NO.	TYPE	6 inch Increments	Last 1 foot	Percent Recov'd				
4941.1	2.50							SM	<p><b>Silty Sand with Gravel</b>, dense, dry to moist, (10 YR 5/4) moderate yellowish brown.</p> <p>Start: 8:30am Finish: 10:20am Partly cloudy and hot.</p> <p>100 psi down pressure unless noted otherwise.</p> <p>Approx. 4.5' fill.</p> <p>----- 4.50</p> <p><b>Poorly graded Gravel with Silt and Sand</b>, very dense, dry to moist, (10 YR 5/4) moderate yellowish brown.</p> <p>B: Refusal, 10 blows with no progress.</p> <p>5'-10' Bulk 1.</p> <p>----- 7.00</p> <p><b>Silty Clayey Sand with Gravel</b>, very dense, moist, (10 YR 8/2) very pale orange.</p> <p>7'-8.5' 200 psi down pressure.</p> <p>----- 14.00</p> <p><b>Poorly graded Sand with Silty Clay and Gravel</b>, very dense, moist, (10 YR 5/4) moderate yellowish brown.</p> <p>----- 16.50</p> <p><b>Silty Clayey Sand with Gravel</b>, very dense, moist, (10 YR 5/4) moderate yellowish brown.</p> <p>G: Refusal on cobble. Fractured rock in sampler shoe.</p> <p>----- 17.50</p> <p><b>B.O.H.</b></p> <p>No groundwater encountered.</p> <p>Backfilled with cuttings after drilling.</p>	
	4.00	A	SPT	10 9 14	23	85	S, PI, W			
	5.00									
	5.60	B	CMS	29	17/0.1'	115	S, PI, W	GP GM		
				17/0.1'						
	7.50							SC SM		
	9.00	C	SPT	11 14 36	50	100	S, PI, W			
	10.00									
	10.40	D	CMS	50/0.4'	50/0.4'	100	S, PI, W			
	12.50							SP SC		
12.90	E	SPT	50/0.4'	50/0.4'	100	S, W				
15.00							SC SM			
15.50	F	CMS	24 32 48	80	100	S, PI, W				
16.50										
17.50	G	SPT	21 50/0.5'	50/0.5'	100	S, PI, W				





START DATE 6/8/10  
 END DATE 6/8/10  
 JOB DESCRIPTION SR160 Cable Barrier Rail  
 LOCATION SR160 MP CL21.89 to NY0.97  
 BORING CBP4  
 E.A. # 73587-1  
 GROUND ELEV. 4669 +/- (ft)  
 HAMMER DROP SYSTEM Automatic

**EXPLORATION LOG**

STATION "X2" 1262+00  
 OFFSET 25' LT  
 ENGINEER Ablahani  
 EQUIPMENT Diedrich D120, Rig #1627  
 OPERATOR White  
 DRILLING METHOD 6" Hollow Stem Auger  
 BACKFILLED Yes DATE 6/8/2010

GROUNDWATER LEVEL		
DATE	DEPTH ft	ELEV. ft
N/A	N/A	0.0

ELEV. (ft)	DEPTH (ft)	SAMPLE		BLOW COUNT			LAB TESTS	USCS Group	MATERIAL DESCRIPTION	REMARKS
		NO.	TYPE	6 inch Increments	Last 1 foot	Percent Recov'd				
4663.5	2.00							SC SM	<p><b>Silty Clayey Sand with Gravel and Cobbles</b> very dense, dry to moist, (10 YR 5/4) moderate yellowish brown.</p> <p>Start: 11:00am Finish: 12:15pm Mostly sunny and hot. Approx. &lt;1' cut. 100 psi down pressure entire depth.</p> <p>0-4' mostly cobbles. A: Fractured rock in sampler shoe. Refusal possibly due to cobble.</p> <p>5'-10' Bulk 1.</p>	
	2.90	A	SPT	14	50/0.4'	110	S, PI, W			
	4.00							SW SM		
	5.00			24						
	6.50	B	SPT	45	88	100	S, PI, W			
4658.5	10.00							SM		
	10.50	C	SPT	54/0.5'	54/0.5'	100	S, W			
	12.00								SP SM	
4653.5	15.00									
	16.50	D	SPT	33	99	100	S, PI, W			
					59					
								<p><b>B.O.H.</b></p> <p>No groundwater encountered.</p> <p>Backfilled with cuttings after drilling.</p>		



GEOTECHNICAL ENGINEERING

START DATE 6/8/10  
 END DATE 6/8/10  
 JOB DESCRIPTION SR160 Cable Barrier Rail  
 LOCATION SR160 MP CL21.89 to NY0.97  
 BORING CBP5  
 E.A. # 73587-1  
 GROUND ELEV. 4558 +/- (ft)  
 HAMMER DROP SYSTEM Automatic

**EXPLORATION LOG**

STATION "X2" 1301+19  
 OFFSET 25' LT  
 ENGINEER Ablahani  
 EQUIPMENT Diedrich D120, Rig #1627  
 OPERATOR Ford  
 DRILLING METHOD 6" Hollow Stem Auger  
 BACKFILLED Yes DATE 6/8/2010

GROUNDWATER LEVEL		
DATE	DEPTH ft	ELEV. ft
N/A	N/A	0.0

ELEV. (ft)	DEPTH (ft)	SAMPLE		BLOW COUNT			LAB TESTS	USCS Group	MATERIAL DESCRIPTION	REMARKS
		NO.	TYPE	6 inch Increments	Last 1 foot	Percent Recov'd				
4553.2	1.00							SC SM	<u>Silty Clayey Sand with Gravel</u> dense to very dense, dry to moist, (5 YR 4/4) moderate brown.	Start: 12:40pm Finish: 2:00pm Mostly sunny and hot.  Approx. OG, <1' fill.  100 psi down pressure entire depth.  B: Refusal, possibly due to cobble.
	2.50	A	SPT	12 14 11	25	85	S, PI, W			
	5.00									
	6.40	B	SPT	9 11 50/.4	50/.4	100	S, PI, W			
	7.50									
4548.2	9.00	C	SPT	33 43 53	96	105	S, PI, W	GC GM	<u>Silty Clayey Gravel with Sand</u> very dense, dry to moist, reddish brown.	5'-10' Bulk 1.
	10.00									
	11.50	D	SPT	26 30 49	79	105	S, PI, W	SC SM		
4543.2	15.00							GC GM	<u>Silty Clayey Gravel with Sand</u> very dense, dry to moist, reddish brown.	
	15.40	E	SPT	50/0.4'	50/0.4'	100	S, W			
									<u>B.O.H.</u> No groundwater encountered. Backfilled with cuttings after drilling.	E: Refusal possibly due to cobble/boulder.



GEOTECHNICAL ENGINEERING

START DATE 6/8/10  
 END DATE 6/8/10  
 JOB DESCRIPTION SR160 Cable Barrier Rail  
 LOCATION SR160 MP CL21.89 to NY0.97  
 BORING CBP6  
 E.A. # 73587-1  
 GROUND ELEV. 4287 +/- (ft)  
 HAMMER DROP SYSTEM Automatic

**EXPLORATION LOG**

STATION "X2" 1379+52  
 OFFSET 37' LT  
 ENGINEER Ablahani  
 EQUIPMENT Diedrich D120, Rig #1627  
 OPERATOR Ford  
 DRILLING METHOD 6" Hollow Stem Auger  
 BACKFILLED Yes DATE 6/8/2010

GROUNDWATER LEVEL		
DATE	DEPTH ft	ELEV. ft
N/A	N/A	0.0

ELEV. (ft)	DEPTH (ft)	SAMPLE		BLOW COUNT			LAB TESTS	USCS Group	MATERIAL DESCRIPTION	REMARKS
		NO.	TYPE	6 inch Increments	Last 1 foot	Percent Recov'd				
4281.8	5.00							SP SM	<p><u>Poorly graded Sand with Silt, Gravel, and Cobbles</u> very dense, dry, yellowish brown.</p> <p>Start: 2:30pm Finish: 3:30pm Sunny, breezy and hot.</p> <p>Approx. OG, &lt;1' cut.</p> <p>100 psi down pressure entire depth.</p>	5'-10' Bulk 1.
		A	SPT	23	89	95	S, PI, W			
	6.50			36						
4276.8	10.00							SP SM	<p><u>Poorly graded Sand with Silt, Gravel, and Cobbles</u> very dense, dry, yellowish brown.</p> <p>Start: 2:30pm Finish: 3:30pm Sunny, breezy and hot.</p> <p>Approx. OG, &lt;1' cut.</p> <p>100 psi down pressure entire depth.</p>	5'-10' Bulk 1.
		B	SPT	16	93	85	S, PI, W			
	11.30			37						
4271.8	15.00							SP SM	<p><u>Poorly graded Sand with Silt, Gravel, and Cobbles</u> very dense, dry, yellowish brown.</p> <p>Start: 2:30pm Finish: 3:30pm Sunny, breezy and hot.</p> <p>Approx. OG, &lt;1' cut.</p> <p>100 psi down pressure entire depth.</p>	5'-10' Bulk 1.
	15.40	C	SPT	50/0.4'	50/0.4'	100	S, W			
									<p><u>B.O.H.</u></p> <p>No groundwater encountered.</p> <p>Backfilled with cuttings after drilling.</p>	C: Refusal possibly due to cobble.



START DATE 6/9/10  
 END DATE 6/9/10  
 JOB DESCRIPTION SR160 Cable Barrier Rail  
 LOCATION SR160 MP CL21.89 to NY0.97  
 BORING CBP7  
 E.A. # 73587-1  
 GROUND ELEV. 3978 +/- (ft)  
 HAMMER DROP SYSTEM Automatic

**EXPLORATION LOG**

STATION "L" 98+02  
 OFFSET 58' LT  
 ENGINEER Ablahani  
 EQUIPMENT Diedrich D120, Rig #1627  
 OPERATOR White  
 DRILLING METHOD 6" Hollow Stem Auger  
 BACKFILLED Yes DATE 6/9/2010

GROUNDWATER LEVEL		
DATE	DEPTH ft	ELEV. ft
N/A	N/A	0.0

ELEV. (ft)	DEPTH (ft)	SAMPLE		BLOW COUNT			LAB TESTS	USCS Group	MATERIAL DESCRIPTION	REMARKS
		NO.	TYPE	6 inch Increments	Last 1 foot	Percent Recov'd				
3972.5	2.50							SP SC	<u>Poorly graded Sand with Silty Clay, Gravel and Cobbles</u> very dense, dry to moist, (10YR 5/4) moderate yellowish brown.	Start: 8:30am Finish: 9:40am Sunny, breezy and hot.  100 psi down pressure entire depth. Easy drilling.  Approx. 4' fill.
	2.95	A	SPT	25/0.45'	25/0.45'	110	PI, W			
	5.00									
3972.5	6.50							GP GM	<u>Poorly graded Gravel with Silt, Sand and Cobbles</u> very dense, dry to moist, (10YR 5/4) moderate yellowish brown.	5'-10' Bulk 1.
	7.50	B	SPT	20	81	100	S, PI, W			
	7.70	C	SPT	20/0.2'	20/0.2'	50				
3967.5	10.00							SP SM	<u>Poorly graded Sand with Silt, Gravel and Cobbles</u> very dense, dry to moist, (10YR 5/4) moderate yellowish brown.	C: Refusal, 10 blows with no progress.
	10.35	D	SPT	50/0.35'	50/0.35'	115	S, W			
3962.5	15.00								<u>B.O.H.</u>  No groundwater encountered.  Backfilled with cuttings after drilling.	E: Refusal, fractured rock chunks in sample.
	15.30	E	SPT	31/0.3'	31/0.3'	35				



START DATE 6/9/10  
 END DATE 6/9/10  
 JOB DESCRIPTION SR160 Cable Barrier Rail  
 LOCATION SR160 MP CL21.89 to NY0.97  
 BORING CBP8  
 E.A. # 73587-1  
 GROUND ELEV. 3780 +/- (ft)  
 HAMMER DROP SYSTEM Automatic

**EXPLORATION LOG**

STATION "L" 198+03  
 OFFSET 25' LT  
 ENGINEER Ablahani  
 EQUIPMENT Diedrich D120, Rig #1627  
 OPERATOR White  
 DRILLING METHOD 6" Hollow Stem Auger  
 BACKFILLED Yes DATE 6/9/2010

GROUNDWATER LEVEL		
DATE	DEPTH ft	ELEV. ft
N/A	N/A	0.0

ELEV. (ft)	DEPTH (ft)	SAMPLE		BLOW COUNT		Percent Recov'd	LAB TESTS	USCS Group	MATERIAL DESCRIPTION	REMARKS
		NO.	TYPE	6 inch Increments	Last 1 foot					
3775.4	2.50							GC GM	<u>Silty Clayey Gravel with Sand and Cobbles</u> very dense, dry, (10 YR 5/4) moderate yellowish brown.	Start: 10:30am Finish: 11:30am Sunny, breezy and hot. 100 psi down pressure entire depth. Approx. 3' fill.
	3.00	A	SPT	62/0.5'	62/0.5'	120	S, PI, W			
3775.4	5.00							GM	<u>Silty Gravel with Sand and Caliche</u> very dense, dry, (10 YR 5/4) moderate yellowish brown.	3'-7' chunks of moderately to strongly cemented caliche in cuttings.  B: 1" thick strongly cemented caliche in sampler shoe.
	6.50	B	SPT	15 20 22	42	95	S, PI, W			
	7.00									
3770.4	10.00							SW SM	<u>Well-graded Sand with Silt, Gravel and Cobbles</u> very dense, dry, (10 YR 5/4) moderate yellowish brown.	5'-10' Bulk 1.
	11.50	C	SPT	45 54 37	91	80	S, PI, W			
3765.4	12.00							GM	<u>Silty Gravel with Sand and Cobbles</u> very dense, dry, (10 YR 5/4) moderate yellowish brown.	C:10.8'-11.1' fractured cobble in sampler.
	15.00 15.40	D	SPT	50/0.4'	50/0.4'	100	S, W			
									<u>B.O.H.</u> No groundwater encountered. Backfilled with cuttings after drilling.	



GEOTECHNICAL  
ENGINEERING

START DATE 6/9/10  
 END DATE 6/9/10  
 JOB DESCRIPTION SR160 Cable Barrier Rail  
 LOCATION SR160 MP CL21.89 to NY0.97  
 BORING CBP9  
 E.A. # 73587-1  
 GROUND ELEV. 3586 +/- (ft)  
 HAMMER DROP SYSTEM Automatic

EXPLORATION LOG

STATION "L" 330+49  
 OFFSET 56' LT  
 ENGINEER Ablahani  
 EQUIPMENT Diedrich D120, Rig #1627  
 OPERATOR White  
 DRILLING METHOD 6" Hollow Stem Auger  
 BACKFILLED Yes DATE 6/9/2010

GROUNDWATER LEVEL		
DATE	DEPTH ft	ELEV. ft
N/A	N/A	0.0

ELEV. (ft)	DEPTH (ft)	SAMPLE		BLOW COUNT			LAB TESTS	USCS Group	MATERIAL DESCRIPTION	REMARKS
		NO.	TYPE	6 inch Increments	Last 1 foot	Percent Recov'd				
3581.1	5.00	A	SPT	50/0.4'	50/0.4'	100	S, PI, W	GC	<u>Clayey Gravel with Sand and Cobbles</u> very dense, dry, yellowish brown.	Start: 12:15pm Finish: 1:15pm Sunny, breezy and hot.  Approx. OG, <1' fill.  100 psi down pressure entire depth. Easy drilling.
	5.40									
3576.1	10.00	B	SPT	19/0.1'	19/0.1'	100	PI, W	SM	<u>Silty Sand</u> very dense, dry, pale yellowish brown.	B: Refusal, 10 blows with no progress. Possibly caliche?
3571.1	15.00	C	SPT	33/0.15'	33/0.15'	100	S, W		<u>B.O.H.</u> No groundwater encountered.  Backfilled with cuttings after drilling.	

# **APPENDIX C: LABORATORY TEST RESULTS**

Summary of Results

Particle Size Distribution Reports

Chemical Analysis Table

Direct Shear Test Report

## SUMMARY OF RESULTS N.D.O.T. GEOTECHNICAL SECTION

EA/Cont # 73587

Job Description SR 160 Cable Barrier Rail

Boring No. CBP 1

Elevation (ft) 5140 +/-

Station "X" 1150 + 22

Date 06/07/2010

SAMPLE NO.	SAMPLE DEPTH (ft)	SAMPLER TYPE	N BLOWS per ft.	SOIL GROUP	W%	DRY UW pcf	% PASS #200	LL %	PL %	PI %	STRENGTH TEST				COMMENTS	
											TEST TYPE	Φ deg.	C psi	Φ deg.		C psi
												Peak		Residual		
A	2.5 - 4.0	SPT	17	SM	9.3		25.5	23	NP	NP						
B1	5.3 - 5.8	CMS	17	ML	21.0	79.9	81.1	26	24	2	DS	33	1.1	30	1.2	
B2	5.8 - 6.5	CMS		ML	18.8		70.1	24	22	2						
C	7.5 - 9.0	SPT	12	SM	8.7		23.9	21	NP	NP						
D	10.0 - 11.5	SPT	22	SM	4.4		13.1	18	NP	NP						
E	12.5 - 14.0	SPT	12	SM	10.1		27.0	18	NP	NP						
F	15.0 - 16.5	SPT	64	SM	2.9		14.8	16	NP	NP						
BULK 1	5.0 - 10.0			SM			34.8	21	NP	NP						Ch, RV = 80

CMS = California Modified Sampler 2.42" ID  
 SPT = Standard Penetration 1.38" ID  
 CS = Continuous Sample 3.23" ID  
 RC = Rock Core  
 PB = Pitcher Barrel  
 CSS = Calif. Split Spoon 2.42" ID  
 CPT = Cone Penetration Test  
 TP = Test Pit  
 P = Pushed, not driven  
 R = Refusal  
 Sh = Shelby Tube 2.87" ID

U = Unconfined Compressive  
 UU = Unconsolidated Undrained  
 CD = Consolidated Drained  
 CU = Consolidated Undrained  
 DS = Direct Shear  
 Φ = Friction  
 C = Cohesion  
 N = No. of blows per ft., sampler  
 N = Field SPT                      N = (N<sub>css</sub>)(0.62)

H = Hydrometer  
 S = Sieve  
 G = Specific Gravity  
 PI = Plasticity Index  
 LL = Liquid Limit  
 PL = Plastic Limit  
 NP = Non-Plastic  
 OC = Consolidation  
 Ch = Chemical  
 RV = R - Value  
 MD = Moisture Density

CM = Compaction  
 E = Swell/Pressure on Expansive Soils  
 SL = Shrinkage Limit  
 UW = Unit Weight  
 W = Moisture Content  
 K = Permeability  
 O = Organic Content  
 D = Dispersive  
 RQD = Rock Quality Designation  
 X = X-Ray Defraction  
 HCpot = Hydro-Collapse Potential

\* = Average of subsamples



## SUMMARY OF RESULTS N.D.O.T. GEOTECHNICAL SECTION

EA/Cont # 73587

Job Description SR 160 Cable Barrier Rail

Boring No. CBP 2

Elevation (ft) 5022 +/-

Station "X2" 1174 + 92

Date 06/07/2010

SAMPLE NO.	SAMPLE DEPTH (ft)	SAMP- LER TYPE	N BLOWS per ft.	SOIL GROUP	W%	DRY UW pcf	% PASS #200	LL %	PL %	PI %	STRENGTH TEST				COMMENTS	
											TEST TYPE	Φ deg.	C psi	Φ deg.		C psi
												Peak		Residual		
A	0.5 - 1.9	SPT	R	GP-GM	1.8		10.7	19	18	1						
B	2.5 - 2.9	SPT	R		2.7		19.0									
C	5.5 - 5.8	SPT	R		3.1			18	15	3						
D	10.0 - 10.3	SPT	R		2.1		29.3									
BULK 1	5.0 - 10.0			SM			21.0	18	15	3					RV = 75	

CMS = California Modified Sampler 2.42" ID  
 SPT = Standard Penetration 1.38" ID  
 CS = Continuous Sample 3.23" ID  
 RC = Rock Core  
 PB = Pitcher Barrel  
 CSS = Calif. Split Spoon 2.42" ID  
 CPT = Cone Penetration Test  
 TP = Test Pit  
 P = Pushed, not driven  
 R = Refusal  
 Sh = Shelby Tube 2.87" ID

U = Unconfined Compressive  
 UU = Unconsolidated Undrained  
 CD = Consolidated Drained  
 CU = Consolidated Undrained  
 DS = Direct Shear  
 Φ = Friction  
 C = Cohesion  
 N = No. of blows per ft., sampler  
  
 N = Field SPT                      N = (N<sub>css</sub>)(0.62)

H = Hydrometer  
 S = Sieve  
 G = Specific Gravity  
 PI = Plasticity Index  
 LL = Liquid Limit  
 PL = Plastic Limit  
 NP = Non-Plastic  
 OC = Consolidation  
 Ch = Chemical  
 RV = R - Value  
 MD = Moisture Density

CM = Compaction  
 E = Swell/Pressure on Expansive Soils  
 SL = Shrinkage Limit  
 UW = Unit Weight  
 W = Moisture Content  
 K = Permeability  
 O = Organic Content  
 D = Dispersive  
 RQD = Rock Quality Designation  
 X = X-Ray Defraction  
 HCpot = Hydro-Collapse Potential

\* = Average of subsamples

## SUMMARY OF RESULTS N.D.O.T. GEOTECHNICAL SECTION

EA/Cont # 73587

Job Description SR 160 Cable Barrier Rail

Boring No. CBP 3

Elevation (ft) 4946 +/-

Station "X2" 1192 + 96

Date 06/08/2010

SAMPLE NO.	SAMPLE DEPTH (ft)	SAMP- LER TYPE	N BLOWS per ft.	SOIL GROUP	W%	DRY UW pcf	% PASS #200	LL %	PL %	PI %	STRENGTH TEST				COMMENTS	
											TEST TYPE	Φ deg.	C psi	Φ deg.		C psi
												Peak		Residual		
A	2.5 - 4.0	SPT	23	SM	5.1		17.2	19	18	1						
B	5.0 - 5.6	CMS	R	GP-GM	3.5		10.3	22	19	3						
C	7.5 - 9.0	SPT	50	SC-SM	11.2		37.0	29	22	7						
D	10.0 - 10.4	CMS	R	SC-SM	5.9		25.7	22	17	5						
E	12.5 - 12.9	SPT	R		3.7		20.8									
F	15.0 - 16.5	CMS	80	SP-SC	3.8		11.5	21	16	5						
G	16.5 - 17.5	SPT	R	SC-SM	4.7		14.2	24	17	7						
BULK 1	5.0 - 10.0			GC-GM			15.4	20	15	5					RV = 65	

CMS = California Modified Sampler 2.42" ID  
 SPT = Standard Penetration 1.38" ID  
 CS = Continuous Sample 3.23" ID  
 RC = Rock Core  
 PB = Pitcher Barrel  
 CSS = Calif. Split Spoon 2.42" ID  
 CPT = Cone Penetration Test  
 TP = Test Pit  
 P = Pushed, not driven  
 R = Refusal  
 Sh = Shelby Tube 2.87" ID

U = Unconfined Compressive  
 UU = Unconsolidated Undrained  
 CD = Consolidated Drained  
 CU = Consolidated Undrained  
 DS = Direct Shear  
 Φ = Friction  
 C = Cohesion  
 N = No. of blows per ft., sampler  
 N = Field SPT      N = (N<sub>css</sub>)(0.62)

H = Hydrometer  
 S = Sieve  
 G = Specific Gravity  
 PI = Plasticity Index  
 LL = Liquid Limit  
 PL = Plastic Limit  
 NP = Non-Plastic  
 OC = Consolidation  
 Ch = Chemical  
 RV = R - Value  
 MD = Moisture Density

CM = Compaction  
 E = Swell/Pressure on Expansive Soils  
 SL = Shrinkage Limit  
 UW = Unit Weight  
 W = Moisture Content  
 K = Permeability  
 O = Organic Content  
 D = Dispersive  
 RQD = Rock Quality Designation  
 X = X-Ray Defraction  
 HCpot = Hydro-Collapse Potential

\* = Average of subsamples

## SUMMARY OF RESULTS N.D.O.T. GEOTECHNICAL SECTION

EA/Cont # 73587

Job Description SR 160 Cable Barrier Rail

Boring No. CBP 4

Elevation (ft) 4669 +/-

Station "X2" 1262 + 00

Date 06/08/2010

SAMPLE NO.	SAMPLE DEPTH (ft)	SAMP- LER TYPE	N BLOWS per ft.	SOIL GROUP	W%	DRY UW pcf	% PASS #200	LL %	PL %	PI %	STRENGTH TEST				COMMENTS	
											TEST TYPE	Φ deg.	C psi	Φ deg.		C psi
												Peak		Residual		
A	2.0 - 2.9	SPT	R	SC-SM	6.8		18.2	25	20	5						
B	5.0 - 6.5	SPT	88	SW-SM	2.7		8.9	15	NP	NP						
C	10.0 - 10.5	SPT	R		2.7		14.0									
D	15.0 - 16.5	SPT	99	SP-SM	3.2		11.5	16	14	2						
BULK 1	5.0 - 10.0			GP-GM			9.4	19	17	2					RV = 77	

CMS = California Modified Sampler 2.42" ID  
 SPT = Standard Penetration 1.38" ID  
 CS = Continuous Sample 3.23" ID  
 RC = Rock Core  
 PB = Pitcher Barrel  
 CSS = Calif. Split Spoon 2.42" ID  
 CPT = Cone Penetration Test  
 TP = Test Pit  
 P = Pushed, not driven  
 R = Refusal  
 Sh = Shelby Tube 2.87" ID

U = Unconfined Compressive  
 UU = Unconsolidated Undrained  
 CD = Consolidated Drained  
 CU = Consolidated Undrained  
 DS = Direct Shear  
 Φ = Friction  
 C = Cohesion  
 N = No. of blows per ft., sampler  
 N = Field SPT                      N = (N<sub>css</sub>)(0.62)

H = Hydrometer  
 S = Sieve  
 G = Specific Gravity  
 PI = Plasticity Index  
 LL = Liquid Limit  
 PL = Plastic Limit  
 NP = Non-Plastic  
 OC = Consolidation  
 Ch = Chemical  
 RV = R - Value  
 MD = Moisture Density

CM = Compaction  
 E = Swell/Pressure on Expansive Soils  
 SL = Shrinkage Limit  
 UW = Unit Weight  
 W = Moisture Content  
 K = Permeability  
 O = Organic Content  
 D = Dispersive  
 RQD = Rock Quality Designation  
 X = X-Ray Defraction  
 HCpot = Hydro-Collapse Potential

\* = Average of subsamples

## SUMMARY OF RESULTS N.D.O.T. GEOTECHNICAL SECTION

EA/Cont # 73587

Job Description SR 160 Cable Barrier Rail

Boring No. CBP 5

Elevation (ft) 4558 +/-

Station "X2" 1301 + 19

Date 06/08/2010

SAMPLE NO.	SAMPLE DEPTH (ft)	SAMP- LER TYPE	N BLOWS per ft.	SOIL GROUP	W%	DRY UW pcf	% PASS #200	LL %	PL %	PI %	STRENGTH TEST				COMMENTS	
											TEST TYPE	Φ deg.	C psi	Φ deg.		C psi
												Peak		Residual		
A	1.0 - 2.5	SPT	25	SC-SM	4.9		27.8	24	18	6						
B	5.0 - 6.5	SPT	R	SC-SM	13.0		36.6	28	22	6						
C	7.5 - 9.0	SPT	96	GC-GM	5.6		26.3	21	16	5						
D	10.0 - 11.5	SPT	79	SC-SM	7.1		30.8	26	21	5						
E	15.0 - 15.4	SPT	R		3.1		14.6									
BULK 1	5.0 - 10.0			GC			28.8	25	17	8					Ch, RV = 53	

CMS = California Modified Sampler 2.42" ID  
 SPT = Standard Penetration 1.38" ID  
 CS = Continuous Sample 3.23" ID  
 RC = Rock Core  
 PB = Pitcher Barrel  
 CSS = Calif. Split Spoon 2.42" ID  
 CPT = Cone Penetration Test  
 TP = Test Pit  
 P = Pushed, not driven  
 R = Refusal  
 Sh = Shelby Tube 2.87" ID

U = Unconfined Compressive  
 UU = Unconsolidated Undrained  
 CD = Consolidated Drained  
 CU = Consolidated Undrained  
 DS = Direct Shear  
 Φ = Friction  
 C = Cohesion  
 N = No. of blows per ft., sampler  
  
 N = Field SPT                      N = (N<sub>css</sub>)(0.62)

H = Hydrometer  
 S = Sieve  
 G = Specific Gravity  
 PI = Plasticity Index  
 LL = Liquid Limit  
 PL = Plastic Limit  
 NP = Non-Plastic  
 OC = Consolidation  
 Ch = Chemical  
 RV = R - Value  
 MD = Moisture Density

CM = Compaction  
 E = Swell/Pressure on Expansive Soils  
 SL = Shrinkage Limit  
 UW = Unit Weight  
 W = Moisture Content  
 K = Permeability  
 O = Organic Content  
 D = Dispersive  
 RQD = Rock Quality Designation  
 X = X-Ray Defraction  
 HCpot = Hydro-Collapse Potential

\* = Average of subsamples

## SUMMARY OF RESULTS N.D.O.T. GEOTECHNICAL SECTION

EA/Cont # 73587

Job Description SR 160 Cable Barrier Rail

Boring No. CBP 6

Elevation (ft) 4287 +/-

Station "X2" 1379 + 52

Date 06/08/2010

SAMPLE NO.	SAMPLE DEPTH (ft)	SAMP-LER TYPE	N BLOWS per ft.	SOIL GROUP	W%	DRY UW pcf	% PASS #200	LL %	PL %	PI %	STRENGTH TEST				COMMENTS	
											TEST TYPE	Φ deg.	C psi	Φ deg.		C psi
												Peak		Residual		
A	5.0 - 6.5	SPT	89	SP-SM	3.6		9.7	19	16	3						
B	10.0 - 11.5	SPT	93	SP-SM	3.2		8.0	15	NP	NP						
C	15.0 - 15.4	SPT	R		2.2		12.7									
BULK 1	5.0 - 10.0			GP-GM			8.7	19	16	3					RV = 81	

CMS = California Modified Sampler 2.42" ID  
 SPT = Standard Penetration 1.38" ID  
 CS = Continuous Sample 3.23" ID  
 RC = Rock Core  
 PB = Pitcher Barrel  
 CSS = Calif. Split Spoon 2.42" ID  
 CPT = Cone Penetration Test  
 TP = Test Pit  
 P = Pushed, not driven  
 R = Refusal  
 Sh = Shelby Tube 2.87" ID

U = Unconfined Compressive  
 UU = Unconsolidated Undrained  
 CD = Consolidated Drained  
 CU = Consolidated Undrained  
 DS = Direct Shear  
 Φ = Friction  
 C = Cohesion  
 N = No. of blows per ft., sampler  
 N = Field SPT                      N = (N<sub>css</sub>)(0.62)

H = Hydrometer  
 S = Sieve  
 G = Specific Gravity  
 PI = Plasticity Index  
 LL = Liquid Limit  
 PL = Plastic Limit  
 NP = Non-Plastic  
 OC = Consolidation  
 Ch = Chemical  
 RV = R - Value  
 MD = Moisture Density

CM = Compaction  
 E = Swell/Pressure on Expansive Soils  
 SL = Shrinkage Limit  
 UW = Unit Weight  
 W = Moisture Content  
 K = Permeability  
 O = Organic Content  
 D = Dispersive  
 RQD = Rock Quality Designation  
 X = X-Ray Defraction  
 HCpot = Hydro-Collapse Potential

\* = Average of subsamples

## SUMMARY OF RESULTS N.D.O.T. GEOTECHNICAL SECTION

EA/Cont # 73587

Job Description SR 160 Cable Barrier Rail

Boring No. CBP 7

Elevation (ft) 3978 +/-

Station "L" 98 + 02

Date 06/09/2010

SAMPLE NO.	SAMPLE DEPTH (ft)	SAMP-LER TYPE	N BLOWS per ft.	SOIL GROUP	W%	DRY UW pcf	% PASS #200	LL %	PL %	PI %	STRENGTH TEST				COMMENTS	
											TEST TYPE	Φ deg.	C psi	Φ deg.		C psi
												Peak		Residual		
A	2.0 - 3.0	SPT	R		7.3			25	19	6						
B	5.0 - 6.5	SPT	81	GP-GM	3.0		10.9	23	21	2						
C	7.5 - 7.7	SPT	R												0.1' recovered, no tests	
D	10.0 - 10.4	SPT	R		1.9		11.0									
E	15.0 - 15.3	SPT	R												0.1' recovered, no tests	
BULK 1	5.0 - 10.0			GP-GC			10.6	22	16	6					RV = 75	

CMS = California Modified Sampler 2.42" ID  
 SPT = Standard Penetration 1.38" ID  
 CS = Continuous Sample 3.23" ID  
 RC = Rock Core  
 PB = Pitcher Barrel  
 CSS = Calif. Split Spoon 2.42" ID  
 CPT = Cone Penetration Test  
 TP = Test Pit  
 P = Pushed, not driven  
 R = Refusal  
 Sh = Shelby Tube 2.87" ID

U = Unconfined Compressive  
 UU = Unconsolidated Undrained  
 CD = Consolidated Drained  
 CU = Consolidated Undrained  
 DS = Direct Shear  
 Φ = Friction  
 C = Cohesion  
 N = No. of blows per ft., sampler  
  
 N = Field SPT                      N = (N<sub>css</sub>)(0.62)

H = Hydrometer  
 S = Sieve  
 G = Specific Gravity  
 PI = Plasticity Index  
 LL = Liquid Limit  
 PL = Plastic Limit  
 NP = Non-Plastic  
 OC = Consolidation  
 Ch = Chemical  
 RV = R - Value  
 MD = Moisture Density

CM = Compaction  
 E = Swell/Pressure on Expansive Soils  
 SL = Shrinkage Limit  
 UW = Unit Weight  
 W = Moisture Content  
 K = Permeability  
 O = Organic Content  
 D = Dispersive  
 RQD = Rock Quality Designation  
 X = X-Ray Defraction  
 HCpot = Hydro-Collapse Potential

\* = Average of subsamples



## SUMMARY OF RESULTS N.D.O.T. GEOTECHNICAL SECTION

EA/Cont # 73587

Job Description SR 160 Cable Barrier Rail

Boring No. CBP 8

Elevation (ft) 3780 +/-

Station "L" 198 + 03

Date 06/09/2010

SAMPLE NO.	SAMPLE DEPTH (ft)	SAMP-LER TYPE	N BLOWS per ft.	SOIL GROUP	W%	DRY UW pcf	% PASS #200	LL %	PL %	PI %	STRENGTH TEST				COMMENTS	
											TEST TYPE	Φ deg.	C psi	Φ deg.		C psi
												Peak		Residual		
A	2.5 - 3.0	SPT	R	GC-GM	8.5		19.7	28	22	6						
B	5.0 - 6.5	SPT	42	GM	6.3		12.9	46	32	14						
C	10.0 - 11.5	SPT	91	SW-SM	1.5		9.5	15	14	1						
D	15.0 - 15.4	SPT	R		1.9		15.4									
BULK 1	5.0 - 10.0			GM			14.5	44	29	15					RV = 68	

CMS = California Modified Sampler 2.42" ID  
 SPT = Standard Penetration 1.38" ID  
 CS = Continuous Sample 3.23" ID  
 RC = Rock Core  
 PB = Pitcher Barrel  
 CSS = Calif. Split Spoon 2.42" ID  
 CPT = Cone Penetration Test  
 TP = Test Pit  
 P = Pushed, not driven  
 R = Refusal  
 Sh = Shelby Tube 2.87" ID

U = Unconfined Compressive  
 UU = Unconsolidated Undrained  
 CD = Consolidated Drained  
 CU = Consolidated Undrained  
 DS = Direct Shear  
 Φ = Fri Φ = Friction  
 C = Cohesion  
 N = No. of blows per ft., sampler  
  
 N = Field SPT                      N = (N<sub>css</sub>)(0.62)

H = Hydrometer  
 S = Sieve  
 G = Specific Gravity  
 PI = Plasticity Index  
 LL = Liquid Limit  
 PL = Plastic Limit  
 NP = Non-Plastic  
 OC = Consolidation  
 Ch = Chemical  
 RV = R - Value  
 MD = Moisture Density

CM = Compaction  
 E = Swell/Pressure on Expansive Soils  
 SL = Shrinkage Limit  
 UW = Unit Weight  
 W = Moisture Content  
 K = Permeability  
 O = Organic Content  
 D = Dispersive  
 RQD = Rock Quality Designation  
 X = X-Ray Defraction  
 HCpot = Hydro-Collapse Potential

\* = Average of subsamples

## SUMMARY OF RESULTS N.D.O.T. GEOTECHNICAL SECTION

EA/Cont # 73587

Job Description SR 160 Cable Barrier Rail.

Boring No. CBP 9

Elevation (ft) 3586 +/-

Station "L" 330 + 49

Date 06/09/2010

SAMPLE NO.	SAMPLE DEPTH (ft)	SAMP-LER TYPE	N BLOWS per ft.	SOIL GROUP	W%	DRY UW pcf	% PASS #200	LL %	PL %	PI %	STRENGTH TEST				COMMENTS	
											TEST TYPE	Φ deg.	C psi	Φ deg.		C psi
												Peak		Residual		
A	5.0 - 5.4	SPT	R	GC	3.2		18.9	34	22	12						
B	10.0 - 10.1	SPT	R		1.8			18	NP	NP						
C	15.0 - 15.2	SPT	R		0.8		23.9									
BULK 1	5.0 - 10.0			GC-GM			17.4	25	18	7					Ch, RV = 50	

CMS = California Modified Sampler 2.42" ID  
 SPT = Standard Penetration 1.38" ID  
 CS = Continuous Sample 3.23" ID  
 RC = Rock Core  
 PB = Pitcher Barrel  
 CSS = Calif. Split Spoon 2.42" ID  
 CPT = Cone Penetration Test  
 TP = Test Pit  
 P = Pushed, not driven  
 R = Refusal  
 Sh = Shelby Tube 2.87" ID

U = Unconfined Compressive  
 UU = Unconsolidated Undrained  
 CD = Consolidated Drained  
 CU = Consolidated Undrained  
 DS = Direct Shear  
 Φ = Friction  
 C = Cohesion  
 N = No. of blows per ft., sampler  
 N = Field SPT                      N = (N<sub>css</sub>)(0.62)

H = Hydrometer  
 S = Sieve  
 G = Specific Gravity  
 PI = Plasticity Index  
 LL = Liquid Limit  
 PL = Plastic Limit  
 NP = Non-Plastic  
 OC = Consolidation  
 Ch = Chemical  
 RV = R - Value  
 MD = Moisture Density

CM = Compaction  
 E = Swell/Pressure on Expansive Soils  
 SL = Shrinkage Limit  
 UW = Unit Weight  
 W = Moisture Content  
 K = Permeability  
 O = Organic Content  
 D = Dispersive  
 RQD = Rock Quality Designation  
 X = X-Ray Defraction  
 HCpot = Hydro-Collapse Potential

\* = Average of subsamples

## SUMMARY OF RESULTS N.D.O.T. GEOTECHNICAL SECTION

EA/Cont # 73587

Job Description SR 160 Cable Barrier Rail

Boring No. CBP 10

Elevation (ft) 3338 +/-

Station "L" 478 + 11

Date 06/09/2010

SAMPLE NO.	SAMPLE DEPTH (ft)	SAMP-LER TYPE	N BLOWS per ft.	SOIL GROUP	W%	DRY UW pcf	% PASS #200	LL %	PL %	PI %	STRENGTH TEST				COMMENTS	
											TEST TYPE	Φ deg.	C psi	Φ deg.		C psi
												Peak		Residual		
A	5.0 - 6.5	SPT	71	GC-GM	4.5		24.3	21	16	5						
B	7.5 - 9.0	SPT	51	GC-GM	3.7		14.1	23	17	6						
C	10.0 - 11.5	SPT	67	GC-GM	4.0		18.8	20	15	5						
D	12.5 - 14.0	SPT	91	GC-GM	3.9		14.1	19	15	4						
E	15.0 - 16.5	SPT	89	GP-GC	3.1		11.7	21	15	6						
BULK 1	5.0 - 10.0			GP-GC			10.3	21	16	5					RV = 74	

CMS = California Modified Sampler 2.42" ID  
 SPT = Standard Penetration 1.38" ID  
 CS = Continuous Sample 3.23" ID  
 RC = Rock Core  
 PB = Pitcher Barrel  
 CSS = Calif. Split Spoon 2.42" ID  
 CPT = Cone Penetration Test  
 TP = Test Pit  
 P = Pushed, not driven  
 R = Refusal  
 Sh = Shelby Tube 2.87" ID

U = Unconfined Compressive  
 UU = Unconsolidated Undrained  
 CD = Consolidated Drained  
 CU = Consolidated Undrained  
 DS = Direct Shear  
 Φ = Friction  
 C = Cohesion  
 N = No. of blows per ft., sampler  
  
 N = Field SPT                      N = (N<sub>css</sub>)(0.62)

H = Hydrometer  
 S = Sieve  
 G = Specific Gravity  
 PI = Plasticity Index  
 LL = Liquid Limit  
 PL = Plastic Limit  
 NP = Non-Plastic  
 OC = Consolidation  
 Ch = Chemical  
 RV = R - Value  
 MD = Moisture Density

CM = Compaction  
 E = Swell/Pressure on Expansive Soils  
 SL = Shrinkage Limit  
 UW = Unit Weight  
 W = Moisture Content  
 K = Permeability  
 O = Organic Content  
 D = Dispersive  
 RQD = Rock Quality Designation  
 X = X-Ray Defraction  
 HCpot = Hydro-Collapse Potential

\* = Average of subsamples

## SUMMARY OF RESULTS N.D.O.T. GEOTECHNICAL SECTION

EA/Cont # 73587

Job Description SR 160 Cable Barrier Rail

Boring No. CBP 11

Elevation (ft) 3430 +/-

Station "L" 684 + 13

Date 06/10/2010

SAMPLE NO.	SAMPLE DEPTH (ft)	SAMP- LER TYPE	N BLOWS per ft.	SOIL GROUP	W%	DRY UW pcf	% PASS #200	LL %	PL %	PI %	STRENGTH TEST				COMMENTS	
											TEST TYPE	Φ deg.	C psi	Φ deg.		C psi
												Peak		Residual		
A	2.5 - 4.0	SPT	59	GW-GC	1.8		6.8	20	14	6						
B	5.0 - 6.0	SPT	R	GP-GM	1.7		8.0	16	14	2						
C	7.5 - 7.6	SPT	R												no recovery	
D	10.0 - 11.4	SPT	R		2.0		10.8									
E	15.0 - 16.1	SPT	R		2.3		13.4									
BULK 1	5.0 - 10.0			GP-GC			5.8	21	15	6					RV = 84	

CMS = California Modified Sampler 2.42" ID  
 SPT = Standard Penetration 1.38" ID  
 CS = Continuous Sample 3.23" ID  
 RC = Rock Core  
 PB = Pitcher Barrel  
 CSS = Calif. Split Spoon 2.42" ID  
 CPT = Cone Penetration Test  
 TP = Test Pit  
 P = Pushed, not driven  
 R = Refusal  
 Sh = Shelby Tube 2.87" ID

U = Unconfined Compressive  
 UU = Unconsolidated Undrained  
 CD = Consolidated Drained  
 CU = Consolidated Undrained  
 DS = Direct Shear  
 Φ = Friction  
 C = Cohesion  
 N = No. of blows per ft., sampler  
 N = Field SPT      N = (N<sub>css</sub>)(0.62)

H = Hydrometer  
 S = Sieve  
 G = Specific Gravity  
 PI = Plasticity Index  
 LL = Liquid Limit  
 PL = Plastic Limit  
 NP = Non-Plastic  
 OC = Consolidation  
 Ch = Chemical  
 RV = R - Value  
 MD = Moisture Density

CM = Compaction  
 E = Swell/Pressure on Expansive Soils  
 SL = Shrinkage Limit  
 UW = Unit Weight  
 W = Moisture Content  
 K = Permeability  
 O = Organic Content  
 D = Dispersive  
 RQD = Rock Quality Designation  
 X = X-Ray Defraction  
 HCpot = Hydro-Collapse Potential

\* = Average of subsamples

## SUMMARY OF RESULTS N.D.O.T. GEOTECHNICAL SECTION

EA/Cont # 73587

Job Description SR 160 Cable Barrier Rail

Boring No. CBP 12

Elevation (ft) 3193 +/-

Station "P" 318 + 79

Date 06/10/2010

SAMPLE NO.	SAMPLE DEPTH (ft)	SAMP-LER TYPE	N BLOWS per ft.	SOIL GROUP	W%	DRY UW pcf	% PASS #200	LL %	PL %	PI %	STRENGTH TEST				COMMENTS	
											TEST TYPE	Φ deg.	C psi	Φ deg.		C psi
												Peak		Residual		
A	1.0 - 2.5	SPT	78	GC	3.6		26.1	30	20	10						
B	5.0 - 6.5	SPT	81	GC-GM	2.5		13.6	23	17	6						
C	10.0 - 10.5	SPT	R		3.0			19	15	4						
D	15.0 - 15.8	SPT	R	GC-GM	2.4		14.0	22	15	7						
BULK 1	5.0 - 10.0			GW			4.6	25	17	8					RV = 83	

CMS = California Modified Sampler 2.42" ID  
 SPT = Standard Penetration 1.38" ID  
 CS = Continuous Sample 3.23" ID  
 RC = Rock Core  
 PB = Pitcher Barrel  
 CSS = Calif. Split Spoon 2.42" ID  
 CPT = Cone Penetration Test  
 TP = Test Pit  
 P = Pushed, not driven  
 R = Refusal  
 Sh = Shelby Tube 2.87" ID

U = Unconfined Compressive  
 UU = Unconsolidated Undrained  
 CD = Consolidated Drained  
 CU = Consolidated Undrained  
 DS = Direct Shear  
 Φ = Friction  
 C = Cohesion  
 N = No. of blows per ft., sampler  
  
 N = Field SPT                      N = (N<sub>css</sub>)(0.62)

H = Hydrometer  
 S = Sieve  
 G = Specific Gravity  
 PI = Plasticity Index  
 LL = Liquid Limit  
 PL = Plastic Limit  
 NP = Non-Plastic  
 OC = Consolidation  
 Ch = Chemical  
 RV = R - Value  
 MD = Moisture Density

CM = Compaction  
 E = Swell/Pressure on Expansive Soils  
 SL = Shrinkage Limit  
 UW = Unit Weight  
 W = Moisture Content  
 K = Permeability  
 O = Organic Content  
 D = Dispersive  
 RQD = Rock Quality Designation  
 X = X-Ray Defraction  
 HCpot = Hydro-Collapse Potential

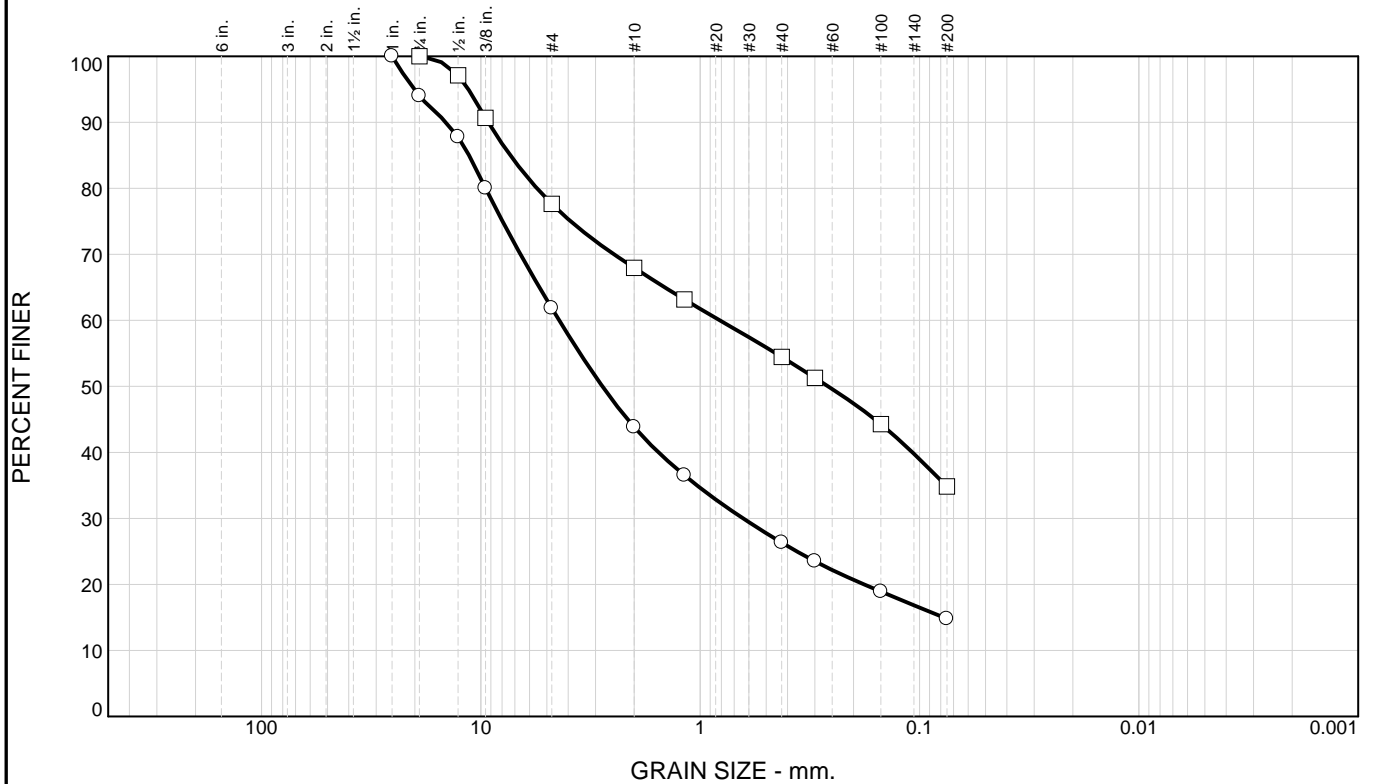
**\* = Average of subsamples**







# Particle Size Distribution Report



	+3"	% GRAVEL	% SAND	% SILT	% CLAY	USCS	AASHTO	PL	LL
○	0.0	38.1	47.1	14.8		SM	A-1-a	NP	16
□	0.0	22.4	42.8	34.8		SM	A-2-4(0)	20	21

SIEVE inches size	PERCENT FINER	
	○	□
1"	100.0	
3/4"	94.0	100.0
1/2"	87.8	97.1
3/8"	80.0	90.7
GRAIN SIZE		
D60	4.3932	0.8132
D30	0.6374	
D10		
COEFFICIENTS		
Cc		
Cu		

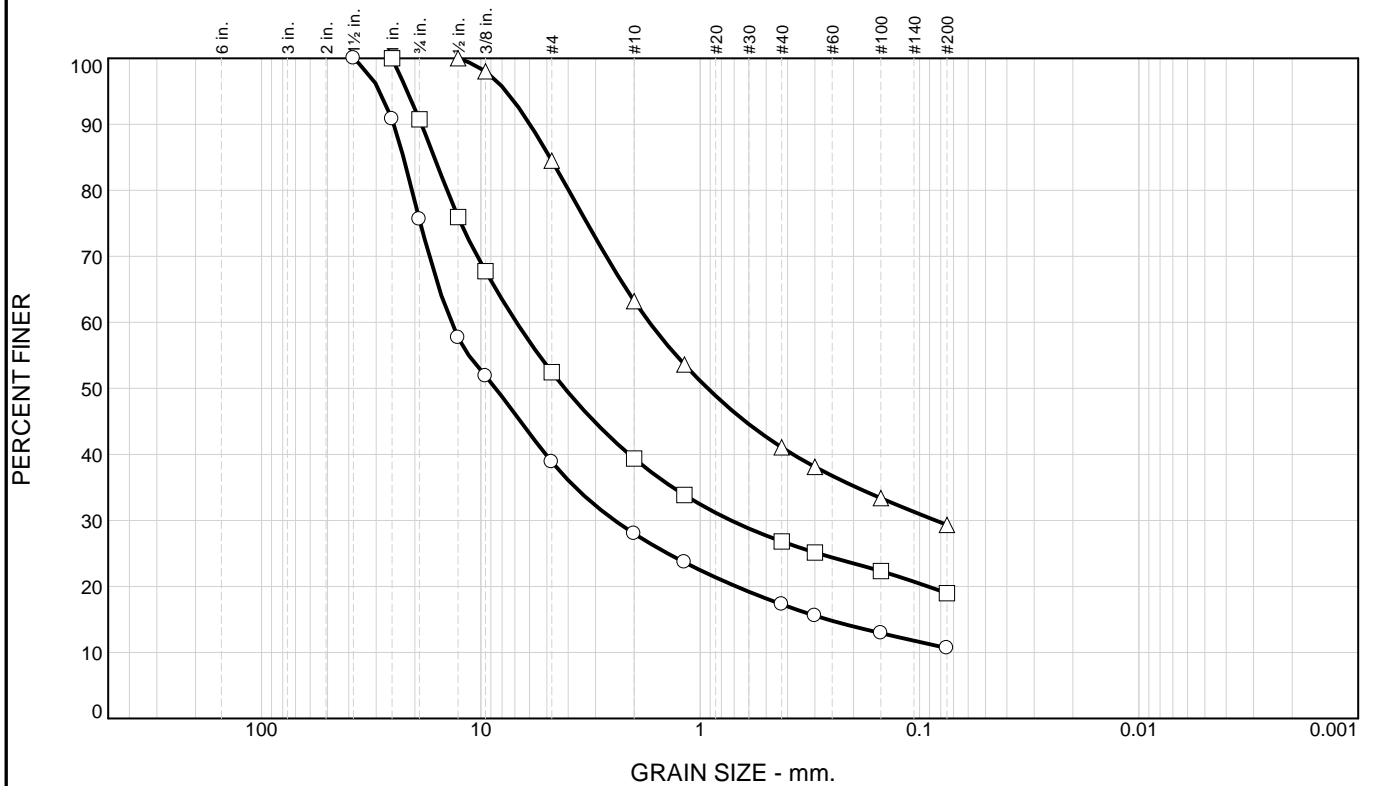
SIEVE number size	PERCENT FINER	
	○	□
#4	61.9	77.6
#10	43.8	68.0
#16	36.5	63.2
#40	26.3	54.5
#50	23.5	51.3
#100	18.9	44.3
#200	14.8	34.8

**Material Description**  
 silty sand with gravel  
  
 silty sand with gravel

**REMARKS:**

○ Source of Sample: CBP1      Depth: 15.0 - 16.5'      Sample Number: F  
 □ Source of Sample: CBP1      Depth: 5.0 - 10.0'      Sample Number: BULK 1

# Particle Size Distribution Report



	+3"	% GRAVEL	% SAND	% SILT	% CLAY	USCS	AASHTO	PL	LL
○	0.0	61.1	28.2		10.7	GP-GM	A-1-a	18	19
□	0.0	47.5	33.5		19.0				
△	0.0	15.5	55.2		29.3				

SIEVE inches size	PERCENT FINER		
	○	□	△
1.5"	100.0		
1"	90.8	100.0	
3/4"	75.6	90.8	
1/2"	57.7	76.0	100.0
3/8"	51.9	67.7	98.0
GRAIN SIZE			
D60	13.6526	6.8815	1.7065
D30	2.4563	0.7237	0.0845
D10			
COEFFICIENTS			
C <sub>c</sub>			
C <sub>u</sub>			

SIEVE number size	PERCENT FINER		
	○	□	△
#4	38.9	52.5	84.5
#10	28.0	39.4	63.2
#16	23.7	33.9	53.6
#40	17.3	26.8	41.1
#50	15.6	25.1	38.1
#100	12.9	22.3	33.4
#200	10.7	19.0	29.3

**Material Description**  
○ poorly graded gravel with silt and sand

□ SIEVE ONLY

△ SIEVE ONLY

**REMARKS:**

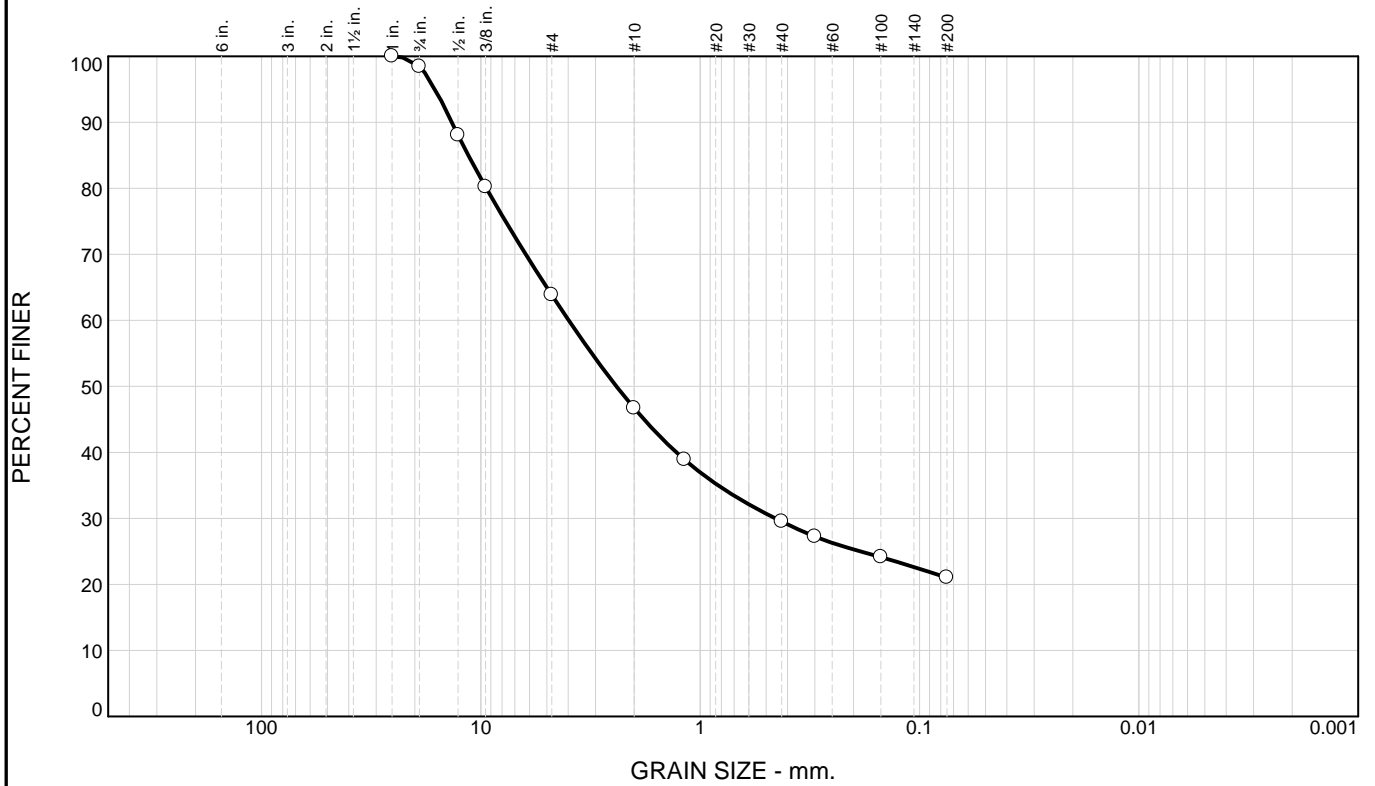
○

□

△

○ Source of Sample: CBP2      Depth: 0.5 - 1.85'      Sample Number: A  
 □ Source of Sample: CBP2      Depth: 2.5 - 2.85'      Sample Number: B  
 △ Source of Sample: CBP2      Depth: 10.0 - 10.3'      Sample Number: D

# Particle Size Distribution Report



	+3"	% GRAVEL	% SAND	% SILT	% CLAY	USCS	AASHTO	PL	LL
○	0.0	36.2	42.8	21.0		SM	A-1-b	15	18

SIEVE inches size	PERCENT FINER		
	○		
1"	100.0		
3/4"	98.5		
1/2"	88.0		
3/8"	80.2		
GRAIN SIZE			
D60	3.9763		
D30	0.4539		
D10			
COEFFICIENTS			
Cc			
Cu			

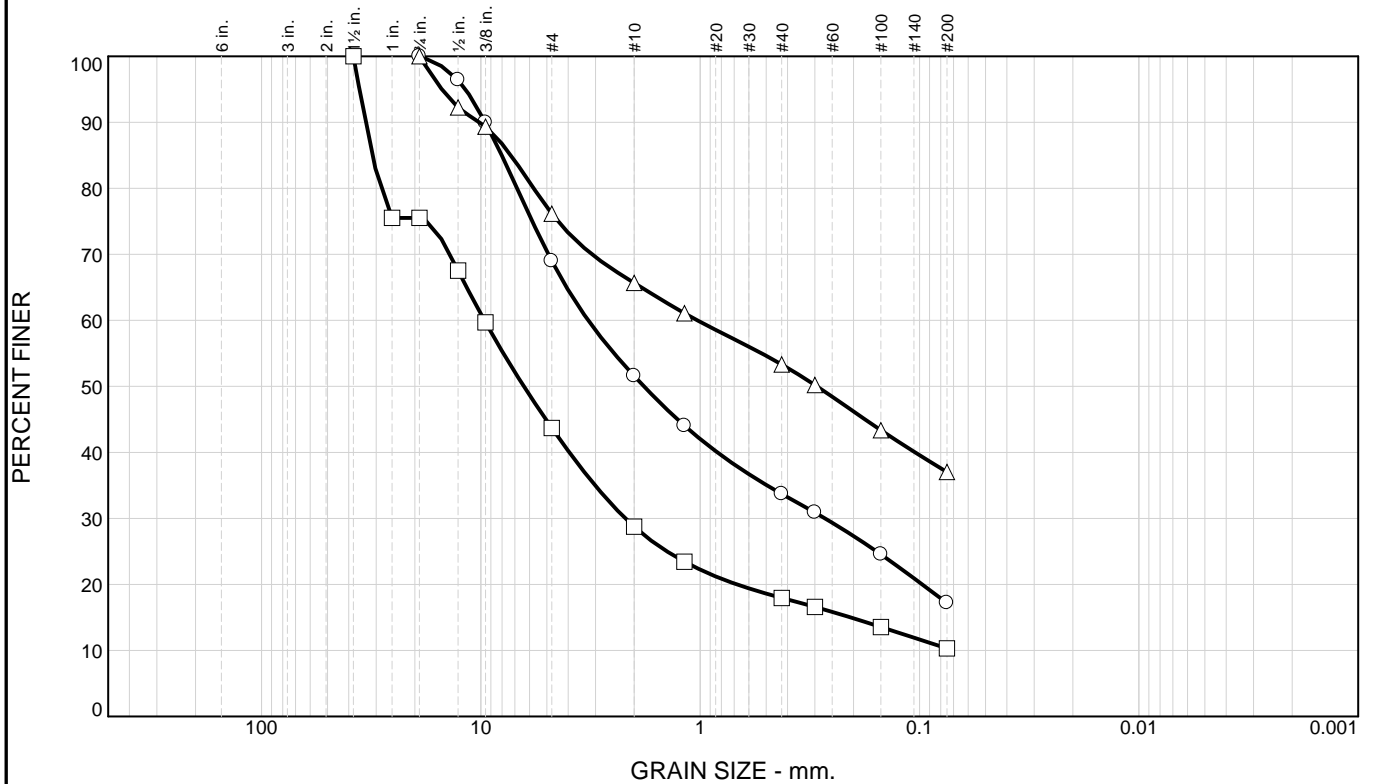
SIEVE number size	PERCENT FINER		
	○		
#4	63.8		
#10	46.7		
#16	38.9		
#40	29.5		
#50	27.2		
#100	24.1		
#200	21.0		

**Material Description**  
○ silty sand with gravel

**REMARKS:**  
○

○ Source of Sample: CBP2      Depth: 5.0 - 10.0'      Sample Number: BULK 1

# Particle Size Distribution Report



	+3"	% GRAVEL	% SAND	% SILT	% CLAY	USCS	AASHTO	PL	LL
○	0.0	31.0	51.8		17.2	SM	A-1-b	18	19
□	0.0	56.3	33.4		10.3	GP-GM	A-1-a	19	22
△	0.0	23.8	39.2		37.0	SC-SM	A-4(0)	22	29

SIEVE inches size	PERCENT FINER		
	○	□	△
1.5"		100.0	
1"		75.5	
3/4"	100.0	75.5	100.0
1/2"	96.4	67.5	92.2
3/8"	89.9	59.7	89.3
GRAIN SIZE			
D60	3.2426	9.6385	1.0297
D30	0.2695	2.1955	
D10			
COEFFICIENTS			
Cc			
Cu			

SIEVE number size	PERCENT FINER		
	○	□	△
#4	69.0	43.7	76.2
#10	51.6	28.7	65.7
#16	44.0	23.4	61.1
#40	33.7	18.0	53.3
#50	30.9	16.6	50.2
#100	24.6	13.6	43.4
#200	17.2	10.3	37.0

**Material Description**

○ silty sand with gravel

□ poorly graded gravel with silt and sand

△ silty, clayey sand with gravel

**REMARKS:**

○

□

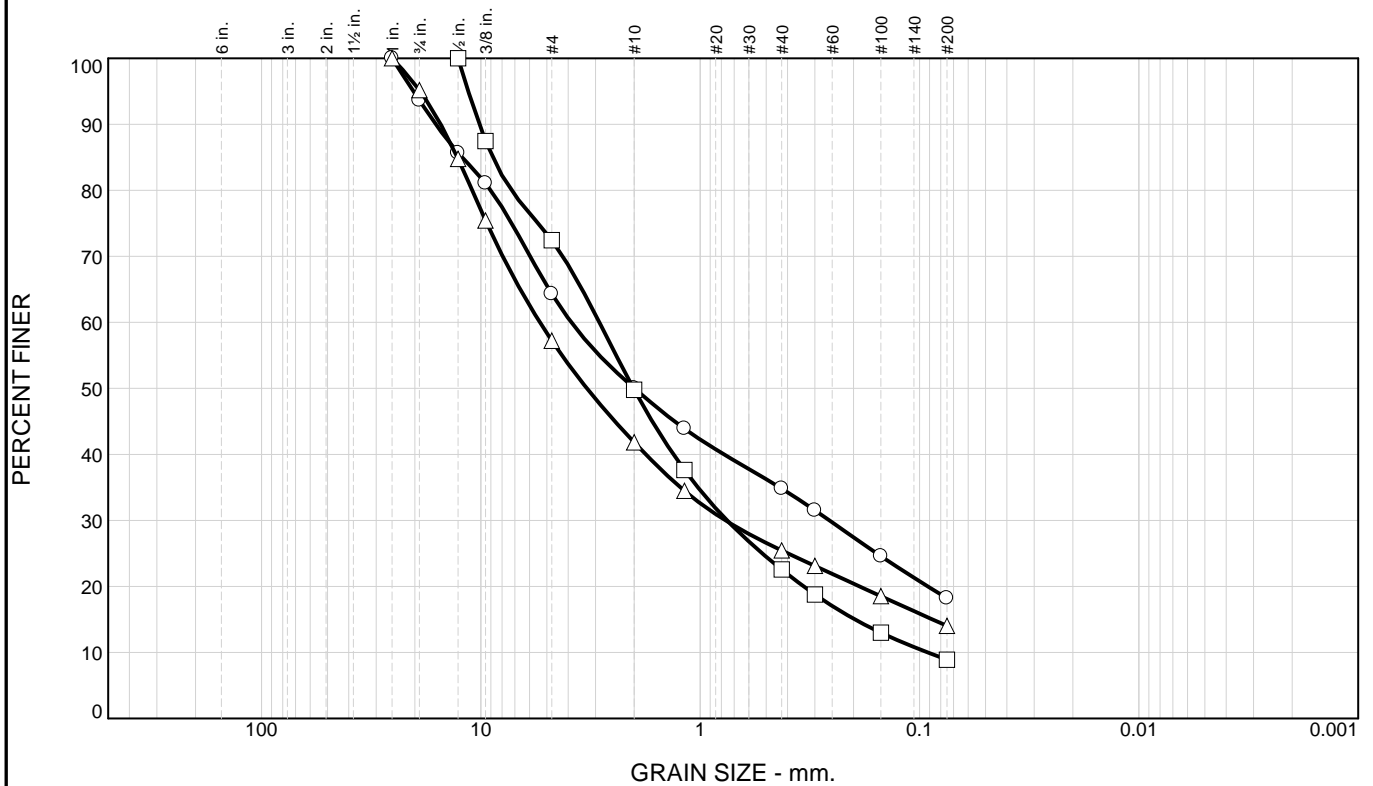
△

○ Source of Sample: CBP3      Depth: 2.5 - 4.0'      Sample Number: A  
 □ Source of Sample: CBP3      Depth: 5.0 - 5.6'      Sample Number: B  
 △ Source of Sample: CBP3      Depth: 7.5 - 9.0'      Sample Number: C





# Particle Size Distribution Report



	+3"	% GRAVEL	% SAND	% SILT	% CLAY	USCS	AASHTO	PL	LL
○	0.0	35.7	46.1		18.2	SC-SM	A-1-b	20	25
□	0.0	27.6	63.5		8.9	SW-SM	A-1-a	NP	15
△	0.0	42.8	43.2		14.0				

SIEVE inches size	PERCENT FINER		
	○	□	△
1"	100.0		100.0
3/4"	93.6		95.2
1/2"	85.7	100.0	84.8
3/8"	81.1	87.5	75.4
GRAIN SIZE			
D60	3.8728	2.8827	5.3852
D30	0.2576	0.7540	0.7666
D10		0.0916	
COEFFICIENTS			
C <sub>c</sub>		2.15	
C <sub>u</sub>		31.47	

SIEVE number size	PERCENT FINER		
	○	□	△
#4	64.3	72.4	57.2
#10	50.0	49.8	43.9
#16	43.9	37.6	34.5
#40	34.8	22.6	25.5
#50	31.5	18.8	23.1
#100	24.6	13.0	18.5
#200	18.2	8.9	14.0

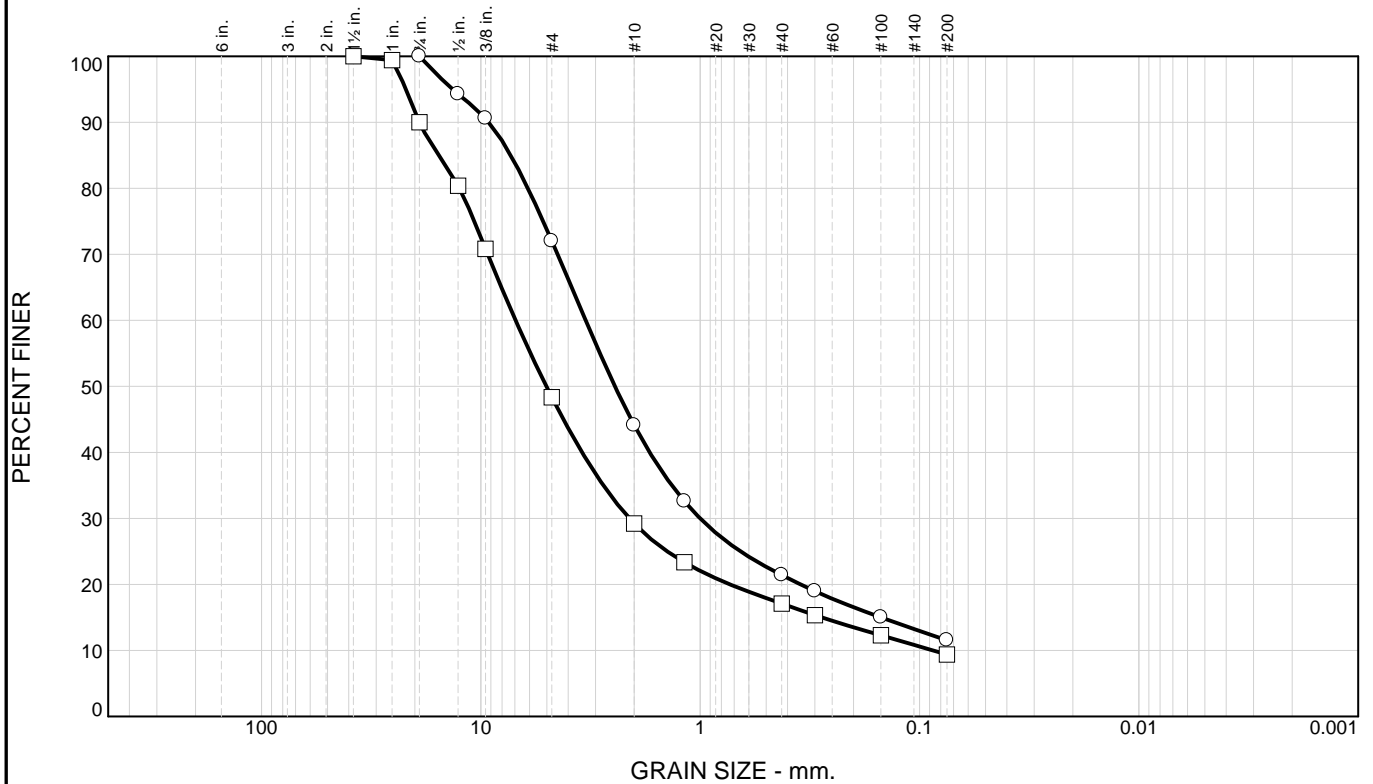
**Material Description**  
 ○ silty, clayey sand with gravel  
 □ well-graded sand with silt and gravel  
 △ SIEVE ONLY

**REMARKS:**  
 ○  
 □  
 △

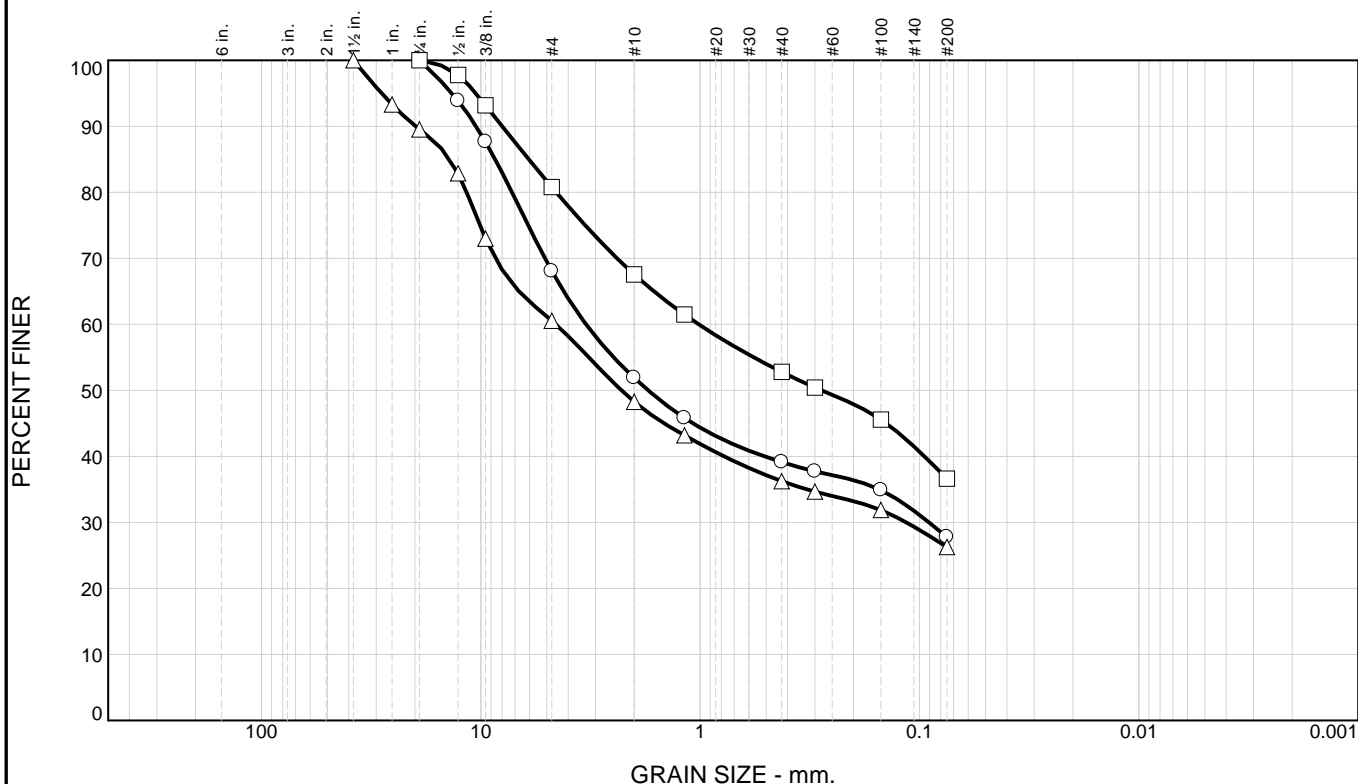
○ Source of Sample: CBP4      Depth: 2.0 - 2.9'      Sample Number: A  
 □ Source of Sample: CBP4      Depth: 5.0 - 6.5'      Sample Number: B  
 △ Source of Sample: CBP4      Depth: 10.0 - 10.5'      Sample Number: C



# Particle Size Distribution Report



# Particle Size Distribution Report



	+3"	% GRAVEL	% SAND	% SILT	% CLAY	USCS	AASHTO	PL	LL
○	0.0	31.9	40.3		27.8	SC-SM	A-2-4(0)	18	24
□	0.0	19.2	44.2		36.6	SC-SM	A-4(0)	22	28
△	0.0	39.5	34.2		26.3	GC-GM	A-2-4(0)	16	21

SIEVE inches size	PERCENT FINER		
	○	□	△
1.5"			100.0
1"			93.3
3/4"	100.0	100.0	89.5
1/2"	93.9	97.8	82.9
3/8"	87.7	93.2	73.0
GRAIN SIZE			
D60	3.3161	1.0145	4.5535
D30	0.0905		0.1146
D10			
COEFFICIENTS			
Cc			
Cu			

SIEVE number size	PERCENT FINER		
	○	□	△
#4	68.1	80.8	60.5
#10	51.9	67.6	48.3
#16	45.8	61.5	43.2
#40	39.2	52.8	36.3
#50	37.7	50.4	34.7
#100	34.9	45.6	31.9
#200	27.8	36.6	26.3

**Material Description**

○ silty, clayey sand with gravel

□ silty, clayey sand with gravel

△ silty clayey gravel with sand

**REMARKS:**

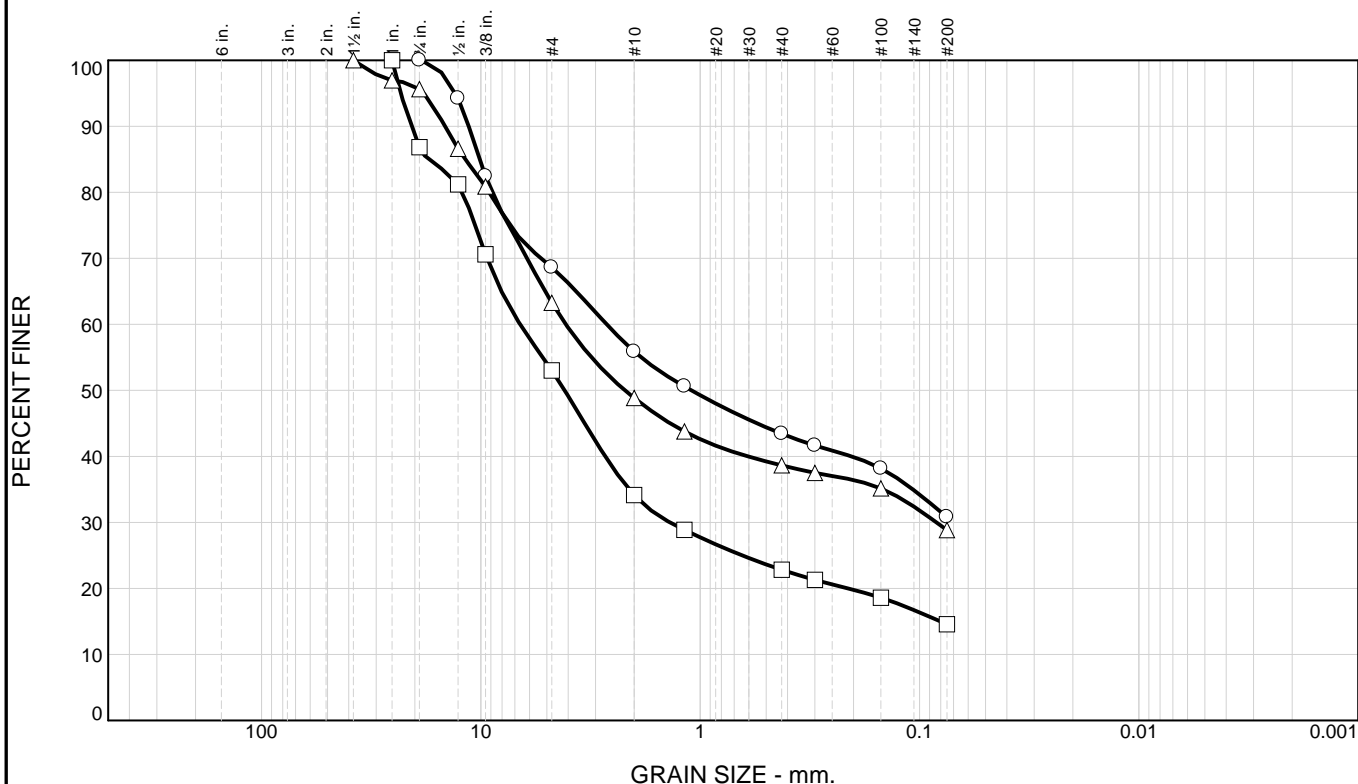
○

□

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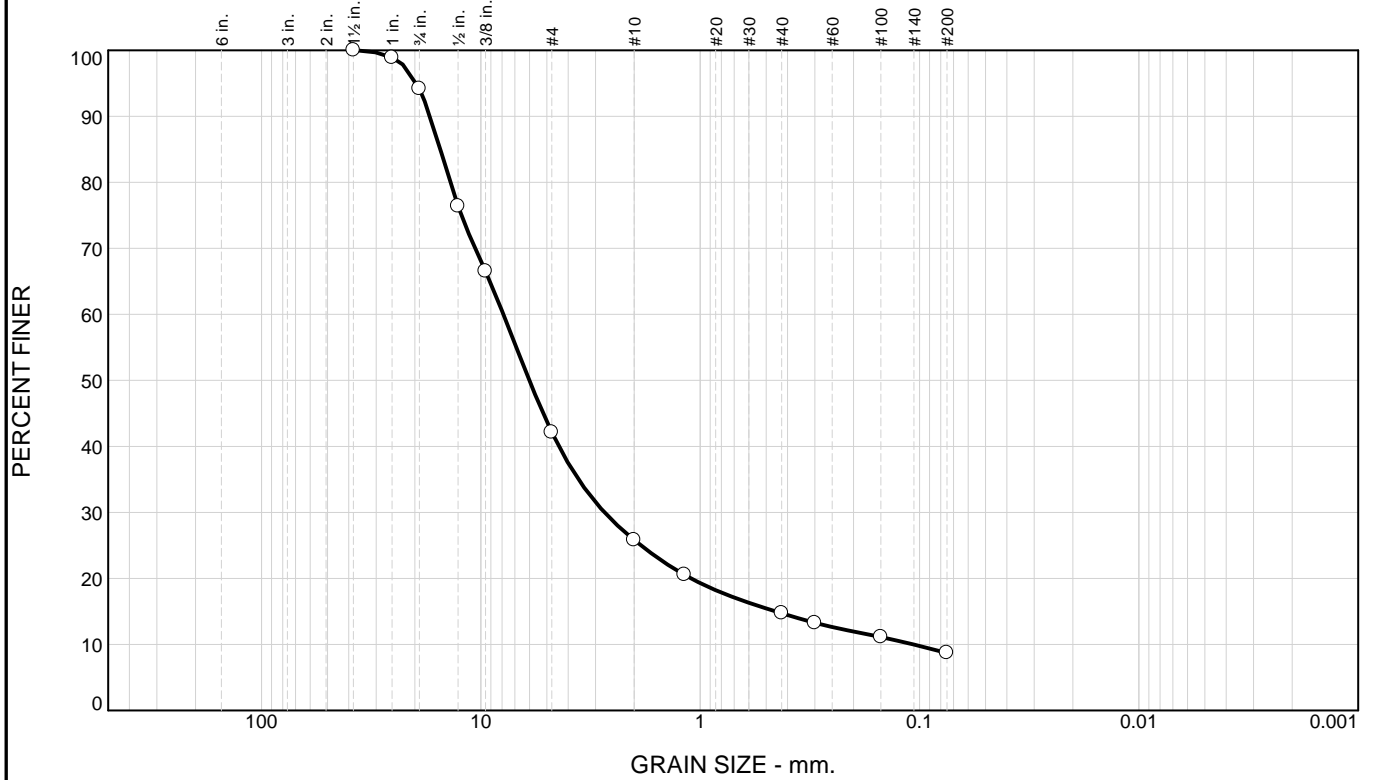
○ Source of Sample: CBP5      Depth: 1.0 - 2.5'      Sample Number: A  
 □ Source of Sample: CBP5      Depth: 5.0 - 6.5'      Sample Number: B  
 △ Source of Sample: CBP5      Depth: 7.5 - 9.0'      Sample Number: C

# Particle Size Distribution Report





# Particle Size Distribution Report



	+3"	% GRAVEL	% SAND	% SILT	% CLAY	USCS	AASHTO	PL	LL
○	0.0	57.9	33.4	8.7		GP-GM	A-1-a	16	19

SIEVE inches size	PERCENT FINER		
	○		
1.5"	100.0		
1"	98.9		
3/4"	94.2		
1/2"	76.4		
3/8"	66.5		
<del>X</del>	GRAIN SIZE		
D60	7.8964		
D30	2.7333		
D10	0.1068		
<del>X</del>	COEFFICIENTS		
Cc	8.86		
Cu	73.94		

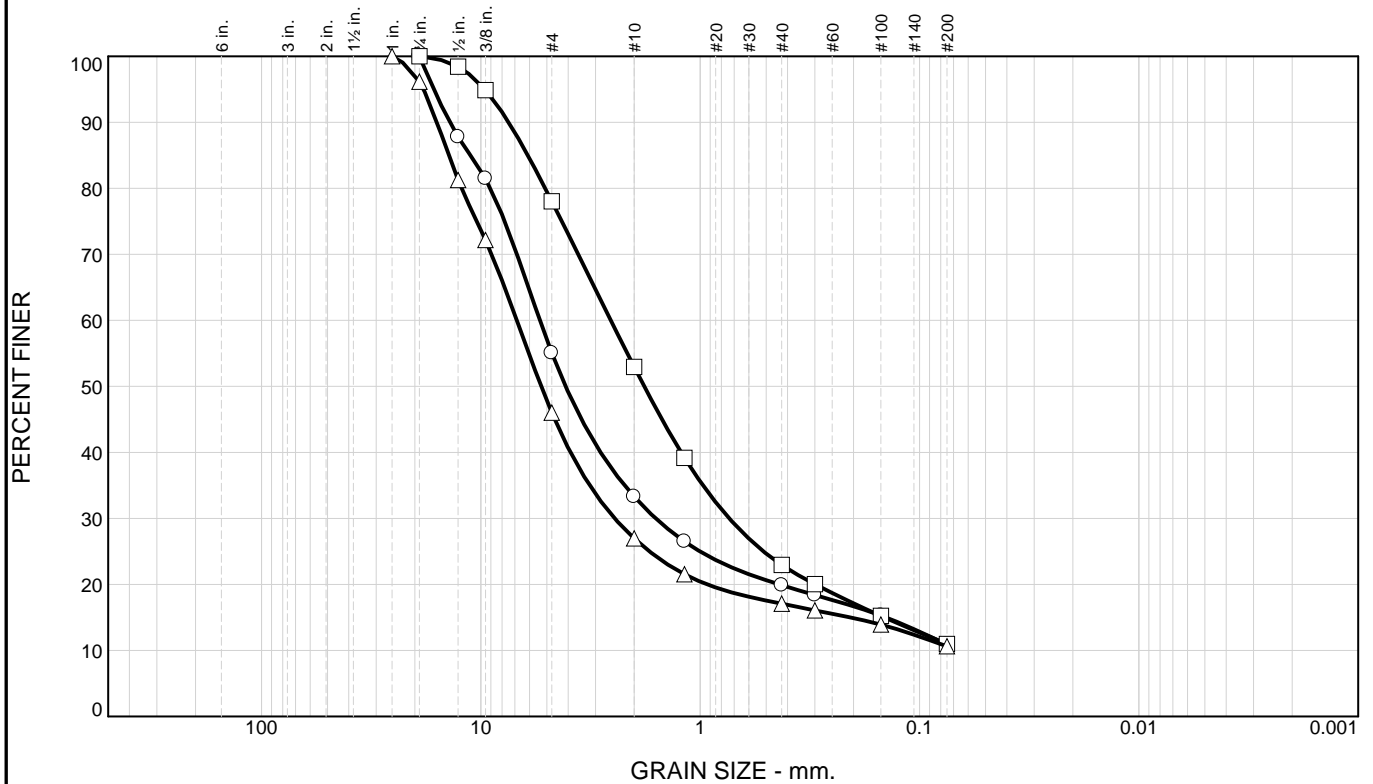
SIEVE number size	PERCENT FINER		
	○		
#4	42.1		
#10	25.8		
#16	20.5		
#40	14.7		
#50	13.3		
#100	11.1		
#200	8.7		

**Material Description**  
○ poorly graded gravel with silt and sand

**REMARKS:**  
○

○ Source of Sample: CBP6      Depth: 5.0 - 10.0'      Sample Number: BULK 1

# Particle Size Distribution Report



	+3"	% GRAVEL	% SAND	% SILT	% CLAY	USCS	AASHTO	PL	LL
○	0.0	44.9	44.2		10.9	GP-GM	A-1-a	21	23
□	0.0	22.0	67.0		11.0				
△	0.0	54.0	35.4		10.6	GP-GC	A-1-a	16	22

SIEVE inches size	PERCENT FINER		
	○	□	△
1"	100.0	100.0	100.0
3/4"	100.0	100.0	96.2
1/2"	87.8	98.4	81.3
3/8"	81.5	94.9	72.2
GRAIN SIZE			
D60	5.3831	2.5567	6.8562
D30	1.5964	0.7369	2.4557
D10			
COEFFICIENTS			
C <sub>c</sub>			
C <sub>u</sub>			

SIEVE number size	PERCENT FINER		
	○	□	△
#4	55.1	78.0	46.0
#10	33.3	53.0	27.0
#16	26.5	39.2	21.5
#40	19.9	23.0	17.1
#50	18.4	20.0	16.1
#100	15.3	15.2	13.9
#200	10.9	11.0	10.6

**Material Description**  
 ○ poorly graded gravel with silt and sand  
 □ SIEVE ONLY  
 △ poorly graded gravel with siltyclay and sand

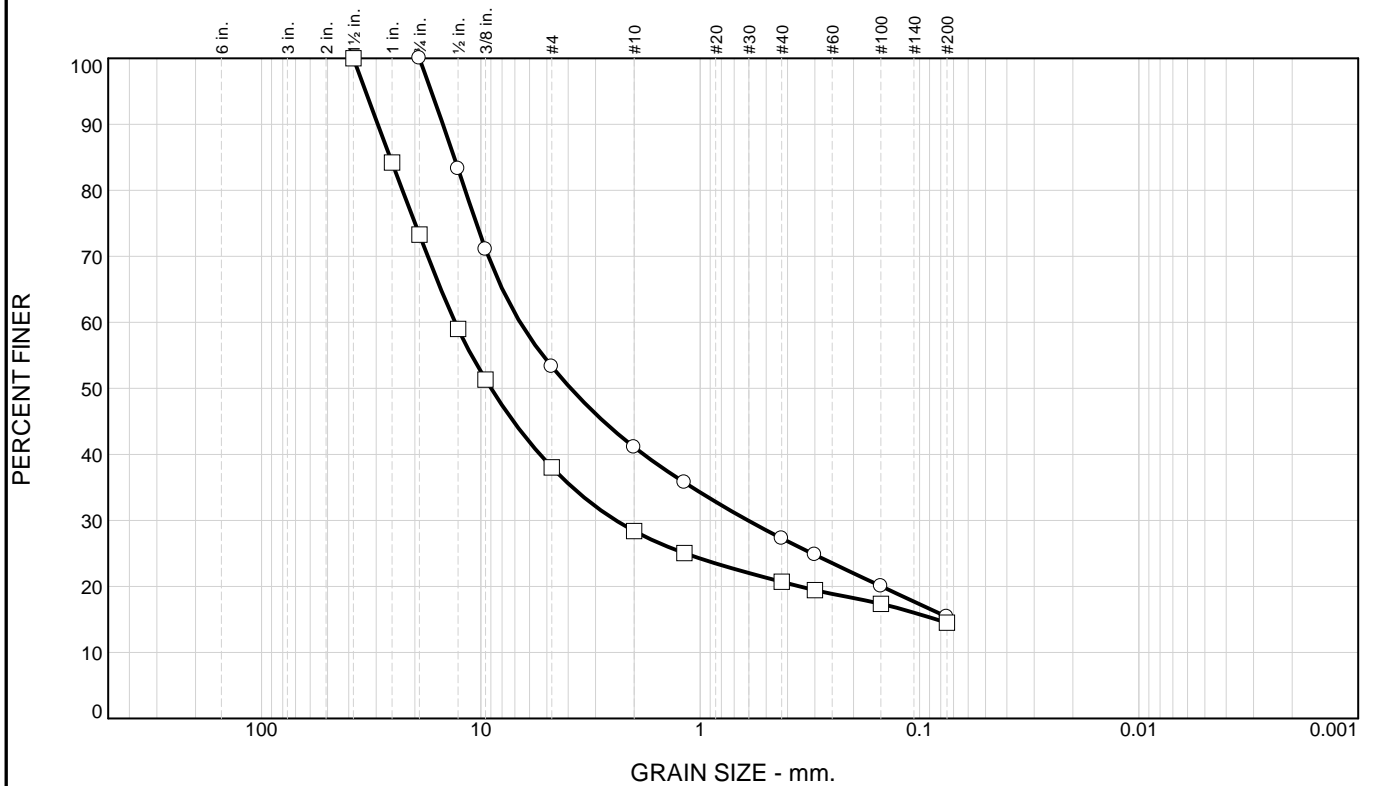
**REMARKS:**  
 ○  
 □  
 △

○ Source of Sample: CBP7      Depth: 5.0 - 6.5'      Sample Number: B  
 □ Source of Sample: CBP7      Depth: 10.0 - 10.35'      Sample Number: D  
 △ Source of Sample: CBP7      Depth: 5.0 - 10.0'      Sample Number: BULK 1





# Particle Size Distribution Report



	+3"	% GRAVEL	% SAND	% SILT	% CLAY	USCS	AASHTO	PL	LL
○	0.0	46.7	37.9	15.4					
□	0.0	62.0	23.5	14.5		GM	A-2-7(0)	29	44

SIEVE inches size	PERCENT FINER	
	○	□
1.5"		100.0
1"		84.2
3/4"	100.0	73.3
1/2"	83.3	59.0
3/8"	71.1	51.3
GRAIN SIZE		
D60	6.6247	13.1133
D30	0.6055	2.4233
D10		
COEFFICIENTS		
Cc		
Cu		

SIEVE number size	PERCENT FINER	
	○	□
#4	53.3	38.0
#10	41.1	28.4
#16	35.7	25.1
#40	27.3	20.7
#50	24.8	19.4
#100	20.0	17.4
#200	15.4	14.5

**Material Description**  
 SIEVE ONLY  
  
 silty gravel with sand

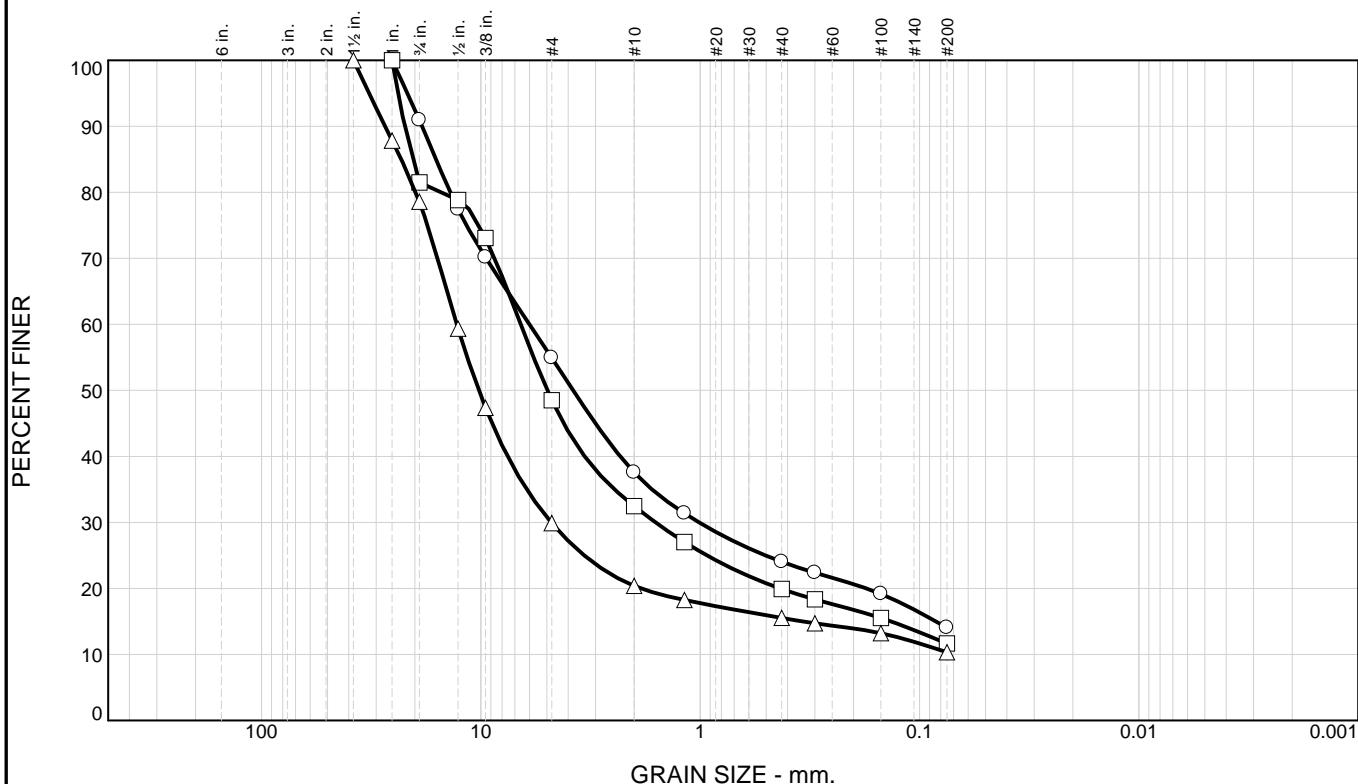
**REMARKS:**

○ Source of Sample: CBP8      Depth: 15.0 - 15.4'      Sample Number: D  
 □ Source of Sample: CBP8      Depth: 5.0 - 10.0'      Sample Number: BULK 1





# Particle Size Distribution Report



	+3"	% GRAVEL	% SAND	% SILT	% CLAY	USCS	AASHTO	PL	LL
○	0.0	45.1	40.8		14.1	GC-GM	A-1-a	15	19
□	0.0	51.5	36.8		11.7	GP-GC	A-1-a	15	21
△	0.0	70.1	19.6		10.3	GP-GC	A-1-a	16	21

SIEVE inches size	PERCENT FINER		
	○	□	△
1.5"	100.0	100.0	100.0
1"	100.0	100.0	87.8
3/4"	90.9	81.5	78.6
1/2"	77.4	78.8	59.4
3/8"	70.1	73.1	47.4
GRAIN SIZE			
D60	6.0061	6.5799	12.8739
D30	1.0111	1.5985	4.7803
D10			
COEFFICIENTS			
Cc			
Cu			

SIEVE number size	PERCENT FINER		
	○	□	△
#4	54.9	48.5	29.9
#10	37.6	32.5	20.4
#16	31.4	27.0	18.3
#40	24.0	19.9	15.5
#50	22.4	18.3	14.7
#100	19.2	15.5	13.2
#200	14.1	11.7	10.3

**Material Description**

○ silty clayey gravel with sand

□ poorly graded gravel with siltyclay and sand

△ poorly graded gravel with siltyclay and sand

**REMARKS:**

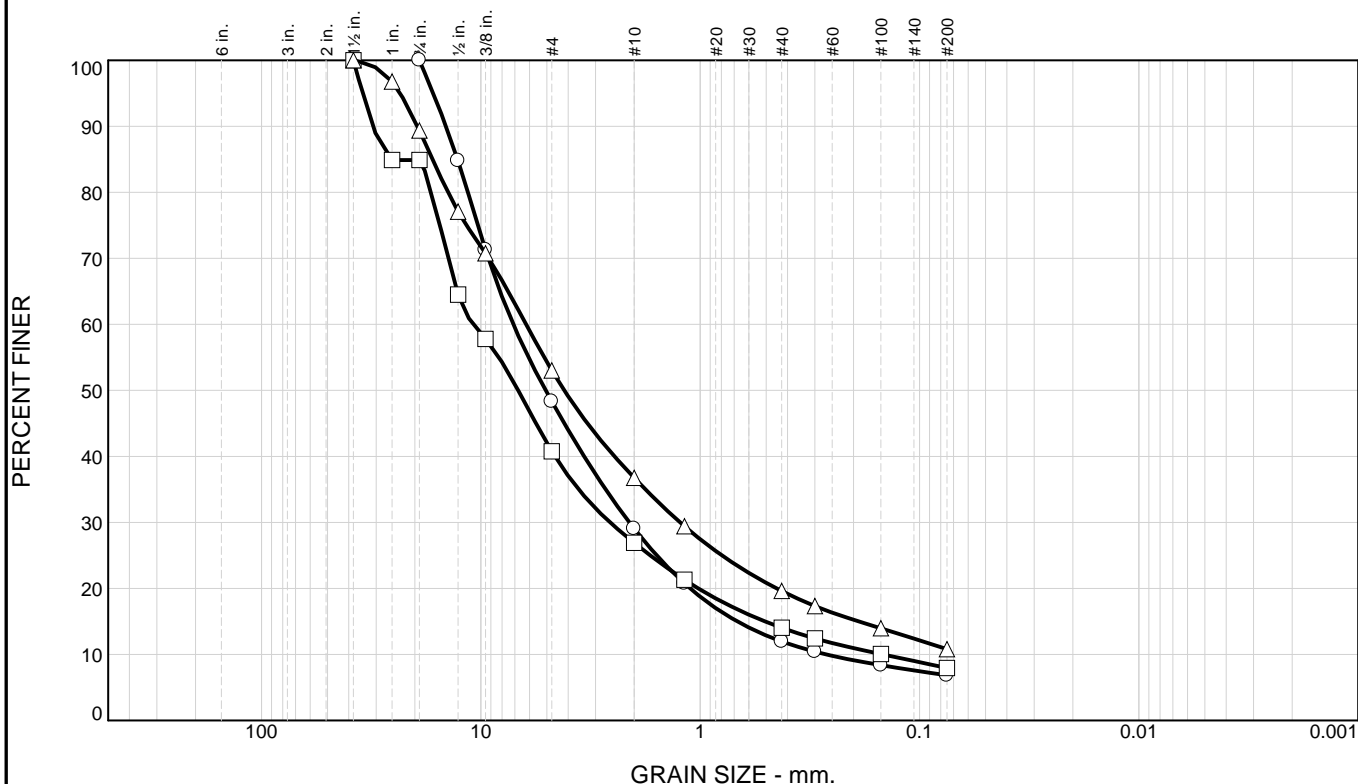
○

□

△

○ Source of Sample: CBP10      Depth: 12.5 - 14.0'      Sample Number: D  
 □ Source of Sample: CBP10      Depth: 15.0 - 16.5'      Sample Number: E  
 △ Source of Sample: CBP10      Depth: 5.0 - 10.0'      Sample Number: BULK 1

# Particle Size Distribution Report



	+3"	% GRAVEL	% SAND	% SILT	% CLAY	USCS	AASHTO	PL	LL
○	0.0	51.7	41.5		6.8	GW-GC	A-1-a	14	20
□	0.0	59.2	32.8		8.0	GP-GM	A-1-a	14	16
△	0.0	47.0	42.2		10.8				

SIEVE inches size	PERCENT FINER		
	○	□	△
1.5"		100.0	100.0
1"		84.9	96.8
3/4"	100.0	84.9	89.4
1/2"	84.8	64.5	77.1
3/8"	71.3	57.8	70.8
GRAIN SIZE			
D60	7.1192	10.8660	6.2204
D30	2.1047	2.5774	1.2359
D10	0.2662	0.1467	
COEFFICIENTS			
C <sub>c</sub>	2.34	4.17	
C <sub>u</sub>	26.74	74.08	

SIEVE number size	PERCENT FINER		
	○	□	△
#4	48.3	40.8	53.0
#10	29.1	26.9	36.8
#16	20.8	21.3	29.4
#40	12.0	14.0	19.6
#50	10.4	12.4	17.3
#100	8.4	10.1	14.0
#200	6.8	8.0	10.8

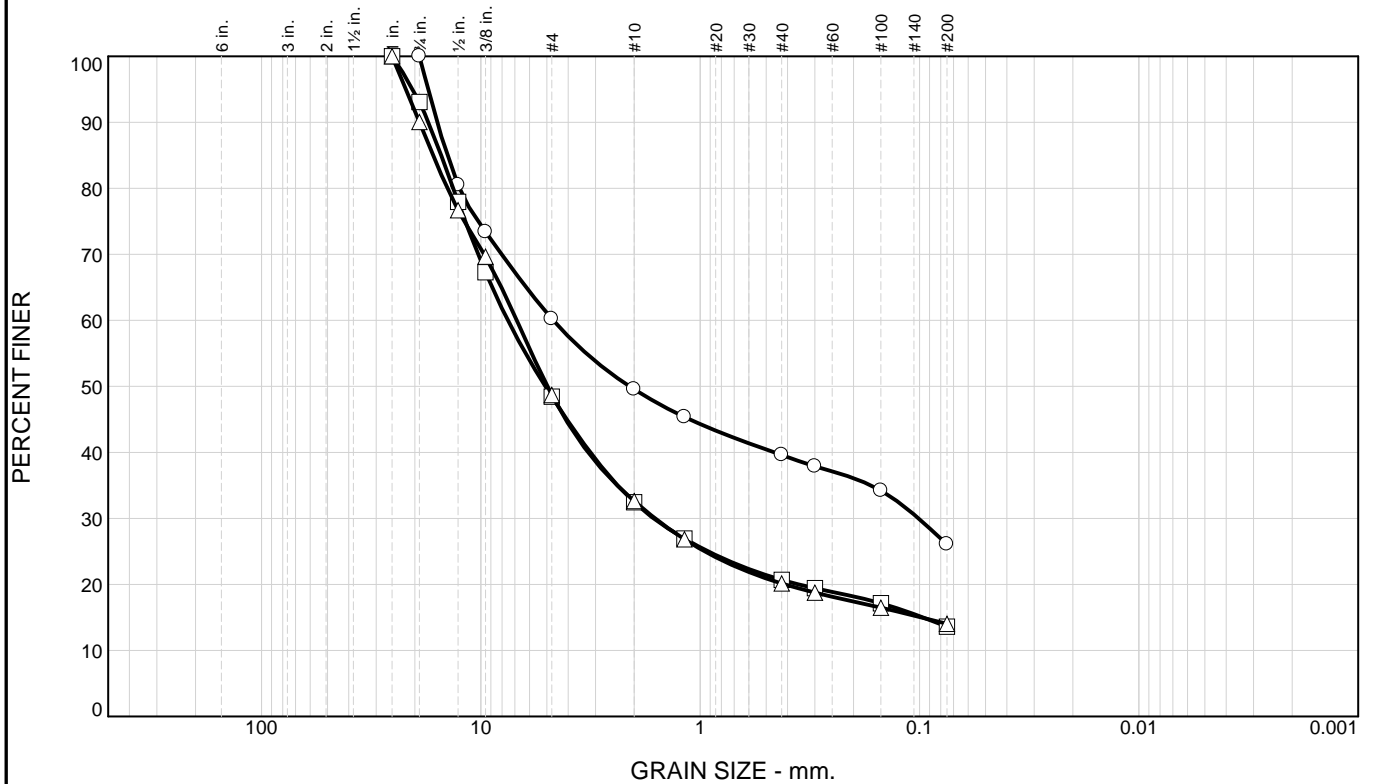
**Material Description**  
 ○ well-graded gravel with siltyclay and sand  
 □ poorly graded gravel with silt and sand  
 △ SIEVE ONLY

**REMARKS:**  
 ○  
 □  
 △

○ Source of Sample: CBP11      Depth: 2.5 - 4.0'      Sample Number: A  
 □ Source of Sample: CBP11      Depth: 5.0 - 6.0'      Sample Number: B  
 △ Source of Sample: CBP11      Depth: 10.0 - 11.35'      Sample Number: D



# Particle Size Distribution Report



	+3"	% GRAVEL	% SAND	% SILT	% CLAY	USCS	AASHTO	PL	LL
○	0.0	39.8	34.1	26.1		GC	A-2-4(0)	20	30
□	0.0	51.6	34.8	13.6		GC-GM	A-1-a	17	23
△	0.0	51.3	34.7	14.0		GC-GM	A-2-4(0)	15	22

SIEVE inches size	PERCENT FINER		
	○	□	△
1"	100.0	100.0	100.0
3/4"	100.0	93.1	90.0
1/2"	80.5	78.0	76.7
3/8"	73.4	67.3	69.7
GRAIN SIZE			
D60	4.6809	7.5367	6.8621
D30	0.1010	1.6324	1.6060
D10			
COEFFICIENTS			
C <sub>c</sub>			
C <sub>u</sub>			

SIEVE number size	PERCENT FINER		
	○	□	△
#4	60.2	48.4	48.7
#10	49.6	32.4	32.6
#16	45.4	26.9	26.8
#40	39.6	20.7	20.1
#50	37.9	19.4	18.8
#100	34.2	17.1	16.5
#200	26.1	13.6	14.0

**Material Description**

○ clayey gravel with sand

□ silty clayey gravel with sand

△ silty clayey gravel with sand

**REMARKS:**

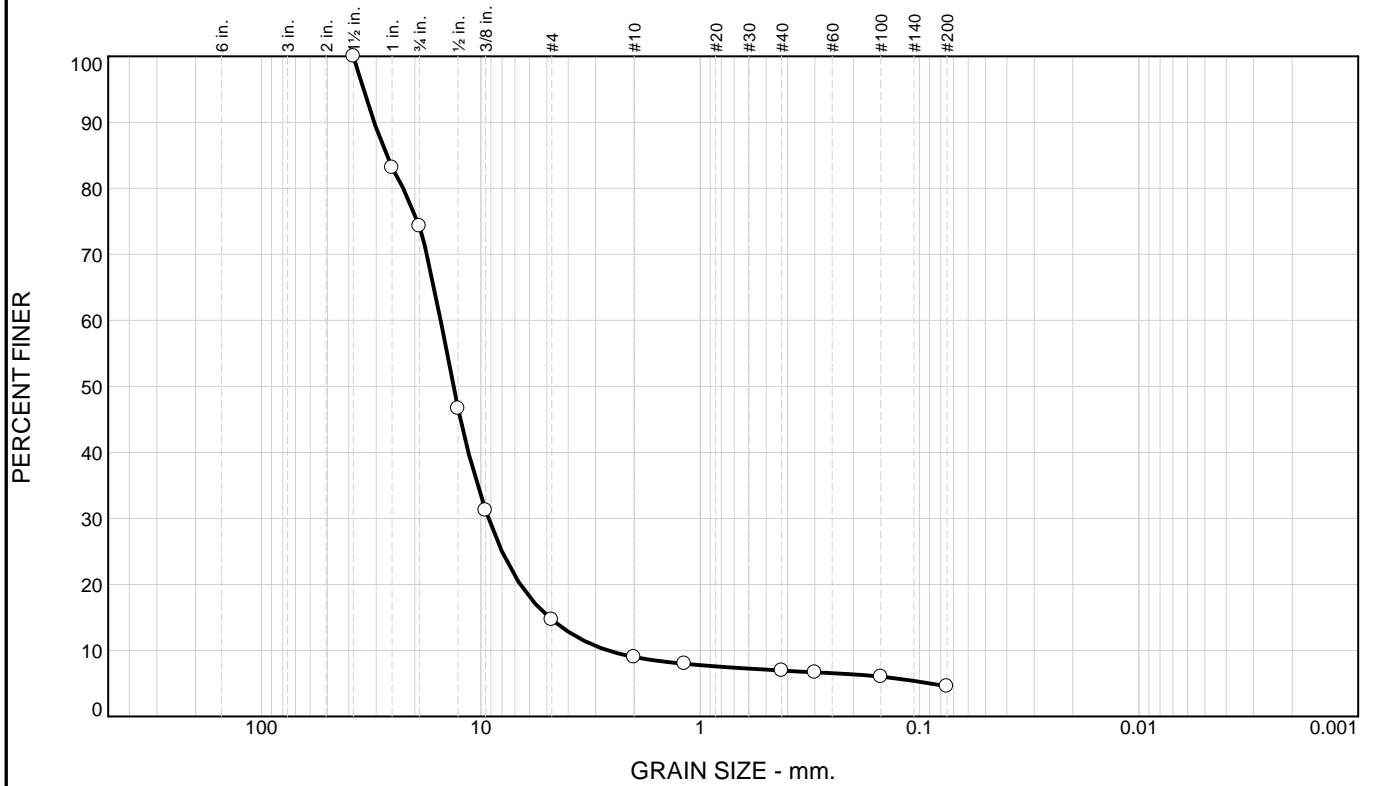
○

□

△

○ Source of Sample: CBP12      Depth: 1.0 - 2.5'      Sample Number: A  
 □ Source of Sample: CBP12      Depth: 5.0 - 6.5'      Sample Number: B  
 △ Source of Sample: CBP12      Depth: 15.0 - 15.8'      Sample Number: D

# Particle Size Distribution Report



	+3"	% GRAVEL	% SAND	% SILT	% CLAY	USCS	AASHTO	PL	LL
○	0.0	85.3	10.1	4.6		GW	A-2-4(0)	17	25

SIEVE inches size	PERCENT FINER		
	○		
1.5"	100.0		
1"	83.1		
3/4"	74.3		
1/2"	46.6		
3/8"	31.2		
<del>X</del>	GRAIN SIZE		
D60	15.2414		
D30	9.2438		
D10	2.6384		
<del>X</del>	COEFFICIENTS		
C <sub>c</sub>	2.12		
C <sub>u</sub>	5.78		

SIEVE number size	PERCENT FINER		
	○		
#4	14.7		
#10	9.0		
#16	8.0		
#40	7.0		
#50	6.7		
#100	6.0		
#200	4.6		

**Material Description**  
○ well-graded gravel

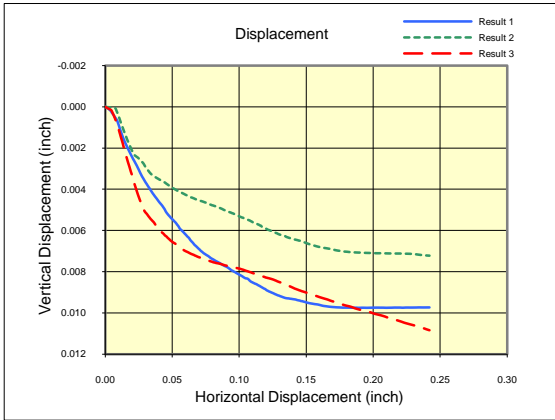
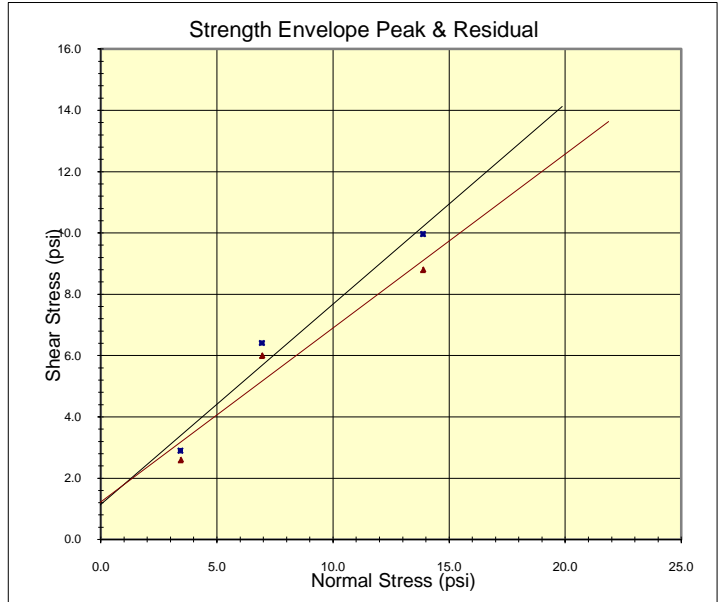
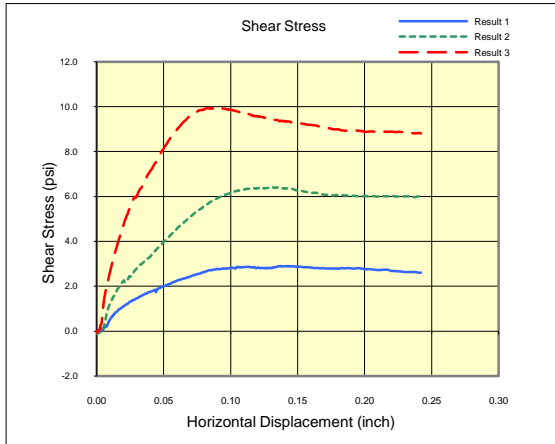
**REMARKS:**  
○

○ Source of Sample: CBP12      Depth: 5.0 - 10.0'      Sample Number: BULK 1





# DIRECT SHEAR TEST REPORT



Strength Parameters		
Friction Angle =	Peak <u>33</u> degrees	Residual <u>30</u>
Cohesion =	1.14 psi	1.23

Project: FL-6-10

Boring: CBP-1

Sample: B1

	Result 1	Result 2	Result 3
Specimen:	a	b	c
Date Tested	06/28/2010	06/28/2010	06/28/2010
Diameter (inch):	2.42	2.42	2.42
Height (inch):	1.00	1.00	1.00
Depth (ft):	5.80	5.80	5.80
Moisture (%):	23.2	22.0	17.8
Dry Unit Wt (pcf)	79.1	78.8	81.9
<b>SHEAR</b>			
Displacement Rate (in/min)	0.0055	0.0054	0.0057
Normal Stress (psi)	3.43	6.94	13.87
<b>Peak Shear Stress (psi)</b>	2.90	6.41	9.96
<b>Residual Shear Stress (psi)</b>	2.6	6.0	8.8
Residual Point Picked @ (in)	0.242	0.242	0.242
Time @ Peak Failure (min)	25.0	23.9	15.5

Specimen Comments

- a Pale Yellowish brown silt with sand shear @ 500 psf
- b Pale Yellowish brown silt with sand shear @ 1000 psf
- c Pale Yellowish brown silt with sand shear @ 2000 psf

